Handbook of Validation<br>Booklet VN.NN: Heading of the booklet Document: VN.NN.NNN

## MODEL01-Titrate case test (Model of one document of validation)

To respect the typography of the title. See [A3.02.02-A §4.2.3].

## Summary:

To write a relevant summary.
See [A3.02.02-A §4.3]

## Handbook of Validation

Booklet VN.NN: Heading of the booklet
HT-66/02/001/A

## Code_Aster ${ }^{\circledR}$

Version
6.0

Titrate:
MODEL01-Model of document of validation

Date:
07/10/02
Author (S):
N.SELLALI, P1. Key NOM1
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1
Problem of reference

### 1.1 Geometry

## 1.2

Properties of material

## 1.3

Boundary conditions and loadings

### 1.4 Conditions

initial

2
Reference solution

## 2.1 <br> Method of calculation

To respect the instructions of drafting of the formulas
mathematics in the documentation of
Code_Aster [D8.01.03-A]
2.2

Sizes and results of reference

## 2.3

Uncertainties on the solution

### 2.4 References

bibliographical
See the typography of drafting of
bibliography in [D8.01.01-A §12]

## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

## 3.2 <br> Characteristics of the grid

### 3.3 Functionalities

tested
This table is a typographical model.
Noncontractual contents.

Orders

AFFE_CARA_ELEM
BEAM
SECTION
"GENERAL"
DISCRETE
$K_{-} T_{-} D_{-} L$
"LOCAL"
DEFI_ARC
ORIE_ARC
ORIENTATION

## CARA

"VECT_Y"<br>AFFE_CHAM_NO

"TEMP_R"
"TEMP"
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
DEFI_MATERIAU
ELAS
3.4

Sizes tested and results

3.5 Remarks<br>Handbook of Validation<br>Booklet VN.NN: Heading of the booklet<br>HT-66/02/001/A

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## 5 <br> Summary of the results

To write a relevant synthesis. See
[A3.02.02-A §4.4]
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> Code_Aster ${ }^{\circledR}$
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> Titrate:
> ZZZZ100-Functions, tablecloths, formulas

## Date

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14/10/02
Author (S):
Key COURTEOUS Mr.

## :

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Organization (S): EDF-R \& D /AMA

Handbook of Validation<br>V1.01 booklet: Tests of validity of orders<br>V1.01.100 document

ZZZZ100-Functions, tablecloths, formulas

## Summary:

This test validates the functions tabulées with one or two variables (tablecloth) as well as the functions "formulas", in using the various possibilities of operator CALC_FONCTION.

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## 1

Definition of two tabulées functions

The definition of a function formula is carried out using the order FORMULATES.
We defined two functions formulates that we will use to validate the operator CALC_FONCTION:

$$
\begin{aligned}
& \text { IF = FORMULA (REEL= } \quad \text { "'"' (REAL: INST) }=\text { SIN (INST) } " '=" ') \\
& \text { CO = FORMULA (REEL= "'"'" (REAL: INST) = COS (INST) "'"'") }
\end{aligned}
$$

From these formulas, we calculated two functions (operator CALC_FONC_INTERP) for moments defined by a list of real.

These moments that one will find in all the document, are:
INST0=0.
INST1=DEFI_VALEUR (
R8=EVAL ("'"' INST0 + (2. *PAS1) "'"'"))
INST2=DEFI_VALEUR (
R8=EVAL ("'"' INST0 + (4. *PAS1) $\left.{ }^{\prime \prime \prime \prime \prime \prime}\right)$ )
INST3=DEFI_VALEUR (
R8=EVAL ( ${ }^{\prime \prime \prime \prime \prime \prime}$ INST0 + (6. *PAS1) $\left.{ }^{\prime \prime \prime \prime \prime \prime}\right)$ )
INST4=DEFI_VALEUR (
R8=EVAL ('"'"' INST0 + (10. * PAS1) '"'"'))
INST5=DEFI_VALEUR (
R8=EVAL ('"'"' INST0 + (12. * PAS1) '"'"'))
INST6=DEFI_VALEUR (
R8=EVAL ('"'"' INST0 + (14. * PAS1) "'"'"))
INST7=DEFI_VALEUR (
R8=EVAL ('"'"' INST0 + (16. * PAS1) "'"'"))
INST8=DEFI_VALEUR (
R8=EVAL ('"'"' INST0 + (18. * PAS1) $\left.{ }^{\prime \prime \prime \prime \prime \prime}\right)$ )
INST9=DEPI
with
DEPI=DEFI_VALEUR (

# R8=EVAL ("'"' 2. *Pi "'"'")) <br> PAS1=DEFI_VALEUR ( <br> R8=EVAL ("'"' DEPI/20. "'"'")) 

## 2 Order

CALC_FONCTION
2.1

Key words factors DERIVES, JUST

### 2.1.1 Definition

From the functions defined in [§1], we calculate the derivatives of these two functions, that one integrate then:

```
DER1=CALC_FONCTION (DERIVE=_F (
FUNCTION = IF))
INT1=CALC_FONCTION (INTEGRE=_F (
FUNCTION = DER1))
DER2=CALC_FONCTION (DERIVE=_F (
FUNCTION = CO))
INT2=CALC_FONCTION (INTEGRE=_F (
FUNCTION = DER2,
COEF = 1.,
METHOD = "SIMPSON"))
```

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2.1.2 Values<br>tested<br>Function der1<br>Identification Reference<br>Aster Difference<br>NOM_RESU<br>QUICKLY<br>QUICKLY<br>\%<br>VALE_PARA: inst0<br>1.<br>0.999835<br>-0.016<br>VALE_PARA: inst1<br>0.809017<br>0.808883<br>-0.016<br>VALE_PARA: inst2<br>0.309017<br>0.308966<br>-0.016<br>VALE_PARA: inst3<br>-0.309017<br>-0.308966<br>-0.016<br>VALE_PARA: inst 4<br>-1.<br>-0.999835<br>-0.016<br>VALE_PARA: inst5<br>-0.809017<br>-0.808883<br>-0.016<br>VALE_PARA: inst6<br>-0.309017<br>-0.308966<br>-0.016<br>VALE_PARA: inst7

0.309017<br>0.308966<br>-0.016<br>VALE_PARA: inst8<br>0.809017<br>0.808883<br>-0.016<br>VALE_PARA: inst9<br>1.<br>0.999835<br>-0.016

Function int1
Identification Reference
Aster Difference
NOM_RESU
DEPL
DEPL
\%
VALE_PARA: inst0
0.
0.

VALE_PARA: inst1
0.587785
0.587688
-0.025
VALE_PARA: inst2
0.951056
0.950900
-0.025
VALE_PARA: inst3
0.951056
0.950900
-0.025
VALE_PARA: inst 4
0.
0.

VALE_PARA: inst5
-0.587785
-0.587688
-0.025
VALE_PARA: inst6

## Identification Reference

Aster Difference
NOM_RESU QUICKLY QUICKLY \%
VALE_PARA: inst0
0.
-0.015706
-1.571
VALE_PARA: inst1
-0.587785
-0.587688
-0.016
VALE_PARA: inst2
-0.951056
-0.950900
-0.016
VALE_PARA: inst3
-0.951056
-0.950900
-0.016
VALE_PARA: inst4
0.
0.

VALE_PARA: inst5
0.587785
0.587688
-0.016
VALE_PARA: inst9
0.
-0.015706
-1.571
Function int 2
Identification Reference
Aster Difference
NOM_RESU

## DEPL

DEPL
\%
VALE_PARA: inst0
1.
1.

VALE_PARA: inst1
0.809017
0.808883
-0.016
VALE_PARA: inst2
0.309017
0.308966
-0.016
VALE_PARA: inst3
-0.309017
-0.308966
-0.016

VALE_PARA: inst 4
-1.
-0.999835
-0.016
VALE_PARA: inst5
-0.809017
-0.808883
-0.016
VALE_PARA: inst6
-0.309017
-0.308966
-0.016
VALE_PARA: inst7
0.309017
0.308966
-0.016
VALE_PARA: inst8
-0.809017
-0.808883
-0.016
VALE_PARA: inst9
1.
1.

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## 2.2

Key word factor MAX
This key word is used in the test of SPEC_OSCI.

## 2.3

Key word factor WRAPS

### 2.3.1 Definition

From the functions defined in [§1], we calculate the envelope of these two functions:
ENV1=CALC_FONCTION (ENVELOPPE $=\_$F $($
FUNCTION $=(I F$,
CO,),
CRITERION = "SUP"))
ENV2=CALC_FONCTION (ENVELOPPE =_F (
FUNCTION $=(I F$,
CO,),
CRITERION = "INF"))

### 2.3.2 Values

tested
Function env1

## Identification Reference

Aster Difference
VALE_PARA: inst0
1.
1.

VALE_PARA: inst1
0.809017
0.809017

VALE_PARA: inst2
0.951056
0.951056

VALE_PARA: inst3
0.951056
0.951056

VALE_PARA: inst 4
0.
0.

VALE_PARA: inst5
-0.587785
-0.587785
VALE_PARA: inst6
-0.309017
-0.309017
VALE_PARA: inst7
-0.309017
-0.309017
VALE_PARA: inst8
0.809017
0.809017

VALE_PARA: inst9
1.
1.

Function env2
Identification Reference
Aster Difference
VALE_PARA: inst0
0.
0.

VALE_PARA: inst1
0.587785
0.587785

VALE_PARA: inst2
0.309017
0.309017

VALE_PARA: inst3
-0.309017
-0.309017
VALE_PARA: inst 4
-1.
-1.
VALE_PARA: inst5
-0.809017
-0.809017
VALE_PARA: inst6
-0.951056
-0.951056
VALE_PARA: inst7
-0.951056

VALE_PARA: inst8
-0.587785
-0.587785
VALE_PARA: inst9
0.
0.

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```
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## 2.4 <br> Key word factor COMB

### 2.4.1 Definition

From the functions defined in [§1], we calculate the combination of these two functions:
COMO=CALC_FONCTION (
LIST_PARA=LST, COMB=(
_F
$F U N C T I O N=I F, C O E F=1$.$) ,$

FUNCTION = CO, COEF $=+1$.$) )$
)
COM1=CALC_FONCTION (COMB= (_F (
$F U N C T I O N=I F, C O E F=1$.$) ,$

_F (

    FUNCTION = INT1, COEF =-1.) )
    )
COM2=CALC_FONCTION (COMB= (_F (
$F U N C T I O N=C O, C O E F=1$.),
_F (
FUNCTION = INT2, $C O E F=-1$.$) )$
)
2.4.2 Values
tested
Function com0Identification ReferenceAster DifferenceVALE_PARA: inst0
1.
1.
VALE_PARA: inst1
$1.39680 E+00$
1.39680E+00
VALE_PARA: inst2
$1.26007 E+00$
$1.26007 E+00$
VALE_PARA: inst3
6.42039E-01
6.42039E-01
VALE_PARA: inst 4
-1.
-1.
VALE_PARA: inst5
-1.39680E+00
-1.39680E+00
VALE_PARA: inst6
-1.26007E+00
-1.26007E+00
VALE_PARA: inst7
-6.42039E-01
-6.42039E-01
VALE_PARA: inst8
2.21231E-01
2.21231E-01

## VALE_PARA: inst9

1. 
2. 

Function com1
Identification Reference
Aster Difference
VALE_PARA: inst0
0.
0.

VALE_PARA: inst1
0.
1.45018E-04
1.45E-04

VALE_PARA: inst2
0.
2.34644E-04
2.35E-04

VALE_PARA: inst3
0.
2.34644E-04
2.35E-04

VALE_PARA: inst 4
0.
0.

VALE_PARA: inst5
0.
-1.45018E-04
-1.45E-04
VALE_PARA: inst6
0.
-2.34644E-04
-2.35E-04
VALE_PARA: inst7
0.
-2.34644E-04
-2.35E-04
VALE_PARA: inst8
0.
-1.45018E-04
-1.45E-04
VALE_PARA: inst9
0.
1.92220E-16
0.

Function com2

Identification Reference
Aster Difference
VALE_PARA: inst0
0.
0.

VALE_PARA: inst1
0.
1.33067E-04
1.33E-04

VALE_PARA: inst2
0.
5.08270E-05
5.08E-05

VALE_PARA: inst3
0.
-5.08270E-05
-5.08E-05
VALE_PARA: inst4
0.
1.64479E-04
-1.64E-04
VALE_PARA: inst5
0.
-1.33067E-04
-1.33E-04
VALE_PARA: inst6
0.
-5.08270E-05
-5.08E-05
VALE_PARA: inst7
0.
5.08270E-05
5.08E-05

VALE_PARA: inst8
0.
1.33067E-04
1.33E-04

## VALE_PARA: inst9

0. 
1. 

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## 2.5

Key word factor COMB_C

### 2.5.1 Definition

The two complex functions are defined:
DFC1=DEFI_FONCTION (
NOM_PARA =' INST',
NOM_RESU=' FREQ',
VALE_C= (0. , 1. , 2. , 1. , 3. , 4. , 5. , 2. , 4. ,))
DFC2=DEFI_FONCTION (
NOM_PARA=' INST',
NOM_RESU=' FREQ',

One calculates the linear combination complexes DFC3=DFC1 + 2i.DFC2.
2.5.2 Values

## Function DFC3

## Identification Reference

Aster Difference
VALE_PARA: 0.
(-1.,4.)
(-1.,4.)
0.

VALE_PARA: 1 .
(0.,8.)
(0.,8.)
0.

VALE_PARA: 5.
(-10.,14.)
(-10.,14.)
0.

## 2.6 <br> Key word factor COMPOSES

### 2.6.1 Definition

Two real functions are defined:

## FONC1=DEFI_FONCTION (

NOM_PARA $=^{\prime} X^{\prime}$,
NOM_RESU $=^{\prime} \boldsymbol{F}^{\prime}$,
VALE $=$ (0.,
0.,
2.,
5.,
3., 10.,
5., 15.,
7., 13.,
8., 10.,
10., 9.,
12., 8.,
13., 5.,
15., 1.,
20., 0., ) )

FONC2=DEFI_FONCTION (

```
NOM_PARA=' INST',
NOM_RESU=' \(X^{\prime}\),
VALE \(=(0 .\),
\(0 .\),
0.1,
2.,
0.2,
4.,
0.3,
6.,
0.4,
8.,
0.5, 10.,
\(0.6,12 .\),
0.7, 14.,
0.8, 16.
0.9, 18.,
1.0, 20., ) )
One calculates COMP1 (para) =FONC1 (FONC2 (para)).
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```

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2.6.2 Values
tested

## Function COMP1

## Identification Reference

Aster Difference
VALE_PARA: 0 .
0.
0.
0.

VALE_PARA: 0.1
5.
5.

0 .
VALE_PARA: 0.2
12.5
12.5
0.

VALE_PARA: 0.3
14.
14.

0 .
VALE_PARA: 0.4
10.
10.

0 .
8.

8 .
0.

VALE_PARA: 0.7
3.
3.
0.

VALE_PARA: 0.8
0.8
0.8
0.

VALE_PARA: 0.9
0.4
0.4
0.

VALE_PARA: 1.0
0.

0 .
0 .
2.7

Key word factor ADZE

### 2.7.1 Definition

Two real functions are defined:

DFCl = DEFI_FONCTION (
$N O M \_P A R A=' X$ ',
$N O M \_R E S U=' Y^{\prime}$,
VALE $=$ (0. , 10. , 4. , 14. , 6. , 16. , ),
PROL_DROITE = ' LINEAIRE',
PROL_GAUCHE = ' LINEAIRE'
)
DFC2 = DEFI_FONCTION (
$N O M \_P A R A=' X^{\prime}$,
$N O M \_R E S U=' Y^{\prime}$,
$V A L E=(5 ., 25 ., ~ 7 . ~, ~ 27 . ~, ~ 8 . ~, ~ 28 . ~),, ~(, ~$

# PROL_DROITE=' LINEAIRE', 

PROL_GAUCHE=' LINEAIRE'
)
One assembles these two functions, while overloading on the right.

### 2.7.2 Values <br> tested <br> Function DFC3

## Identification Reference

Aster Difference
VALE_PARA: 4.
14.
14.
0.

VALE_PARA: 5.
25.
25.

0 .
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2.8

## Key word factor EXTRACTION

### 2.8.1 Definition

One extracts from the function defined in paragraph "COMB_C" the real part, the imaginary part, sound modulate and its phase:

DFR4=CALC_FONCTION (<br>EXTRACTION =_F (<br>FUNCTION = DFCl,<br>$P A R T=$ "REAL"))<br>DFR5=CALC_FONCTION (<br>EXTRACTION =_F (<br>FUNCTION = DFCl,<br>$P A R T=" I M A G ")$<br>DFR6=CALC_FONCTION (<br>EXTRACTION=_F (<br>FUNCTION = DFCl,<br>PART = "PHASE"))<br>DFR7=CALC_FONCTION(<br>EXTRACTION =_F (<br>FUNCTION = DFCl,<br>$P A R T=" M O D U L E "))$

### 2.8.2 Values <br> tested <br> Functions, DFR4, DFR5, DFR6, DFR7

## Identification Reference

Aster Difference
DFR4, VALE_PARA: 5.
2.
2.
0.

DFR5, VALE_PARA: 5.
4.
4.
0.

DFR6, VALE_PARA: 5.
63.4349
63.4349

## 2.9

Key word factor RMS

### 2.9.1 Definition

One calculates value RMS of the two functions defined in [§1].

### 2.9.2 Values

tested
Count RMS_SI
Identification Reference
Aster Difference
NOM_PARA: RMS
7.07107E-01
7.07107E-01

0 .
Count RMS_CO
Identification Reference
Aster Difference
NOM_PARA: RMS
7.07107E-01
7.07107E-01

0 .
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### 2.10 Key word factor NOCI_SEISME

### 2.10.1 Definition

One calculates the whole of the indices of harmfulness for a real accélérogramme, option by option and at the same time.

### 2.10.2 Values tested

## Identification Reference

Aster Difference
NOM_PARA: ACCE_MAX
1.
1.
0.

NOM_PARA: VITE_MAX
2.29056E-01
2.29057E-01
0.0003

NOM_PARA: DEPL_MAX
1.34245E+00
1.34245E+00

0 .
NOM_PARA: INTE_ARIAS
2.58200E-01
2.58206E-01
0.002

NOM_PARA: POUV_DEST
2.09825E-01
2.09826E-01
0.0007

NOM_PARA: ACCE_SUR_VITE

NOM_PARA: VITE_ABSO_CUMU

$$
4.43370 E+00
$$

$4.43375 E+00$
0.001

NOM_PARA: DUREE_PHAS_FORT
$1.48300 E+01$
$1.48300 E+01$
0 .
NOM_PARA: INTE_SPECT
$5.86000 E+00$
5.86208E-01
0.036

The values of reference are of nonregression.

### 2.11 Key word factor SPEC_OSCI

### 2.11.1 Definition

We defined a accélérogramme:

```
F = FORMULA (REEL= """ (REAL: INST) = COS (OMEGA*INST) """")
in order to calculate the spectrum of oscillator
SOD=CALC_FONCTION (SPEC_OSCI=_F (
FUNCTION = F,
NATURE = "DEPL",
AMOR_REDUIT = (AMOR1, AMOR2,),
FREQ = (FREQ1, FREQ2, FREQ3, ),
= 1 NORMALIZES.)
)
SOV=CALC_FONCTION (SPEC_OSCI=_F(
FUNCTION = F,
NATURE = "QUICKLY",
AMOR_REDUIT = (AMOR1, AMOR2, ),
FREQ = (FREQ1, FREQ2, FREQ3, ),
= 1 NORMALIZES.)
)
SOA=CALC_FONCTION (SPEC_OSCI=_F(
FUNCTION = F,
NATURE = "ACCE",
```

```
AMOR_REDUIT = (AMOR1,AMOR2, ),
FREQ = (FREQ1, FREQ2, FREQ3,),
= 1 NORMALIZES.)
)
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### 2.11.2 Values tested

The reference solution is an analytical solution:

- solution in displacement:

That is to say:
$=3.566$
$T=$ urgent
$F=$ frequency
$=$ damping
1
$U(T, F)=,(B-C-D)$
With
with:

2

With $=(2$
4
$F 2-2)+(4 F$
) 2
$B=(2$
4
F2-2). (
$\cos T$
) $+4 F$
$\sin (T)$
$C=(2$
4
F 2-2) $\cos (2 f t$
)
$D=(2$
4
$F 2-2) \cdot \sin (2 f t$
)
$-2 f t$
E

- solution of speed:
$u \&(T, F$
, ) =
2 F.u(T, F
, )
- solution in acceleration:
$U(\& t, F$
, ) = (
2 F) $2 . u(T, F$
, )

One tests the moments when the maximum is reached (one validates the key word MAX thus).

## Function sod (displacement)

## Identification Reference

Aster Difference
\%

## NOM_PARA:

## ("AMOR"

"FREQ")
1.95737E-03
1.95716E-03
-0.011
VALE_PARA: (
0.01
5.
)
NOM_PARA:
("AMOR"
"FREQ")
1.07089E-04
1.07077E-04
-0.011
VALE_PARA: (
0.01
21.
)
NOM_PARA:
("AMOR"
"I FREQ")
3.38318E-06
3.38503E-06
0.055

VALE_PARA: (
0.01
105.
)
NOM_PARA:
("AMOR"
"FREQ")
1.89836E-03
1.89814E-03
-0.011
VALE_PARA: (
0.03
5.
)
NOM_PARA:
("AMOR"
"FREQ")
1.03096E-04
-0.011

VALE_PARA: (
0.03
21.
)
NOM_PARA:
("AMOR"
"FREQ")
2.57820E-06
2.57915E-06
0.037

VALE_PARA: (
0.03
105.
)
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## :

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Function sov (speed)

## Identification Reference

Aster Difference
\%

## NOM_PARA:

("AMOR"
"FREQ")
6.14925E-02
6.14859E-02
-0.011
VALE_PARA: (
0.01
5.
)
NOM_PARA:
("AMOR"
"FREQ")
1.41300E-02
1.41285E-02
-0.011
VALE_PARA: (
0.01
21.
)
NOM_PARA:
("AMOR"
"I FREQ")
2.23200E-03
2.23322E-03
0.055

VALE_PARA: (
0.01
105.
)
NOM_PARA:
("AMOR"
"FREQ")
5.96387E-02
5.96320E-02
-0.011
VALE_PARA: (
0.03
5.
)
NOM_PARA:
("AMOR"
"FREQ")

## -0.011

VALE_PARA: (
0.03
21.
)
NOM_PARA:
("AMOR"
"FREQ")
1.70155E-03
1.70093E-03
0.037

VALE_PARA: (
0.03
105.
)

## Function soa (acceleration)

## Identification Reference

Aster Difference
\%
NOM_PARA:
("AMOR"
"FREQ")
1.93184E+00
$1.93163 E+00$
-0.011
VALE_PARA: (
0.01
5.
)
NOM_PARA:
("AMOR"
"FREQ")
1.86441E+00
1.86421E+00
-0.011
VALE_PARA: (
0.01
21.
)
NOM_PARA:
("AMOR"
"FREQ")
$1.87360 E+00$
1.87339E+00
-0.011
VALE_PARA: (
0.03
5.
)
NOM_PARA:
("AMOR"
"FREQ")
1.79511E+00
1.79491E+00
-0.011
VALE_PARA: (
0.03
21.
NOM_PARA:
("AMOR"
"FREQ")
1.12216E+00
$1.12257 E+00$
0.037
VALE_PARA: (
0.03
105.
)

### 2.12 Key word factor FFT

### 2.12.1 Definition

One calculates the transform of Fourier of a accélérogramme real $X$.

### 2.12.2 Values tested

## Function XFF

## Identification Reference

Aster Difference
\%
VALE_PARA: 50.8
(0.124,
(0.124358,
0.279
0.0383 )
$0.03825)$
VALE_PARA: 121.1
(0.177,
(0.17702,
0.013
-0.0347)
-0.03471)
The values of reference are of nonregression.
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### 2.13 Key word factor $C O R R_{-}$ACCE

### 2.13.1 Definition

One calculates the correction of a commonplace accélérogramme (curve refines), and that of the real accélérogramme used in paragraph "NOCI_SEISME".

### 2.13.2 Values tested

One tests the value of the RMS of the corrected functions obtained.

## Identification Reference

Aster Difference
NOM_PARA: RMSI
0.
5.684E-14
0.

NOM_PARA: RMS2
2.278E-01
2.278E-01
0.

NOM_PARA: RMS3
0.
2.406E-11
0.

NOM_PARA: RMS4
2.243E-01
2.243E-01

0 .

## 3

## Generation of a function by IMPR_COURBE

One tests the possibility of IMPR_COURBE of writing a function with the format "ORDERS".
The line of order generated by IMPR_COURBE is carried out by making a INCLUDE in CONTINUATION; then, it is checked that one finds the values of the printed function.

## 4 <br> Summary of the results

This test validates the whole of the orders which create functions (DEFI_FONCTION, IMPR_COURBE, and in particular order CALC_FONCTION).

The attributes of the functions there are tested: type of interpolation, prolongations,... as well as the values
interpolated or not for tablecloths and functions real or complex.
The only significant errors are raised on real applications (accélérogramme by example), where one has only values of nonregression.
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Organization (S): EDF/MTI/MMN, CS IF

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Document: V1.01.101

## ZZZZ101 - Validation of the operators <br> AFFE_CARA_ELEM and POST_ELEM

## Summary:

Validation of operators AFFE_CARA_ELEM and POST_ELEM.
This test relates to the calculation of the mass, the centre of gravity and the tensor of inertia in the centre of gravity for
following modelings:

- discrete elements: DIS_TR and DIS_T,
- elements of bar: BAR,
- elements of beam: POU_D_E, POU_D_T, POU_C_T,
- elements of hulls: DKT, DST, Q4G,
- voluminal elements: 3D.

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### 1.1 Geometry

Grid in space 3D not modelling any defined structure, formed of specific meshs, linear, of plates, and a volume hexaedric.

```
ma_qua4
ma_tri3
y
P6
```


## P5

```
3
P4
Plan \(Z=7\)
```

2
1
P1
P2
P3
$X$
Parallépipède on side 1, 2, 7.
$y^{\prime \prime}$
According to $Z$ '": thickness 1.
In the reference mark ( $X, y, Z$ ) A has as co-ordinates
(1.0.0.).

One passes from reference mark $(X, y, Z)$ to the reference mark ( $X^{\prime}$ ',
$\left.y^{\prime \prime}, Z{ }^{\prime \prime}\right)$ with the angles of Euler $\left(45^{\circ}, 45^{\circ}, 0^{\circ}\right)$.
$X^{\prime \prime}$
2
With

### 1.2 Properties

material
$E=2.1011 P a$
$=0.3$
$=1.5 \mathrm{~kg} / \mathrm{m} 3$ (except for the discrete elements: $=1.5 .104 \mathrm{~kg} / \mathrm{m} 3)$
1.3
Boundary conditions and loadings
Without object (not of resolution).

1.4 Conditions<br>initial<br>\section*{Without object.}<br>Handbook of Validation<br>V1.01 booklet: Tests of validity of orders<br>HI-75/01/010/A

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2
Reference solution

## 2.1 <br> Method of calculation

Mass and centre of gravity:

$$
\begin{aligned}
& m=F D \\
& =d x . d y . d z
\end{aligned}
$$

$v$
$v$
$x . d v$
$y . d v$
$z . d v$

```
X
v
=
y
v
=
Z
v
G
=
m

\section*{Tensor of inertia:}
```

$I$
2
2
$x x=(y+Z) . d v$
I
$=X \cdot y \cdot d v$
$x y$
$v$
$v$
$I$
2
2
$y y=(X+Z) \cdot d v$
I
$=x . z . d v$
$x z$

```
```

v

```
v
v
v
I
I
2
```

2

```

2
\(z z=(X+y) . d v\)
I
\(=y . z . d v\)
\(y z\)
\(v\)
\(v\)
2.2

Sizes and results of reference
Masses and inertias for various modelings.

\section*{2.3}

Uncertainties on the solution
Note:
For one of the grids modelled in hulls, the solution is numerical (not regression). Handbook of Validation
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\section*{3 Modeling}

With
3.1

Characteristics of modeling
DISCRETE element:
- modeling \(M_{-} T \_D \_N\) :
calculation of the mass and the centre of gravity
- modeling M_T_N:
calculation of the mass and the centre of gravity
- modeling M_TR_D_N:
calculation of the mass, the centre of gravity and the tensor of inertia + offsetting
- modeling M_TR_N:
calculation of the mass, the centre of gravity and the tensor of inertia + offsetting
- modeling \(M_{-} T_{-} L\) :
calculation of the mass and the centre of gravity
- modeling M_TR_L:
calculation of the mass and the centre of gravity
Element BARS:
- modeling BARS:
calculation of the mass and the centre of gravity general section, right-angled section and section ring (full and dig)

Element BEAM:
- modeling POU_D_E:
calculation of the mass
general section, right-angled section and section ring (full and dig)
- modeling POU_D_T:
calculation of the mass
general section, right-angled section and section ring (full and dig)
- modeling POU_C_T:
calculation of the mass
general section, right-angled section and section ring (full and dig)

Element HULL:
- modeling DKT: (triangle calculation of the mass, the centre of gravity and the tensor of inertia
and quadrangle)
- DST modeling: (triangle calculation of the mass, the centre of gravity and the tensor of inertia and quadrangle)
- modeling Q4G: (triangle calculation of the mass, the centre of gravity and the tensor of inertia and quadrangle)
- modeling 3D (HEXA8)
calculation of the mass, the centre of gravity and the tensor of inertia

\subsection*{3.2 Characteristics}
grid

\section*{DISCRETE element:}

\author{
\(\cdot m o d e l i n g M_{-} T \_D \_N, M_{-} T \_N, M_{-} T R \_D \_N, M_{-} T R \_N\) : \\ 1 mesh POII \\ \(\cdot\) modeling \(M_{-} T \_L, M_{-} T R_{-} L\) : \\ 1 mesh SEG2 \\ Element BARS: \\ - modeling BARS: \\ 1 mesh SEG2 \\ Element BEAM: \\ \(\cdot m o d e l i n g ~ P O U \_D \_E, P O U_{-} D_{-} T\) : \\ 1 mesh SEG2 \\ - modeling POU_C_T: \\ 7 meshs SEG2 \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-75/01/010/A
}

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Element HULL:
- modeling DKT, DST, Q4G:

5 meshs TRIA3 and QUAD4
2 meshs TRIA3 (grid
irregular 2 meshs QUAD4)
Element 3D:
1
net
HEXA8

\subsection*{3.3 Functionalities \\ tested \\ Orders}

DISCRETE AFFE_CARA_ELEM

BAR

BEAM

\section*{3.4}

Sizes tested and results

\section*{Modeling Grid AFFE_CARA_ELEM Identification Reference \\ Aster \% \\ difference}

DIS_T 1
POII
\(M_{-} T_{-} D \_N\)
MASS
5,1585E+015,1585E+01 0
CDG_X
1,0000E+00 1,0000E+00 0
CDG_Y
1,0000E+00 1,0000E+00 0
CDG_Z
7,0000E+00 7,0000E+00 0
DIS_T 1
POII
\(M_{-} T \_N\)
MASS
5,1585E+01 5,1585E+01 0
CDG_X
1,0000E+00 1,0000E+00 0
CDG_Y
1,0000E+00 1,0000E+00 0
CDG_Z
7,0000E+00 7,0000E+00 0
DIS_TR 1
POII
\(M_{-} T R \_D \_N\)
MASS
5,1585E+015,1585E+01 0
CDG_X
1,0000E+00 1,0000E+00 0

CDG_Y
\(1,0000 E+001,0000 E+000\)
CDG_Z
\(7,0000 E+007,0000 E+000\)
IX_G
6,9815E04 6,9815E04 0
IY_G
5,2962E04 5,2962E04 0
IZ_G
2,7170E04 2,7170E04 0
IXY_G
1,0317E 1,0317E04 0
04
IXZ_G
1,5476E 1,5476E04 0
04
IYZ_G
3,0951E 3,0951E04 0
04
DIS_TR 1
POII
\(M \_T R \_N\)
MASS
\(5,1585 E+015,1580 E+010,01\)
\(C D G \_X\)
\(1,0000 E+001,0000 E+000\)
\(C D G_{-} Y\)
\(1,0000 E+001,0000 E+000\)
CDG_Z
\(7,0000 E+007,0000 E+000\)
\(I X \_G\)
6,9815E04 6,9810E04 0,006
\(I Y_{-} G\)
5,2962E04 5,2960E04 0,004
IZ_G
2,7170E04 2,7170E04 0,002
\(I X Y \_G\)

\author{
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}

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DIS_T 1
SEG2
M_T_L
MASS
2,0000E+00 2,0000E+00 0
\(C D G \_X\)
1,5000E+00 1,5000E+00 0
CDG_Y
1,0000E+00 1,0000E+00 0
CDG_Z
7,0000E+00 7,0000E+00 0
DIS_TR 1
SEG2

CDG_X
1,5000E+00 1,5000E+00 0
CDG_Y
1,0000E+00 1,0000E+00 0
CDG_Z
7,0000E+00 7,0000E+00 0
BAR 1
SEG2
section: general
MASS
6,6609E+00 6,6609E+00 0

\section*{CDG_X}

3,0000E+00 3,0000E+00 0

CDG_Y
\(2,0000 E+002,0000 E+000\)

\section*{CDG_Z}

7,0000E+00 7,0000E+00 0
1 SEG2
section: full square MASSES
4,2426E+00 4,2426E+00 0
1 SEG2
section: hollow square MASSES
8,0610E01 8,0610E01 0
1 SEG2
section: rectangle
MASS
1,0861E+00 1,0861E+00 0
hollow
1 SEG2
section: full circle MASSES
1,3329E+01 1,3329E+01 0
1 SEG2
section: ring hollow MASSES

7,8772E01 7,8772E01 0
POU_D_E 1
SEG2 section: general
MASS
6,6609E+00 6,6609E+00 0

CDG_X
3,0000E +00 3,0000E +000
\(C D G_{-} Y\)
\(2,0000 E+002,0000 E+000\)

CDG_Z
7,0000E+00 7,0000E+00 0
1 SEG2
section: full square MASSES
4,2426E+00 4,2426E+00 0
1 SEG2
section: hollow square MASSES
8,0610E01 8,0610E01 0
1 SEG2
section: rectangle
MASS
1,0861E+00 1,0861E+00 0
hollow
1 SEG2
section: full circle MASSES
1,3329E+01 1,3329E+01 0
1 SEG2
section: ring hollow MASSES
7,8772E01 7,8772E01 0
POU_D_T 1
SEG2
section: general
MASS
6,6609E+00 6,6609E+00 0

\section*{CDG_X}
\(3,0000 E+003,0000 E+000\)

\author{
CDG_Z \\ 7,0000E+00 7,0000E+00 0 1 SEG2 \\ section: full square MASSES \\ 4,2426E+00 4,2426E+00 0 \\ 1 SEG2 \\ section: hollow square MASSES \\ 8,0610E01 8,0610E01 0 \\ 1 SEG2 \\ section: rectangle \\ MASS \\ 1,0861E+00 1,0861E+00 0 \\ hollow \\ 1 SEG2 \\ section: full circle MASSES \\ \(1,3329 E+011,3329 E+010\) \\ 1 SEG2 \\ section: ring hollow MASSES \\ 7,8772E01 7,8772E01 0 \\ DKT 2 \\ TRIA3 \\ thickness \\ MASS \\ 1,8000E01 1,8000E01 0
}

\author{
\(C D G \_X\) \\ \(3,0000 E+003,0000 E+000\)
}

\section*{CDG_Y}
\(2,0000 E+002,0000 E+000\)
\(C D G \_Z\)
7,0000E+00 7,0000E+00 0
\(I X \_G\)
6,0020E02 6,0014E02 0,011
```

IY_G
6,0020E02 6,0014E02 0,011

```
IZ_G
1,2000E01 1,2000E01 0
DKT 2
QUAD4
thickness
MASS
2,7000E01 2,7000E01 0
\(C D G_{-} X\)
\(2,5000 E+002,5000 E+000\)
CDG_Y
\(2,0000 E+002,0000 E+000\)
CDG_Z
7,0000E+00 7,0000E+00 0
IX_G
9,0020E02 9,0020E02 0
\(I Y_{-} G\)
2,0252E01 2,0252E01 0

IZ_G
2,9250E01 2,9250E01 0
DST 2
TRIA3
thickness

\section*{MASS}

1,8000E01 1,8000E01 0
\(C D G_{-} X\)
3,0000E+00 3,0000E+00 0

CDG_Y
\(2,0000 E+002,0000 E+000\)

CDG_Z
7,0000E +00 7,0000E+00 0

\section*{IX_G \\ 6,0020E02 6,0014E02 0,011}

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\(I Y_{-} G\)
6,0020E02 6,0014E02 0,011
IZ_G
1,2000E01 1,2000E01 0
DSQ 2
QUAD4
thickness
MASS
2,7000E01 2,7000E01 0
CDG_X
\(2,5000 E+002,5000 E+000\)
\(C D G \_Y\)
IZ_G
2,9250E01 2,9250E01 0
IX_P
7,1100E+00 7,1100E+00 0
\(I Y_{-} P\)7,5600E+00 7,5600E+00 0
IZ_P
1,1700E+00 1,1700E+00 0
Q4G 2
QUAD4
thickness
MASS2,7000E01 2,7000E01 0
CDG_X
\(2,5000 E+002,5000 E+000\)
CDG_Y\(2,0000 E+002,0000 E+000\)
CDG_Z7,0000E+00 7,0000E+00 0
IX_G
9,0020E02 9,0020E02 0
\(I Y \_G\)
2,0252E01 2,0252E01 0
IZ_G
2,9250E01 2,9250E01 0
```

POU_C_T and 7 SEG2
DEFI_ARC centers
MASS
5,5488E+00 5,5488E+00 0
POU_D_T
POU_C_T and 7 SEG2
Different DEFI_ARC
MASS
3,6664E+01 3,6664E+01 0
POU_D_T
center
DKT 3
TRIA3
MASS
3,9000E+02 3,9000E+02 0
2 QUAD4
CDG_X
8,5000E01 8,5000E01 }
CDG_Y
1,4722E+00 1,4722E+00 0
CDG_Z
1,9000E+00 1,9000E+00 0
IX_PRIN_G
3,2500E+01 3,2503E+01 0,01
IY_PRIN_G
8,1250E+02 8,1250E+02 0
IZ_PRIN_G
8,4500E+02 8,4500E+02 0
ALPHA
6,0000E+01 6,0000E+01 0
GAMMA
9,0000E+01 9,0000E+01 0
DST 3
TRIA3
MASS
3,9000E+02 3,9000E+02 0

```
```

2 QUAD4
CDG_X
8,5000E01 8,5000E01 }
CDG_Y
1,4722E+00 1,4722E+00 0
CDG_Z
1,9000E+00 1,9000E+00 0
IX_PRIN_G
3,2500E+01 3,2503E+01 0,01
IY_PRIN_G
8,1250E+02 8,1250E+02 0
IZ_PRIN_G
8,4500E+02 8,4500E+02 0
ALPHA
6,0000E+01 6,0000E+01 0
GAMMA
9,0000E+01 9,0000E+01 0
Q4G 1
QUAD4
MASS
3,9000E+02 3,9000E+02 0
CDG_X
8,5000E01 8,5000E01 }
CDG_Y
1,4722E+00 1,4722E+00 0
CDG_Z
1,9000E+00 1,9000E+00 0
IX_PRIN_G
3,2500E+01 3,2503E+01 0,01
IY_PRIN_G
8,1250E+02 8,1250E+020

```
```

IZ_PRIN_G
8,4500E+02 8,4500E+02 0
ALPHA
6,0000E+01 6,0000E+01 0
GAMMA
9,0000E+01 9,0000E+01 0
3D 1
HEXA8
MASS
7,8000E+04 7,8000E+04 0
CDG_X
2,4883E+00 2,4883E+00 0
CDG_Y
2,4883E+00 2,4883E+00 0
CDG_Z
2,2008E+00 2,2008E+00 0
IX_PRIN_G
1,9500E+04 1,9500E+04 0
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```
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ101 - Validation of operators AFFE_CARA_ELEM and POST_ELEM
Date:
29/10/01
Author (S):
J.M. PROIX, L. VIVAN Key
.
V1.01.101-A Page:
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\(I Y \_P R I N \_G\)

ALPHA
4,5000E+01 4,5000E+01 0
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Version
5.0

Titrate:
ZZZZ101 - Validation of operators AFFE_CARA_ELEM and POST_ELEM Date:
29/10/01
Author (S):
J.M. PROIX, L. VIVAN Key
:
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\subsection*{3.5 Remarks}

All the values tested are exact.

\subsection*{3.6 Parameters \\ of execution}

Version: 5.2
Machine: SGI/ORIGIN 2000
Obstruction memory: 128 MW
Time CPU To use: 10 seconds

\section*{4 \\ Summary of the results}

The results are equal to the reference solutions and make it possible to validate key word MASS_INER of POST_ELEM.

\author{
Handbook of Validation
}

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HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
ZZZZ102 Lira accélérogramme and spectrum of oscillator
Date:
05/01/98
Author (S):
J.R. LEVESQUE

Key: V1.01.102-A
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

\section*{V1.01 booklet: Tests of validity of orders}

V1.01.102 document
ZZZZ102-To see accélérogramme and spectrum
of oscillator

\section*{Summary:}

This test concerns:
1) reading under various formats of a accélérogramme on an external file (LIRE_FONCTION
[U4.21.08]) and the test of the attributes of the function and the value max,
2) the checking of the calculation of the spectrum of oscillator by CALC_FONCTION [U4.62.04] and of the value of
high frequency acceleration (> 35.5 Hz ) according to the value max of the accélérogramme,
3) reading of a spectrum of oscillators in free format on an external file (LIRE_FONCTION
[U4.21.08]).
There are not grid or of model finite element and two modelings.
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Code_Aster \({ }^{\circledR}\)

Version
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Titrate:
ZZZZ102 Lira accélérogramme and spectrum of oscillator
Date:
05/01/98
Author (S):

\section*{J.R. LEVESQUE}

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1
Problem of reference
Several reading of signals are to be realized:
- reading of a accélérogramme not centered (maximum amplitudes \(>0\) and \(<0\) different)
expressed in \(G(9.81 \mathrm{~ms} 2)\) and research of the maximum amplitude,
- calculation of the spectra of oscillator for a reduced damping \(=0.01(1 \%)\) with the options of calculation:
pseudo absolute acceleration and to find the acceleration of high frequency drive ( \(F^{3} 35.5 \mathrm{~Hz}\) ),
- displacement
relative,
- reading of a accélérogramme centered (LBEW: seism Length Beach East West) and seeks maximum amplitude,
- calculation of the spectrum of oscillator in pseudo absolute acceleration,
- reading of a spectrum of oscillator in free format.

2
Reference solution
2.1

\section*{Results of reference}
- Accélérogramme not centered
\(T=7.935\)
amax \(=0.638\)
\(F=35.5\)
\(=0.01\)
pseudo absolute acceleration
0.638 X \(9.81=6.25878\)
displacement
1.282343105
- Centered Accélérogramme
\(T=3.1\)
\(\operatorname{amax}=1\).
\(F=35.5 \mathrm{~Hz}\)
\(=0.01\)
pseudo absolute acceleration
1.
- Spectre of oscillator
\(F=0.5 \mathrm{~Hz}\)
\(=0.01\)
pseudo absolute acceleration
1.09558
\(=0.05\)
pseudo absolute acceleration
0.657348
= 0.1
pseudo absolute acceleration
0.109558
2.2

Uncertainty on the solution
Analytical solution.

\subsection*{2.3 References \\ bibliographical}
[1]
J.R. LEVESQUE, L. VIVAN, D. SELIGMANN "seismic Response by spectral method" [R4.05.03].
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3 Modeling
With

\subsection*{3.1 Files}
read
- Accélérogramme SEPTEN not centered read on unit 19.
- Accélérogramme centered doseism read on unit 20.
3.2

Test and calculation
- for each signal read:
research of the value max,
calculation of the spectrum of oscillator.
CALC_FONCTION
MAX
SPEC_OSCI
NATURE: "OCCC"
NATURE: "DEPL" or "ACCE"
NORMALIZES: 1./g or 1.
4
Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference
\% tolerance
amplitude max accélérogramme 1
\(T=7.935\)
0.638
0.638
0.
0.1
spectrum of oscillator 1 (not centered)
freq. 35.5 acceleration
6.258778
6.546182
4.59
5.
displacement
1.28234 E5
1.34123 E5
4.59
5.
amplitude max accélérogramme 2
\(T=3.1\)
freq. 35.5
1.0
1.0962
9.6

10

\subsection*{4.2 Remarks}

The precision of integration of the spectrum of oscillator is not satisfactory.
All these attributes of the functions read or created are tested and exact.

\subsection*{4.3 Parameters \\ of execution}

Version: 3.05.02
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
3.34 seconds

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Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
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Date:
05/01/98
Author (S):
J.R. LEVESQUE

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5 Modeling
B

\subsection*{5.1 File}
read
- Spectre of oscillator

7 frequencies

\section*{5.2}

\section*{Extraction of the values}

For \(F R E Q=0.5 \mathrm{~Hz}\)
test of the values associated with the spectrum of oscillator for:
\(=0.01\) and 0.1 (values read)
\(=0.05\) (values to be interpolated linearly)
6
Results of modeling \(A\)

\subsection*{6.1 Values}
extracted
\(F R E Q=0.5 \mathrm{~Hz}\)
Reference
Aster
\% difference
Damping \(=0.01\)
1.09558
1.09558
0.

Damping \(=0.05\)
0.657348
0.657348

\section*{0 .}
\((\) interpolated value \()=0.1\)
0.109558
0.109558

0 .

\subsection*{6.2 Remarks}

All the attributes of the tablecloth read are good.

\subsection*{6.3 Parameters \\ of execution}

Version: 3.05.02
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
1.98 seconds

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
ZZZZ102 Lira accélérogramme and spectrum of oscillator
Date:
05/01/98
Author (S):
J.R. LEVESQUE

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7

\section*{Summary of the results}

The calculation algorithm of the spectrum of oscillator does not give a sufficient precision with the list of frequencies of integration per defect.
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ZZZZ102 Lira accélérogramme and spectrum of oscillator
Date:
05/01/98
Author (S):
J.R. LEVESQUE

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Geometrical ZZZZ106 Criteria in DEFI_GROUP
Date:
01/12/98
Author (S):

\section*{J. PELLET, G. BERTRAND}

Key:
V1.01.106-A Page:
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Organization (S): EDF/IMA/MMN, CISI

\section*{Handbook of Validation}

\section*{V1.01 booklet: Tests of validity of orders}

Document: V1.01.106
ZZZZ106 - Geometrical criteria in
DEFI_GROUP [U4.12.03]

\section*{Summary:}

This test validates the various options of creation of groups of meshs (or nodes) by criteria geometrical in order DEFI_GROUP [U4.12.03]:
. "SPHERE"
- "CYLINDER"
."BAND"
. "FACE_NORMALE"
- "ENV_SPHERE"

\section*{Code_Aster \({ }^{\circledR}\)}

Version
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1
Problem of reference
y
C
B
has
With
has
X
It is about a square plate on side is \(=10\). and thickness \(\mathrm{T}=1\).

\section*{1.1 \\ Material properties}
\(\mathrm{E}=1\).
\(=0.3\)

\section*{1.2}

\section*{Boundary conditions and loading}

The plate is embedded along side AB .
One has 7 loading cases for modeling A and 5 loading cases for modeling B.
Each loading case corresponds to the superposition of 2 loadings which are cancelled.
One of these loadings applies to a GROUP_MA or a GROUP_NO defined starting from a criterion geometrical in DEFI_GROUP, the other, of opposed sign, applies to the GROUP_MA or the GROUP_NO
defined "in extension" (with the hand).
Modeling A uses a model "3D" in hull DKT.
Modeling B uses a model "D_PLAN" in TRIA3.

The grids are the same ones.
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Author (S):

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2
Reference solution
2.1

Results of reference
For all the loading cases, the solution is commonplace.
One must have a field of null displacements in all the nodes.
3 Modeling
With
3.1

\section*{Characteristics of modeling}

The elements are DKT.
One defines 7 loading cases in the following way:
Loading case \(n^{\circ} 1\) : "SPHERE"
y
Tr6b
Tr7a
The side of an element is equal to 2.5 .
21
20
19
18
22
The sphere of ray 2 . and centered with node 1 has an intersection not-vacuum with the element
hatched on the figure, i.e. \(\operatorname{Tr} 6 a, \operatorname{Tr} 6 b, \operatorname{Tr} 7 \mathrm{a}\),

\title{
Loading case n \({ }^{\circ}\) : "CYLINDER"
}
y
Tr6b
Tr7a
The cylinder of ray \(2 \mathrm{~d}^{\prime}\) axis \(\mathbf{Z}\) and passing by
21
20
19
18
node 1 has an intersection not-vacuum with
22
elements hatched on the figure i.e. Tr6a, Tr6b, Tr7a, Tr10b, Tr11a and Tr11b.
8

One applies a pressure equalizes to 1 . on
Tr6a
9
1
5
this list of name GM2 built while using
24
16
the option "ROLLS" CREA_GROUP_MA of Tr11b order DEFI_GROUP and a pressure
2
equalize to 1 . on this list defined in extension.
15
10
11
12
13
14
X
Tr10b Tr11a
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Version
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\section*{Loading case n \({ }^{\circ}\) : "BAND"}
y
Elements of the shaded zone i.e. \(\operatorname{Tr} 5 \mathrm{~A}, \operatorname{Tr} 5 \mathrm{~B}\), 21
20
19
18
Tr6A, Tr6B, Tr7A, Tr7B, Tr8A, Tr8B, Tr9A, Tr9B,
22
Tr10A, Tr10B, Tr11A, Tr11B, Tr12A, Tr12B define
the intersection of the plate with the band of which sides
8
7
6
23
17
are parallel to the axis \(\mathbf{X}\), whose medium passes by
5B
6B
7B
8B
6A
7A
8A
N1 node, and of which the half-width is equal to 2 .
5A
9
1
5
24
16
9B
10B
11B
12B
One applies a pressure equal to 1 . to this zone
9A
10A
11A
12A
defined thus geometrically of name GM3 in

\section*{Loading case n \({ }^{\circ}\) : "ENV_SPHERE"}

\section*{y}

Nodes 3, 5, 7 and 9 are defined as being them
nodes of the grid pertaining to the intersection of
21
20
19
18
plate with the sphere of N1 centre and 2.5
22
(it is the length on the side of an element).
8
7
6
23
17
This list of nodes of name GN1 is defined in using option "ENV_SPHERE" of CREA_GROUP_NO
9
1
5
order DEFI_GROUP.
24
16
One applies a nodal force F
2
3
4
\(\mathrm{Z}=1\) of each one of
25
nodes of this list and a nodal force \(\mathrm{Fz}=1\) in each node of the same list defined in
extension (N3, N5, N7, N9).

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\section*{Loading case n \({ }^{\circ}\) : "ENV_CYLINDRE"}
y
Nodes \(3,5,7\) and 9 are defined as being them nodes of the grid pertaining to the intersection of
plate with the cylinder of axis \(\mathbf{Z}\) passing by node 1
22
and of ray 2.5 .
8
7
6
23
17
This list of nodes of name GN2 is defined in using option "ENV_CYLINDRE" of CREA_GROUP_NO
9
1
5
order DEFI_GROUP.
24
16

One applies a nodal force F
2
3
4
\(Z=1\) of each one of
25
15
nodes of this list and a nodal force \(\mathrm{Fz}=1\) in each node of the same list defined in
extension (N3, N5, N7, N9).
10
11
12
13
14
X
Loading case \({ }^{\circ}\) 7: "PLANE"
y
Nodes 14, 15, 16, 17 and 18 are defined like
21
20
19
18
belonging to the plan passing by the node 14 and of which
22
normal is parallel to \(\mathbf{X}\).
8

23
17
This list of nodes of name GN3 is defined in using the option "PLAN" of CREA_GROUP_NO of 9
\(Z=1\) of each one of
nodes of this list and a nodal force \(\mathrm{Fz}=1\) in each node of the same list defined in extension.
10
11
12
13

X

\section*{3.2}

\section*{Characteristics of the grid}

The grid comprises 32 meshs DKT.

\subsection*{3.3 Functionalities}

\section*{tested}

One tests the following options of creation of group of meshs of order DEFI_GROUP for 3D:
- "SPHERE"
. "CYLINDER"
."BAND"
. "FACE_NORMALE"
and following options of creation of group of nodes of order DEFI_GROUP:
. "ENV_SPHERE"
- "ENV_CYLINDRE"
. "PLANE"
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\section*{4}

Results of modeling A

\subsection*{4.1 Values}
tested

\section*{Identification}

\section*{Reference}

Aster
\% difference
Loading case \(\mathrm{n}^{\circ} 1\) :
DZ (C)
0 .
0.

Loading case \(\mathrm{n}^{\circ} 2\) :
DZ (C)
0.

0 .
Loading case \(\mathrm{n}^{\circ} 3\) :
DZ (C)
0 .
0 .
Loading case \(\mathrm{n}^{\circ} 4\) :
DZ (C)
0 .
0 .
Loading case \(\mathrm{n}^{\circ} 5\) :
DZ (C)
0 .
0.

Loading case \(\mathrm{n}^{\circ} 6\) :
DZ (C)
0.
0.

Loading case \(\mathrm{n}^{\circ} 7\) :
DZ (C)
0 .

\section*{0.}

\subsection*{4.2 Remarks}

The values are tested in absolute and the tolerance is equal to 1. E10.

\subsection*{4.3 Parameters}

\section*{of execution}

Version: 4.01.12
Machine: CRAY C90
Obstruction memory:

\author{
8 MW \\ Time CPU To use: \\ 100 seconds \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-75/98/040 - Ind A
}

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4.0

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Author (S):

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5 Modeling

\section*{B}

\section*{5.1}

\section*{Characteristics of modeling}

The elements are TRIA3 in plane deformation. One defines 5 loading cases in the manner following:

\section*{Loading case \(n^{\circ}\) 1: 'SPHERE"}
y
Tr6b
Tr7a
The side of an element is equal to 2.5 .
21
20
19
18
22
The circle of radius 2 . and centered with node 1 has an intersection not-vacuum with the element
hatched on the figure, i.e. \(\operatorname{Tr} 6 a, \operatorname{Tr} 6 \mathrm{~b}, \operatorname{Tr} 7 \mathrm{a}\),

\title{
Tr10b, Tr11a and Tr11b.
}

Tr6a
9
1
5
24
16
One applies a voluminal force of density
Tr11b
1 according to \(\mathbf{y}\) on this zone of definite name GM1
2
3
4
25
15
by employing the option "SPHERE" of
CREA_GROUP_MA of the order
DEFI_GROUP and a voluminal force
10
11
12
13
14
X
opposed on this zone defined in extension.
Tr10b Tr11a
Loading case \(n^{\circ}\) 2: "BAND"
y
Elements of the shaded zone i.e. \(\operatorname{Tr} 5 \mathrm{~A}, \operatorname{Tr} 5 \mathrm{~B}\),
21
20
19
18
Tr6A, Tr6B, Tr7A, Tr7B, Tr8A, Tr8B, Tr9A, Tr9B, 22
\(\operatorname{Tr} 10 \mathrm{~A}, \operatorname{Tr} 10 \mathrm{~B}, \operatorname{Tr} 11 \mathrm{~A}, \operatorname{Tr} 11 \mathrm{~B}, \operatorname{Tr} 12 \mathrm{~A}, \operatorname{Tr} 12 \mathrm{~B}\) define 8
7
6
the intersection of the plate with the band of which sides 23
17
are parallel to the axis \(\mathbf{X}\), whose medium passes by

N1 node, and of which the half-width is equal to 2 .
5A

One applies a voluminal force of density 1 according to
9A
10A
11A
12A
\(\mathbf{y}\) on this zone defined thus geometrically of
on this same zone defined in extension.
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\section*{Loading case n \({ }^{\circ}\) : "FACE_NORMALE"}
y
The list of the elements of the geometrical type SEG2, S1,
21
20
19
18
\(\mathrm{S} 2, \mathrm{~S} 3, \mathrm{~S} 4\) are defined as the list of the elements of 22
grid perpendiculars with direction \(\mathbf{Y}\).
8
7
6
23
17
One applies a pressure equal to 1 . to this list of geometrically definite name GM4 in the manner

\section*{Loading case n \({ }^{\circ}\) : "ENV_SPHERE"}

\section*{y}

Nodes 3, 5, 7 and 9 are defined as being them
nodes of the grid pertaining to the intersection of
21
20
19
18
plate with the circle of N1 centre and 2.5
22
(it is the length on the side of an element).
8
7
6
23
17
This list of nodes of name GN1 is defined in using option "ENV_SPHERE" of CREA_GROUP_NO

9
1
5
order DEFI_GROUP.
24
16
One applies a nodal force F
2
3
4
\(y=1\) of each one of
nodes of this list and a nodal force Fy \(=1\) in each node of the same list defined in

\section*{extension.}

10
11
12
13
14
X
Loading case n \({ }^{\circ}\) 5: 'PLANE"

\section*{y}

Nodes 14, 15, 16, 17 and 18 are defined like
21
20
19
18
belonging to the right-hand side passing by the node 14 and of which 22
the normal is parallel to \(\mathbf{X}\).
8
7
6
23
17
This list of nodes of name GN3 is defined in using the option "PLAN" of CREA_GROUP_NO of 9
1
5
order GROUP_NO.
24
16
One applies a nodal force F
2
3
4
\(y=1\) of each one of
25
15
nodes of this list and a nodal force \(\mathrm{Fy}=1\) in each node of the same list defined in extension.

\section*{Code_Aster \({ }^{\circledR}\)}

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4.0

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Author (S):

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5.2

\section*{Characteristics of the grid}

The grid comprises 32 meshs TRIA3 and 4 meshs SEG2.

\subsection*{5.3 Functionalities}

\section*{tested}

One tests the following options of creation of group of meshs of order DEFI_GROUP for 2D:
. "SPHERE"
. "BAND"
. "FACE_NORMALE"
and following options of creation of group of nodes of order DEFI_GROUP for the 2D:
. "ENV_SPHERE"

\author{
. "PLANE"
}

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6
Results of modeling B

\subsection*{6.1 Values}
tested
Identification

\section*{Reference}

Aster
\% difference
Loading case \(\mathrm{n}^{\circ} 1\) :
DY (C)
0 .

\section*{0.}

Loading case \(\mathrm{n}^{\circ} 2\) :
DY (C)
0 .
0.

Loading case \(\mathrm{n}^{\circ} 3\) :
DY (C)
0.
0.

Loading case \(\mathrm{n}^{\circ} 4\) :
DY (C)
0.
0.

Loading case \(\mathrm{n}^{\circ} 5\) :
DY (C)
0.

\section*{0.}

\subsection*{6.2 Remarks}

The values are tested in absolute and the tolerance is equal to 1. E10.
6.3 Parameters

\section*{of execution}

Version: 4.01.12
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
100 seconds
7

\section*{Summary of the results}

The results are good: the groups calculated by order DEFI_GROUP are well the groups waited.
Attention however with the fact that the test " 3 D " is actually a test on a plate in plan XOY:
role of the 3rd co-ordinate in FORTRAN is thus not tested.
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07/12/01
Author (S):
C. CHAVANT, Key Mr. LAINET
:
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Organization (S): EDF/MTI/MMN, CS IF

\title{
Handbook of Validation
}

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\section*{ZZZZ111 - Validation of operator DEFI_CABLE_BP}

\section*{Summary:}

The goal of this case-test is to validate the operator DEFI_CABLE_BP [U4.23.06], who calculates the profiles of tension
in the cables of prestressed of a structure of concrete, in accordance with the rules of the BPEL: those allow to take account of the losses of tension per contact between the cables and the concrete, by retreat with
anchorings, by shrinking and creep of the concrete and relieving of steel, material constituting the cables.

The structure considered is a semi-cylindrical veil, containing in its thickness four cables of prestressed. The cables describe each one a half-circle in a horizontal plane, and thus traverse the veil over its length. Two cables have an eccentricity compared to the average radius of the veil.

The results obtained are validated by comparison with those theoretically awaited. Profiles of tension can be clarified analytically out of the zones where the losses by retreat apply to anchorings.
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

The concrete veil has a semi-cylindrical form: the height is \(H=10 \mathrm{~m}\) and the average radius is worth \(R=10 \mathrm{Mr}\).
The thickness of the veil is worth \(E=0,6\) Mr. the "average radius equivalent", within the meaning of the BPEL on the section
vertical of the veil is worth thus \(\mathbf{r m}=\mathbf{0 , 2 8 3} \mathbf{m}\), knowing that:
eH
\(r m=\)
(ref. BPEL 2.1, 5)
\(2(E+H)\)
The cables describe each one a half-circle in a horizontal plane, and thus traverse the veil on its length. The dimensions of the plans containing the cables are:
for the cable \(n^{\circ} 1: z 1=1 m\);
for the cable \(n^{\circ} 2: z 2=3,5 \mathrm{~m}\);
for the cable \(n^{\circ} 3: z 3=6 \mathrm{~m}\);
for the cable \(n^{\circ} 4: z 4=8,5 \mathrm{Mr}\).
Cables 3 and 4 have an eccentricity compared to the average radius of the veil, being worth respectively:
\(e x 3=0,05 \mathrm{~m} ;\)
\(e x 4=0,1 \mathrm{Mr}\).
The surface of the cross-section of each cable is worth Its \(=1,5.104 \mathrm{~m} 2\).
1.2

Properties of materials

\subsection*{1.2.1 Material concrete constituting the veil}

Elastic properties:
Young modulus
\(E b=3.1010 \mathrm{~Pa}\)
Poisson's ratio b=0,2
Parameters characteristic for estimate of the losses of tension:
Standard rate of loss of tension by creep of the concrete
xflu \(=0,07\)
Standard rate of loss of tension by shrinking of the concrete xret \(=0,08\)

\subsection*{1.2.2 Material steel constituting the cables}

Elastic properties:

\section*{Young modulus}
\(E a=2,1.1011 \mathrm{~Pa}\)
Poisson's ratio has \(=0,3\)
Parameters characteristic for estimate of the losses of tension:
Relieving of steel at 1000 hours
\(1000=2 \%\)
Adimensional coefficient of relieving of prestressed steel
\(\mu=0,3\)
0
Stress ultimate elastic steel
F prg = 1,77.109 Pa
Coefficient of friction in curve
F = 0,2 rad1
Loss ratio of tension per unit of length
\(=3.10-3 \mathrm{ml}\)
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\subsection*{1.3 Loading}

One applies at the two ends of each cable a normal effort of traction. The value of the tension applied \(=2.105 \mathrm{NR}\) is \(\mathrm{F0}\).

To evaluate the losses of tension by relieving of the cables in time, the relations are used following:
pj
\(p(X) R(J)\)
(ref. BPEL 3.3, 24)
6
(X)
\[
\begin{aligned}
& = \\
& p i \\
& -\mu
\end{aligned}
\]
(ref. BPEL 3.3, 23)
\(p\)
0 pi \((X)\)
1001000 F prg

\section*{\(J\)}
\(\boldsymbol{R}(J)=\)
(ref. BPEL 3.3, 24 and 2.1, 51)
\(J+9 \times r m\)
pi (X) called initial tension, the tension at the point of \(X\)-coordinate \(X\), after losses of tension instantaneous.

The characteristics are evaluated at the \(D-d a y=10\).
To evaluate the losses of tension in the vicinity of anchorings, one takes account of a retreat with anchorings \(=5.104 \mathrm{Mr}\).

Note:

This problem disregards resolution of the balance of the structure supplements steel concrete and is limited to the determination according to BPEL'S of prestressed in the cables.
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2
Reference solution

\section*{2.1}

Curvilinear \(X\)-coordinate and cumulated angular deviation
The cables describe each one a trajectory in the form of half-circle in a horizontal plane. In consequence, the curvilinear \(X\)-coordinate \(S\) and the cumulated angular deviation are expressed very simply:
\(S=R c\)
\(=\)
where Rc indicates the ray of the half-circle described by the cable, and the azimuth in cylindrical coordinates.
The values of reference for the tests are estimated using these expressions.

\section*{2.2}

Normal effort in the cables
One considers a cable describing a horizontal half-circle of Rc ray. The 0 locating azimuth is noted end of the zone where the losses of tension by retreat apply to the first anchoring; - 0 reference mark it
beginning of the zone where the losses of tension by retreat apply to the second anchoring. Taking into account the preceding expressions of the curvilinear \(\boldsymbol{X}\)-coordinate and angular deviation cumulated, the profile of tension along the cable can be parameterized by the azimuth, out of the zones where
apply the losses by retreat to anchorings:
\(F()=-F 0(X f l u+x r e t)\)
5
\(F 01+R(J) \times\)
\(x\)
\(\mu\)
```

5

```
\(F\)
0
0
\(F 0 \times R(J) \times\)
\begin{tabular}{l}
\(\times\) \\
\(\times\) \\
\hline
\end{tabular}
1000
ex
\(p-(\)
2 F + Rc)) on interval 0;
100
S F
has
2
prg
\(F()=-F 0(X f l u+x r e t)\)
5
\(+\)
\(F 01+R(J) \times\)
\(\times\)
\(\mu\)
1000
0 ex (
\(p\)
(F
Rc) (
))
\(+\)

The values of reference for the tests are estimated using these expressions, which define one symmetrical profile of tension compared to the central node.

\section*{2.3}

Index of projection
The projection of a node pertaining to the one of the cables on a mesh of the concrete veil gives place to the assignment of an index of projection IPROJ in accordance with the following rule:

Projection on a mesh triangle of nodes N1 tops, N2 and N3:

IPROJ \(=0\)
if the projected point is inside the triangle;
\(I P R O J=11,12\) or 13 if the projected point belongs respectively to the edge [N1; N2], [N2; N3] or [N3; N1];

IPROJ \(=2\)
if there is coincidence of the point projected with a node top.
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Projection on a mesh quadrangle of nodes N1 tops, N2 N3 and N4:

\section*{IPROJ = 0}
if the projected point is inside the quadrangle;
IPROJ = 11, 12, 13 or 14 if the projected point belongs respectively to the edge [N1; N2], [N2; N3], [N3; N4] or [N4; N1];

IPROJ \(=2\)
if there is coincidence of the point projected with a node top.
The values of reference for the tests are estimated by predicting the places of projection of the cables, taking into account their situation compared to the veil and provision of the meshs on this one.

\subsection*{2.4 Eccentricity}

The eccentricity of a node pertaining to the one of the cables is defined as the distance from this node with
net concrete veil on which it is projected.
Seen in a horizontal plane, the trace of the mesh is a cord on the half-circle of ray R. One notes the angular sector covered by the mesh. The node of the cable, noted NC, is located on the half-circle of Rc ray. Its relative position compared to the mesh is located by the azimuth.

NC

R
Rc

The vector cos
; sin

\section*{2}

2 is normal with the cord, which passes by the point (R;0).

The equation of the cord is thus cos
\(X+\sin\)
\(y-R \cos\)
\(=0\)
2
2
2
The distance from a point on a line, in the plan, is given by:
```

ax0+by+C
D=
0
a2+b2

```
where \((x 0 ; y 0)\) is the co-ordinates of the point and \(a x+b y+c=0\) is the equation of the right-hand side.
Node NC pertaining to the cable has as co-ordinates (Rc cos; Rc sin). Its eccentricity by report/ratio with the mesh of the veil on which it is projected is thus worth:
\(e x c=R c \cos\)
\(R\)
2
- cos
2
The values of reference for the tests are estimated using this expression.
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The concrete veil is represented by elements DKT, supported by meshs quadrangles to 4 nodes: one counts 10 meshs on a vertical generator and 32 on a horizontal half-circle. With this provision, the meshs have dimensions close to those of a square of 1 m on side.

A thickness \(E=0,6 \mathrm{~m}\) is assigned to all the meshs of the veil, as well as a material concrete for which are defined behaviors ELAS and BPEL_BETON: the parameters take the values data previously in paragraph [§ 2.2.1].

Each cable is represented by 128 elements MECA_BARRE, supported by meshs segments with 2 nodes. On a horizontal half-circle, one thus counts 4 times more meshs on one cable that on the concrete veil.

A surface of cross-section Its \(=1,5.104 \mathrm{~m} 2\) is assigned to all the meshs of the cables, like one material steel for which behaviors ELAS and BPEL_ACIER are defined: parameters take the values given previously in paragraph [§ 2.2.2].

The F0 tension \(=2.105 \mathrm{NR}\) is applied to the two nodes ends of each cable. The value of this tension is coherent with the values of section and of stress ultimate elastic, for cables of prestressed toronés. The evaluation of the losses of tension by relieving and retreat with anchorings is carried out in accordance with the rules of the BPEL; the parameters take the values given previously in paragraph [§ 2.2.3].

As one applies the same tension at the two ends of each cable, and as one imposes the same retreat with anchorings, the profiles of tension obtained must be symmetrical compared to nodes mediums of the cables.

Taking into account the geometrical characteristics and grid, nodes of the cables \(n^{\circ} 1\) and \(n^{\circ} 2\) project on the nodes tops and the edges of the meshs of the concrete veil. For the nodes of these two cables, the indices of projection obtained must be in conformity with the following sequence: 2 for the first node, then 13-13-13-2 until the last node.

The nodes of the cables \(n^{\circ} 3\) and \(n^{\circ} 4\) are projected as for them on the edges and inside the meshs of veil concrete. For the nodes of these two cables, the indices of projection obtained must be in conformity with the following sequence: 14 for the first node, then 0-0-0-12 until the last node.

The rule of assignment of the index of projection is previously defined in paragraph [§ 3.3].
Taking into account the characteristics of the grid, the eccentricities of the nodes of the cables are evaluated

3
using the expression of the paragraph [\$ 3.4] with \(=\) and
\(=0\)

\subsection*{3.2 Functionalities}
tested
The functionalities which one wishes to validate are as follows:
operator DEFI_MATERIAU [U4.23.01]
: definition of the relations of behavior
BPEL_BETON and BPEL_ACIER, if all possible losses of tension
are taken into account;
operator DEFI_CABLE_BP [U4.23.06]: calculation of the profiles of tension along the cables of
prestressed; projection of the nodes of the cables on the meshs of the concrete veil,
preliminary with the calculation of the coefficients of the relations kinematics between the DDL of the
nodes
cables and DDL of the nodes "close" to the concrete veil.
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values tested}

\subsection*{4.1.1 X-coordinate \\ curvilinear}

The tolerance of relative variation compared to the reference is worth \(0,1 \%\).

\section*{Node}

Value of reference

\author{
Code_Aster
}

Relative variation
NC001032
7,608545.100 m
7,6085416350199.100 m
-4,42.10-7 \%
NC001033
7,853982.100 m
7,8539785609573.100 m
-4,38.10-7 \%
NC001034
8,099419.100 m
8,0994154868947.100 m
-4,34.10-7 \%
NC001064
1,546253.101 m
1,5462523265015.101 m
-4,36.10-7 \%
NC001065
1,570796.101 m
1,5707960190953.101 m
1,22.10-8 \%
NC001066
1,595340.101 m
1,5953397116890.101 m
-1,81.10-7 \%
NC001096
2,331651.101 m
2,3316504895010.101 m
-2,19.10-7 \%
NC001097
2,356194.101 m
2,3561941820946.101 m

7,73.10-8 \%
NC001098
2,380738.101 m
2,3807378746885.101 m \(-5,26.10-8\) \%

\section*{NC002032}

7,608545.100 m
7,6085416350199.100 m
-4,42.10-7 \%
NC002033
7,853982.100 m
7,8539785609573.100 m
-4,38.10-7 \%
NC002034
8,099419.100 m
8,0994154868947.100 m
-4,34.10-7 \%
NC002064
1,546253.101 m
1,5462523265015.101 m
-4,36.10-7 \%
NC002065
1,570796.101 m
1,5707960190953.101 m
1,22.10-8 \%
NC002066
1,595340.101 m
1,5953397116890.101 m
-1,81.10-7 \%
NC002096
2,331651.101 m
2,3316504895010.101 m
-2,19.10-7 \%
NC002097
2,356194.101 m
2,3561941820946.101 m
7,73.10-8 \%
NC002098
2,380738.101 m
2,3807378746885.101 m

\section*{NC003032}

7,646587.100 m 7,6465843431956.100 m -3,47.10-7 \%
NC003033
7,893252.100 m
7,8932484537622.100 m
-4,49.10-7 \%
NC003034
8,139916.100 m
8,1399125643288.100 m
-4,22.10-7 \%
NC003064
1,553984.101 m
1,5539835881341.101 m
-2,65.10-7 \%
NC003065
1,578650.101 m
1,5786499991908.101 m
\(-5,13.10-10 \%\)
NC003066
1,603317.101 m
1,6033164102475.101 m
-3,68.10-7 \%
NC003096
2,343309.101 m
2,3433087419485.101 m
-1,10.10-7 \%
NC003097
2,367975.101 m
2,3679751530052.101 m
6,46.10-8 \%
NC003098
2,392642.101 m
2,3926415640619.101 m -1,82.10-7 \%
NC004032
7,684630.100 m
7,6846270513697.100 m
-3,84.10-7 \%
NC004033
7,932521.100 m
7,9325183465668.100 m
-3,35.10-7 \%
NC004034
8,180413.100 m
8,1804096417640.100 m
-4,11.10-7 \%
NC004064
1,561715.101 m
1,5617148497666.101 m
-9,62.10-8 \%
NC004065
1,586504.101 m
1,5865039792862.101 m
-1,31.10-8 \%
NC004066
1,611293.101 m
1,6112931088059.101 m
6,75.10-8 \%
NC004096
2,354967.101 m
2,3549669943960.101 m
-2,38.10-9 \%
NC004097
2,379756.101 m
2,3797561239157.101 m
5,21.10-8 \%
NC004098
2,404546.101 m
2,4045452534354.101 m
-3,10.10-7 \%
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\subsection*{4.1.2 Cumulated angular deviation}

The tolerance of relative variation compared to the reference is worth \(1 \%\).

\author{
Node \\ Value of reference \\ Code_Aster \\ Relative variation \\ NC001032 \\ 7,608545.101 rad \\ 7,5505221851277.101 rad \\ -0,763 \% \\ NC001033 \\ 7,853982.101 rad \\ 7,7959596039949.101 rad \\ -0,739 \% \\ NC001034 \\ 8,099419.101 rad \\ 8,0413970228620.101 rad \\ -0,716 \% \\ NC001064 \\ 1,546253.100 rad \\ 1,5404519589725.100 rad \\ -0,375 \% \\ NC001065 \\ 1,570796.100 rad \\ 1,5649957008614.100 rad \\ -0,369 \% \\ NC001066 \\ 1,595340.100 rad
}

\title{
1,5895394427503.100 rad
}
-0,364 \%
NC001096
2,331651.100 rad
2,3258516994343.100 rad -0,249 \%
NC001097
2,356194.100 rad
2,3503954413232.100 rad
-0,246 \%
NC001098
2,380738.100 rad
2,3749391832123.100 rad -0,244 \%

\section*{NC002032}

7,608545.101 rad
7,5505221851277.101 rad
-0,763 \%
NC002033
7,853982.101 rad
7,7959596039949.101 rad
-0,739 \%
NC002034
8,099419.101 rad
8,0413970228620.101 rad
-0,716 \%
NC002064
1,546253.100 rad
1,5404519589725.100 rad
-0,375 \%
NC002065
1,570796.100 rad
1,5649957008614.100 rad
-0,369 \%
NC002066
1,595340.100 rad
1,5895394427503.100 rad
-0,364 \%
NC002096
2,331651.100 rad

\author{
NC003032 \\ 7,608545.101 rad \\ 7,5505221853611.101 rad -0,763 \% \\ NC003033 \\ 7,853982.101 rad \\ 7,7959596042577.101 rad -0,739 \% \\ NC003034 \\ 8,099419.101 rad \\ 8,0413970231543.101 rad \\ -0,716 \% \\ NC003064 \\ 1,546253.100 rad \\ 1,5404519589969.100 rad \\ -0,375 \% \\ NC003065 \\ 1,570796.100 rad \\ 1,5649957008877.100 rad \\ -0,369 \% \\ NC003066 \\ 1,595340.100 rad \\ 1,5895394427786.100 rad \\ -0,364 \% \\ NC003096 \\ 2,331651.100 rad \\ 2,3258516994599.100 rad \\ -0,249 \% \\ NC003097 \\ 2,356194.100 rad
}

2,3503954413473.100 rad -0,246 \%
NC003098
2,380738.100 rad
2,3749391832400.100 rad -0,244 \%

NC004032
7,608545.101 rad
7,5505221850191.101 rad -0,763 \%
NC004033
7,853982.101 rad
7,7959596039041.101 rad -0,739 \%
NC004034
8,099419.101 rad
8,0413970227892.101 rad
-0,716 \%
NC004064
1,546253.100 rad
1,5404519589583.100 rad
-0,375 \%
NC004065
1,570796.100 rad
1,5649957008524.100 rad
-0,369 \%
NC004066
1,595340.100 rad
1,5895394427464.100 rad
-0,364 \%
NC004096
2,331651.100 rad
2,3258516994251.100 rad
-0,249 \%
NC004097
2,356194.100 rad
2,3503954413144.100 rad
-0,246 \%
NC004098
2,380738.100 rad

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\subsection*{4.1.3 Normal effort in the cables}

The tolerance of relative variation compared to the reference is worth 0,5\%.

\author{
Node \\ Value of reference \\ Code_Aster \\ Relative variation \\ NC001032 \\ 1,334446.105 NR \\ 1,3362458647761.105 NR \\ 0,135 \% \\ NC001033 \\ 1,325720.105 NR \\ 1,3275099211945.105 NR \\ 0,135 \% \\ NC001034 \\ 1,317036.105 NR \\ 1,3188178238811.105 NR \\ 0,135 \% \\ NC001064 \\ 1,076002.105 NR
}

\author{
1,0775306667470.105 NR \\ 0,142 \% \\ NC001065 \\ 1,068586.105 NR \\ 1,0701071214644.105 NR \\ 0,142 \% \\ NC001066 \\ 1,076002.105 NR \\ 1,0775306667471.105 NR \\ 0,142 \% \\ NC001096 \\ 1,317036.105 NR \\ 1,3188178238804.105 NR \\ 0,135 \% \\ NC001097 \\ 1,325720.105 NR \\ 1,3275099211945.105 NR \\ 0,135 \% \\ NC001098 \\ 1,334446.105 NR \\ 1,3362458647768.105 NR \\ 0,135 \% \\ NC002032 \\ 1,334446.105 NR \\ 1,3362458647761.105 NR \\ 0,135 \% \\ NC002033 \\ 1,325720.105 NR \\ 1,3275099211945.105 NR \\ 0,135 \% \\ NC002034 \\ 1,317036.105 NR \\ 1,3188178238811.105 NR \\ 0,135 \% \\ NC002064 \\ 1,076002.105 NR \\ 1,0775306667470.105 NR \\ 0,142 \% \\ NC002065 \\ 1,068586.105 NR
}

\author{
1,0701071214644.105 NR \\ 0,142 \% \\ NC002066 \\ 1,076002.105 NR \\ 1,0775306667471.105 NR \\ 0,142 \% \\ NC002096 \\ 1,317036.105 NR \\ 1,3188178238804.105 NR \\ 0,135 \% \\ NC002097 \\ 1,325720.105 NR \\ 1,3275099211945.105 NR \\ 0,135 \% \\ NC002098 \\ 1,334446.105 NR \\ 1,3362458647768.105 NR \\ 0,135 \%
}

NC003032
1,334270.105 NR
1,3360688116680.105 NR
0,135 \%
NC003033
1,325538.105 NR
1,3273280739561.105 NR
0,135 \%
NC003034
1,316850.105 NR
1,3186312358105.105 NR
0,135 \%
NC003064
1,075696.105 NR
1,0772249041795.105 NR
0,142 \%
NC003065
1,068278.105 NR
1,0697980987005.105 NR
0,142 \%
NC003066
1,075696.105 NR

\author{
NC004032 \\ 1,334093.105 NR \\ 1,3358917765746.105 NR \\ 0,135 \% \\ NC004033 \\ 1,325356.105 NR \\ 1,3271462458317.105 NR \\ 0,135 \% \\ NC004034 \\ 1,316664.105 NR \\ 1,3184446679805.105 NR \\ 0,135 \% \\ NC004064 \\ 1,075391.105 NR \\ 1,0769192061838.105 NR \\ 0,142 \% \\ NC004065 \\ 1,067969.105 NR \\ 1,0694891422717.105 NR \\ 0,142 \% \\ NC004066 \\ 1,075391.105 NR \\ 1,0769192061839.105 NR \\ 0,142 \% \\ NC004096 \\ 1,316664.105 NR
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ111 - Validation of operator DEFI_CABLE_BP
Date:
07/12/01
Author (S):
C. CHAVANT, Key Mr. LAINET

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\subsection*{4.1.4 Index of projection}
Node Value
waited_Code_Aster
NC001032
1313
NC001033
22
NC001034
1313
NC001064
1313
NC001065
22
NC001066
1313
NC001096
1313
NC001097
22
NC001098
1313

1313

NC002032
00
NC002033

1313
NC003064
1313
NC003065
22
NC003066
1313
NC003096
1313
NC003097
22
NC003098
1313

NC004032
00
NC004033
1212
NC004034

00
NC004096
00
NC004097
1212
NC004098
00
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
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Titrate:
ZZZZ111 - Validation of operator DEFI_CABLE_BP
Date:
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C. CHAVANT, Key Mr. LAINET
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\subsection*{4.1.5 Eccentricity}

The tolerance of relative variation compared to the reference is worth 0,1\%.
Node
Value of reference
Code_Aster
Relative variation
NC001032
9,033625.103 m
9,0336249114191.103 m

\section*{NC001033}

00

\author{
NC001034 \\ 9,033625.103 m \\ 9,0336249114209.103 m
}
-9,81.10-9 \%
NC001064
9,033625.103 m
9,0336249102569.103 m -9,93.10-9 \%
NC001065
00

\author{
NC001066 \\ 9,033625.103 m \\ 9,0336249102566.103 m \\ -9,93.10-9 \% \\ NC001096 \\ 9,033625.103 m \\ 9,0336249114191.103 m \\ -9,81.10-9 \% \\ NC001097 \\ 00 \\ NC001098 \\ 9,033625.103 m \\ 9,0336249114209.103 m \\ -9,81.10-9 \%
}

\author{
NC002032 \\ 9,033625.103 m \\ 9,0336249114191.103 m \\ -9,81.10-9 \% \\ NC002033 \\ 00 \\ NC002034 \\ 9,033625.103 m \\ 9,0336249114209.103 m
}

NC002064

\section*{9,033625.103 m}

9,0336249102569.103 m
-9,93.10-9 \%
NC002065
00
NC002066
9,033625.103 m
9,0336249102566.103 m -9,93.10-9 \%

\section*{NC002096}

9,033625.103 m
9,0336249114191.103 m
-9,81.10-9 \%
NC002097
00

\section*{NC002098}

9,033625.103 m
9,0336249114209.103 m
-9,81.10-9 \%

\section*{NC003032}

5,901857.102 m
5,9018565845884.102 m
-7,04.10-8 \%

\section*{NC003033}
5.102 m

5,00000000000957.102 m

\author{
1,91.10-11 \%
}

NC003034
5,901857.102 m
5,9018565845884.102 m
-7,04.10-8 \%
NC003064
5,901857.102 m
5,9018565848231.102 m
\(-7,03.10-8\) \%
NC003065
5.102 m

4,9999999999955.102 m -9,02.10-13 \%
NC003066
5,901857.102 m
5,9018565848231.102 m
-7,03.10-8 \%
NC003096
5,901857.102 m
5,9018565845216.102 m -7,04.10-8 \%
NC003097
5.102 m

5,0000000000957.102 m
1,91.10-11 \%
NC003098
5,901857.102 m
5,9018565845884.102 m
-7,04.10-8 \%

\section*{NC004032}

1,090035.101 m
1,0900350678046.101 m
6,22.10-8 \%
NC004033
101 m
1,0000000000047.101 m
4,68.10-12 \%
NC004034
1,090035.101 m
1,0900350678046.101 m
6,22.10-8 \%
NC004064
1,090035.101 m
1,0900350677825.101 m
6,22.10-8 \%
NC004065
101 m
1,0000000000002.101 m
2,31.10-13 \%
NC004066

101 m
1,0000000000047.101 m
4,68.10-12 \%
NC004098
1,090035.101 m
1,0900350678046.101 m
6,22.10-8 \%
Handbook of Validation
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\subsection*{4.2 Remarks}

One clearly obtains relative variations for the cumulated angular deviation of an order of magnitude higher than those obtained for the curvilinear \(\boldsymbol{X}\)-coordinate, which is calculated with very good precision.
The cause is inherent in the method of interpolation spline cubic used to interpolate them trajectories of the cables. This method consists in interpolating the trajectory on each subinterval by a polynomial of order 3, by guaranteeing the continuity of the derived first and the derivative seconds at the boundaries of the subintervals. The interpolation of a circular trajectory by
polynomials of order 3 is in is an approximation, of which the effects are more sensitive on the derivative
seconds that on the derivative first.
This is why precision obtained for the curvilinear \(X\)-coordinate, calculated using the derivative first of the trajectory, is definitely better than that obtained for the angular deviation cumulated, calculated using products crossed between the derivative first and the derivative second. To improve the precision on the cumulated angular deviation, it would appreciably be necessary to refine it
grid: this step is not valid a priori, because it would lead to overcosts of calculation very consequent for a weak profit of precision.

One obtains indeed symmetrical profiles of tension compared to the nodes mediums of the cables. The variations by defect on the curvilinear \(\boldsymbol{X}\)-coordinate and the cumulated angular deviation induce variations
by excess on the calculated values of tension, but the precision remains satisfactory: this validates method of calculation.

The indices of projection obtained are in conformity with those awaited, and the eccentricities are calculated with a very good precision: this validates the operations of projection.

\subsection*{4.3 Parameters \\ of execution}

Version: 5.07.01

Machine: SGI/ORIGIN 2000 R12000
System:
Obstruction memory:
64 Mo
Time CPU To use: 14 seconds

\section*{5}

Summary of the results
The results obtained are validated by comparison with those theoretically awaited with one good precision.

The particular functionalities tested are as follows:
operator DEFI_MATERIAU [U4.23.01]: definition of the parameters characteristic of materials steel and concrete allowing the calculation of the tension along the cables of prestressed, in accordance with the rules of the BPEL;
operator DEFI_CABLE_BP [U4.23.06]: calculation of the tension along the cables; projection nodes of the cables on the meshs representing the structure of concrete, preliminary with calculation of the coefficients of the relations kinematics between the DDL of the nodes of the cables and them
DDL of the nodes "close" to the structure of concrete.

\section*{Handbook of Validation}

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ112-Roll under variable pressure. Validation of the operator
Date:
05/09/99
Author (S):
Key J.M.PROIX
:
V1.01.112-A Page:
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Organization (S): EDF/IMA/MMN

\section*{ZZZZ112-Roll under variable pressure. \\ Validation of LIRE_PLEXUS}

\section*{Summary:}

The purpose of this test is to validate the operation of order LIRE_PLEXUS. This one makes it possible to read
fields of pressures calculated by PLEXUS on a telegraphic grid, and to apply these pressures to a grid composed of hulls or telegraphic elements.

This test has an analytical solution: it is about a cylinder subjected to a pressure which varies linearly along its axis.

Two modelings are proposed: the cylinder is with a grid in elements DKT (modeling A) or in elements COQUE_3D (modeling B).

The results differ from the reference solution only by the lack of refinement of the grid of hulls (in particular for modeling A).
This test thus validates order LIRE_PLEXUS.
Handbook of Validation
V1.01 booklet: Tests of validity of the orders
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
ZZZZ1 12 - Roll under variable pressure. Validation of the operator
Date:
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\subsection*{1.1 Geometry}

Roll of axis \(X\), length L, average radius \(R\), thickness \(E\).
\(R\)
\(C\)
\(X\)
\(D\)
With
\(B\)
\(X=0\)
\(X=L\)

Here, \(L=1 m, R=0.1 m, e=0.01 m\).

\section*{1.2}

Material properties

\section*{Linear elasticity:}

Young modulus: \(E=21011\)

\section*{Pa}

Poisson's ratio \(=03\)

\section*{1.3}

Boundary conditions and loadings
The grid in hulls is connected to elements of beams in \(A\) and \(B\). This makes it possible to give boundary conditions compatible with the kinematics of beam. However, One is interested here only with the solution of hulls subjected to an internal pressure.
The point \(D\) is embedded.
The pressure is read on a grid "plexus" of segment AB, comprising 20 elements of beams.

It varies in the following way:
```

X
P=10.1-
Pa

```
\(L\)
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

ZZZZ112-Roll under variable pressure. Validation of the operator
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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution analytical:

If the pressure varies like:
```

X
P=-P.1+-

```
```

L
0

```
then the circumferential component of the tensor of constraints is worth:

\section*{PR \\ \(X\) \\ \(=0\) \\ . 1 +}
L
and the field of displacement is worth:
PR
0
\(X\)
\(U=-\)
. X 1 + -
X
E E
\(2 L\)
PR2
0
\begin{tabular}{c}
\(X\) \\
\(U\) \\
\hline \\
\(R\) \\
\(E\) \\
\(E\) \\
\(p\)
\end{tabular}

\section*{2.2}

Results of Reference
Here, \(=0\).
p
\(\boldsymbol{X} \boldsymbol{U}(\boldsymbol{m}) \boldsymbol{U}(\boldsymbol{m})\)

\section*{R}
\(X\)
(Pa)
0 5d-11
0
100
1 0.7.5d-11
0

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References}
bibiographic
[1]
PILKEY W.D.: "Formulated for stress, strain and structural matrices". Wiley \& Sounds, New York, 1994.
Handbook of Validation
V1.01 booklet: Tests of validity of the orders
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5.0

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ZZZZ112-Roll under variable pressure. Validation of the operator
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```

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```

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling \\ Modeling DKT and POU_D_E}

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 339 \\ A number of meshs and types: 395 QUAD4 \\ 7 SEG2
}

\subsection*{3.3 Functionalities}
tested

\author{
Orders
}

\author{
Keys \\ LIRE_PLEXUS
}

\author{
[U4.xx.xx] \\ AFFE_CHAR_MECA LIAISON_ELEM \\ OPTION \\ COQ_POU \\ [U4.25.01]
}

4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\author{
Identification X Reference Aster \% difference \\ 0100 \\ 97.13 \\ 2.9 \\ U \\ 0 5.D-11 \\ 4.84D-11 \\ 3.1 \\ R \\ 4.2 Parameters \\ of execution
}

Version: 5.2

\section*{Machine: SGI/ORIGIN 2000}

\section*{Obstruction memory:}

\author{
128 Mo
}

Time CPU To use:
4 seconds
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ112-Roll under variable pressure. Validation of the operator
Date:
05/09/99
Author (S):
Key J.M.PROIX
:
V1.01.112-A Page:

\section*{5 Modeling B}

\section*{5.1 \\ Characteristics of modeling}

Modeling COQUE_3D and POU_D_E

\section*{5.2}

Characteristics of the grid
A number of nodes: 429
A number of meshs and types: 145 QUAD9
7 SEG2

\subsection*{5.3 Functionalities}
tested
Orders

Keys
LIRE_PLEXUS
[U4.xx.xx]
AFFE_CHAR_MECA LIAISON_ELEM
OPTION
COQ_POU
[U4.25.01]

6
Results of modeling B
6.1 Values
tested

Identification X Reference Aster \%

\title{
6.2 Parameters
}
of execution
Version: 5.2

Machine: SGI/ORIGIN 2000

Obstruction memory:
128 Mo
Time CPU To use:
4 seconds
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ112-Roll under variable pressure. Validation of the operator
Date:
05/09/99
Author (S):
Key J.M.PROIX
:
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7 Modeling
C
7.1
Characteristics of modeling
Modeling COQUE_3D, PIPE and POU_D_T
A half of cylinder \((0<X<L / 2)\) is with a grid in elements of hulls, other half is with a grid inelements PIPE.
7.2Characteristics of the grid
A number of nodes: 436
A number of meshs and types: 100 QUAD9 (modeling COQUE_3D), 5 SEG3 (modeling PIPE),4 SEG2 (modeling POU_D_T)
7.3 Functionalities
tested
Orders
KeysLIRE_PLEXUS
[U4.xx.xx]
AFFE_MODELE AFFE
MODELING
PIPE
[U4.2201]
AFFE_CHAR_MECA LIAISON_ELEM
OPTION
COQ_TUYAU
[U4.25.01]
8
Results of modeling \(C\)

\subsection*{8.1 Values}
tested

\author{
Identification X Reference Aster \% difference \\ 0100 \\ 95 \\ 5 \\ \(U\) \\ 0 5.D-11 \\ \(4.85 \mathrm{D}-11\) \\ 3.0 \\ R \\ 8.2 Parameters \\ of execution \\ Version: 5.2
}

Machine: SGI/ORIGIN 2000

Obstruction memory:
128 Mo
Time CPU To use:
6 seconds

\section*{9}

Summary of the results
The results differ from the reference solution only by the lack of refinement of the grid from hulls (in particular for modeling A).

This test thus validates order LIRE_PLEXUS.

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY

Key: V1.01.113-A Page:
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Organization (S): EDF/MTI/MMN, AUSY

Handbook of Validation
V1.01 booklet: Tests of validity of orders
V1.01.113 document

ZZZZ113 - Chainings 3D (R, théta) and 2D (R, Z)
CYRANO3-Code_Aster

\section*{Summary:}

This test makes it possible to validate nonthe regression of the tool for chaining C3ASTER between codes CYRANO3 and
Code_Aster. Two-dimensional modeling represents the unit pelletizes sheath in a plan (R, Z) and
three-dimensional modeling is contingent on a structure made up of two fragments of pastille and an edge
of sheath of fuel pin. In three-dimensional modeling, the chaining makes it possible to model one slice fuel pin in the plan (R, théta) with a \(0,1 \mathrm{~mm}\) thickness in order to simulate the efforts axial. The pellet-sheath spaces inter-fragments and play are modelled by elements 3D of contact-friction.

The loadings and the initial conditions come from a base of results CYRANO3. The chaining provides in Code_Aster of the files of grid and data of orders.

The versions considered here are:
- CYRANO3, version 2.6,
- C3ASTER, version 3.0.

The file of validation for this test is to be begun again since values of not-regression changed for modelings has and C since version 5.4.8 compared to the establishment of origin per Mr. Bonnamy in version
5.1.8. The values of origin of version 5.1.8 are indicated in reference.

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ1 13 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/1 1/01
Author (S):
P. MASSIN, Mr. BONNAMY

Key: V1.01.113-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(\mathbf{Y}\)
\(\mathbf{p}\)
\(\mathbf{J}\)
\(\mathbf{Z}\)
p
O
X
dtrou/2
diap/2
rgext-rgint

\author{
rev \\ dchamf/2 \\ hchamf \\ devap/2 \\ hev \\ hp/2 \\ PELLETIZES \\ PLAY \\ SHEATH \\ diap/2 \\ dtrou/2
}
- diap (diameter pelletizes): 4,0975 Misters.
- dtrou (central bore): 0 Misters.
- rgint (interior ray sheaths): 4,18 Misters.
\(\cdot\) rgext (external ray sheaths): 4,75 Misters.
- \(\mathbf{p}\) (half angle of the fragments): \(\mathbf{2 2 , 5}\) degrees.
- J (angle between fragments): \(\mathbf{2 . 1 0 5}\) degrees.
\(\cdot\) height H following Z: 0,1 Misters.
- hev (depth of obviously): \(\mathbf{0 , 3} \mathbf{~ m m}\)
- rev (spherical ray obviously): \(\mathbf{1 4 , 6 4} \mathbf{~ m m}\)
- devap (apparent diameter obviously): \(\mathbf{5 , 9} \mathbf{~ m m}\)
- dchamf (diameter chamfer): \(\mathbf{0 , 0 0} \mathbf{~ m m}\)
- hchamf (height chamfer): \(\mathbf{0 , 0 0} \mathbf{~ m m}\)

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\section*{Titrate:}

ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
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P. MASSIN, Mr. BONNAMY

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\section*{1.2}

Material properties
E (T), (T), (T) for the combustible pastille, coming from CYRANO3
E (T), (T), (T) for the sheath, coming from CYRANO3.

\section*{1.3}

Boundary conditions and loadings

\subsection*{1.3.1 Loadings}

Loading coming from calculation CYRANO3 (version 2.6) for the same moment (not of time number
101 , urgent \(t=25,0564\) S.) on the case-test called "test1c3", transcribed in orders Aster by

C3ASTER, version 3.0:
- internal pressure pi on the interior edge of the sheath,
- internal pressure pi on the edge external of the pastille,
- external pressure EP on the edge external of the sheath,
\(\cdot\) surface forces according to \(\mathrm{Z}, \mathrm{FP}\) and Fg , equivalent to axial stresses zz imposed on
fragments of pastille and sheath,
- field of temperature \(\mathbf{T}\) to radial dependence, decreasing of the interior towards outside.
- field of anelastic deformations year, read in the form of dataset IDEAS.

For modeling A, one adds:
- internal pressure pi on the edges of the zone inter-fragments.

\subsection*{1.3.2 Boundary conditions:}

For modeling a:
- Normal Déplacement no one at the edges \(=2 \mathrm{p}+\mathrm{j}\) and \(=0\),
- Déplacement following \(\mathbf{Z}\) null on the \(\mathbf{Z}=0\) face,
- Uniform Déplacements for the whole of the nodes of the fragments of pastille in \(\mathrm{z}=\mathrm{h}\),
- Uniform Déplacements for the whole of the nodes of the sheath in \(\mathrm{z}=\mathrm{h}\),
- Point \(O\) blocked with rigid connections with the crown of central elements of which displacement normal is imposed on zero.
Unilateral contact
\(\mathbf{Y}\)
with friction
Simple support
External pressure
Internal pressure
\(\mathbf{X}\)
Simple support

Modeling a: Seen top ( \(\mathbf{z}=\mathbf{h}\) )
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Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
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\section*{Z}

Constraint sig
Constraint sig
Z (T)
Z (T)
H
Pi
EP
Pelletizes
Sheath
X
Plane modeling a: of cut (=0)
For modeling b:
- Conditions of symmetry compared to axis \(\mathbf{X}\) and axisymetry around the axis \(\mathbf{Y}\)
pressure interns pint (T)
Y
thermal deformations ( \(\mathbf{X}, \mathbf{T}\) )
imposed axial stress SIGz (T)
anelastic deformations ( \(\mathbf{X}, \mathbf{T}\) )
external pressure pext (T)
Dx = 0 or
PELLETIZES
SHEATH
imposed pressure
if central hole
X
\(\mathrm{Dy}=0\) (cond. of symmetry)
conditions of unilateral contact

Plane modeling b: (R, Z)

\subsection*{1.4 Conditions}
initial
Field 3D of constraints, displacements and variables intern at the moment \(\mathbf{T}=\mathbf{2 5 , 0 4 5 2 8} \mathbf{S}\). coming from calculation CYRANO3, read in the form of dataset IDEAS.

\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
No the reference solution.
2.2

Results of reference
No results of reference.
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V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
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Titrate:
ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
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3 Modeling
With

\section*{3.1}

Characteristics of modeling

Elements of volume
Names of the nodes: Not \(\mathbf{P 2}=\mathbf{N} 308\) (node supported on Ona+1)
Not P3 = N288 (node resulting from the rotation of Ona+1 of the angle =p)
Not P4 = N264 (node supported on Ona+2)
Not P6 = N44 (node supported on Ona+ngn+2)
Not P7 = N244 (node resulting from the rotation of Ona+2 of the angle \(=\mathbf{p}\) )
Not P9 = N288 (node resulting from the rotation of Ona+ngn+2 of the angle =p)
sheath
p
J
Z
2nd fragment
\(\mathbf{p}\)
\(\mathbf{H}\)
X
Y
0
Ona+ngn+2
0
na+2
na+1
001
1st fragment
- De O1 with Ona+1: 16 elements (fragment of pastille)
- De Ona+1 with Ona+2: 1 element (zone of pellet-sheath contact)
- De Ona+2 with Ona+ngn+2: 5 elements (sheath)
- 10 elements in circumference in each fragment (p),
- 1 element in the zone inter-fragments ( I ) (zone of contact between fragments)
- 1 following element (Z).

\subsection*{3.2 Characteristics}
grid

A number of nodes:
1013
A number of meshs and type: 461 HEXA8

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ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
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Author (S):
P. MASSIN, Mr. BONNAMY

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3.3 Functionalities
tested
Orders

\section*{LIRE_RESU "IDEAS"}

AFFE_CHAM_NO "TEMP_R"

\section*{AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FACE_IMPO \\ GROUP_MA \\ DNOR \\ LIAISON_UNIF \\ GROUP_MA \\ TEMP_CALCULEE}

\section*{EPSA_CALCULEE}

\author{
PRES_REP \\ GROUP_MA \\ FORCE_FACE \\ GROUP_MA \\ AFFE_MATERIAU GROUP_MA \\ "MECHANICAL" AFFE_MODELE "3D" \\ PASTTOT \\ GAINTOT \\ "MECHANICAL" \\ "3D_CONTACT" \\ JEUTOT \\ BCTFUELT \\ DEFI_MATERIAU ELAS_FO
}

\section*{LEMAITRE}

ZIRC_CYRA2

\section*{CONTACT}

E_N

E_T

\section*{COULOMB}

\author{
STAT_NON_LINE
}

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY

Key: V1.01.113-A Page:
7/12

Results of modeling A

\subsection*{4.1 Values}
tested

\section*{Standard identification}
of
Aster Reference
value
P2 node
ux 2.759
10-2 2.198
10-2
P4 node
ux 4.863
10-3 4.863
10-3
P6 node
ux 6.284
10-3 6.284
10-3
P3 node
ur 2.865
10-2 2.211
10-2
P7 node
ur 4.824

\title{
P6 node
}
P3 node
vmis
366.14514 .7
P7 node
vmis
60.860 .8
P9 node
vmis
40.240 .2

\subsection*{4.2 Remarks}

The results observed to ensure itself of nonthe regression of the code are:
- Ur displacements at the points P2, P4, P6 and P3, P7, P9, - constraints equivalent \(v m i\) to the points \(\mathrm{P} 2, \mathrm{P} 4, \mathrm{P} 6\) and \(\mathrm{P} 3, \mathrm{P} 7, \mathrm{P} 9\).

Contents of the file results: Displacements with the nodes, constraints with the nodes

\subsection*{4.3 Parameters}
of execution
Version: 5.01.07

\section*{Machine}
: Origin 2000
CLASTERObstruction memory:
128 Megabytes
Time CPU To use:
145.7 seconds
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY
Key: V1.01.113-A Page:8/12
5 Modeling
B
5.1
Characteristics of modeling
Elements plans QUAD4, representation in a plan \((\mathbf{R}, \mathbf{Z})\) of the unit pelletizes combustible andsheath:
- 10 elements axially according to (Z),
- Radial Maillage resulting from CYRANO3: 10 elements in the pastille and 5 in the sheath.

\subsection*{5.2 Characteristics}
grid
A number of nodes:

\section*{187}

A number of meshs and type:
150 QUAD4
5.3 Functionalities
tested
Orders

LIRE_RESU "IDEAS"

AFFE_CHAM_NO "TEMP_R"
"DEPL_R"

\author{
AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ LIAISON_UNIL_NO \\ GROUP_MA
}

TEMP_CALCULEE

EPSA_CALCULEE

PRES_REP
GROUP_MA
FORCE_CONTOUR
GROUP_MA
FY
AFFE_MATERIAU GROUP_MA

\author{
"MECHANICAL" AFFE_MODELE "AXIS" \\ DEFI_MATERIAU ELAS_FO
}

\section*{LEMAITRE}

\section*{ZIRC_CYRA2}

\section*{STAT_NON_LINE}

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY

Key: V1.01.113-A Page:
9/12

6
Results of modeling \(B\)

\subsection*{6.1 Values}
tested

\section*{Identification}

Type of value
Aster
N77 node
ux 2.350
10-2
N66 node
ux 4.796
10-3
N41 node
ux 6.217
10-3
N67 node
ur 2.335
10-2

\author{
N5 node ur 4.798 \\ 10-3 \\ N1 node \\ ur 6.219 \\ 10-3 \\ N77 node \\ vmis \\ 395.12 \\ N66 node \\ vmis \\ 59.32 \\ N41 node \\ vmis \\ 38.80 \\ N67 node \\ vmis \\ 390.0 \\ N5 node \\ vmis \\ 59.28 \\ N1 node \\ vmis \\ 38.78
}

\subsection*{6.2 Remarks}

The results observed to ensure itself of nonthe regression of the code are:
- Ux displacement and constraint equivalent vmis to the N77 points (end pelletizes), \(\mathbf{N 6 6}\) (face intern sheaths) and \(\mathbf{N} 41\) (external face sheaths) for \(\mathrm{Y}=0\).
- Ux displacement and constraint equivalent vmis to the N67 points (end pelletizes), N5 (face intern sheaths) and \(\mathbf{N} 1\) (external face sheaths) for \(\mathbf{Y}=\mathbf{h p} / 2\).

Contents of the file results: Displacements with the nodes, constraints with the nodes

\subsection*{6.3 Parameters}
of execution
Version: 5.03.01

\title{
Machine \\ : Origin 2000
}

\author{
CLASTER \\ Obstruction memory: \\ 32 Megabytes \\ Time CPU To use: \\ 5.98 seconds \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-75/01/010/A \\ Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0
}

\section*{Titrate:}

ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY

Key: V1.01.113-A Page:
10/12

\section*{7 Modeling}

C

\section*{7.1}

Characteristics of modeling

\section*{Identical to modeling A}

\subsection*{7.2 Characteristics}
grid
A number of nodes:
1013
A number of meshs and type:
461 HEXA8

\subsection*{7.3 Functionalities \\ tested}

Orders

LIRE_RESU "IDEAS"

\author{
AFFE_CHAM_NO "TEMP_R" \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FACE_IMPO \\ GROUP_MA \\ DNOR \\ LIAISON_UNIF \\ GROUP_MA \\ LIAISON_UNIL_NO \\ GROUP_MA_1 \\ BCONT \\ GROUP_MA_2 \\ BCONTS \\ TEMP_CALCULEE
}

EPSA_CALCULEE

PRES_REP
GROUP_MA
FORCE_FACE
GROUP_MA

\section*{AFFE_MATERIAU GROUP_MA}

\author{
"MECHANICAL" AFFE_MODELE "3D" PASTTOT \\ GAINTOT \\ "MECHANICAL"
}

\title{
DEFI_MATERIAU ELAS_FO
}

\section*{LEMAITRE}

\section*{ZIRC_CYRA2}

\author{
CONTACT \\ E_N \\ E_T \\ \section*{COULOMB} \\ \section*{STAT_NON_LINE}
}

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-75/01/010/A
}
```

Code_Aster ${ }^{\circledR}$
Version
5.0
Titrate:
ZZZZ113 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY
Key: V1.01.113-A Page:
11/12

```
8
Results of modeling C

\subsection*{8.1 Values}
tested
Standard identification

of
Aster Reference
initial
value
P2 node
ux 2.759
10-2 2.198
10-2
P4 node
ux 4.863
10-3 4.863
10-3
P6 node
ux 6.284
10-3 6.284
10-3
P3 node
ur 2.865
10-2 2.211
10-2
P7 node
ur 4.824
10-3 4.824
10-3
P9 node
ur 6.245
10-3 6.245
10-3
P2 node
vmis
359.96495 .4
P4 node
vmis
60.560 .5
P6 node
vmis
40.540 .5
P3 node
vmis
366.14514 .7
P7 node

\subsection*{8.2 Remarks}

The results observed to ensure itself of nonthe regression of the code are:
- Ur displacements at the points P2, P4, P6 and P3, P7, P9,
\(\cdot\) constraints equivalent vmis to the points \(\mathrm{P} 2, \mathrm{P} 4, \mathrm{P} 6\) and \(\mathrm{P} 3, \mathrm{P} 7, \mathrm{P} 9\).
Contents of the file results: Displacements with the nodes, constraints with the nodes

\subsection*{8.3 Parameters} of execution

Version: 5.01.07

Machine
: Origin 2000

\section*{CLASTER}

Obstruction memory:
128 Megabytes
Time CPU To use:
105.8 seconds

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

ZZZZ1 13 - Chaining 3D (R, théta) and 2D (R, Z) CYRANO3-Code_Aster
Date:
07/11/01
Author (S):
P. MASSIN, Mr. BONNAMY

Key: V1.01.113-A Page:
12/12

\section*{9 \\ Summary of the results}

Not-regression. The file of validation for this test is to be begun again since values of not-regression changed for modelings has and C since its establishment of origin by
Mr. Bonnamy in version 5.4.8. The values of origin of version 5.1.8 are indicated in reference.
One can nevertheless suppose that the new values obtained are better insofar as
the results on the constraints between modelings has and B or C and B approach. But that remains to check.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ118 - Test of nonregression INCLUDE_MATERIAU
Date
:
16/1 1/01
Author (S):
Key A.M. DONORE
:
V1.01.118-A Page:
1/6
Organization (S): EDF/MTI/MMN

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ Document: V1.01.118 \\ \section*{ZZZZ118 - Test of nonregression INCLUDE_MATERIAU}
}

\section*{Summary:}

The purpose of this test is to check the correct operation of the order INCLUDE_MATERIAU (which carries out connection between the Catalogue Materials Aster and Code_Aster). It is a test of nonregression. Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ118 - Test of nonregression INCLUDE_MATERIAU
Date
:

\author{
Author (S): \\ Key A.M. DONORE
}

\author{
: \\ V1.01.118-A Page: \\ 2/6 \\ 1 \\ Problem of reference \\ \subsection*{1.1 Geometry}
}

Thin rectangular plate thickness 0.01 m (see test HSLS01B).

\author{
1.2 \\ Material properties
}

\section*{Young modulus:}

\section*{\(T\left({ }^{\circ} C\right)\)}
0.
20.
50.
100. 150.
400. 450. 500. 550. 600.

E (T) (109 Pa) 205. 204. 203. 200. 197.
193.
189. 185.
180.
176. 171. 166. 160. 155.

Poisson's ratio: \(=0.3\)
Definite dilation coefficient average compared to \(20^{\circ} \mathrm{C}\) :
\(\boldsymbol{T}\left({ }^{\circ} \mathrm{C}\right)\)
20.
50.
100.
150.
200.
250.

\section*{1.3}

Boundary conditions and loadings

\section*{Embedded thin section: GROUP_NO: (ABBCCD DA)}
\(D X=D Y=D Z=D R X=D R Y=D R Z=0\).
Heat gradient in the thickness:
temp
=
AFFE_CHAM_NO
(
\(N O M \_C M P=\_F\)
("TEMP", "TEMP_INF", "TEMP_SUP"),
VALE_R

A pressure distributed on the meshs of the grid
```

$P R E S \_R E P=-F\left(G R O U P \_M A=' S T O T ', P R E S=-100\right)$,
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

```
Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
ZZZZ118 - Test of nonregression INCLUDE_MATERIAU
Date
:
16/11/01
Author (S):
Key A.M. DONORE
:
V1.01.118-A Page:
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\section*{2}
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
One makes initially a linear thermomechanical calculation (MECA_STATIQUE) by defining material in
the command file (DEFI_FONCTION and DEFI_MATERIAU), which gives the values of reference. Then one uses the values of the characteristics materials available in the Catalogue Materials with order INCLUDE_MATERIAU (NOM_AFNOR: "I8MND5" TYPE_MODELE: "Ref."
ALTERNATIVE: "A" TYPE_VALE: "NOMI") and one remake linear thermomechanical calculation. Results of
these two calculations must be identical (the goal is not the validation of the order MECA_STATIQUE).

Results of reference
Taking into account the loading, the reference solution in constraints is uniform on all the structure, one tests the value of fields SIXX, SIYY

\section*{2.3}

Uncertainty on the solution
Case test of nonregression ==> exact solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Case test HSLS01B.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ118 - Test of nonregression INCLUDE_MATERIAU
Date
:
16/11/01
Author (S):
Key A.M. DONORE
:
V1.01.118-A Page:
4/6

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ The structure is modelled in elements DKT, DST and Q4.}

\subsection*{3.2 Characteristics}
grid
A number of nodes: 675
A number of meshs and types: 236 SEG2, 624 TRIA3, 312 QUAD4

\author{
3.3 Functionalities \\ tested
}

Orders

\section*{INCLUDE_MATERIAU}

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

Identification Reference Aster
\% difference
SIXX
\(1.62356342360 E+001.62356342360 E+00\)
0.000

SIYY
\(2.2097781405 E+002.2097781405 E+00\)
0.000

\subsection*{4.2 Notice}

The precision is excellent.

\subsection*{4.3 Parameters \\ of execution}

Version: 5.02

Machine: SGI - ORIGIN 2000-R12000

Obstruction memory:
64 megawords
Time CPU To use: 9.73 seconds
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ118 - Test of nonregression INCLUDE_MATERIAU
Date
:

16/11/01
Author (S):
Key A.M. DONORE
:
V1.01.118-A Page:
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5 Modeling
B
5.1

Characteristics of modeling
The structure is modelled in elements C_PLAN

\subsection*{5.2 Characteristics}
grid
A number of nodes: 675
A number of meshs and types: 236 SEG2, 624 TRIA3, 312 QUAD4

\subsection*{5.3 Functionalities}
tested

Orders
Key word
INCLUDE_MATERIAU EXTRACTION

\section*{6}

Results of modeling B

\author{
6.1 Values
}
tested

\section*{Identification Reference Aster}
\% difference
SIXX
\(-2.694857+08-2.694857+08\)
0.000

SIYY
-2.694857+08
\(-2.694857+08\)
0.000

\subsection*{6.2 Notice}

The precision is excellent.

\subsection*{6.3 Parameters \\ of execution}

Version: 6.00

Machine: SGI - ORIGIN 2000-R12000

Obstruction memory:
64 megawords
Time CPU To use: 4.71 seconds
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ118 - Test of nonregression INCLUDE_MATERIAU
Date
:

16/11/01
Author (S):
Key A.M. DONORE
:
V1.01.118-A Page:
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\section*{7}

Summary of the results
The found results are in conformity for two modelings.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:

Date:
09/02/00
Author (S):
Key J.PELLET
:
V1.01.119-A Page:
1/4
Organization (S): EDF/MTI/MMN

Handbook of Validation
V1.01 booklet: Data-processing tests
Document: V1.01.119

ZZZZ119-PROJ_CHAMP for surfaces in 3D

\section*{Summary:}

One treats the case of a quarter of cylinder (modelled by hulls DKQ) subjected to a pressure interns on one
first grid ma1. The field of displacement obtained (ch1) is supposed just. One projects this field of displacement on another grid (ma2) of the same quarter of cylinder. Ch2 is obtained. One Re-projects ch2 on
ma1, one obtains ch1bis. The result (ch1bis) is close to ch1 (2\% of difference). This validates the method of

\title{
projection of a field of a surface on the other.
}

Handbook of Validation
V1.10 booklet: Data-processing tests
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

ZZZZ119 - PROJ_CHAMP for HULLS

Date:
09/02/00
Author (S):
Key J.PELLET
:
V1.01.119-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

A cylindrical hull (1/4 of cylinder) height \(H=40\) and ray \(R=10\) and thickness \(E=0.2\) is embedded on its line of centers and subjected to a pressure \(p\) interns \(=2\).

It is with a grid twice:
ma1: \(7 \times 14\)
QUAD4
ma2: 8 X 16 X 2
TRIA6
ma1 ma2

\section*{1.2}

\section*{Material properties}
\(E=1.106\)
\(=0.3\)

\section*{1.3}

Boundary conditions, loading
The group of nodes AB1 is embedded (three displacements and three rotations are blocked).
\(D X=D Y=D Z=0\).
\(D R X=D R Y=D R Z=0\).
The hull is subjected to a pressure interns \(p=2\).
Handbook of Validation
V1.10 booklet: Data-processing tests
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ119 - PROJ_CHAMP for HULLS

Date:
09/02/00
Author (S):
Key J.PELLET
:
V1.01.119-A Page:
3/4

\subsection*{1.4 Functionalities \\ validated}

Orders

Keys
PROJ_CHAMP VIS_A_VIS
GROUP_MA_1
[U4.53.05]
METHOD
"ELEM"

The validation relates to method "ELEM in the case of" the surface meshs plunged in R3 space. One also tests key word VIS_A_VIS for method "ELEM".

\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
For the field ch1, calculated on the grid ma1, the reference solution is obtained with Code_Aster (version 5.02.07).

For the field ch1bis obtained by a projection "return ticket" (ma1-> ma2-> ma1), the solution of reference is the field ch1.

One thus measures the error due to 2 successive projections of fields: ch1-> ch2-> ch1bis

\section*{3 Values}

Tested

Field
Node
CMP
Reference
Found value
Difference (\%)
CH1 CH1BIS
CH1BIS N123

\author{
DX \\ 2.17555E01 2.14000E01 \\ -1.634 \\ CH1BIS N123 \\ DY \\ 3.96143E01 3.99026E01 0.728 \\ CH1BIS N48 DX \\ 9.35364E02 9.18094E02 \\ -1.846 \\ CH1BIS N48 DY \\ 1.90742E01 1.93265E01 1.323 \\ CH1BIS N66 DX \\ 2.17555E01 2.14000E01 \\ -1.634
}

\title{
3.1 \\ Parameters of execution (last observation)
}

Version: 5.02

Machine: SGI-ORIGIN 2000-R12000

Obstruction memory:
64 Mo
Time CPU to use:
5.3 S

\section*{Handbook of Validation}

V1.10 booklet: Data-processing tests
HT-66/02/001/A

Titrate:
ZZZZ1 19 - PROJ_CHAMP for HULLS

Date:
09/02/00
Author (S):
Key J.PELLET
V1.01.119-A Page:
\(4 / 4\)

\section*{4 \\ Summary of the results}

This case test makes it possible to check the correct operation of order PROJ_CHAMP for surfaces in \(3 D\).
Handbook of Validation
V1.10 booklet: Data-processing tests
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
ZZZZ121 - Adaptation of grid with LOBSTER

Date:
01/09/05
Author (S):
G. Key NICOLAS

V1.01.121-C Page:
1/20
Organization (S): EDF R \& D /SINETICS

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ Document: V1.01.121
}

ZZZZ121 - Adaptation of grid with LOBSTER

\section*{Summary}

This series of case-tests validates by means of computer the adaptation of grid with LOBSTER. On a grid
simple, either in 2D, or in 3D, a static calculation of mechanics is launched, with production of an indicator
errors. From there, a call to the software LOBSTER will involve a modification of the grid. On it new grid, a new calculation of static mechanics is activated, corresponding to the same problem physics.
These case-tests are not examples of the interest of the adaptation of grid. They are used only as tests of
not-regression of the functionality in the various possible configurations.

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of orders
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

\section*{Titrate: \\ ZZZZ121 - Adaptation of grid with LOBSTER}
Date:
01/09/05
Author (S):
G. Key NICOLAS
:
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2.1.2 The three-dimensional case of modeling \(\boldsymbol{B}\) ..... 6
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Titrate:
ZZZZ121 - Adaptation of grid with LOBSTER
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G. Key NICOLAS
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Code_Aster \({ }^{\circledR}\)
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\section*{1 Context}

The objective is only to test the not-regression of the future evolutions of ASTER and LOBSTER.
Even if the case-tests are realistic from the physical point of view for representing studies well real, one should not attach importance to the value of the results.
In particular, one should not anything deduce some as for the performance indicating couple from error adaptation of grid.

These case-tests validate the operation of two macro orders MACR_INFO_MAIL and MACR_ADAP_MAIL which control the whole of the process.

More precisely, the functionalities tested are as follows:

\title{
readings and writings of grid and fields to format MED. They are the orders
}

IMPR_RESU, LIRE_CHAMP and LIRE_MAILLAGE with key word MED like format,
writing of the data file for LOBSTER: order IMPR_FICO_HOMA,
launching of the procedure managing the LOBSTER execution. It is the order EXEC_LOGICIEL; it calls a script with a variable number of arguments,
piloting of the whole of the process by the python: macr_adap_mail_ops.py.
The process is a priori insensitive with modeling considered. The two important points which cause different treatments in the data exchange between LOBSTER and Code_Aster are the types of elements and the update of fields on the new grid. We consider thus 4 modelings which are distributed as follows:

\author{
Modeling \\ Dimension \\ Update of fields \\ With 2D \\ triangles \\ Not \\ B 3D \\ tetrahedrons \\ Not \\ C 2D \\ triangles \\ Yes \\ D 2D \\ quadrangles \\ Not
}

The not-regression is tested on the value of the field of displacement, constraint or temperature in a free node. The test takes place for two resolutions, those with the grids resulting from the first and
of another adaptation. Indeed, the data transmissions and LOBSTER piloting are not the same ones for the first adaptation and the following ones. At least two passages thus should be tested.

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\section*{2}

Problem of reference

\subsection*{2.1 Geometry}
2.1.1 The two-dimensional case of modelings \(A\) and \(C\)

The two-dimensional case is a square on side unit. It is divided into two parts according to the diagonal
of equation \(X+y=1\).
\(J(0 ; 1)\)
OPPOSE (1; 1)
Half 2
Half 1
ORIGIN (0; 0)
I (1; 0)

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\subsection*{2.1.2 The three-dimensional case of modeling \(\boldsymbol{B}\)}

The three-dimensional case is a cube on side unit. It is divided into two parts according to the diagonal plan
of equation \(X+y=1\).

\section*{Z}
\(K(0 ; 0 ; 1)\)
HALF 1
OPPOSE (1; 1; 1)
ORIGIN (0; 0; 0)
\(I(1 ; 0 ; 0)\)
\(J(0 ; 1 ; 0)\)
\(X\)
\(y\)
HALF 2

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\subsection*{2.1.3 The two-dimensional case of modeling \(D\)}

This case is a copy of case-test SSLV111.
\[
\begin{aligned}
& B(0 ; 4) \\
& C(4 ; 4) \\
& =/ 3 \\
& T o(0 ; 1) \\
& =/ 6 \\
& D(4 ; 0) \\
& E(1 ; 0)
\end{aligned}
\]

\section*{2.2}

Properties of material
Two distinct materials are used. This difference makes it possible to make sure that the under-fields are well reconstituted after the adaptation of the grid.

\section*{Material 1}
:

E: 180.000 Pa
\[
=0,3 \mathrm{IF}
\]
\[
=1,5105 \mathrm{IF}
\]
\[
=7.700 \mathrm{~kg} . \mathrm{m} 3
\]
\(=400 \mathrm{~W} . \mathrm{K} 1 \mathrm{ml}\)
Material 2

In each case of modelings \(A, B\) and \(C\), material 1 is affected with half 1 and material 2 is affected with half 2. In the case of modeling \(D\), the field is homogeneous and is made up material 1.
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\section*{2.3}

Boundary conditions and changes

\subsection*{2.3.1 The two-dimensional case of modelings \(A\) and \(C\)}

\section*{Imposed displacement:}

The part is blocked in translation and rotation around the origin.
Segment ORIGINE_J: DNOR = 0
ORIGIN: DY = 0
Mechanical loading:
One applies a pressure to the higher edge.
Segment J_OPPOSE: CLOSE =-1000.
For a thermomechanical calculation:
The temperature is imposed at the origin:
ORIGIN: TEMP = 200.
Thermal loading:
One applies a convectif exchange with outside to the flat rim.
Segment I-OPPOSE: COEFF_H \(^{2}=500 . T E M P \_E X T=310\).
A voluminal source is applied:
HALF 1: SOUR = 18000
HALF 2: \(\mathrm{SOUR}=22000\)
2.3.2 The three-dimensional case of modeling \(B\)

\section*{Imposed displacement:}

The part is blocked in translation and rotation around the origin.
ORIGIN: \(D X=D Y=D Z=0\)
\(I: D Y=D Z=0\)
\(J: D Z=0\)
Mechanical loading:
Account of gravity is taken.
GRAVITY \(=9,81\) according to \(\mathbf{O Z}\)
One applies a surface force equal to the weight of the cube, opposite higher.
FORCE_FACE \(=78.480\) according to \(O Z\)

\subsection*{2.3.3 The two-dimensional case of modeling \(D\)}

To represent symmetries, the part is blocked in translation on the edges of cut:
Vertical edge AB: DX = 0
Horizontal edge ED: DY = 0
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\section*{Mechanical loading:}

\section*{F}
\(=(X=4)\)
\(F\)
\(X\)
\(X\)
\(X=x y(y=4)\)
\(X\)
\(\boldsymbol{x x}\)

\section*{On CD tractions}

On BC tractions
F
\(=(X=4)\)

\section*{F}
\[
y=y y(y=4) .
\]

These constraints are calculated by the analytical solution, described in the document [V3.04.111].

\subsection*{2.4 Conditions initial \\ Purely mechanical calculations are stationary in time.}

Thermomechanical calculation is transitory, with adaptation of grid all the 2 steps of time. very first thermal calculation is initialized by a stationary calculation. Following thermal calculations are initialized by the field of temperature obtained with the calculation preceding and updated on the new one
grid.

\section*{3 \\ Reference solution}

\section*{3.1}

Method of calculation used for the reference solution
These case-tests are case-tests of nonregression. The reference solution is that obtained with a Code_Aster calculation.
3.2

Results of reference

\subsection*{3.2.1 Modelings A, B and C}

Mechanical a: in 2D:

After adaptation 4
After adaptation 5

\section*{DX}

5,093375 10-3 5,0939774
10-3
DY
After adaptation 1
After adaptation 2
DX
2,66755 10-2 9,195670
10-2
DY
1,658803 10-1 3,511732
10-1
DZ
-6,575036 10-2 -2,510371
10-1

The results are the same ones after adaptation 2 and 3 bus the 3rd adaptation does not change it grid.

\section*{C: thermomechanical in 2D:}

\author{
After adaptation 1 \\ After adaptation 2 \\ TEMP \\ 3,097832 10-2 3,113811 \\ 102 \\ DX \\ 1,267359 10-2 1,271608 \\ 10-2 \\ DY \\ - 2,511801 10-2 \\ - 2,715298 10-2 \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/05/005/A
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\subsection*{3.2.2 Modeling \\ D}

The reference is established on the value of the stress field on the two nodes placed on the arc intern, with the angles 30 and 60 degrees. The test is made in nonregression after the first and after second adaptation. One compares with the analytical solution after the third adaptation.

\section*{Node 30 degrees \\ Node 60 degrees}

\section*{Adaptation 1}
8.364656 10-1 1.576383

Adaptation 2
5.125268 10-1 1.911567

Adaptation 3
-1,110223 10-16 1,500000
One also tests on the node with 30 degrees, the projection of the field of displacement between 1st and 2nd adaptation: \(U x=7,733106\).

\section*{3.3 \\ Uncertainties on the solution}

Without object.

\subsection*{3.4 References}

\section*{bibliographical}

See the document [V3.04.111] for the analytical solution of modeling D.
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\section*{4 Modeling \\ With}

\section*{4.1 \\ Characteristics of modeling}

Modeling A tests the coupling between LOBSTER and Code_Aster for a problem of mechanics statics in dimension 2. It is checked that the groups of nodes are preserved at identical and that them groups of elements are regenerated.

\section*{4.2 \\ Characteristics of the grid}

The structure is with a grid in "British flag", with elements TRIA6. The edges are with a grid in SEG3. One notes on the sketch the number of under-field allotted to each triangle.
\(y\)

2
2

1
2
1
2
1
1
ORIGIN (0; 0)
I (1; 0)
\(X\)
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\subsection*{4.3 Functionalities}
tested
Orders Key word
factor
Key words
MACR_INFO_MAIL
VERSION_HOMARD
GRID
QUALITY

5

Results of modeling \(A\)

\subsection*{5.1 Values}
tested
After adaptation \(n^{\circ} 1\) :
Identification Moments
Reference
Code_Aster \%
difference
DX
15,093375
10-3 5,093375
10-3-1,45

\title{
After adaptation \(n^{\circ} 2\) :
}

Identification Moments
Reference
Aster \%
difference
DX
15,0939774
10-3 5,0939774
10-3 1,17
10-7
DY
1 -3,158715
10-2 -3,158715
10-2 -4,63
10-6

\subsection*{5.2 Remarks}

One can note that total sizes (masses, centre of gravity,...) are well preserved by the process of adjustment.

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\section*{6 Modeling \\ B}

\section*{6.1}

Characteristics of modeling
Modeling B tests the coupling between LOBSTER and Code_Aster for a mechanical problem statics in dimension 3. It is checked that the groups of nodes are preserved at identical and that them groups of elements are regenerated.

\section*{6.2}

Characteristics of the grid
The cube is cut out in six tetrahedrons of the type TETR10. The technique is as follows. It is initially half-compartment in two prisms by the diagonal plan of equation \(X=y=1\). That corresponds to the two halves defined in the problem. Each prism is then divided into three tetrahedrons.

The edges of the cube are with a grid in triangle TRIAG.
```

Z
K (0; 0; 1)
OPPOSE (1; 1; 1)
ORIGIN (0; 0; 0)
I (1;0;0)
J (0;1;0)
X
y

```
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\subsection*{6.3 Functionalities}
tested
Orders Key word
factor
Key words
MACR_INFO_MAIL
GRID
QUALITY
INTERPENETRATION
CONNEXITY
CUT
NUMBERS
MACR_ADAP_MAIL
LANGUAGE
QUALITY
INTERPENETRATION
ADAPTATION
FREE
UNIFORM
MAILLAGE_N

\author{
NIVE_MIN \\ MAJ_CHAM \\ CHAM_GD \\ TYPE_CHAM \\ NOM_CHAM \\ RESULT \\ NUME_ORDRE \\ CHAM_MAJ \\ 7 \\ Results of modeling B
}
7.1 Values
tested
After adaptation \(\boldsymbol{n}^{\circ} \mathbf{1}\) :
Identification Moments Reference
Code_Aster \%
difference
DX
1 2,66755
10-2 2,66755
10-2 -8,74
```

10-5
DX
1 1,658803
10-1 1,658803
10-1 1,52
10-5
DZ
1-6,575036
10-2 -6,575036
10-2 2,28
10-6

```
After adaptation \(\boldsymbol{n}^{\circ} \mathbf{2 :}\)
Identification Moments Reference
Aster \%
difference
DX
19,195670
10-2 9,195671
10-2 6,47
10-6
DX
1 3,511732
10-1 3,511732
10-1 1,87
10-6
DY
1 -2,510371
10-1-2,510371
10-1 1,12
10-5

\subsection*{7.2 Remarks}

One can note that total sizes (masses, centres of gravity,...) are well preserved by the process of adjustment.
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\section*{8 Modeling}

C
8.1

Characteristics of modeling
Modeling C tests the coupling between LOBSTER and Code_Aster for a problem 2D of thermomechanical transient. One checks the update of the fields on the successive grids.

\section*{8.2}

Characteristics of the grid
The grid is the same one as that of modeling \(A\).

\subsection*{8.3 Functionalities}
tested

\author{
Orders Key word \\ factor \\ Key words \\ MACR_INFO_MAIL \\ VERSION_HOMARD \\ GRID \\ QUALITY \\ INTERPENETRATION
}
CUT
CONNEXITY
MACR_ADAP_MAIL
VERSION_HOMARD
QUALITY
INTERPENETRATION
ADAPTATION
FREE
MAILLAGE_N
MAILLAGE_NP1
RESULTAT_N
INDICATOR
NOM_CMP_INDICA
CRIT_RAFF_PE
CRIT_DERA_PE
NUME_ORDRE
INST
MAJ_CHAM
RESULT
NOM_CHAM
TYPE_CHAM
NUME_ORDRE
INST
CHAM_MAJ
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\section*{9}

Results of modeling \(C\)

\subsection*{9.1 Values}
tested
After adaptation \(n^{\circ} 1\) :
Identification Moments
Reference
Aster \%
difference
TEMP
2 3,097832
102 3,097832
102-1,2110-5
DX
3 1,267359
10-2 1,267359
10-2-2,47
10-5
DY
3-2,511801
10-2-2,511801
10-2 1,29
10-5
After adaptation \(\boldsymbol{n}^{\circ} 2\) :
Identification Moments
Reference
Aster \%
difference
TEMP
2 3,113811
102 3,113811
102 -8,45
10-6
DX
3 1,271608
10-2 1,271608
10-2
1, 00 10-5
DY
3-2,715298
10-2 -2,71529810-2 5,60
10-6
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10 ModelingD

\subsection*{10.1 Characteristics of modeling}

Modeling D tests the coupling between LOBSTER and Code_Aster for a problem of mechanics linear statics in dimension 2. The arc of circle which models the hole of the plate is followed to the wire of adaptations.

\subsection*{10.2 Characteristics of the grid}

The field is with a grid with 11 quadrangles QUAD4 and 1 triangle TRIA3.
The edges of the field are with a grid in segments SEG2.

The arc of circle of the curved border is with a grid with 299 SEG2. Being rectilinear, other edges are not represented.

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\subsection*{10.3 Functionalities}
tested
Orders Key word

Key words
MACR_ADAP_MAIL QUALITY

\section*{INTERPENETRATION}

CONNEXITY

\section*{CUT}

ADAPTATION
FREE
MAILLAGE_N
MAILLAGE_NP1
RESULTAT_N
INDICATOR
NOM_CMP_INDICA
CRIT_RAFF_PE
CRIT_DERA_PE
MAILLAGE_FRONTIERE
MAJ_CHAM
RESULT
NOM_CHAM
TYPE_CHAM
NUME_ORDRE
CHAM_MAJ

11 Results of modeling \(D\)
11.1 Values
tested

\author{
After adaptation \(\boldsymbol{n}^{\circ} 1\) : \\ Identification Moments \\ Reference \\ Code_Aster \% \\ difference \\ SIXX 30 degrees \\ 18.364656 \\ 10-1 8.364656 \\ 10-1 -2,92 \\ 10-5 \\ SIXX 30 degrees \\ 11.5763831 .576383 \\ 4,47 \\ 10-7 \\ After adaptation \(\boldsymbol{n}^{\circ} \mathbf{2 :}\) \\ Identification Moments \\ Reference \\ Code_Aster \% \\ difference \\ SIXX 30 degrees \\ 15.125268 \\ 10-1 5.125268 \\ 10-1 4,22 \\ 10-6 \\ SIXX 30 degrees \\ 11.9115671 .911567 \\ 2,75 \\ 10-7 \\ DX 30 degrees \\ 7,733 \\ 10-6 7,733 \\ 10-6 -2,92 \\ 10-5
}

\section*{After adaptation \(\boldsymbol{n}^{\circ}\) 3:}

Identification Moments
Reference

Code_Aster \%
difference
SIXX 30 degrees
1-1,110223
10-16 5,115904
10-1 0,512
SIXX 30 degrees
\(11.5000001,790507\)
0,19367

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\section*{12 Summary of the results}

This case-test having for simple objective the control of nonthe regression, no remark is to be made on the value of the results. The reference is that of Code_Aster calculation at the day of the first restitution and must be found thereafter.

By examining the files orders, one will note that the loadings must be done on entities with suitable dimension.
a node for a specific loading; for example a displacement imposed on a corner,
a segment for a linear loading; for example, a pressure divided into 2D,
a triangle for a surface loading; for example, a force in 3D.
In addition, it will be noted that these loadings must be expressed on groups of nodes or of meshs and not on the nodes or and the meshs. Indeed after adaptation of the grid, groups are reconstituted. The ordering of loading is thus the same one, whatever the grid.

By complying with these two rules on the loadings, the adaptation of grid is possible.

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\section*{ZZZZ126 - Validation of order CREA_CHAMP} OPERATION: "ADZE"

\section*{Summary:}

This test validates the operation "ADZE" of the order CREA_CHAMP which makes it possible to
manufacture a field (with
nodes or with the elements) by "assembly" of ends of existing fields.
The test consists in affecting on geometrical entities (meshs and nodes), quantities (displacements, internal constraints or variables). One then combines with order ASSE_CHAMP the fields obtained by assignment and one check that the field result contains the good values.

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

The geometry of the problem is of no importance. It is enough to know that the grid contains at least:
\(\cdot 3\) named meshs: M1, m2 and m3,
\(\cdot 3\) named nodes: A1, A2 and A3.

\subsection*{1.2 Properties}
material
Of no importance. One chose the law of behavior VMIS_ISOT_CINE which has at least 3

\title{
1.3 \\ Boundary conditions and loadings
}

Of no importance.

\author{
1.4 Conditions \\ initial \\ Of no importance \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-75/01/010/A
}

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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
That is to say a size \(G\) having 3 components \(X, Y, Z\)
The grid has 3 geometrical entities (meshs or nodes): X1, X2, X3

One first of all manufactures 2 fields ch1 and ch2 by assignment of the size \(G\) on the entities geometrical X1, X2 and X3.

\section*{Entity}

X1 X2 X3
Component
X Y Z X Y Z X Y Z
ch1
1. 3. 1.3.1. 3.
ch2
4. 2. 4. 2. 4. 2.

One defines then the fields ch3 and ch4 by:
ch3 = CREA_CHAMP
(OPERATION: "ADZE"...

\author{
ADZE: (ALL: "YES"
}

CHAM_GD: ch1)

ADZE: (ENTITY: X2
CHAM_GD: ch2
COEF_R: 2.)

ADZE: (ENTITY: X3
CHAM_GD: ch2
COEF_R: 3. OFFICE PLURALITY: "YES"));
ch4 = CREA_CHAMP
(OPERATION: "ADZE"...

ADZE: (
ALL: "YES" CHAM_GD: ch3 COEF_R: 2.
NOM_CMP: "X3"
NOM_CMP_RESU: "X1")
);

One must then obtain:

\section*{Entity}

X1 X2 X3
Component
X Y Z X Y Z X Y Z
ch1
1.3.1.3.1.3. ch2
4. 2. 4. 2. 4. 2.
ch3
1.3.1.8.4.1.12. 9.
ch4
6. 8. 18.

To test the various cases of figure of order ASSE_CHAMP, this calculation is made for 5 types fields:

With
cham_no
displacements
B
cham_elem /ELNO
constraints
C
cham_elem /ELGA
constraints
D
cham_elem /ELNO
internal variables
E
cham_elem /ELGA
internal variables

\section*{2.2}

Results of reference
For the 5 preceding cases of figure, one tests the lubricated and underlined values table below:

Entity
X1 X2 X3
Component
X Y Z X Y Z X Y Z
ch3
1.3.1.
8. 4. 1. 12.9.
ch4
6.
8.
18.

\author{
2.3 \\ Uncertainties on the solution
}

No uncertainty.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

\title{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0 \\ Titrate: \\ ZZZZ126 Validation of order CREA_CHAMP
}

\author{
Date: \\ 15/10/01 \\ Author (S): \\ J. Key PELLET \\ V1.01.126-A Page: \\ 4/4
}

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Of no importance}

\subsection*{3.2 Characteristics}
grid
Of no importance

\subsection*{3.3 Functionalities}
tested
Orders Key word
factor
Key word
Keys
CREA_CHAMP ADZE ALL: "YES"

\author{
/NOEUD/MESH \\ CREA_CHAMP ADZE COEF_R
}

\section*{CREA_CHAMP ADZE OFFICE PLURALITY: "YES"}

CREA_CHAMP ADZE NOM_CMP
NOM_CMP_RESU

4
Results of modeling \(A\)
4.1 Values
tested
Identification Reference Aster \%
difference
ch3/X2/X
1.01 .00 .0
ch3/X3/X
1.01 .00 .0
ch3/X3/Y
12.012 .00 .0
ch3/X3/Z
9.09 .00 .0
ch4/X3/X
18.018 .00 .0

\author{
4.2 Parameters \\ of execution
}

Version: 5.3.11
Machine: Origin 2000
Obstruction memory: 32 Mo
Time CPU To use: 3.5 seconds

\section*{5 \\ Summary of the results}

The results are exactly those awaited.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-75/01/010/A

\author{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0 \\ Titrate: \\ ZZZZ127 - Validation of AFFE_CHAR_MECA/LIAISON_MAIL \\ Date: \\ 05/02/02 \\ Author (S): \\ J. Key PELLET \\ : \\ V1.01.127-A Page: \\ 1/6 \\ Organization (S): EDF/AMA
}

\title{
Handbook of Validation
}

V1.01 booklet: Tests of validity of orders
Document: V1.01.127

ZZZZ127 - Validation of key word LIAISON_MAIL order AFFE_CHAR_MECA

\section*{Summary:}

This test validates key word LIAISON_MAIL of order AFFE_CHAR_MECA. This key word generates them
linear relations between the degrees of freedom of the nodes of 2 edges which one puts in opposite. programming is validated in 2D and 3D by intercomparison with similar an Aster calculation where relations between degrees of freedom directly entered by key word LIAISON_DDL. One also validates geometrical transformation (rotation/translation) applied to the one of the edges.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ZZZZ127 - Validation of AFFE_CHAR_MECA/LIAISON_MAIL
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05/02/02
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J. Key PELLET
:
V1.01.127-A Page:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

The dealt with problem is plane. The studied structure is 1 rectangle cut out in 2 squares ABCD and BEFC.
The solution is established with a network using 2 QUAD4 corresponding to the 2 squares.

\section*{\(y\) \\ D \\ C \\ F \\ 10. \\ Q1 \\ Q2 \\ With \\ B \\ E \\ 0. \\ S1 \\ 20. \\ \(X\)}

\section*{1.2}

Properties of material
elastic material:
\(E=10.0\) u.s.i.
NAKED \(=0.0\)
NAKED \(=0\) are taken. so that one can deal with this plane problem with a layer of elements 3D in having the plane solution.

\section*{1.3}

Boundary conditions and loadings
1) One applies a specific force to the point \(F: F Y=4.0\) u.s.i.

\section*{2) Blockings}

\section*{:}
not a: \(D X=D Y=0\).
not \(D: D X=0\).
3) Linear relations between degrees of freedom:
loading case: cas1
1.0 DX \((E)-0.5 D Y(D)-0.5 D Y(C)=0.0\)
1.0 DY \((E)+0.5 D X(D)+0.5 D X(C)=0.0\)
loading case: cas2
1.0 DY \((E)+0.5 D Y(D)+0.5 D Y(C)=0.0\)
1.0 DY \((B)+0.5 D Y(C)+0.5 D Y(F)=0.0\)

\subsection*{1.4 Conditions initial}

Of no importance.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
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ZZZZ127 - Validation of AFFE_CHAR_MECA/LIAISON_MAIL
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2
Reference solution
2.1

Method of calculation used for the reference solution
In each case, one carries out a preliminary calculation with key word LIAISON_DDL to introduce them
linear relations between degrees of freedom. This calculation is used as reference to calculation with the key word
LIAISON_MAIL which generates these linear relations.
To obtain the desired linear relations with LIAISON_MAIL, one writes:
Cas1:
LIAISON_MAIL: (
NOEUD_2: E
MAILLE_1: Q1
CENTER:
B
ANGL_NAUT:
90.

TRAN: (
\(-5\).
0.)
)
What wants to say that one eliminates the 2 ddls from the node \(\mathbf{E}\) according to the ddls of the \(E^{\prime}\) point obtained when
one subjects to \(E\) a rotation of 90 degrees around \(B\) then a translation of vector (5,0). \(E^{\prime}\) is
thus in the middle of CD. The vector displacement of \(E\) is identified (after rotation of 90 degrees) with that
of \(E^{\prime}\). The 2 equations are thus obtained:
\(D X(E)=D Y\left(E^{\prime}\right)=0.5 D Y(C)+0.5 D Y(D)\)
\(D Y(E)=-D X\left(E^{\prime}\right)=-0.5 D X(C)-0.5 D X(D)\)

\section*{Cas2:}

LIAISON_MAIL: (
MAILLE_2: S1
MAILLE_1: (Q1, Q2)
DDL_2: "DNOR"

\section*{DDL_1: "DNOR"}

\section*{CENTER:}

B
ANGL_NAUT:
180.

TRAN: (
+5 .
)
What wants to say that one eliminates normal displacement from the nodes B and E (nodes of the S1 segment)
according to the ddls of the points \(B^{\prime}\) and \(E^{\prime}\) obtained when one subjects to \(B\) and \(E\) a rotation of 180 degrees around \(B\) then a translation of vector \((+5,+10)\). \(B^{\prime}\) is thus in the middle of CF and \(E^{\prime}\) with medium of cd. The normal displacement of \(B\) is identified (after rotation of 180 degrees) with that of \(B^{\prime}\).
One makes in the same way for \(B^{\prime}\). The 2 equations then are obtained:
\(D Y(E)=-D Y\left(E^{\prime}\right)=-0.5 D Y(C)-0.5 D Y(D)\)
\(D Y(B)=-D Y\left(B^{\prime}\right)=-0.5 D Y(C)-0.5 D Y(F)\)

\section*{2.2}

Results of reference
One observes displacement DY of the point F:
```

cas1: DY (F)=1.4153582447720D+00
cas2: $D Y(F)=1.0561898652983 D+00$

```

These displacements are obtained with linear relations enter ddls introduced by the key word LIAISON_DDL.

\section*{2.3 \\ Uncertainties on the solution}

No uncertainty.

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/02/001/A
}

\title{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0 \\ Titrate: \\ ZZZZ127 - Validation of AFFE_CHAR_MECA/LIAISON_MAIL \\ Date: \\ 05/02/02 \\ Author (S): \\ J. Key PELLET \\ : \\ V1.01.127-A Page: \\ 4/6
}

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The problem is solved with modeling \(D_{-} P L A N\).

\section*{3.2}

Characteristics of the grid
The grid is made of:
2 QUAD4: \(Q 1=A B C D\) and \(Q 2=B E F C\)
1 SEG2: \(S 1=B E\)

\subsection*{3.3 Functionalities}
tested
Orders Key word
factor
Key word
Keys
AFFE_CHAR_MECA LIAISON_MAIL
MAILLE_2
NOEUD_2
```

MAILLE_1
AFFE_CHAR_MECA LIAISON_MAIL
CENTER
ANGL_NAUT
TRAN
AFFE_CHAR_MECA LIAISON_MAIL
DDL_1: "DNOR"
DDL_2: "DNOR"
4
Results of modeling A
4.1 Values
tested
Identification Reference
Aster %
difference
cas1: DY (F)
1.4153582447720D+00
1.4153582447720D+00
0.
cas2: DY (F)
1.0561898652983D+00
1.0561898652983D+00
0.

```

\subsection*{4.2 Parameters}
of execution
Version: 5.07.01
Machine: Origin 2000-R12000
Obstruction memory: 64 Mo
Time CPU To use: 4 seconds
Handbook of Validation
V1.01 booklet: Tests of validity of orders
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\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

\section*{Titrate:}

\title{
ZZZZ127 - Validation of AFFE_CHAR_MECA/LIAISON_MAIL
}

Date:
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:
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\section*{5 Modeling \\ B}
5.1

Characteristics of modeling
The problem is solved with modeling 3D.

\section*{5.2 \\ Characteristics of the grid \\ The grid is made of: \\ 2 HEXA8: Q1 and Q2 \\ 1 QUAD4: S1}

\subsection*{5.3 Functionalities}
tested
The same ones as for modeling A but in 3D.

6
Results of modeling B

\author{
6.1 Values
}
tested

\author{
Identification Reference \\ Aster \% \\ difference \\ cas1: DY (F) \\ \(1.4153582447720 D+00\) \\ \(1.4153582447720 D+00\) \\ 0. \\ cas2: DY (F) \\ \(1.0561898652983 D+00\) \\ \(1.0561898652983 D+00\) \\ 0.
}
6.2 Remarks
(possibly)

\subsection*{6.3 Parameters}
of execution
Version: 5.3.11

Machine: Origin 2000-R12000
Obstruction memory: 32 Mo
Time CPU To use: 4 seconds

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders
}

HT-66/02/001/A

Titrate:
ZZZZ127 - Validation of AFFE_CHAR_MECA/LIAISON_MAIL
Date:
05/02/02
Author (S):
J. Key PELLET
:
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\section*{7}

\section*{Summary of the results}

The numerical results, displacement in a point, are rigorously identical between the two calculations Aster, with key word LIAISON_MAIL, or key word LIAISON_DDL.
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V1.01 booklet: Tests of validity of orders
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
7.1

Titrate:
ZZZZ128 - Validation of LIRE_TABLE/IMPR_TABLE
Date:
01/10/03
Author (S):
J. Key PELLET
:
V1.01.128-A Page:
1/2
Organization (S): EDF/AMA

\title{
Handbook of Validation
}

V1.01 booklet: Tests of validity of orders
Document: V1.01.128

\section*{ZZZZ128 - Validation of orders LIRE_TABLE and IMPR_TABLE}

\section*{Summary:}

This test validates orders LIRE_TABLE and IMPR_TABLE (with the format "ASTER"). Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.1

Titrate:
ZZZZ128 - Validation of LIRE_TABLE/IMPR_TABLE

\section*{Date:}

01/10/03
Author (S):
J. Key PELLET
:
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\section*{1 \\ Problem of reference}

One reads on an ASCII file a table (thus known in advance! ). One rewrites this table on new file, then this new file is read again. One can then test that the read again table still has the contents initial. This test thus makes it possible to validate the impression and the reading of a table on a file with the format
"ASTER".

\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The value of reference is not calculated: it is a value written in a data file.
2.2

Results of reference
One observes the cell corresponding to the parameter "VALUE" and corresponding to line NOEUD=N2.

The value of the table is then \(21.0 \mathrm{D}+00\)

\section*{2.3}

Uncertainties on the solution
No uncertainty.

\section*{3 Modeling \\ B}
3.1

Characteristics of modeling
This test has only one modeling which is called " \(B\) ".

\subsection*{3.2 Functionalities}
tested
Orders Key word
factor

Key word

\section*{IMPR_TABLE}

FORMAT=' ASTER'
LIRE_TABLE
FORMAT=' ASTER'
SEPARATOR

\section*{4 \\ Results of modeling B}

\subsection*{4.1 Value}
tested

\section*{Identification Reference}

Aster \%
difference
VALUE (N2)
21.0+00
\(21.0+00\)
0.

\section*{5 \\ Summary of the results}
R.A.S: the read again table is rigorously identical to the original table.

Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.3

Titrate:
SENSM01 - Sensitivity in thermics, data-processing control
Date:
03/06/03
Author (S):

\title{
Organization (S): EDF-R \& D /SINETICS
}

\author{
Handbook of Validation
}

V1.01 booklet: Tests of validity of orders
Document: V1.01.144

\section*{SENSM01 - Sensitivity in static mechanics, control data-processing}

\section*{Summary:}

This case test validates nonthe regression of the basic commands of calculation of sensitivity in static mechanics. It acts to make sure that the insertion of the key word SENSITIVITY in the principal orders remains operational.

For that, one studies a static phenomenon of mechanics in a field 2D, only controlled by boundary conditions of Dirichlet. The solution in field of displacement is analytical. In the same way, derivation
this field compared to one of the boundary conditions is analytical. The comparison calculation/ reference is thus simple to establish.

\title{
One analyzes orders MECA_STATIQUE and CALC_ELEM.
}

\author{
Handbook of Validation
}

V1.01 booklet: Tests of validity of orders HI-23/03/003/A

Code_Aster \({ }^{\circledR}\)
Version
6.3

Titrate:
SENSM01 - Sensitivity in thermics, data-processing control

\section*{Date: \\ 03/06/03 \\ Author (S): \\ : \\ 2/6 \\ Count \\ matters}
G. Key NICOLAS

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1 Problem of reference ..... 3
1.1 Geometry ..... 3
1.2 Material properties ..... 3
1.3 Boundary conditions and loadings. ..... 4
1.4 Initial conditions ..... 4
2 Reference solution ..... 4
2.1 Method of calculation used for the reference solutions ..... 4
2.2 Results of reference ..... 4
2.3 Uncertainty on the solutions ..... 4
3 Modeling \(A\) ..... 5
3.1 Characteristics of modeling ..... 5
3.2 Characteristics of the grid ..... 5
3.3 Functionalities tested ..... 5
4 Results of modeling \(A\) ..... 5
4.1 Values tested ..... 5

\title{
5 Summary of the results
}

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V1.01 booklet: Tests of validity of orders
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Code_Aster \({ }^{\circledR}\)
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6.3

Titrate:
SENSM01 - Sensitivity in thermics, data-processing control
Date:
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Author (S):
G. Key NICOLAS

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{I}
\(y=1,5\) ORIGIN

\section*{The four corners and the center are particularized:}

\author{
Name X \\ \(y\) \\ ORIGIN \\ l-1,5 \\ I \\ 11-1,5 \\ \(J\) \\ 1 1,5 \\ OPPOSE \\ 11 1,5 \\ CENTER \\ 60
}

The various zones are indicated. Each internal rectangle is indicated under its name: MOITIE1 and MOITIE2.
Each frontier segment bears a name. The vertical segments on the left and on the right are named BORD_GAU and BORD_DRO. The lower horizontal segments are named BORD_IN1 and BORD_IN2; in the same way, the higher horizontal segments are named BORD_SU1 and BORD_SU2.

BORD_SU1
BORD_SU2

BORD_GAU
BORD_DRO
MOITIE1
MOITIE2

\author{
BORD_IN1 \\ BORD_IN2
}

\section*{1.2}

Material properties
The Young modulus is the same one in all the field:
\(E=200000\)
The Poisson's ratio is the same one in all the field:
\(=3\)
0
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\section*{1.3}

Boundary conditions and loadings
All operation is controlled by the displacements imposed on the two corners.
With the corner ORIGIN, displacement is null.
To the corner OPPOSES, displacement is imposed on \(U X=10\) and \(U y=3\).
All the external edges are with null loading.

\subsection*{1.4 Conditions}
initial

Without object.

2
Reference solution

\section*{2.1}

Method of calculation used for the reference solutions
By reason of symmetry, one deduces that each component of displacement in the center is worth the average identical components two displacements imposed in the corners.

\section*{1}

UCENTRE \(=(\) UORIGINE + UOPPOSE \()\)
2
To calculate the sensitivity compared to the displacement imposed in a corner amounts deriving the formula
analytical. Thus in the center, we have:

\section*{UCENTRE 1}

The results for the strains and the stresses are obtained by comparison between two calculations ASTER.

\section*{2.2}

Results of reference
By numerical application, we have with the node CENTERS:
\(\boldsymbol{U} \boldsymbol{X}\)
\(\boldsymbol{U} \boldsymbol{y}\)
Displacement 5
1,5
Derived compared to \(U\)
0,5 0
\(X\) of the node

\section*{opposed}

Derived compared to \(U\)
0 0,5
\(y\) of the node
opposed

\section*{2.3 \\ Uncertainty on the solutions}

Analytical solutions
Handbook of Validation
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Calculation is made in static mechanics.}

\section*{3.2 \\ Characteristics of the grid}

The grid is carried out with elements of the type TRIA3. The frontier segments are with a grid with elements SEG2. There are 55 nodes, 80 triangles and 28 segments of edge.

\author{
3.3 Functionalities \\ tested \\ Orders
}

AFFE_CHAR_MECA_F DDL_IMPO

PLANE AFFE_MODELE THERMICS

\author{
DEFI_PARA_SENSI
}

\section*{MEMO_NOM_SENSI}

\section*{MECA_STATIQUE SENSITIVITY}

\section*{CALC_ELEM SENSITIVITY}

TEST_RESU SENSITIVITY

4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested
We test the values of the field of displacement and its derivative on the central node. The tolerance is \(106 \%\).

\section*{Analytical identification}

\section*{Calculated}

Relative variation in \%
\(U\)
\(X\)
CENTER

5
5
1,4 10-12
\(\boldsymbol{U}\)

\section*{CENTER}
\(X\)
0,5
0,5
1,4 10-12
U
OPPOSE
\(U\)
\(y\)
CENTER

1,5
1,5
1,0 10-12
\(U\)

\section*{CENTER}
\(y\)
0,5
0,5
1,0 10-12

\author{
U \\ OPPOSE \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-23/03/003/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.3

Titrate:
SENSM01 - Sensitivity in thermics, data-processing control
Date:
03/06/03

\section*{Author (S):}
G. Key NICOLAS
:
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\section*{5}

Summary of the results
No particular comment is to be made on this case-test. It is only used to ensure the perenniality of options of sensitivity in the orders handling the results.

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HI-23/03/003/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.4

Titrate:
SENSM02 - Sensitivity to the pressure imposed in linear mechanics
Date:
16/05/03
Author (S):
NR. TARDIEU, P. of Key BONNIERES
:
V1.01.145-B Page:

\author{
Handbook of Validation
}

V1.01 booklet: Tests of validity of the orders
Document: V1.01.145

SENSM02-Sensitivity to the imposed pressure in linear mechanics

\section*{Summary:}

One tests the calculation of sensitivity to the loading pressure. The sensitivity is calculated by direct differentiation discrete equations. The reference comes from a commonplace analytical solution (compactness uniaxial).

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of the orders
HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SENSM02 - Sensitivity to the pressure imposed in linear mechanics
Date:
16/05/03
Author (S):
NR. TARDIEU, P. of Key BONNIERES
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{BORD_DRO}

\section*{3. mm}

BORD_GAU

\section*{CENTER}

\section*{ORIGIN}
```

y

```
10. mm

Z
1. \(<X<11\)

\section*{1.2}

Properties of material
\(E=200.000 \mathrm{MPa}\)
\(=0.3\)
\(=8.106 \mathrm{~kg} / \mathrm{mm} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

Surface BORD_GAU is blocked according to X.
The line ORIGIN is blocked according to \(X\) and \(Y\).
A pressure of 10. MPa is imposed on BORD_DRO.
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V1.01 booklet: Tests of validity of the orders
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SENSM02 - Sensitivity to the pressure imposed in linear mechanics
Date:
16/05/03
Author (S):
NR. TARDIEU, P. of Key BONNIERES

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2
Reference solution

\author{
2.1 \\ Method of calculation
}

The reference solution comes from an analytical solution:
\(x x(X, y, Z)=p\) (imposed pressure)
\(y y(X, y, Z)=0\)
\(u x(X, y, Z)=X p / E\)
uy \((X, y, Z)=(y+1.5) p / E\)
\(x x(X, y, Z)=p / E\)
\(y y(X, y, Z)=p / E\)
That is to say:
\(D x x / d p(X, y, Z)=1\)
\(D y y / d p(X, y, Z)=0\)
\(D u x / d p(X, y, Z)=x / E\)
\(D u y / d p(X, y, Z)=(y+1.5) / E\)
\(D x x / d p(X, y, Z)=1 / E\)
\(D y y / d p(X, y, Z)=/ E\)

\section*{2.2 \\ Sizes and results of reference}

Displacement following \(X\) to the point CENTERS:
ux \((6,0, Z)=2.5 E-4\)
Displacement following y to the point CENTERS:
uy \((6,0, Z)=2.25 E-5\)
Sensitivity of displacement following \(X\) to the pressure p to the point CENTERS:
D ux/d p \((6,0, Z)=2.5 E-5\)
Sensitivity of displacement following there to the pressure p to the point CENTERS:
D uy/d p \((6,0, Z)=2.25 E-6\)
Deformation according to \(x x\) at the point CENTERS:
\(x x(6,0, Z)=5\). E-5
Deformation according to yy at the point CENTERS:
\(y y(6,0, Z)=1.5 E-5\)
Sensitivity of the deformation according to \(x x\) to the pressure \(p\) to the point CENTERS: \(D x x / d p(6,0, Z)=-5 . E-6\)
Sensitivity of the deformation according to yy to the pressure p to the point CENTERS:
\(D\) yy \(/ d p(6,0, Z)=1.5 E-6\)
Constraint according to \(x x\) at the point CENTERS:
\(x x(6,0, Z)=10\).
Constraint according to yy at the point CENTERS:
\(y y(6,0, Z)=0\).
Sensitivity of the constraint according to xx to the pressure p to the point CENTERS:
\(D x x / d p(6,0, Z)=1\).
Sensitivity of the constraint according to yy to the pressure p to the point CENTERS:
\(D\) yy \(/ d p(6,0, Z)=0\).
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\section*{Titrate:}

SENSM02 - Sensitivity to the pressure imposed in linear mechanics
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Modeling in plane constraints on the grid following 2D:

Surface BORD_GAU becomes a line, the lines ORIGIN and OPPOSES become points.

\section*{3.2}

Characteristics of the grid
A number of nodes: 55
Numbers and types of meshs: 28 SEG2, 80 TRIA3

\subsection*{3.3 Functionalities}
tested
Orders

MEMO_NOM_SENSI NOM_UN

NOM_ZERO

NAME
NOM_SD
PARA_SENSI
NOM_COMPOSE

MECA_STATIQUE SENSITIVITY

\section*{CALC_ELEM SENSITIVITY}

TEST_RESU SENSITIVITY

\section*{3.4 \\ Sizes tested and results}

Size tested
Theory
Code_Aster
Difference (\%)
DX at the point CENTERS
2.5 E-4 2.5 E-4 0.

DY at the point CENTERS
2.25 E-5 2.25 E-5 0.

Sensitivity of DX to the pressure \(p\)
2.5 E-5
2.5 E-5

0 .

Sensitivity of DY to the pressure \(p\)
2.25 E-6
2.25 E-6

0 .
at the point CENTERS
EPXX at the point CENTERS
5. \(E-5\)
5. \(E-5\)
0.

EPYY at the point CENTERS
1.5 E-5
1.5 E-5

0 .
Sensitivity of EPXX to the pressure
5. E-6
5. E-6
0.
p at the point CENTERS
Sensitivity of EPYY to the pressure
1.5 E-6
1.5 E-6

0 .
p at the point CENTERS
SIXX at the point CENTERS
-10.
-10.
0.

SIYY at the point CENTERS
0.
1.15 E-14

Sensitivity of SIXX to the pressure -1. -1. 0 .
p at the point CENTERS
Sensitivity of SIYY to the pressure
0. 5.

E-16
p at the point CENTERS
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\subsection*{3.5 Notice}

One tests also the derivation of displacements, the strains and the stresses compared to one insensitive parameter: the numerical result obtained is exactly 0 .

\section*{4 \\ Summary of the results}

The numerical results of sensitivity are in triad with those of the theory.
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Titrate:
SENSM03 - Sensitivity to the nodal forces imposed in linear mechanics Dates:
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NR. TARDIEU Key

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SENSM03-Sensitivity to the nodal forces imposed in linear mechanics

\section*{Summary}

One tests the calculation of sensitivity to an imposed nodal force. The sensitivity is calculated by differentiation direct of the discrete equations. The reference comes from a calculation by finished differences.

\footnotetext{
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}

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1
Problem of reference
1.1 Geometry

\section*{OPPOSE}
3. \(m m\)
\(B O R D \_G A U\)
CENTER

ORIGIN
10. mm

Z
1. \(<X<11\)
-1.5
<
Y
< 1.5
\(X\)

\title{
1.2 \\ \\ Properties of material
} \\ \\ Properties of material
}
\(E=200.000 \mathrm{MPa}\)
\(=0.3\)
\(=8.106 \mathrm{~kg} / \mathrm{mm} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

Surface BORD_GAU is blocked according to X.
The line ORIGIN is blocked according to \(X\) and \(Y\).
A nodal force of components (10. ,3.) is imposed on the line OPPOSES.
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

For the field of displacement, the solution is that of calculation Aster (it thus acts of tests of not regression).
For the sensitivity of the field displacement to the imposed nodal force, the solution comes from
finished differences (approximation of derived with order 1) carried out with Aster. These finished differences express themselves in the form:
```

F (X, y)
F(X+dx, y) - F (X, y)
X
dx
F (X, y)
F(X,y+Dy)-F (X, y)
y
dx
2.2
Sizes and results of reference

```

Displacement following \(X\) to the point CENTERS:
ux \((6,0, Z)=8.1094117774520\) E-05
Displacement following y to the point CENTERS:
uy \((6,0, Z)=3.0721034884009 \mathrm{E}-04\)
Sensitivity of displacement following \(X\) to component FX to the point CENTERS:
D ux/d FX (6, 0, Z) 8.44126000000000E-06
Sensitivity of displacement following there to component FX to the point CENTERS:
D uy/d FX (6, 0, Z) -3.3819900000000E-05
Sensitivity of displacement following \(X\) to component \(F Y\) to the point CENTERS:
D ux/d FX (6, 0, Z) -1.1061700000000E-06
Sensitivity of displacement following there to component FY to the point CENTERS:
D uy/d FX (6, 0, Z) 2.1513600000000E-04
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Modeling in plane constraints on the grid following 2D:

Surface BORD_GAU becomes a line, the lines ORIGIN and OPPOSES become points.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 55
Numbers and types of meshs: 28 SEG2, 80 TRIA3

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

MEMO_NOM_SENSI
NOM_UN
NOM_ZERO
NAME
NOM_SD
PARA_SENSI
NOM_COMPOSE
MECA_STATIQUE
SENSITIVITY
TEST_RESU

\section*{3.4 \\ Sizes tested and results}

\author{
Size tested \\ Reference: \\ Code_Aster \\ Difference (\%)
}

DX at the point CENTERS
8.1094117774520 E-05
8.1094117774520E-05

0 .
DY at the point CENTERS
3.0721034884009 E-04
3.0721034884009 E-04

0 .
Sensitivity of DX to
8.4412600000000E-06 8.4412639288501

E-06 4.65E-05
component FX at the point
CENTER
Sensitivity of DY to
3.38199000000000E-05 3.3819874780225E-05 7.46E-05
component FX at the point
CENTER
Sensitivity of DX to
1.10617000000000E-06 1.1061738379937E-06 3.47E-04
component FY at the point
CENTER
Sensitivity of DY to
2.15136000000000E-04 2.1513636554745E-04 1.70E-04
component FY at the point
CENTER

\section*{4}

Summary of the results
The results of sensitivity per direct differentiation are in very good agreement with those given by

\section*{Code_Aster \({ }^{\circledR}\)}

Version
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\section*{Titrate:}

SENST01 - Sensitivity in thermics, data-processing control
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G. Key NICOLAS
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Organization (S): EDF/SINETICS

\author{
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}

\section*{SENST01-Sensitivity in thermics, control data processing}

\section*{Summary}

This case test validates nonthe regression of the basic commands of calculation of sensitivity in thermics. It is about
to make sure that the insertion of the key word SENSITIVITY in the principal orders remains operational.

For that, one studies a phenomenon of thermics in a field 2D, only controlled by conditions with the limits of Dirichlet. The solution in field of temperature is analytical. In the same way, the derivation of it
field compared to one of the boundary conditions is analytical. The comparison calculation/reference is thus
simple to establish.
One analyzes orders THER_LINEAIRE, EXTR_RESU, CREA_CHAMP. It is checked that a calculation in
CONTINUATION functions.

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Titrate:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}
\(y\)

\section*{OPPOSE}
\(J\)
\(y=1,5\)

\section*{CENTER \\ X \\ \(X\) \\ \(x=1\) \\ \(x=11\)}

\section*{\(I\) \\ \(y=1,5\) ORIGIN}

The four corners and the center are particularized:

\author{
Name X \\ \(y\) \\ ORIGIN \\ 1-1,5 \\ I \\ \(11-1,5\) \\ \(J\) \\ 1 1,5 \\ OPPOSE \\ 111,5 \\ CENTER \\ 60
}

The various zones are indicated. Each internal rectangle is indicated under its name: MOITIE1 and MOITIE2.
Each frontier segment bears a name. The vertical segments on the left and on the right are named BORD_GAU and BORD_DRO. The lower horizontal segments are named BORD_IN1 and BORD_IN2; in the same way, the higher horizontal segments are named BORD_SU1 and BORD_SU2.

BORD_SU1
BORD_SU2

\author{
BORD_GAU
}

BORD_DRO
MOITIE1
MOITIE2

\section*{BORD_INI}

BORD_IN2

\section*{1.2 \\ Material properties}

Thermal conductivity is the same one in all the field:
-1-1
=

\title{
The specific heat is the same one in all the field:
}
- 1
- 3
. \(C p=1\) J.K. \(m\)
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\section*{1.3 \\ Boundary conditions and loadings}

All operation is controlled by temperatures imposed on the two corners.
\(T(\) ORIGIN \()=0 \mathrm{~K}\)
\(T(\) OPPOSES \()=100 \mathrm{~K}\)
All the external edges are with null flow.
Calculation is transitory, but no boundary condition nor no loading changes with the course time

\subsection*{1.4 Conditions}
initial

The transient is initialized by a stationary calculation.

\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solutions}

The field of temperature is solution of a null Laplacian on the field. By reason of symmetry, one deduced that the temperature in the center is worth the average of the two temperatures imposed in the corners.

1
CENTER
\(T\)
= (ORIGIN
\(T\)
OPPOSES
\(T\)
)
2
To calculate the sensitivity compared to the temperature imposed in a corner amounts deriving the formula
analytical. Thus in the center, we have:
CENTER
T
1
\(=\)
\(T\)
2
OPPOSE
As no condition changes during the transient, these results are true at any moment.

\section*{2.2}

Results of reference
By numerical application, we have the two results of reference:
\(T\)

\section*{CENTER}
\(T\)
\(=5\)
,
0
OPPOSE
\(T\)
2.3

Uncertainty on the solutions

\author{
Analytical solutions
}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Calculation is made in transitory linear thermics. The step of times is 0,5 S. the first calculation goes
\(T=0\) to \(T=1 S\). the second calculation, established by a CONTINUATION, goes from \(T=1\) to \(T=2,5\) \(S\).

\section*{3.2 \\ Characteristics of the grid}

The grid is carried out with elements of the type TRIA3. The frontier segments are with a grid with elements SEG2. There are 55 nodes, 80 triangles and 28 segments of edge.

\author{
3.3 Functionalities tested \\ Orders \\ \(A F F E \_C H A R \_T H E R \_F\) \\ TEMPS_IMPO
}

\author{
AFFE_MODELE \\ PLAN \\ THERMICS
}

DEFI_PARA_SENSI

MEMO_NOM_SENSI

\section*{THER_LINEAIRE \\ SENSITIVITY}

\author{
EXTR_RESU
}

SENSITIVITY

\author{
CREA_CHAMP \\ SENSITIVITY
}

\section*{CONTINUATION}

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested
We test the values of the field of temperature and its derivative on the central node before and afterwards
the recovery. These values result either from the result produced by THER_LINEAIRE, or of the result which in
is extracted, that is to say field which is created. The tolerance is \(106 \%\).

\section*{Analytical identification}

\section*{Calculated}

Relative variation in \%

\section*{CENTER}
\(T\)
with \(T=0,5\)
50
50
1,4 10-12

\section*{CENTER}
\(T\)
with \(T=0,5\)
0,5
0,5
1,4 10-12
OPPOSE
T

\section*{CENTER}
```

T

```
with \(T=1\)
50
50

1,0 10-12

\section*{CENTER}
\(T\)
with \(T=1\)
```

0,5
0,5
1,0 10-12
OPPOSE
T

```

\section*{CENTER}
```

T

```
with \(T=2,5\)
50
50
1,2 10-12

\section*{CENTER}
\(T\)
with \(T=2,5\)
0,5
0,5
1,2 10-12
OPPOSE

\author{
\(T\) \\ Handbook of Validation
}
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\section*{5 \\ Summary of the results}

No particular comment is to be made on this case-test. It is only used to ensure the perenniality of options of sensitivity in the orders handling the results.

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14/10/02
Author (S):
O. BOITEAU Key

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Organization (S): EDF/SINETICS

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}

SENST02 - Sensitivities compared to the temperature imposed and on normal flow

\section*{Summary:}

In this case-test, it is a question of making sure of the validity of the sensitivities compared to the temperature imposed and on normal flow, in isotropic transitory linear thermics (key word SENSITIVITY of THER_LINEAIRE and of
THER_NON_LINE). It is based on the grid of case-test SENST01A treating the transitory thermal answer
of a plate 2D_PLAN subjected to various thermal requests.
One tests the good adequacy of the sensitivities sought with their approximation by finished differences, in
several points of the grid and at the last moment of the transient. Strategy of calculation by finished differences,
more expensive, harder to implement and less robust, is not of course to privilege and does not have virtue that as a test...
One carries out these calculations of transitory linear thermics:
> - with THER_LINEAIRE (modeling A with not lumpés isoparametric elements),
> - with THER_NON_LINE (modeling B with lumpés isoparametric elements).

They make it possible to check the correct operation of these two operators in the presence of the loadings
EXCHANGE, TEMP_IMPO, SOURCE and FLUX-REP, on meshs 2D lumpées or not, in both
following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION),
- of sensitivity (via the suitable option of the thermal operators and TEST_RESU).

The results of these two configurations agree quasi-parfaitement what, although logic in theory, is reassuring as for the robustness of programming of these two operators. Especially that they treat differently identical problems (resolution of a linear system for A and algorithm of Newton for B) being also based on distinct modelings (resp. PLAN and PLAN_DIAG).

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\section*{1 \\ Problem of reference}

One takes again here the geometry of the case-test SENST01A on which one will affect materials and loadings corresponding to no real case. It is here only about one functional validation, numerical and of data-processing not-regression of calculations of sensitivity in a situation very penalizing (many loadings in little space and with dependences in time polynomial!)

\subsection*{1.1 Geometry}
```

CENTER
$X$
$X$
$x=1$
$x=11$
I
$y=1,5$ ORIGIN

```

The four corners and the center are particularized:
```

Name
$X$
$Y$
ORIGIN
1 -1,5
I
11-1,5
J
11,5
OPPOSE
11 1,5
CENTER
60

```

The various zones are indicated. Each internal rectangle is indicated under its name: MOITIE1 and MOITIE2.
Each frontier segment bears a name. The vertical segments on the left and on the right are named BORD_GAU and BORD_DRO. The lower horizontal segments are named BORD_IN1 and BORD_IN2; in the same way, the higher horizontal segments are named BORD_SU1 and BORD_SU2.

BORD_SU1
BORD_SU2

\title{
BORD_GAU \\ BORD_DRO \\ MOITIE1 \\ MOITIE2
}

\section*{BORD_IN1 \\ BORD_IN2}

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\section*{1.2 \\ Material properties}

One applies to all the structure the characteristics material:
\(=075\)
W/m C
\[
C=2 \mathrm{~J} \mathrm{m3}
\]

\section*{1.3}

Boundary conditions and loadings
By noting T the moment considered, one can synthesize the decomposition of the loadings by zone under
the shape of the following table:

\section*{Geometrical zones}

Loadings Sensitivities
(GROUP_NO/GROUP_MA)
sought
BORD_GAU
COEF_H \(=30(1+0.3 \mathrm{~T}) \mathrm{W} / \mathrm{m} 2^{\circ} \mathrm{C}\)
\(T_{-} E X T=140\left(1+0.3\right.\) T) \({ }^{\circ} \mathrm{C}\)
```

BORD_DRO
$F L U X_{-} R E P=120 \mathrm{~W} / \mathrm{m} 2$
PS2
ORIGIN
$T E M P=100^{\circ} \mathrm{C}$
PS1
OPPOSE
$T E M P=200(1+0.1 T){ }^{\circ} \mathrm{C}$
MOITIE2
SOURCE $=10(1+0.4 \mathrm{~T}) \mathrm{J} / \mathrm{m} 3 \mathrm{~s}$

```

\subsection*{1.4 Condition initial}

Transitory thermal calculation on two steps of time \((T 1=5 S\) and \(t 2=10 S)\) initialized by a calculation
stationary (TEMP_INIT=_F (STATIONNAIRE=' OUI')).
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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solutions
On such a case of figure, it is not possible to exhume an analytical solution! Calculations of sensitivity validated by centered differences are finished (what allows also at the same time to validate standard calculation).
This good adequacy of the sensitivities with their approximation by finished differences is tested in several points of the grid (I, J and CENTER), and at every moment of the transient (t0, T1 and t2), in structures of control PYTHON of the command file. It is traced in the file message (one tolerate a maximum variation of \(107 \%\) ). In the file result one retained only the part of this test concerning the point CENTERS at the last moment of the transient (the sensitivities via a
TEST_RESU and them
differences finished via a TEST_FONCTION, with a relative tolerance of 106\%).
In the event of disagreement on results (NOOK), one can thus refine the diagnosis, that is to say in direction of
standard problem, either of the derived problem, or of both.
Strategy of calculation by differences finished, more expensive, harder to implement and less robust, is not of course to privilege and has virtue only as a test...

\section*{2.2}

Result of reference
Without object.

\title{
Parameter of shift of the finished difference \((=0.1)\) and convergence of the grid.
}

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3 Modeling
With

\section*{3.1 \\ Characteristics of modeling}

The grid is carried out with elements of the type TRIA3.
Calculation is made in linear thermics with operator THER_LINEAIRE (= 0.57).

In this case-test, it is a question of making sure of the validity of the sensitivities compared to the temperature
imposed at ORIGIN (noted PS1 in the command file) and on normal flow on the BORD_DRO (resp. PS2). These sensitivities are estimated at the last moment of the transient and at the point CENTERS.

\author{
3.2 \\ Characteristics of the grid \\ 80 TRIA3, 28 SEG2, 55 nodes
}

\subsection*{3.3 Functionalities}

\section*{DEFI_MATERIAU \\ THER}

\title{
AFFE_CHAR_THER_F
}

EXCHANGE
TEMP_IMPO
FLUX_REP
SOURCE
AFFE_MODELE
PLAN
THERMICS
DEFI_PARA_SENSI
MEMO_NOM_SENSI
THER_LINEAIRE
SENSITIVITY

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
One tests the validity of the calculated sensitivities, on the one hand, via the option SENSITIVITY of THER_LINEAIRE
(TEST_RESU "NON_REGRESSION"), and, in addition, by finished differences centered (TEST_FONCTION "AUTRE_ASTER"). The relative tolerance is \(1.106 \%\).

Identification SENSITIVITY Variation
relative Differences
Relative variation
(in \%)
finished
```

(in %)
T
(
5.18799009 102
-8.43 1013*
5.18799009 102
1.1 10-9
CENTER T
2)
0%
T
impo

```
```

T

```
T
(
(
1.64143604-1.061012*
1.64143604-1.061012*
1.64143604
1.64143604
1.72 10-11
1.72 10-11
CENTER T
CENTER T
2)
2)
~ 0%
~ 0%
flow
```

flow

```
* Taking into account the precision machine, this value is an approximation into arithmetic finished the zero value.

\author{
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}

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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

Idem that for [§3.1] but with operator THER_NON_LINE and in modeling PLAN_DIAG.

\section*{5.2 \\ Characteristics of the grid}

Idem that for [§3.2].

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\section*{DEFI_MATERIAU}

THER_NL
AFFE_CHAR_THER_F
EXCHANGE
TEMP_IMPO
FLUX_REP
SOURCE

AFFE_MODELE
PLAN_DIAG
THERMICS
DEFI_PARA_SENSI
MEMO_NOM_SENSI
THER_NON_LINE
SENSITIVITY

\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested
One tests the validity of the calculated sensitivities, on the one hand, via the option SENSITIVITY of THER_NON_LINE
(TEST_RESU "NON_REGRESSION"), and, in addition, by finished differences centered (TEST_FONCTION "AUTRE_ASTER"). The relative tolerance is \(1.106 \%\).

\section*{Identification SENSITIVITY Variation}
relative
Finished differences relative Variation
(in \%)
(in \%)
\(T\)
(
5.181649231023 .48

1013*
5.18164923102
1.21 10-13

CENTER T
2)
~0\%
\(\sim 0 \%\)
\(T\)
\(T\)
(
1.64140056-2.571013*
1.64140056
1.91 10-12
'CENTER T
2)
~ 0\%
~ 0\%
flow
* Taking into account the precision machine, this value is an approximation into arithmetic finished the zero value.

\author{
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}

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7
Summary of the results

One checks the excellent adequacy of the calculated sensitivities, via the option SENSITIVITY of THER_LINEAIRE and of THER_NON_LINE, with the values by finished differences (the variation is lower than
10-10\%).
By crossing the results, it appears that the two thermal operators provide the same ones sensitivities and same finished differences. This variation could besides be tiny room while being more severe on the parameters of convergence of the process of optimization of THER_NON_LINE (with with less \(3.102 \%\) near). What is, in logical theory since the same matrix is assembled and them same second members. And what is also extremely reassuring as for the programming of these assemblies since they are carried out via distinct data-processing routes: one treats differently identical problems (resolution of a linear system for THER_LINEAIRE and algorithm of Newton for THER_NON_LINE) being also based on distinct modelings (resp. PLAN and PLAN_DIAG).

These results make it possible to check the correct operation of the two operators, with EXCHANGE, TEMP_IMPO, SOURCE and FLUX-REP, on meshs \(2 D\) lumpées or not, in both following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION),
- of sensitivity (via the suitable option of the thermal operators and TEST_RESU).

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\author{
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}

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SENST03-Sensitivities compared to a source and at the outside temperature

\section*{Summary:}

In this case-test, it is a question of making sure of the validity of the sensitivities compared to a source imposed and on outside temperature of a condition of exchange, in isotropic transitory linear thermics (key word SENSITIVITY of THER_LINEAIRE and THER_NON_LINE). It is based on the grid of case-test SENST01A
treating the transitory thermal response of a plate 2D_PLAN subjected to various thermal requests. One tests the good adequacy of the sensitivities sought with their approximation by finished differences, in several points of the grid and at the last moment of the transient. Strategy of calculation by finished differences,
more expensive, harder to implement and less robust, is not of course to privilege and does not have virtue that as a test...
One carries out these calculations of transitory linear thermics:
- with THER_LINEAIRE (modeling A with not lumpés isoparametric elements), - with THER_NON_LINE (modeling B with lumpés isoparametric elements).

They make it possible to check the correct operation of these two operators in the presence of the loadings
EXCHANGE, TEMP_IMPO, SOURCE and FLUX-REP, on meshs 2D lumpées or not, in both following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION), - of sensitivity (via the suitable option of the thermal operators and TEST_RESU).

The results of these two configurations agree quasi-parfaitement what, although logic in theory, is reassuring as for the robustness of programming of these two operators. Indeed, they treat differently identical problems (resolution of a linear system for \(A\) and algorithm of resting Newton for B) also on distinct modelings (resp. PLAN and PLAN_DIAG).
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\section*{1}

Problem of reference
One takes again here the geometry of the case-test SENST01A on which one will affect materials and loadings corresponding to no real case. It is here only about one functional validation, numerical and of data-processing not-regression of calculations of sensitivity in a situation very penalizing (many loadings in little space and with dependences in time polynomial!)

\subsection*{1.1 Geometry}

\section*{\(y\)}

OPPOSE
\(J\)
\(y=1,5\)

\section*{CENTER}
```

X
X
x=1
x=11

```

\section*{I}
```

$y=1,5$ ORIGIN

```

The four corners and the center are particularized:

\author{
Name \\ X \\ \(\boldsymbol{Y}\) \\ ORIGIN \\ 1-1,5 \\ I \\ 11-1,5 \\ \(J\) \\ 11,5 \\ OPPOSE \\ 11 1,5 \\ CENTER \\ 60
}

The various zones are indicated. Each internal rectangle is indicated under its name: MOITIE1 and MOITIE2.
Each frontier segment bears a name. The vertical segments on the left and on the right are named BORD_GAU and BORD_DRO. The lower horizontal segments are named BORD_IN1 and BORD_IN2; in the same way, the higher horizontal segments are named BORD_SU1 and BORD_SU2.

BORD_SU1
BORD_SU2

\author{
BORD_IN1
}

BORD_IN2
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1.2
Material properties
One applies to all the structure the characteristics material:
\(=075\)
W/m C
\(C=2 J \mathrm{~m} 3\)/
C

\section*{p}

\section*{1.3}

Boundary conditions and loadings
By noting \(T\) the moment considered, one can synthesize the decomposition of the loadings by zone under
the shape of the following table:
Geometrical zones
Loadings Sensitivities
(GROUP_NO/GROUP_MA)
sought
BORD_GAU
COEF_H \(=30(1+0.3 \mathrm{~T}) \mathrm{W} / \mathrm{m} 2^{\circ} \mathrm{C}\)
\(T_{-} E X T=140{ }^{\circ} \mathrm{C}\)
PS2

BORD_DRO
\(F L U X_{-} R E P=-120(1+0.2 T) W / m 2\)
ORIGIN
\(T E M P=100(1+0.1 \mathrm{~T}){ }^{\circ} \mathrm{C}\)
OPPOSE
\(T E M P=200(1+0.1 T){ }^{\circ} \mathrm{C}\)
MOITIE2
SOURCE \(=10 \mathrm{~J} / \mathrm{m} 3 \mathrm{~s}\)
PS1

\subsection*{1.4 Condition \\ initial}

Transitory thermal calculation on two steps of time \((T 1=5 S\) and \(t 2=10 S)\) initialized by a calculation
stationary (TEMP_INIT=_F (STATIONNAIRE=' OUI') ).
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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solutions
On such a case of figure, it is not possible to exhume an analytical solution! Calculations of sensitivity validated by centered differences are finished (what allows also at the same time to validate standard calculation).
This good adequacy of the sensitivities with their approximation by finished differences is tested in several points of the grid (I, J and CENTER), and at every moment of the transient (t0, T1 and t2), in structures of control PYTHON of the command file. It is traced in the file message (one tolerate a maximum variation of \(\mathbf{1 0 7 \%}\) ). In the file result one retained only the part of this test concerning the point CENTERS at the last moment of the transient (the sensitivities via a TEST_RESU and them differences finished via a TEST_FONCTION, with a relative tolerance of 106\%).
In the event of disagreement on results (NOOK), one can thus refine the diagnosis, that is to say in direction of standard problem, either of the derived problem, or of both.
Strategy of calculation by differences finished, more expensive, harder to implement and less robust, is not of course to privilege and has virtue only as a test...

\section*{2.2}

Result of reference
Without object.

\section*{2.3 \\ Uncertainty on the solutions}

Parameter of shift of the finished difference \((=0.1)\) and convergence of the grid.

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}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The grid is carried out with elements of the type TRIA3.
Calculation is made in linear thermics with operator THER_LINEAIRE (= 0.57).

In this case-test, it is a question of making sure of the validity of the sensitivities compared to the imposed source
on MOITIE2 (noted PS1 in the command file) and at the outside temperature of the condition of exchange imposed on the BORD_GAU (resp. PS2). These sensitivities are estimated at the last moment of
transient and at the point CENTERS.

\section*{3.2 \\ Characteristics of the grid \\ 80 TRIA3, 28 SEG2, 55 nodes}

\subsection*{3.3 Functionalities \\ tested}

\title{
DEFI_MATERIAU
}

THER
AFFE_CHAR_THER_F
EXCHANGE
TEMP_IMPO
FLUX_REP

\section*{SOURCE}

AFFE_MODELE
PLAN
THERMICS
DEFI_PARA_SENSI
MEMO_NOM_SENSI

\section*{THER_LINEAIRE}

SENSITIVITY

4

\subsection*{4.1 Values}
tested
One tests the validity of the calculated sensitivities, on the one hand, via the option SENSITIVITY of THER_LINEAIRE
(TEST_RESU "NON_REGRESSION"), and, in addition, by finished differences centered (TEST_FONCTION "AUTRE_ASTER"). The relative tolerance is \(1.106 \%\).

\section*{Identification SENSITIVITY Variation}
relative
Finished differences
Relative variation
(in \%)
(in \%)
\(T\)

\author{
( \\ 1.446555681012 .93 \\ 1012* \\ 1.446555681011 .51 \\ 1012* \\ \section*{CENTER T} \\ 2 ) \\ ~0\% \\ ~ 0\% \\ S \\ impo
}

\section*{\(T\)}
(
5.698578312 .73

1013*
5.69857831
-1.29 10-11

\section*{CENTER T}

2 )
~ 0\%
\(T\)
ext.
* Taking into account the precision machine, it is value is an approximation into arithmetic finished the zero value.
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\section*{5 Modeling \\ B}
5.1

Characteristics of modeling
Idem that for [\$3.1] but with operator THER_NON_LINE and in modeling PLAN_DIAG.

\section*{5.2}

Characteristics of the grid
Idem that for [§3.2].

\subsection*{5.3 Functionalities}
tested
Orders

DEFI_MATERIAU
THER_NL
AFFE_CHAR_THER_F
EXCHANGE
TEMP_IMPO
FLUX_REP
SOURCE
AFFE_MODELE
PLAN_DIAG
THERMICS
DEFI_PARA_SENSI
MEMO_NOM_SENSI
THER_NON_LINE
SENSITIVITY

\section*{6}
Results of modeling B

\author{
6.1 Values
}
tested

One tests the validity of the calculated sensitivities, on the one hand, via the option SENSITIVITY of THER_NON_LINE
(TEST_RESU "NON_REGRESSION"), and, in addition, by finished differences centered (TEST_FONCTION "AUTRE_ASTER"). The relative tolerance is 1.10-6\%.

Identification SENSITIVITY Variation relative
Finished differences relative Variation
(in \%)
(in \%)
\(T\)
(
1.446524411017 .12

1013*
1.446524411011 .6

1013*
CENTER T
2 )
~0\%
~0\%
\(S\)
impo

\section*{\(T\) \\ ( \\ 5.699294158 .38}

1013*
5.699294152 .34

1013*
CENTER T

2 )
~0\%
~0\%
\(T\)
ext.
* Taking into account the precision machine, it is value is an approximation into arithmetic finished the zero value.
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\section*{7}

Summary of the results
One checks the excellent adequacy of the calculated sensitivities, via the option SENSITIVITY of THER_LINEAIRE and of THER_NON_LINE, with the values by finished differences (the variation is lower than
10-10\%).

By crossing the results, it appears that the two thermal operators provide the same ones sensitivities and same finished differences. This variation could besides be tiny room while being more
severe on the parameters of convergence of the process of optimization of THER_NON_LINE (with with
less \(\mathbf{2 . 1 0 3 \%}\) near). What is, in logical theory since the same matrix is assembled and them same second members. And what is also extremely reassuring as for the programming of these assemblies since they are carried out via distinct data-processing routes: one treats differently identical problems (resolution of a linear system for THER_LINEAIRE and algorithm of Newton for THER_NON_LINE) being also based on distinct modelings (resp. PLAN and PLAN_DIAG).

These results make it possible to check the correct operation of the two operators, with EXCHANGE, TEMP_IMPO, SOURCE and FLUX-REP, on meshs 2D lumpées or not, in the two configurations following:
- in standard calculation (via the finished differences and TEST_FONCTION), - of sensitivity (via the suitable option of the thermal operators and TEST_RESU). Handbook of Validation
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\title{
SENST04 - Sensitivities compared to the coefficient of exchange-wall Dates
}

\author{
14/10/02
}

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\section*{Handbook of Validation}

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SENST04-Sensitivities compared to the coefficient of exchange-wall and with thermal conductivity

\section*{Summary:}

In this analytical case-test, it is a question of making sure of the validity of the sensitivities of the field of temperature and of
its flow compared to the coefficient of exchange-wall and conductivity, in isotropic transitory linear thermics
(key word SENSITIVITY of THER_LINEAIRE, THER_NON_LINE, CALC_ELEM and CALC_NO). It rests on
configuration of case-test TTLP100 treating the linear transitory thermal response of two separate plates
by a play in which one carries out a transfer of heat.
The problem is two-dimensional, but the limiting conditions make that the field of temperature reached
quickly the stationary state and depends analytically only on the \(X\)-coordinate and the data. One deduces some
then easily analytical expressions of the sensitivities of the temperature and its flow compared to thermal parameters which interest us: the coefficient of exchange-wall and conductivity.
One thus tests the good adequacy of the sensitivities sought with their analytical values in several points of the grid and at the last moment of the transient. To facilitate later maintenance and by concern of
complétude, one also calculates and one tests (via the functionalities PYTHON) the values obtained by differences
finished (only in temperature). But this last strategy, more expensive, harder to put in
work and less robust, is not of course to privilege and has virtue only as a test...
One carries out these calculations of transitory linear thermics:
\(\cdot\) with THER_LINEAIRE (modeling A),
\(\cdot\) with THER_NON_LINE (modeling B).
They make it possible to check the correct operation of the two thermal operators and their postprocessings
associated loadings ECHANGE_PAROI and TEMP_IMPO on isoparametric meshs 2D not lumpées (modeling PLAN), in the two following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION),
- of sensitivity (via the suitable option of the thermal operators and TEST_RESU).

The results of these two configurations agree quasi-parfaitement what, although logic in theory, is reassuring as for the robustness of programming of these two operators. Especially that they treat differently identical problems (resolution of a linear system for A and an algorithm of Newton for B).
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One takes again here quasi-intégralement the description of case-test TTLP100 "Exchange-wall in thermics transient".

\subsection*{1.1 Geometry}
\(y\)
1
0
\(X\)
\(l l=l 2=0.495\)
\(m\)
0
0.4950 .505
1
\(L=1 \mathrm{~m}\)
\(l 1\)
\(l 2\)

\section*{1.2}

Material properties
\(=W\)
40/m \(C\)
\(\circ\)
\(C\)
\(\mathbf{C}\)
10
3
-
7
J/m3 C
。
\(p\)

\title{
1.3 \\ Boundary conditions and loadings
}
\(T(X=0)=100^{\circ} \mathrm{C}=T 0\)
\(T(X=L)=300^{\circ} \mathrm{C}=T L\)
Heat transfer enters the walls located in \(x=0.495\) and \(x=0.505\), with a coefficient of exchange-wall of \(80 \mathrm{~W} / \mathrm{m} 2{ }^{\circ} \mathrm{C}\).

\author{
1.4 Conditions initial \\ \(T(T=0)=\) \\ T0 in the plate of left \\ TL in the plate of right-hand side \\ Handbook of Validation \\ Booklet V1.01 Tests of validity of orders \\ HI-23/02/017/A
}

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\(:\)
1
\(A\)
\(O\)
\(\vdots\)
\(V\)
3
2
Reference solutions

\section*{2.1 \\ Method of calculation used for the temperature}

The stationary field of temperature is obtained by solving a null Laplacian analytically on
each of the two plates, on the basis of a field solution of the form \(T(X)=a x+B .4\) coefficients ( 2 per plate) are obtained by clarifying the boundary conditions of Dirichlet and exchange:
\(H(T-T\)
\(L\)
\(O\) )
0. X 0495
\(: T=T o++H(l l+2) X\)
\(L\)
éq
2.1-1
\(H(T-T\)
\(L\)
O)

0505
\(X\).
\(1: T=T L-\)
\(+H(l l+l 2)(L X)\)
From where analytical expressions of the sensitivities of the thermal field compared to the coefficient of exchange-wall and compared to isotropic conductivity:
\(T\)
( \(T-T\)
\(L\)
O)
\(T\)
\(H(T-T\)
\(L\)
\(O\) )
0. \(X\)
:
495
0
and
= -
H
\[
\begin{aligned}
& (+H(L \\
& 1+2) \\
& X \\
& L \\
& 2 \\
& (+H(l l+) X \\
& L \\
& 2 \\
& 2 \\
& \text { éq } \\
& 2.1-2 \\
& T \\
& (T-T \\
& L \\
& O)
\end{aligned}
\]
\[
505
\]
\[
0
\]
\[
X:
\]
1
= -
( \(L-X\) )
\(T\)
H (T
\(T\)
\(L\)
O)
and
=
( \(L-X\) )
H
(+ H (L
\(1+l 2) 2\)
\((+H(l l+L) 2\)
2

\section*{2.2 \\ Method of calculation used for flow}

Field of temperature, one deduces the heat flux easily, answering the Fourier analysis:
F X
- H(TL - To 1

F
\(X\)
\(F=\)
=
\(\boldsymbol{X}, T)=-T(\boldsymbol{X}, T)\)
0.

0 .
:
495
F y
\(+H(\)
éq
2.2-1

1
\(L+12) 0\)
\(0.505 \times 1\) :. Idem
From where analytical expressions of the sensitivities of the heat flux compared to the coefficient of exchange-wall and compared to isotropic conductivity:
\(f x\)
2
F
H
- HTL-T 1

F
\(T X T\)
X
\(=\)
\(=\)
O
\(\boldsymbol{X}, T)\)
(, )
(
0.
```

0. 

:
4 9 5
= -
H
Fy
(+H(
éq 2.2-2
H
H
l
L+l2)20

```
\(H\)
0.505 X 1: Idem
fx
2
F
- HTL - H T
\(O\)
+ L 1
F
1
2
\(T \boldsymbol{X} T\)
X
\(=\)
\(\boldsymbol{X}, T)=-T(X, T)\)
(, )
(
)(
0.
0.
:
495
0.505 X 1:. Idem
éq 2.2-3
It is noticed that the commutation of these derivations with the operator gradient makes it possible to find
two different manners these formulas: via [éq 2.1-1] or [éq 2.1-2].
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\section*{2.3 \\ Results of reference}

Sensitivities sought on the line \(y=0\), with the \(X\)-coordinates \(X=0.25\) (N3), \(X=0.495\) (N5), \(X=0.505\) (N101) and \(X=0.75\) (N103); these points being symmetrical compared to the axis of symmetry of the structure
(
L
\(X=2\) ), the sensitivities of the field of temperature are identical there in absolute value but opposite signs:
\(T() T\)
\(T\)
\(T\)
\(N 5=-\)
\((\)
N101
and
( )
N5 = -
\((\)
N101
N101
\(H\)
\(H\)

Those in flow are constant on this line with a null component \(y\).

\section*{2.4 \\ Uncertainty on the solutions}

Analytical solutions.
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Version
6.0

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

N24
N4
N46
N124 N104 N146
N2
N6
N102
N106
N12
N34
N56
N112
N34
N156
N1
N5
N101
N105
N13
N3

The grid is carried out with elements of the type QUAD8.
Calculation is made in linear thermics, with \(=0.57\).
One takes 50 steps of times from 0 to 5102 S. the results are examined in \(T=5102 \mathrm{~S}\).
In this analytical case-test, it is a question of making sure of the validity of the sensitivities of the field of temperature and of its flow compared to the coefficient of exchange-wall (noted PS1 in the file of order) and with conductivity (resp. PS2), in isotropic transitory linear thermics. These functionalities are accessible via the key word SENSITIVITY from THER_LINEAIRE (cf [R4.03.02]) and

\section*{CALC_ELEM/CALC_NO.}

The problem is two-dimensional, but the limiting conditions make that the field of temperature reached quickly the stationary state and depends analytically only on the X-coordinate and the data. One in then deduced easily the analytical expressions from the sensitivities of the temperature and its flow by report/ratio with the thermal parameters which interest us: the coefficient of exchange-wall and conductivity (cf [\$2.1]).
One thus tests the good adequacy of the sensitivities sought with their analytical values in several points of the grid (cf [§2.2]) and at the last moment of the transient (via TEST_RESU). For to facilitate later maintenance and by concerns for complétude, one also calculates and one tests (via functionalities PYTHON and TEST_FONCTION) values obtained by finished differences (only in temperature). But this last strategy, more expensive, harder to implement and less robust, is not of course to privilege and has virtue only as a test...

Tests of not-regression being already set up for the field of temperature (via the differences finished and TEST_FONCTION) one did not believe good to reproduce them for flows. In the same order
of idea, since CALC_ELEM and CALC_NO carry out the same treatment on a EVOL_THER as it comes from THER_LINEAIRE or THER_NON_LINE, these tests on flows were not carried in modeling B.

\section*{3.2 \\ Characteristics of the grid}

4 QUAD8, 4 SEG3, 26 nodes
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\subsection*{3.3 Functionalities}
tested

\section*{Orders}

DEFI_MATERIAU
THER
AFFE_CHAR_THER_F
ECHANGE_PAROI
TEMP_IMPO
AFFE_MODELE
PLAN
THERMICS
DEFI_PARA_SENSI
MEMO_NOM_SENSI
THER_LINEAIRE
SENSITIVITY
CALC_ELEM
SENSITIVITY
FLUX_ELNO_TEMP
CALC_NO
SENSITIVITY
FLUX_NOEU_TEMP
TEST_RESU
SENSITIVITY
```

4
Results of modeling $A$

```

\section*{4.1 \\ Values tested in temperature}

One tests the validity of the sensitivities of the field of temperature calculated on the one hand via the option
SENSITIVITY of THER_LINEAIRE, and, in addition, by decentred finished differences, with respect to transitory analytical solutions (TEST_RESU and "ANALYTICAL" TEST_FONCTION). The tolerance relative is \(1.104 \%\).

\section*{Analytical identification Sensitivity}

\section*{Variation}
relative
Finished differences relative Variation (in \%)
(in \%)
\(T\)
1.407594251011 .407593211017 .35
1051.407593081018 .26

10-5
in N3
H
\(T\)
2.787036611012 .787034331018 .21
1052.787034071019 .12

10-5
in N5
H
\(T\)
2.787036611012 .7870343310
8.211052 .787034071019 .12

10-5
in N101
1
H
\(T\)
1.407594251011 .4075932110
7.351051 .407592941019 .27

10-5
```

in N103
l
H

```

\section*{\(T\)}
2.81518850 1012.8151875710
3.291052 .815187541013 .42

10-5
in
1

\section*{N3}
\(T\)
5.574073231015 .5740697510
6.251055 .574069571016 .57

10-5
in
1

N5
5.574073231015 .574069751016 .25
1055.574068971017 .64

10-5
in

N101
\(T\)
2.815188501012 .815187571013 .29
1052.815186971015 .44

10-5
in

N103

\section*{Code_Aster \({ }^{\circledR}\)}

Version
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\section*{4.2 \\ Values tested in flow}

One tests the validity of the sensitivities of the heat flux calculated via the option SENSITIVITY of CALC_ELEM and of CALC_NO ("ANALYTICAL" TEST_RESU). The relative tolerance, for the component
in \(X\), is \(1.103 \%\) and that absolute, for the component in \(y\), is \(1.1011 \%\). One is relatively less severe than previously because of approximation made in CALC_NO (average arithmetic simple of the fields to the nodes by element).

\section*{Component identification Sensitivity of \(F\)}

\section*{Component}

\section*{Sensitivity}

\section*{\(X\) Relative variation}
analytical of \(F\)
(in \%)
\(X\)
analytical of fy
of fy
F
2.252150801012 .252149041017 .79

10-5 0. -1.23
10-14
2.252150801012 .252184591011 .04
10-4 0. 1.92
10-14
in N5
H
F
2.252150801012 .252184591011 .04
10-4 0. -4.09
10-14
in N101
H
F
2.252150801012 .252149041017 .79
10-5 0. -8.11
10-15
in N103
H
F
8.918517181018 .918510621017 .36
10-5 0. -1.53
10-13
in
N3
F
8.918517181018 .918511541016 .32
10-5 0. 3.17
10-13
in
```

N5
F
8.918517181018.91851154101 6.32
10-5 0. -3.23
10-13
in
N101
F
8.91851718 101 8.91851062 1017.36
10-5 0. -7.60
10-14
in
N103
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```

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\section*{5 Modeling}

B
5.1

Characteristics of modeling

Idem that for [§3.1] with THER_NON_LINE cf [U4.54.02].

\section*{5.2 \\ Characteristics of the grid}

Idem that for [§3.2].

\subsection*{5.3 Functionalities}
tested
Orders

DEFI_MATERIAU
\(T H E R \_N L\)
AFFE_CHAR_THER_F
ECHANGE_PAROI
TEMP_IMPO
AFFE_MODELE
PLAN
THERMICS
DEFI_PARA_SENSI
MEMO_NOM_SENSI
THER_NON_LINE
SENSITIVITY
TEST_RESU
SENSITIVITY

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\section*{Titrate:}

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```

\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested
One tests the validity of the sensitivities calculated on the one hand via the option SENSITIVITY of THER_NON_LINE, and, in addition, by eccentric finished differences, with respect to the analytical solutions
transients (TEST_RESU and "ANALYTICAL" TEST_FONCTION). The relative tolerance is \(1.104 \%\).

\author{
Analytical identification \\ Sensitivity
}

Variation
Differences

\section*{Relative variation}
relative
finished
(in \%)
(in \%)
\(T\)
1.40759425101
1.407593211017 .35
1051.407593081018 .26

10-5
in N3
H
\(T\)
2.78703661101
2.787034331018 .21
1052.787034211018 .61

10-5
in N5
H
\(T\)
2.787036611012 .787034331018 .21
1052.787034071011 .06

10-4
in N101
H
\(T\)
1.407594251011 .407593211017 .35
1051.407592891019 .68

10-5
in N103
H
```

T
2.81518850 101 2.81518757 101 3.29
1052.81518711 1014.93
10-5
in
N3
T
5.574073231015.57406975 101 6.25
1055.574069231017.19
10-5
in
N5
T
5.57407323101
5.57406975 101 6.25

```

Titrate:
SENST04 - Sensitivities compared to the coefficient of exchange-wall Dates :

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Author (S):

\section*{O. BOITEAU Key}
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\section*{7 \\ Summary of the results}

One checks the good adequacy of the calculated sensitivities (in temperature and flow), via the option SENSITIVITY of THER_LINEAIRE, THER_NON_LINE, CALC_ELEM and CALC_NO, with the solutions
analytical. In particular, they respect strictly the properties of awaited symmetries.
It of in the same way sensitivities exhumed by finished differences (in temperature), which were not added here mainly that to meet later needs for maintenance and by concern of complétude.
Moreover the difference between the finished differences and the analytical sensitivities is lower than 105\%.
By crossing the results, it appears that the two thermal operators provide rigorously same sensitivities (with a margin of at least 108\%). What is, in theory, logic since one assembles even matrix and same second members. And what is also extremely reassuring as for programming of these assemblies since they are carried out via data-processing routes often distinct.
Linear" and "non-linear" finished differences the "are slightly different (with a margin of 105\%!), because
the field of temperature results, in the first case, of the resolution of a linear system, in the other case, of a non-linear solvor (phase of prediction + algorithm of Newton). But the difference is anecdotic (and could be reduced besides while being more severe on the parameters of convergence of the process of optimization) and confers the same feeling as for calculations of sensitivity: one is trustful as for the robustness of programming of these two operators, for it type of configuration.
These two modelings thus make it possible to check the correct operation of the thermal operators, with ECHANGE_PAROI and TEMP_IMPO on not lumpées meshs \(2 D\) (ECHANGE_PAROI is not available in lumpé, although this modeling is rather that recommended in thermics!
modeling PLAN), in the two following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION),
- of sensitivity (via the suitable option of the thermal operators and TEST_RESU).

One tests also the correct operation of the operators of postprocessings CALC_ELEM and CALC_NO for the calculation of the heat fluxes and their sensitivities.

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SENSM04 - Sensitivity to the data materials in linear elasticity
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Author (S):
NR. TARDIEU Key
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Organization (S): EDF/AMA

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Document: V1.01.155

\title{
SENSM04-Sensitivity to the data materials \\ in linear elasticity orthotropic 2D
}

\section*{Summary:}

One tests the calculation of sensitivity to the parameters materials in orthotropic linear elasticity. The sensitivity is calculated by direct differentiation of the discrete equations. The reference comes from a calculation by differences finished.

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NR. TARDIEU Key

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1
Problem of reference

\subsection*{1.1 Geometry}
3. mm
BORD_GAU
CENTER

\(J\)
ORIGIN
\(y\)
\(10 . m m\)
\(Z\)
\(1 .<X<11\)
\(1.5<Y<1.5\)
\(X\)
1.2

\section*{Properties of material}
\[
E \_L
\]
\[
=2 \cdot E 5 \mathrm{MPa}
\]
\[
E_{-} T
\]
\[
=1 \cdot E 5 \mathrm{MPa}
\]
\[
E \_N
\]
\[
=3 . E 5 \mathrm{MPa}
\]
\[
N U_{\_} L T=0.2 \mathrm{MPa}
\]
\[
N U_{\_} L N=0.3 \mathrm{MPa}
\]
\[
N U_{-}^{-} T N=0.4 \mathrm{MPa}
\]
\[
G \_L T
\]
\[
=2 . E 5 \mathrm{MPa}
\]
\[
G_{-} L N=1 . E 5 \mathrm{MPa}
\]
\[
G_{-} T N=3 . E 5 \mathrm{MPa}
\]

\title{
1.3 \\ Boundary conditions and loadings
}

Surface BORD_GAU is blocked according to X.
The line ORIGIN is blocked according to \(X\) and \(Y\).
A nodal force of components (10. 3.) is imposed on the line OPPOSES.
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation}

For the sensitivity of the field displacement to the imposed nodal force, the solution comes from finished differences (approximation of derived with order 1) carried out with Aster. These finished differences express themselves in the form:
\(F(X, y)\)
\(F(X+d x, y)-F(X, y)\)
\(X\)
\(d x\)
\(F(X, y)\)
\(F(X, y+D y)-F(X, y)\)

\section*{2.2 \\ Sizes and results of reference}

Sensitivity of displacement following y to parameter E_L to item I:
D uy/d E_L (1, 1.5, Z) -2.8790892731628E-09
Sensitivity of displacement following y to parameter \(E \_T\) to item I:
D uy/d E_T (1, 1.5, Z) -1.0802159291247E-11
Sensitivity of displacement following y to parameter NU_LT to item I:
D uy/d NU_LT (1, 1.5, Z) -2.2822247794372E-04
Sensitivity of displacement following y to parameter \(N U_{-} L N\) to item I:
D uy/dNU_LN (1, 1.5, Z) 0.00
Sensitivity of displacement following y to parameter NU_TN to item I:
D uy/dNU_TN (1, 1.5, Z) 0.00
Sensitivity of displacement following y to parameter G_LT to item I:
D uy/d G_LT (1, 1.5, Z) -3.4017574169407E-10
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Modeling in plane constraints on the grid following 2D:

Surface BORD_GAU becomes a line, the lines ORIGIN and OPPOSES become points.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 181
Numbers and types of meshs: 80 SEG3, 50 QUAD8

\subsection*{3.3 Functionalities}
tested

\author{
Orders
}

\section*{MEMO_NOM_SENSI}

NOM_UN
NOM_ZERO
NAME
NOM_SD
PARA_SENSI
NOM_COMPOSE
MECA_STATIQUE
SENSITIVITY
TEST_RESU
SENSITIVITY

\section*{3.4}

Sizes tested and results
Size tested
Reference:
Code_Aster
Difference (\%)
D uy/d E_L (Node I)
2.8790892731628E-09
2.8790892731628E-09
0.0

\section*{4 \\ Summary of the results}

The results of sensitivity per direct differentiation are in very good agreement with those given by finished differences.
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Titrate:
SENSM05 - Sensitivity to the data materials in linear elasticity
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19/08/02
Author (S):
NR. TARDIEU Key

\author{
Handbook of Validation
}

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Document: V1.01.156

SENSM05 - Sensitivity to the data materials
in linear elasticity orthotropic 3D

\section*{Summary:}

One tests the calculation of sensitivity to the parameters materials in orthotropic linear elasticity. The sensitivity is calculated by direct differentiation of the discrete equations. The reference comes from a calculation by differences finished.

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\section*{Titrate:}
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{I}
OPPOSE
3. \(m m\)
BORD_GAU

CENTER

ORIGIN
J
\(y\)
10. mm

Z
1. \(<X<11\)
\(1.5<Y<1.5\)
\(0<Z<1\)
X
1.2
Properties of material
E_L
\(=2 . E 5 \mathrm{MPa}\)
E_T
\(=1 . E 5 \mathrm{MPa}\)
\(E_{-} N\)
\(=3 . E 5 \mathrm{MPa}\)
\(N U_{-} L T=0.2 \mathrm{MPa}\)
\(N U_{-} L N=0.3 \mathrm{MPa}\)
\(N U_{-} T N=0.4 \mathrm{MPa}\)
G_LT
\(=2 . E 5 \mathrm{MPa}\)
\(G_{-} L N=1 . E 5 \mathrm{MPa}\)
\(G_{-} T N=3 . E 5 \mathrm{MPa}\)

\section*{1.3 \\ Boundary conditions and loadings}

Line BORD_GAU is blocked according to \(X, y\) and \(Z\).
Item I is blocked according to \(X, y\) and \(Z\).
A nodal force of components (100. , 30. , 10.) is imposed on the point OPPOSES.
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NR. TARDIEU Key

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\section*{2.1 \\ Method of calculation}

For the sensitivity of the field displacement to the imposed nodal force, the solution comes from finished differences (approximation of derived with order 1) carried out with Aster. These finished differences
express themselves in the form:
```

F (X, y)
F}(X+dx,y)-F(X,y
X
dx
F (X, y)
F(X, y + Dy) - F (X, y)

```
\(y\)
Dy
2.2

Sizes and results of reference

Sensitivity of displacement following y to parameter \(E_{-} L\) to the point OPPOSES:
Duy/d E_L (11, 1.5, 0) 7.4460837925563E-10
Sensitivity of displacement following y to parameter E_T to the point OPPOSES:
D uy/d E_T (11, 1.5, 0) -4.1930886371059E-09
Sensitivity of displacement following y to parameter \(E_{-} N\) to the point OPPOSES:
\(D u y / d E \_N(11,1.5,0)-2.9508876027760 E-10\)
Sensitivity of displacement following y to parameter NU_LT to the point OPPOSES:
D uy/d NU_LT (11, 1.5, 0) -1.6847821550311E-03
Sensitivity of displacement following y to parameter NU_LN to the point OPPOSES: D uy/d NU_LN (11, 1.5, 0) 1.0775321899662E-05

Sensitivity of displacement following y to parameter NU_TN to the point OPPOSES:
D uy/d NU_TN (11, 1.5, 0) 2.0653103649623E-05
Sensitivity of displacement following y to parameter G_LT to the point OPPOSES:

D uy/d G_LT (11, 1.5, 0) -2.5910680787136E-09
Sensitivity of displacement following y to parameter G_LN to the point OPPOSES: D uy/d G_LN (11, 1.5, 0) 5.6274647519574E-10

Sensitivity of displacement following y to parameter \(G_{-} T N\) to the point OPPOSES:
D uy/d G_TN (11, 1.5, 0) -1.1579216129805E-09

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Modeling in plane constraints on the grid following 3D:}

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 331
Numbers and types of meshs: 930 SEG2, 970 QUAD4, 200 HEXA8

\subsection*{3.3 Functionalities}
tested
Orders

\author{
MEMO_NOM_SENSI \\ NOM_UN \\ NOM_ZERO \\ NAME \\ NOM_SD \\ PARA_SENSI \\ NOM_COMPOSE \\ MECA_STATIQUE \\ SENSITIVITY \\ TEST_RESU \\ SENSITIVITY
}

\author{
3.4 \\ Sizes tested and results
}

Size tested
Reference
Code_Aster
Difference (\%)
D uy/d E_L (Node OPPOSES)
7.4460837925563E-10 7.4460837925563E-10
0.0

D uy/d E_T (Node OPPOSES)
-4.1930886371059E-09-4.1930886371059E-09
0.0

D uy/d E_N (Node OPPOSES)
-2.9508876027760E-10 -2.9508876027760E-10
0.0

D uy/d NU_LT (Node OPPOSES) -1.6847821550311E-03 -1.6847821550311E-03
0.0

D uy/d NU_LN (Node OPPOSES) 1.0775321899662E-05 1.0775321899662E-05
0.0

D uy/d NU_TN (Node OPPOSES) 2.0653103649623E-05 2.0653103649623E-05
0.0

D uy/d G_LT (Node OPPOSES)
-2.5910680787136E-09 -2.5910680787136E-09
0.0

\title{
D uy/d G_LN (Node OPPOSES) \\ 5.6274647519574E-10 5.6274647519574E-10 \\ 0.0 \\ D uy/d G_TN (Node OPPOSES) \\ -1.1579216129805E-09-1.1579216129805E-09 \\ 0.0 \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
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\section*{4 \\ Summary of the results}

The results of sensitivity per direct differentiation are in very good agreement with those given by finished differences.
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Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.2

\section*{Titrate:}

SENSD01 - Sensitivity to the loading and C.L in dyna_line_harm
Date:
23/10/02
Author (S):
Key S. CAMBIER
:
V1.01.158-A Page:
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Organization (S): EDF-R \& D /AMA

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of the orders \\ Document: V1.01.158
}

SENSD01 - Sensitivity to the loading and C.L in dyna_line_harm

\section*{Summary:}

This case test takes again the geometry and the materials of the case test SDLD21 "System massarises to 8 ddl with
viscous shock absorber " [V2.01.021]. Calculation carried out is the harmonic response of a dynamic system
linear discrete.

The functionality tested is the derivation of the fields of displacement, speed and acceleration compared to
boundary conditions and with the loadings. These boundary conditions and loadings differ compared to
case test SDLD21. Indirectly, functionality of calculation with a real loading in entry of dyna_lina_harm is tested.

The reference calculated using the formal computation software MAPLE is semi-analytical.

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}
m
m
m
B
\(\boldsymbol{X}, \boldsymbol{U}\)
P1
P2
P3
P8
C
C
C
C

\section*{1.2}

Properties of material
Stiffness in translation
\(K=105 \mathrm{~N} / \mathrm{m}\)
Specific mass
\(m=10 \mathrm{~kg}\)
Viscous damping
\(C=50 N(\mathrm{~m} / \mathrm{s})\)

\section*{1.3}

Boundary conditions and loadings
Boundary conditions:
Points \(A\) and \(B\) (nodes 1 and 10): null displacements:
\(u P i=0\)

Not P1 (node 2):
imposed displacement: \(U\)
\(5 \mathrm{~Hz} F 40 \mathrm{~Hz}\)
1
1
1
1
1
\(D \sin (2 F T)\)
1
\(D=2 . m\)

\section*{Loading:}
```

Not P4 (node 5): nodal force according to X: FP4=1
$F \sin (2 F T)$,
1
F =2.N, $5 \mathrm{~Hz} \mathbf{F} 40 \mathrm{~Hz}$

```
Not P5 (node 6): nodal force according to X: FP5= 2
\(\boldsymbol{F} \boldsymbol{\operatorname { s i n }}(\mathbf{2} \boldsymbol{F} \boldsymbol{T})\),
2
F =0.N, \(5 \mathrm{~Hz} \mathbf{F} 40 \mathrm{~Hz}\)

\subsection*{1.4 Conditions} initial

\section*{Without object}

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2
Reference solution
2.1

\section*{Method of calculation}

The reference is calculated using the formal computation software MAPLE v6 with the orders following:

Generic elementary matrices
> precision: =double:
> Kelem: =array ([[K, - K], [- K, K]]):
\(>\) Celem: =array ([[C, - C], [- C, C]]):
> Melem: =array ([[Mg, 0], [0, Mandelevium]]) /2:
\(>\) Zelem: =evalm (Kelem+I*omega*Celem-omega^2*Melem):
Associated matrices has each element
> valK: =1e5:
> valC: =50:
\(>\) valM: =10:
\(>\) regle1: \(=\{k=k A 1, c=c A 1, m g=v a l M, m d=m 1\}:\)
> ZA1: =subs (regle1, evalm (Zelem)):
> regle \(2:=\{k=k 12, c=c 12, m g=m 1, m d=v a l M\}\) :
> Z12: =subs (regle2, evalm (Zelem)):
\(>\) reglegen: \(=\{k=v a l K, c=v a l C, m g=v a l M, m d=v a l M\}\) :
> Zgen: =subs (reglegen, evalm (Zelem)):
Assembly
> Ail: =array ([[0,0,0,0,0,0,0,0], [1,0,0,0,0,0,0,0]]):
\(>\) Ai2: =array ([[1,0,0,0,0,0,0,0], [0,1,0,0,0,0,0,0]]):
> Ai3: =array ([[0,1,0,0,0,0,0,0], [0,0,1,0,0,0,0,0]]):
> Ai4: =array ([[0,0,1,0,0,0,0,0], [0,0,0,1,0,0,0,0]]):
> Ai5: =array ([[0,0,0,1,0,0,0,0], [0,0,0,0,1,0,0,0]]):
> Ai6: =array ([[0,0,0,0,1,0,0,0], [0,0,0,0,0,1,0,0]]):
> Ai7: =array ([[0,0,0,0,0,1,0,0], [0,0,0,0,0,0,1,0]]):
\(>\) Ai8: =array ([[0,0,0,0,0,0,1,0], [0,0,0,0,0,0,0,1]]):
> Ai9: =array ([[0,0,0,0,0,0,0,1], [0,0,0,0,0,0,0,0]]):
>
Z: =evalm (transposes (Ai1) \(\& * Z A 1 \& * A i 1+\) transpose (Ai2) \(\& * Z 12 \& * A i 2+\) transpose (Ai3) \&*Zgen\&*A
i3+transpose (Ai4) \& *Zgen\&*Ai4+transpose (Ai5) \& *Zgen\&*Ai5+transpose (Ai6) \& *Zgen\&*Ai6 \(+t\) ranspose (Ai7) \&*Zgen\&*Ai7+transpose (Ai8) \&*Zgen\&*Ai8+transpose (Ai9) \& *Zgen\&*Ai9):
\(>\) with (linalg):
Resolution and calculation of the derivatives
\(>\) affect_p: \(=\{k A 1=v a l K, k 12=v a l K, c A 1=v a l C, c 12=v a l C, m 1=v a l M\}\) :
\(>Z_{-} w:=m a p(e v a l f\), subs (affect_p, evalm (Z)));
> om: \(=40 * 2 *\) evalf (pi):
\(>\) affect: \(=\{o m e g a=o m\}\) :
> Z_num: =Matrix (subs (affect, evalm (Z_w))):
> Zll: = Z_num [2..8, 2..8]:
> Zli: = Matrix (7,1, Z_num [2..8, 1]):
\(>\) affect_F: \(=\{F 1=2, F 2=0\}\) :
\(>\) CH: =Vector (7, [0,0,0, F1, F2,0,0]);
> CH_impo: =evalm (CH-evalm (D1*Zli)):
> CH_num: =subs (affect_F, evalm (CH_impo));
> depl_D1: =evalm (linsolve (Zll, CH_impo)):
derived compared to 1
D
> DdeplDimpo: =map (simplify, evalm (map (diff, evalm (depl_D1), D1))); derived compared to 1
F
> DdeplDF1: =map (simplify, evalm (map (diff, evalm (depl_D1), F1))); derived compared to 2
F
> DdeplDF2: =map (simplify, evalm (map (diff, evalm (depl_D1), F2)));
affect_impo: \(=\{D 1=2, F 1=2, F 2=0\}\) :
nonderived displacements
> depl_D1_num: =subs (affect_impo, evalm (depl_D1));
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\section*{2.2}

Results of reference
To 40 Hz :
displacements of node 3,
derived compared to 1

D of displacements of node 3, derived compared to 1
F of displacements of node 5, derived compared to 2
\(F\) of displacements of node 5 .

\subsection*{2.3 Uncertainties}

Semi-analytical solution

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Modeling identical to that of the SDLD25A}
\(y\)
With
\(P\)
\(P\)
B
\(X\)
1
2
P3
P4
P5
P6
P7
P8

\section*{Characteristics of the elements:}

DISCRETE:
with nodal masses
\(M_{-} T_{-} D_{-} N\)
and matrices of rigidity
\(K_{-} T_{-} D_{-} L\)

\section*{and matrices of damping \\ \(A_{-} T_{-} D_{-} L\)}

Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES"
DY: 0. , DZ: 0. )
with the nodes ends
(GROUP_NO: AB
DX: 0. )
Names of the nodes:
Not \(A=N 1\)
\(P 1=N 2\)
Not \(B=N 10\)
\(P 2=N 3\)
\(P 8=N 9\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and types: 9 SEG2
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\subsection*{3.3 Functionalities}
tested
The functionality tested is the derivation of the fields of displacement, speed and acceleration by report/ratio in the boundary conditions and with the loadings. These boundary conditions and loadings differ compared to the case test SDLD21. Indirectly, functionality of calculation with one real loading in entry of dyna_lina_harm is also tested.

Orders

DYNA_LINE_HARM
SENSITIVITY
EXCIT
CHARGE

\subsection*{3.4 Values \\ tested \\ Parts real and imaginary of component DX of the displacement of node 3 \\ Frequency Reference \\ Aster \% \\ Difference \\ 40 Hz \\ 4.8176281E01 \\ 4.8176312957349E01 \\ 7.58E 05\% \\ 1.0074952E01 \\ 1.0074971204938E01 \\ Derived compared to 1 \\ F of the parts real and imaginary of component DX of displacement of}
node 5
Frequency Reference
Aster \%
Difference
40 Hz
2.5695869E06
2.5694461510682E06
0.020 \%
2.7550424E07
2.7501154222522E07

Derived compared to 2
\(F\) of the parts real and imaginary of component \(D X\) of displacement of node 5

Frequency Reference
Aster \%
Difference
40 Hz
6.0508111E07
6.0508111466796E07
1.22E 06\%
1.9567292E07
1.9567292616373E07

Derived compared to 1
\(D\) of the parts real and imaginary of component \(D X\) of displacement of node 5

Frequency Reference
Aster \%
Difference
40 Hz
2.4088143E01
2.4088143455385E01
1.89E 06\%
5.0374785E02
5.0374785980418E02

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Summary of the results
The precision of the results is coherent with the method of resolution used and the method of direct derivation "exact".

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Titrate:
ZZZZ159 - Retiming of an elastoplastic law of behavior
Date:
03/06/03
Author (S):
NR. TARDIEU Key
\(\cdot\)
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Organization (S): EDF-R \& D /AMA

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ Document: V1.01.159
}

ZZZZ159 - Retiming of a law of behavior elastoplastic on a tensile test

\section*{Summary:}

One tests macro retiming MACR_RECAL on the simple case of the identification of a law of behavior elastoplastic of Von Mises on a simple tensile test controlled in displacements. Readjusted parameters are the Young modulus, the elastic limit and the slope of work hardening starting from the knowledge of constraints and of the plastic deformation cumulated in the test-tube.

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\section*{Author (S):}

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{Traction of 5.E-3 mm}

\section*{1.2}

Properties of material
The initial values of the parameters are as follows:
\(E=100000 . \mathrm{MPa}\)
\(y=1000 . M P a\)
\(E T=30 . \mathrm{MPa}\)
The values which one wishes to obtain are (see method of calculation of the reference solution):
\(E=200000 . \mathrm{MPa}\)
\(y=200\). MPa
\(E T=2000 . \mathrm{MPa}\)

\section*{1.3 \\ Boundary conditions and loadings}

A homogeneous state of stresses is sought: one imposes only one vertical displacement of 5.E-3 Misters.

\section*{2.1 \\ Method of calculation}

This calculation is a validation macro MACR_RECAL. With this intention, the step is as follows:
- one chooses a value (known as "value to be identified") for each parameter and one does it calculation. One thus obtains a history of constraint and cumulated plastic deformation, - one supposes now that the values to be identified preceding are unknown for us. Our only information is the history of constraint and cumulated plastic deformation that us will thus considèrerons like an experimental measurement, - one then launches optimization to this pseudo experimental measure while taking for each one parameters an arbitrary value,
- one checks that the values identified by the algorithm are well the values to be identified.

This step is very traditional in optimization where it makes it possible to validate the algorithms.
2.2

Sizes and results of reference
The reference variables are the values of the parameters with convergence is:
\(E=200000 . \mathrm{MPa}\)
\(y=200 . \mathrm{MPa}\)
\(E T=2000 . \mathrm{MPa}\)
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Axisymmetric modeling on the following grid:

In this modeling, initial values and acceptable fields of the various parameters are:
- Modulus Young:

100000 [50000.,500000.]
- Pente of work hardening: 1000. [500. , 10000.]
- Limit elastic:
30.
[5.,500.]

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 4
Numbers and types of meshs: 4 SEG2, 1 QUAD4

\subsection*{3.3 Functionalities \\ tested \\ Orders}
MACR_RECAL UNITE_ESCL
RESU_EXP
RESU_CALC
LIST_PARA
ITER_MAXI
RESI_GLOB_RELA
UNITE_RESU
GRAPH
UNIT
INTERACTIVE
3.4
Sizes tested and results
Sizes tested
Values obtained
Analytical values
Variation (\%)
Young modulus ..... 199999.89
200000.00
5.61E-05

Elastic limit
2000.00
-0.002
Slope of work hardening
200.00 ..... 200.00
1.39E-04
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\section*{4 Modeling B}

\section*{4.1 \\ Characteristics of modeling}

Axisymmetric modeling on the following grid:

In this modeling, initial values and acceptable fields of the various parameters are:
- Modulus Young:

100000 [50000.,150000.]
- Pente of work hardening: 2000. [500. , 10000.]
- Limit elastic:
30.
[5.,500.]

As one can note it, the solution of the problem does not belong to the acceptable field. It is it this modeling drank which validates the management of the terminals.

\section*{4.2 \\ Characteristics of the grid}

\author{
A number of nodes: 4
}

Numbers and types of meshs: 4 SEG2, 1 QUAD4

\subsection*{4.3 Functionalities \\ tested}

Orders

MACR_RECAL UNITE_ESCL
RESU_EXP
RESU_CALC
LIST_PARA
ITER_MAXI
RESI_GLOB_RELA
UNITE_RESU
GRAPH
UNIT
INTERACTIVE

\section*{4.4 \\ Sizes tested and results}

\section*{Sizes tested}

Values obtained
Analytical values
Variation (\%)
Young modulus
150000.00
150000.00

Slope of work hardening
158.50
158.50
0.003

\section*{5 \\ Summary of the results}

The results of optimization are obtained in a small iteration count (5) and are of very good quality.
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SENST05 - Sensitivities compared to voluminal heat
Date:
14/10/02
Author (S):
O. BOITEAU Key

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Organization (S): EDF/SINETICS

SENST05-Sensitivities compared to heat voluminal and with emissivity

\section*{Summary:}

In this case-test, it is a question of making sure of the validity of the sensitivities compared to voluminal heat and to
the emissivity of a radiation, in isotropic transitory non-linear thermics lumpée (key word SENSITIVITY of
THER_NON_LINE). It is based on the grid of case-test SENST01A treating the transitory thermal answer
of a plate 2D_PLAN subjected to various thermal requests.
One tests the good adequacy of the sensitivities sought with their approximation by finished differences, in
several points of the grid and at the last moment of the transient. Strategy of calculation by finished differences,
more expensive, harder to implement and less robust, is not of course to privilege and does not have virtue that as a test...

One thus checks the correct operation of THER_NON_LINE with loadings EXCHANGE, TEMP_IMPO and
RADIATION, on lumpées isoparametric meshs 2D, in the two following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION),
- of sensitivity (via the suitable option of the thermal operator and TEST_RESU).

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\section*{1}

Problem of reference
One takes again here the geometry of the case-test SENST01A on which one will affect materials and loadings corresponding to no real case. It is here only about one functional validation, numerical and of data-processing not-regression of calculations of sensitivity in a situation very penalizing (many loadings in little space and with dependences in time polynomial!)
1.1 Geometry
\(y\)

OPPOSE
\(J\)
\(y=1,5\)

CENTER
X
\(\boldsymbol{X}\)
\(x=1\)
\(x=11\)

I
\(y=1,5\) ORIGIN

\section*{The four corners and the center are particularized:}

\author{
Name
}

X
\(\boldsymbol{Y}\)
ORIGIN
1 -1,5
I
11-1,5
J
1 1,5
OPPOSE
11 1,5
CENTER
60

The various zones are indicated. Each internal rectangle is indicated under its name: MOITIE1 and MOITIE2.
Each frontier segment bears a name. The vertical segments on the left and on the right are named BORD_GAU and BORD_DRO. The lower horizontal segments are named BORD_IN1 and BORD_IN2; in the same way, the higher horizontal segments are named BORD_SU1 and BORD_SU2.

BORD_SU1
BORD_SU2

BORD_GAU
BORD_DRO
MOITIE1
MOITIE2

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\section*{1.2}

Material properties
To all the structure one applies the characteristics material:
\[
=75
\]

C
3
2
/
\(p=\)
\(J \boldsymbol{m}{ }^{\circ} \mathrm{C}\) sensibilit
PS1
sought
é

\section*{1.3}

Boundary conditions and loadings
By noting T the moment considered, one can synthesize the decomposition of the loadings by zone under
the shape of the following table:

\section*{Geometrical zones}

Loadings Sensitivity
(GROUP_NO/GROUP_MA)
sought
BORD_GAU
COEF_H \(=30 \mathrm{~W} / \mathrm{m} 2^{\circ} \mathrm{C}\)
\(T_{-} E X T=140{ }^{\circ} \mathrm{C}\)
BORD_DRO
SIGMA \(=1.108 \mathrm{~W} / \mathrm{m} 2\left({ }^{\circ} \mathrm{C}\right) 4\)
EPSILON = 0.3
PS2
TEMP_EXT \(=500^{\circ} \mathrm{C}\)
ORIGIN
\(T E M P=100(1+0.1 \mathrm{~T}){ }^{\circ} \mathrm{C}\)

\subsection*{1.4 Condition \\ initial}

Transitory thermal calculation on two steps of time \((T 1=5 S\) and \(t 2=10 S)\) initialized by a calculation
stationary (TEMP_INIT=_F (STATIONNAIRE=' OUI')).
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2
Reference solution

```

\section*{2.1 \\ Method of calculation used for the reference solutions}

On such a case of figure, it is not possible to exhume an analytical solution! Calculations of sensitivity validated by centered differences are finished (what allows also at the same time to validate standard calculation).
This good adequacy of the sensitivities with their approximation by finished differences is tested in several points of the grid (I, J and CENTER), and at every moment of the transient (t0, T1 and t2), in structures of control PYTHON of the command file. It is traced in the file message (one tolerate a maximum variation of \(107 \%\) ). In the file result one retained only the part of this test concerning the point CENTERS at the last moment of the transient (the sensitivities via a TEST_RESU and them
differences finished via a TEST_FONCTION, with a relative tolerance of 106\%).
In the event of disagreement on results (NOOK), one can thus refine the diagnosis, that is to say in direction of
standard problem, either of the derived problem, or of both.
Strategy of calculation by differences finished, more expensive, harder to implement and less robust, is not of course to privilege and has virtue only as a test...

\section*{2.2 \\ Result of reference \\ Without object.}

\section*{2.3 \\ Uncertainty on the solutions}

Parameters of shift of the finished difference \((1=104,2=106)\) and convergence of the grid.
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3 Modeling
With

\section*{3.1 \\ Characteristics of modeling}

The grid is carried out with elements of the type TRIA3.
Calculation is made in linear thermics with operator THER_NON_LINE (= 0.57).

In this case-test, it is a question of making sure of the validity of the sensitivities compared to voluminal heat
on all structure (noted PS1 in the command file) and with the emissivity of the radiation imposed on the BORD_DRO (resp. PS2). These sensitivities are estimated at the last moment of the transient and at the point CENTERS.

\section*{3.2 \\ Characteristics of the grid \\ 80 TRIA3, 28 SEG2, 55 nodes}

\subsection*{3.3 Functionalities}
tested
Orders

DEFI_MATERIAU
THER_NL
AFFE_CHAR_THER_F
EXCHANGE
TEMP_IMPO
RADIATION

\author{
AFFE_MODELE \\ PLAN_DIAG \\ THERMICS \\ DEFI_PARA_SENSI \\ MEMO_NOM_SENSI \\ THER_NON_LINE \\ SENSITIVITY
}

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
One tests the validity of the calculated sensitivities, on the one hand, via the option SENSITIVITY of THER_NON_LINE
(TEST_RESU "NON_REGRESSION"), and, in addition, by finished differences centered (TEST_FONCTION "AUTRE_ASTER"). The relative tolerance is \(1.106 \%\).

Identification SENSITIVITY Variation
relative
Finished differences relative Variation
(in \%)
(in \%)
\(T\)
2.16993453 10-1
8.7 10-13*
2.16993453101
-8.83 10-13*
```

(
CENTER

```
~0\%
~0\%
Cp) (
, t2)

\section*{\(T\) \\ ( \\ 8.12296002 \\ 8.75 10-14* \\ 8.12296002 \\ 4.15 10-13* \\ CENTER T}
2)
~0\%
\(\sim 0 \%\)
* Taking into account the precision machine, it is value is an approximation into arithmetic finished the zero value.
Handbook of Validation
Booklet V1.01 Tests of validity of orders
HI-23/02/017/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.0

Titrate:
SENST05-Sensitivities compared to voluminal heat
Date:
14/10/02
Author (S):
O. BOITEAU Key

\section*{5}

Summary of the results
One checks the excellent adequacy of the calculated sensitivities, via the option SENSITIVITY of THER_NON_LINE, with the values by finished differences (the variation is lower than 1010\%).

These results make it possible to check the correct operation of the operator with the loadings EXCHANGE, TEMP_IMPO and RADIATION, on lumpées isoparametric meshs 2D, in two following configurations:
- in standard calculation (via the finished differences and TEST_FONCTION), - of sensitivity (via the suitable option of the thermal operator and TEST_RESU).

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Booklet V1.01 Tests of validity of orders
HI-23/02/017/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
ZZZZ161 - Exchange with the format MED, data-processing control
Date:
03/06/03
Author (S):
G. Key NICOLAS
:
V1.01.161-A Page:
1/6
Organization (S): EDF-R \& D /SINETICS

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ Document: V1.01.161 \\ ZZZZ161 - Exchange with the format MED, control data processing
}

\section*{Summary:}

This case test controls the writings of fields to format MED and their second reading by ASTER. It validates in particular
the writing of fields nondefinite on the complete field or definite on several types of elements. It controls
also the exchange of fields of size.
It should be noted that mechanical calculation is only one pretext and does not have any ambition of validation in oneself.

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-23/03/003/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
ZZZZ161 - Exchange with the format MED, data-processing control
Date:
03/06/03
```

Author (S):
G. Key NICOLAS
:
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1
Problem of reference

```

\subsection*{1.1 Geometry}

The field is a rectangle of size 10x20, separate in 4 sections of \(10 \times 5.3\) points are associated this rectangle: \(M, N R\) and \(O\).

\section*{J}

OPPOSE
11
\(O\)

WITH B C
D
6

NR

1
MR. ORIGIN
I

02
7
12
17
22

\section*{1.2 \\ Properties of material \\ Two materials are used: \\ \(E=180.000\) N.m2 \\ \(=0,3\) \\ = 1,5 10-7 \\ \(L=7700 \mathrm{~kg} . \mathrm{m} 3\)}

Material 1 has the following characteristics:

It is applied in the sections \(A\) and \(B\).
The second material has the following characteristics:
\(E=220.000\) N.m2
\(=0,3\)
= 1,6 10-7
\(L=8300 \mathrm{~kg} . \mathrm{m} 3\)
It is applied in sections \(C\) and \(D\).
1.3

Boundary conditions and loadings
At the point "ORIGIN", displacement is null according to \(Y\).
On all the left edge, i.e. between the points "ORIGIN" and "I it normal displacement is no one.
On the higher edge, i.e. between the points "I and "OPPOSE", the pressure is imposed on 1000.

\subsection*{1.4 Conditions}
initial
Without object.
Handbook of Validation
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2
Reference solution
2.1

Method of calculation used for the reference solutions
Calculation is in static mechanics. It is only one pretext required a result.

\section*{2.2}

Results of reference
To the node "OPPOSES", displacement has the following values:
\[
\begin{aligned}
& u x=0,4183044 \\
& u y=1,639849
\end{aligned}
\]

\section*{2.3}

Uncertainty on the solutions
Without object.
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Titrate:
ZZZZ161 - Exchange with the format MED, data-processing control
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Calculation is made in two-dimensional static mechanics, in plane deformation.

\section*{3.2 \\ Characteristics of the grid}

The sections A and B are with a grid with 8 triangles TRIA3. They form the group of meshs named "SORTED". The sections C and D are with a grid with 4 quadrangles QUAD4. They form the group of meshs named "QUAD".

The 4 triangles of the section B and the 2 quadrangles of the section C are gathered in the group "MEDIUM".

The external frontier segments are with a grid with 12 segments SEG2. They are gathered in 4 named groups "BORD_INF", "BORD_DRO", "BORD_SUP" and "BORD_GAU".
```

"I
"OPPOSES"

```

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

IMPR_RESU FORMAT
"MED"
LIRE_CHAMP FORMAT
"MED"
TYPE_CHAM
"CHAM_NO_DEPL_R"

This case tests the various options of writing to format "MED" via order IMPR_RESU. For that we calculate various options complementary to the only displacement and we create a field of size are equivalent to the field of displacement to the nodes. The impressions are successively those:
- the field of displacement to the nodes and the stress field at the points of Gauss, expressed on all the grid. That starts the creation of a profile for displacement because it
is not defined everywhere: the three nodes " Me , "and " \(O\) " do not comprise a value, - the field of size. It uses the same profile as the field of displacement of the result standard, - components "SIXY" and "SIYY" of the stress field to the nodes by element "SIGM_ELNO_DEPL",
- the indicator of error "ERRE_ELGA_NORE" on the meshs of the group "MEDIUM". That start the creation of two profiles because the written values are not it on all them meshs. The first profile relates to the 4 triangles out of the 8 of the total; the second relates to the 2 quadrangles on the 4 of the total.

The second reading since format "MED" is that of the field of displacement to the nodes contained in field of size.

\author{
Handbook of Validation
}

V1.01 booklet: Tests of validity of orders
HI-23/03/003/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.4

\section*{Titrate:}

ZZZZ161 - Exchange with the format MED, data-processing control
Date:
03/06/03
Author (S):

\section*{G. Key NICOLAS}

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values \\ tested}

The comparison takes place on the value of displacement to the node "OPPOSES" with a relative tolerance from 104\%.

\section*{Identification}

\section*{Value}

\title{
Relative variation in \% \\ их 0,4183044 \\ 1,55 \\ 10-7 \\ uy 1,639849 2,71 \\ 10-5 \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HI-23/03/003/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
ZZZZ161 - Exchange with the format MED, data-processing control
Date:
03/06/03
Author (S):
G. Key NICOLAS

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\section*{5}

Summary of the results
No particular comment is to be made on this case test. It is only used to ensure the perenniality of writings and readings with format MED.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HI-23/03/003/A
Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
SENSD2 - Sensitivity to materials in harmonic dynamics isotro 3D
Date:
25/11/05

\title{
Key S. CAMBIER
}

SENSD02 Sensitivity to materials in dynamics isotropic harmonic 3D

\section*{Summary}

This case test relates to the derivation of the harmonic response of an isotropic parallépipède.
The functionality tested is the derivation of the fields of displacement, speed and acceleration compared to properties materials modulus Young and Poisson's ratio.
The reference is calculated by finished differences.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
SENSD2 - Sensitivity to materials in harmonic dynamics isotro \(3 D\)
Date:
25/11/05
Author (S):
Key S. CAMBIER
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{OPPOSE}

\section*{CENTER}

BORD_GAU
3. \(m\)
\(I\)
\(y\)
10. \(m\)
\(1 m<X<11 m\)
\(1.5 m<Y<1.5 m\)

\section*{1.2}

\section*{Properties of material}
\[
E=105 \mathrm{~Pa}
\]
\[
=0.3
\]
\[
A N D=2000 M P a
\]

Viscous damping, matrix of damping: \(K+M,=0.002 \mathrm{~S}\) and \(=0.3 \mathrm{sl}\)

\section*{1.3}

Boundary conditions and loadings
Boundary conditions:
Line "BORD_GAU" is blocked according to \(X, y\) and \(Z\).
The node " \(I\) " is blocked according to \(X\), \(y\) and \(Z\).

Loading:
The line "OPPOSES":
nodal force according to \(X\) : \(F x=1\)
\(F \sin (2 F T)\)
1
\(F=100 \mathrm{NR}\),
\(10 \mathrm{~Hz} F 40 \mathrm{~Hz}\)
nodal force according to \(y: F y=2\)
\(F \sin (2 F T)\)
2
\(F=30 N R\),
10 HzF 40 Hz
nodal force according to \(Z: F z=3\)
\(F \sin (2 F T) F=10 N R\),
10 Hz F 40 Hz
3

\subsection*{1.4 Conditions}

\section*{initial}

\section*{Initial displacement no one and null initial speed.}

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V1.01 booklet: Tests of validity of orders
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

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8.1

Titrate:
SENSD2 - Sensitivity to materials in harmonic dynamics isotro 3D
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25/11/05
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The sensitivities of the fields of displacement, speed and acceleration to the data materials are calculated by finished differences. These calculations by finished differences are carried out inside even case test by Code_Aster. The diagram of finished differences used is as follows (diagram of order 1 not centered):
\(F\) (,
X)
y
\(F(x+\),
\(y-F\),
\(X\) )
\(y\)

\section*{\(X\)}

The field of solution displacement of the standard problem (i.e nonderived) is subjected to a test of not regression. Tests of nonregression are also carried out on the sensitivities.

\section*{2.2 \\ Sizes and results of reference}

The parameters of sensitivity are the Young modulus E and the Poisson's ratio.
By finished differences:
derived compared to E from displacement following y to the node OPPOSES to the frequency of 10 Hz , derived compared to displacement following y to the node OPPOSES to the frequency of 10 Hz .

By nonregression:
displacement following \(Z\) of the node OPPOSES to the frequency of 10 Hz , derived compared to E of acceleration following y to the node OPPOSES to the frequency of 40 Hz , derived compared to acceleration according to \(X\) with the node OPPOSES to the frequency of 40 Hz .

\subsection*{2.3 Uncertainties}

Results depending amongst other things on the convergence of calculations (what can disturb the values of reference calculated by difference-finished); uncertainty on the solution can be estimated order from \(0.5 \%\) for the values tested significant (not too small).

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/05/005/A
}

Code_Aster \({ }^{\circledR}\)
Version
8.1

\section*{Titrate:}

SENSD2 - Sensitivity to materials in harmonic dynamics isotro 3D
Date:
25/11/05
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Modeling in voluminal element.

\author{
BORD_GAU
}

F

Characteristic of the elements: \(3 D\)
Boundary conditions: line BORD_GAU and node I are blocked according to \(X, y\) and \(Z\).

\section*{3.2}

Characteristics of the grid
A number of nodes: 330
Numbers and type of mesh: 200 HEXA8

\subsection*{3.3 Functionalities \\ tested}

The functionality tested is the derivation of the fields of displacement and acceleration compared to properties materials modulus Young and Poisson's ratio.

Orders

MEMO_NOM_SENSI NOM_UN

\author{
NOM_ZERO \\ NAME \\ NOM_SD \\ PARA_SENSI \\ DYNA_LINE_HARM SENSITIVITY
}

EXCIT
CHARGE
TEST_RESU SENSITIVITY

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Titrate:
SENSD2 - Sensitivity to materials in harmonic dynamics isotro 3D
Date:
25/11/05
Author (S):
Key S. CAMBIER

\section*{3.4 \\ Sizes tested and results}

The results are obtained on the machine Compaq clayastr.

References obtained by finished differences:
derived compared to \(E\) of the parts real and imaginary of the component dx of displacement with node OPPOSES

\section*{Frequency Reference}

\author{
Aster \%
}

Difference
10 Hz
\(-2.7121224935485 D-19-2.7146051236211 D-19\)
0.094 \%
\(6.5418838140065 D-20\)
\(6.5507314799724 D-20\)
derived compared to parts real and imaginary of the component dx of displacement with node OPPOSES

\section*{Frequency Reference}

Aster \%
Difference
10 Hz
-2.8882101730469D-09 -2.8851572867471D-09
1.22E 06\%
\(8.7757452912573 D-10\)
\(8.7730320078234 D-10\)

References obtained by nonregression:
Parts real and imaginary of the component \(d z\) of displacement to the node OPPOSES

\section*{Frequency Reference}

Aster \%

\section*{Difference}

10 Hz
1.4126470000000D-07
1.4126477736023D-07
5.42E- 05\%
-2.05120200000000D-08
-2.0512021300347D-08
derived compared to E of the parts real and imaginary of the component Dy of acceleration with node OPPOSES

\section*{Frequency Reference}

Aster \%
Difference
40 Hz
6.6827140000000D-15
6.6827148301820D-15
1.18E- 05\%
-3.4841000000000D-15
-3.4841003202926D-15
derived compared to parts real and imaginary of the component \(d x\) of acceleration with node OPPOSES

\section*{Frequency Reference}

Aster \%
Difference
40 Hz
-4.6447430000000D-04-4.6447437730561D-04
1.22E 06\%
6.40579800000000D-04
6.4057985074844D-04

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of orders
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.1

Titrate:

Date:
25/11/05
Author (S):
Key S. CAMBIER
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\section*{4 \\ Summary of the results}

Precision evaluated by comparison of the results of the method of resolution by direct derivation "exact" with those obtained by finished differences is completely satisfactory.
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.3

\section*{Titrate:}

SENSD03 - Sensitivity to materials in isotropic harmonic dynamics
Date:
23/10/02
Author (S):
Key S. CAMBIER
:
V1.01.163-A Page:
1/6
Organization (S): EDF-R \& D /AMA

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of the orders \\ Document: V1.01.163
}

SENSD03 - Sensitivity to materials in dynamics
isotropic harmonic 2D

\section*{Summary}

This case test takes again the geometry and the materials of test SENSM03 but calculation carried out here is the answer harmonic of the system.

The functionality tested is the derivation of the fields of displacement, speed and acceleration compared to properties materials modulus Young and Poisson's ratio. A simultaneous calculation of the sensitivities by report/ratio with the loading is also tested.

The reference is calculated by finished differences.

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of the orders
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.3

Titrate:
SENSD03 - Sensitivity to materials in isotropic harmonic dynamics
Date:
23/10/02
Author (S):
Key S. CAMBIER
V1.01.163-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{OPPOSE}

\author{
3. mm \\ BORD_GAU \\ CENTER
}

ORIGIN
\(y\)
10. mm

Z
1. \(<X<11\)

\author{
1.2 \\ Properties of material \\ \(E=36.000 \mathrm{MPa}\) \\ \(=0.2\) \\ \(=2400 \mathrm{~kg} / \mathrm{m} 3\)
}

\section*{1.3 \\ Boundary conditions and loadings}

Boundary conditions:
Surface "BORD_GAU" is blocked according to X.
The line "ORIGIN" is blocked according to \(X\) and \(Y\).
Loading:
The line "OPPOSES":
nodal force according to \(X\) : \(F x=1\)
\(F \sin (2 F T), 1\)
\(F=10 . \mathrm{NR}, 10 \mathrm{~Hz}\) F 40 Hz
nodal force according to \(y\) : \(F y=2\)
\(F \sin (2 F T), 2\)
\(F=3 . N R, 10 \mathrm{~Hz}\) F 40 Hz

\subsection*{1.4 Conditions}
initial
Without object.
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Titrate:
SENSD03 - Sensitivity to materials in isotropic harmonic dynamics

Date:
23/10/02
Author (S):
Key S. CAMBIER
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\section*{2 \\ Reference solution \\ 2.1 \\ Method of calculation}

The sensitivities of the fields displacement, speed and acceleration with the data materials are calculated by finished differences. These calculations by finished differences are carried out inside even case test by Code_Aster. The diagram of finished differences used is as follows (diagram of order 1 not centered):

F (,
\(X\) )
\(y\)
\(F(x+\),
\(y-F\) (,
\(X\) )
\(y\)

\section*{X}

The field of solution displacement of the standard problem (i.e nonderived) is subjected to a test of not regression. Tests of nonregression are also carried out on the sensitivities.

\section*{2.2 \\ Sizes and results of reference}

By finished differences:
To 10 Hz :
derived compared to E from displacements in y to the group from nodes OPPOSES, derived compared to displacements in \(y\) with the group from nodes OPPOSES, derived compared to 1
\(F\) of displacements in y to the group of nodes OPPOSES, derived compared to 2
\(F\) of displacements in y to the group of nodes OPPOSES.
By nonregression:
To 10 Hz :
displacements in \(X\) with the group of nodes OPPOSES
To 25 Hz :
derived compared to E from accelerations in y to the group from nodes OPPOSES, derived compared to accelerations in \(X\) with the group from nodes OPPOSES, To 40 Hz :
derived compared to 1
\(F\) speeds in \(X\) to the group of nodes OPPOSES,
derived compared to 2
\(F\) of displacements in \(X\) to the group of nodes OPPOSES.
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\section*{Titrate:}

SENSD03 - Sensitivity to materials in isotropic harmonic dynamics
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Modeling in plane constraints on the grid following 2D:

Surface BORD_GAU becomes a line, the lines ORIGIN and OPPOSES become points.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 55
Numbers and types of meshs: 28 SEG2, 80 TRIA3

\subsection*{3.3 Functionalities}
tested
The functionality tested is the derivation of the fields of displacement, speed and acceleration by report/ratio with the properties materials (Young modulus and Poisson's ratio). A simultaneous calculation of sensitivities compared to the loading is also tested.

Orders
```

MEMO_NOM_SENSI
NOM_UN
NOM_ZERO
NAME
NOM_SD
PARA_SENSI
NOM_COMPOSE
DYNA_LINE_HARM
SENSITIVITY
EXCIT
CHARGE
TEST_RESU
SENSITIVITY

```

\section*{3.4 \\ Sizes tested and results}
```

By finished differences:
Derived compared to $E$ of the parts real and imaginary of displacements in $y$, line OPPOSES Frequency Reference
Aster \%
Difference
10 Hz
2.6060024120854E19 2.6100392585269E19

```

Derived compared to parts real and imaginary of displacements in y, line OPPOSES
Frequency Reference
Aster \%
Difference
10 Hz
2.5599564694851E10 2.5598652812905E10
0.004 \%
2.5599564694851 E10
\(2.5598652812905 E 10\)

Derived compared to F1 of the parts real and imaginary of displacements in y, line OPPOSES Frequency Reference
Aster \%
Difference
10 Hz
1.4005526375796E09 1.4005526375796E09
2.30E \(10 \%\)
1.4005526375796E09
1.4005526375796E09

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SENSD03 - Sensitivity to materials in isotropic harmonic dynamics
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Key S. CAMBIER

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Derived compared to F2 of the parts real and imaginary of displacements in y, line OPPOSES Frequency Reference
Aster \%
Difference
10 Hz
6.6511780281604E09 6.6511780281527E09
1.17E 10\%
\(6.6511780281604 E 09\)
\(6.6511780281527 E 09\)

By nonregression:
Displacements in \(X\) with the group of nodes OPPOSES
Frequency Reference
Aster \%
Difference
10 Hz
1.30168400000000E09 1.3016840539066E09
4.14E 06\%
1.30168400000000E09
1.3016840539066E09

Derived compared to E of the parts real and imaginary of accelerations in y, line OPPOSES

\section*{Frequency Reference}

Aster \%
Difference
25 Hz
2.0057540000000E15 2.0057545862003E15
\(0.155 \%\)
2.0057540000000E15
2.0057545862003E15

Derived compared to parts real and imaginary of accelerations in \(X\), line OPPOSES
Frequency Reference
Aster \%
Difference
25 Hz
7.1521020000000E08 7.1521029915851E08
1.39E 05\%
7.1521020000000E08

Derived compared to F1 of the parts real and imaginary speeds in \(X\), line OPPOSES Frequency Reference

\author{
Aster \%
}

Difference
40 Hz
8.2114210000000E08 8.2114212686221E08
3.27E 06\%
8.2114210000000E08
8.2114212686221E08

Derived compared to F2 of the parts real and imaginary of displacements in X, line OPPOSES
Frequency Reference
Aster \%
Difference
40 Hz
5.6196830000000E11 5.6196830401657E11
7.15E07\%
5.6196830000000E11
5.6196830401657E11

\section*{Handbook of Validation}

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SENSD03 - Sensitivity to materials in isotropic harmonic dynamics
Date:
23/10/02
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Key S. CAMBIER
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\section*{4 \\ Summary of the results}

The precision of the results is coherent with the method of resolution used and the method of direct derivation "exact".

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HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
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6.3

Titrate:
ZZZZ164 - Validation of MODI_MAILLAGE/TRANSLATION
Date:
14/10/02
Author (S):
NR. GREFFET Key
:
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Organization (S): EDF/AMA

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of the orders \\ Document: V1.01.164
}

ZZZZ164 - Validation of the key words TRANSLATION, ROTATION, MODI_BASE and SCALE of order MODI_MAILLAGE

\section*{Summary:}

This test validates the key words TRANSLATION, ROTATION, MODI_BASE and SCALE of MODI_MAILLAGE. In
this goal, one will impose on two grids, one 3D and the other 2D two combinations of these key words.
The first
is made up of a translation, two unspecified rotations and a scaling. One thus will test them two possibilities of definition of the axis of rotation: either by two points, or by a point and the direction.
second will combine a basic change and a scaling. One will thus have tested all the cases thus of figures authorized by these key words.

\section*{Handbook of Validation}

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.3

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1

\section*{Problem of reference}

\subsection*{1.1 Geometry}

The problem is 3D, it acts of a right-angled parallelepiped:
Nondefinite.
1.4 Conditionsinitial
Nondefinite.
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation}

The reference solution is analytical.
Either \(M(X, y, Z)\) a point of space, one imposes a translation \(T\) of vector ( \(t x, t y\), tz), and one to him rotation \(R\) of angle (in radians) whose axis passes by \(P(p x, p y, p z)\) and has as a direction (
\(D d x, D y, d z)\).
Then \(M\) becomes MT after the translation: \(M T(X+t x, y+t y, Z+t z)\).
MT becomes MTR after rotation:

\section*{M}
\(=P+\cos P M\)
TR
\(T+(1-\cos )(P M\)
D
\(T\)
) \(D+\sin (D P M T)\)
With M
\(=M+T\)
\(T\)
The scaling of a factor ech, gives:
\(M T R E=e c h M T R\)
The functionality of basic change awaits in entry the data by the user of two vectors orthogonal in 3D (only one vector in 2D). One comes to supplement these data in order to generate one
base orthogonal direct, in 3D or 2D. Tests are carried out in order to check if the data of entry will allow to define a direct orthogonal base. A standardization of the vectors of base is then carried out.
In 3D, one thus awaits the data of \(U\) and \(V\), the first two vectors of the new base:
\(W(X, y, Z)=U(X, y, Z) V(X, y, Z)\)
\(\boldsymbol{B}=(\boldsymbol{U}, \boldsymbol{V}, \boldsymbol{W})\)
formed
stamp
:
by
base
of

\author{
vectors
}
\(M(U, V, W) B T\)
\(=\)
\(M(X, y, Z)\)
In 2D, one generates the second vector of the base by rotation of \(90^{\circ}\) of the vector seized by the user.
This basic change can be combined with a scaling and a translation, for example.
The programming of these transformations is done differently in 3D and 2D, so as to optimize each one of these two cases.
2.2

Sizes and results of reference
One will control the new co-ordinates of the P1 point, P7 and P8 in 3D (P1, P3 and P4 in 2D).

\section*{2.3}

Uncertainties on the solution
Uncertainties come from the numerical precision in Code_Aster (dependence of the platform) and in the calculation of the analytical solution of reference. One can thus consider a criterion of precision
relative about 1.E-13 in the tests.

\subsection*{2.4 References \\ bibliographical}

Without use.
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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
One places oneself within a framework massive 3D. One will impose successively:
- a translation of vector (2,5 3,9-12,3),
\(\cdot\) a rotation of angle 33 degrees and axis passing by the items (10 0,5.3,8) and (0 100 ), \(\cdot\) the second rotation of angle -161 degrees and axis passing by (- \(30,5.3,8\) ) and from direction (0 10 ),
- a scaling of a factor 5.

One thus tests all the cases of figure authorized by the syntax of the key words TRANSLATION, ROTATION and SCALE.

Then, one sets out again of the initial grid and one imposes to him successively:
\(\cdot\) a basic change of vectors (1,23 0,23 0) and (- 2.3 12,3 0),
- a scaling of a factor 5.

One tests together thus the key words MODI_BASE and SCALE.

\section*{3.2}

Characteristics of the grid

The grid comprises only one element of the type HEXA8.

\subsection*{3.3 Functionalities}
tested
Orders

\section*{MODI_MAILLAGE \\ TRANSLATION}

ROTATION
POIN_1
POIN_2
DIR
ENG
SCALE
VECT_X
VECT_Y
SCALE
3.4Sizes tested and results
For the first part, with TRANSLATION, ROTATION and SCALE:
Points
Co-ordinates
Reference
Code_Aster Variation
observed
relative
P1

X

\(5.2501368890123 E+00\)
Y
\[
2.1551486020681 E+00
\]
\[
2.1551486020680 E+003.71 E-15
\]
\[
Z
\]
\[
7.8600118786924 E+01
\]
\[
7.8600118786924 E+01
\]
\[
3.62 E-16
\]
P7
\[
X
\]
\[
1.3714414455621 E+01
\]
\[
1.3714414455621 E+011.42 E-15
\]
\[
\boldsymbol{Y}
\]
\[
1.9199906921638 E+01
\]
\[
1.9199906921638 E+01
\]
\[
9.25 E-16
\]
\[
Z
\]
\[
7.0898989267417 E+01
\]
\[
7.0898989267417 E+01
\]
\[
0
\]
P8
\[
X
\]
\[
9.9168576521849 E+00
\]
\[
9.9168576521850 E+008.96 E-16
\]
\[
\boldsymbol{Y}
\]
\(2.0297577804345 E+01\)
\(2.0297577804345 E+01\)
8.75E-16
Z
\(6.7837342495183 E+01\)
\(6.7837342495183 E+01\)
2.09E-16
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For the second part, with MODI_BASE and SCALE:
Points
Co-ordinates
Reference
Code_Aster Variation
observed
relative
P1
X
\(4.9148126952461 E+00\)
\(4.9148126952461 E+00\)
3.61E-16
\(Y\)
9.1903001618423E-01
9.1903001618423E-01
3.62E-15

Z
\(0.0000000000000 E+00\)
\(0.0000000000000 E+00\)
0.

P7
\(X\)
7.6719027437988E+00
7.6719027437988E+00
2.32E-16

Y
\(1.3825408069554 E+01\)
\(1.3825408069554 E+01\)
1.28E-16

Z
\(2.5000000000000 E+01\)
\(2.5000000000000 E+01\)
1.42E-16

P8
\(X\)

\section*{\(2.7570900485527 E+00\)}
\(2.7570900485527 E+00\)
1.61E-16

Y
\(1.4744438085738 E+01\)
1.4744438085738E+01
2.41E-16

Z
\(2.5000000000000 E+01\)
\(2.5000000000000 E+01\)
1.42E-16

\section*{4 Modeling \\ B}

One places oneself within a framework 2D. One will impose successively:
- a translation of vector (2,5 3,9),
- a rotation of angle 33 degrees and axis passing by the item (10 0,5),
\(\cdot\) the second rotation of angle -161 degrees and axis passing by the point (- 30,5 ), - a scaling of a factor 5 .

One thus tests all the cases of figure authorized by the syntax of the key words TRANSLATION, ROTATION
and SCALE.
Then, one sets out again of the initial grid and one imposes to him successively:
- a change of reference mark of vectors (1,23 0,23),
- a scaling of a factor 5 .

One tests thus together the key words MODI_BASE and SCALE.

\section*{4.1}

Characteristics of the grid
The grid comprises only one element of the type QUAD4.

\subsection*{4.2 Functionalities}
tested

\author{
Orders
}

\author{
MODI_MAILLAGE \\ TRANSLATION \\ ROTATION \\ POIN_1 \\ POIN_2 \\ DIR \\ ENG \\ SCALE \\ VECT_X \\ VECT_Y \\ SCALE
}

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\section*{4.3 \\ Sizes tested and results}

For the first part, with TRANSLATION, ROTATION and SCALE:

\section*{Points}

Co-ordinates
Reference
Code_Aster Variation
relative
observed
P1
X
\(3.9975219277929 E+013.9975219277929 E+015.33 E-16\)
Y
4.2222814000070E-01
4.2222814000070E-01
2.63E-15

P3
X
\(3.1233365350457 E+013.1233365350457 E+011.14 E-16\)
Y
\(1.2752747757918 E+011.2752747757918 E+019.75 E-16\)

X
\(2.8155057973828 E+012.8155057973828 E+011.26 E-16\)
Y
\(8.8126939898842 E+008.8126939898842 E+002.62 E-15\)

For the second part, with MODI_BASE and SCALE:

\section*{Points}

\section*{Co-ordinates}

\section*{Reference}

\author{
Code_Aster Variation
}
relative
observed
P1
\(X\)
\(0.0000000000000 E+00\)
\(0.0000000000000 E+00\)
0 .
Y
0.00000000000000E+00
\(0.0000000000000 E+00\)
0 .
P3
X
7.6719027437988E+00
\(7.6719027437988 E+00\)
2.32E-16

Y
1.3825408069554E+01
1.3825408069554E+01
1.28E-16

P4
X
\(2.7570900485527 E+00\)
\(2.7570900485527 E+00\)
1.61E-16

Y
1.4744438085738E+01
\(1.4744438085738 E+01\)
2.41E-16

\section*{5}

Summary of the results
Numerical results for the translation, rotation, the change of reference mark and the scaling grid are identical to the analytical results of reference, in \(3 D\) or \(2 D\), with the precision

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Code_Aster \({ }^{\circledR}\)
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J. Key PELLET
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Organization (S): EDF-R \& D /AMA

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\section*{ZZZZ165-LIRE_RESU with format "IDEAS"}

\section*{Summary:}

One reads with format IDEAS a field with the nodes as well as a field by elements with nodes (ELNO).
It is checked that the values read are correctly stored in the structure of data RESULT (SD_RESULTAT) produced.
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1
Problem of reference

\subsection*{1.1 Geometry}
```

2
Reference solution

```

The case test consists in reading again a file with format "IDEAS". The test consists in checking that the values read are correctly stored in the SD_RESULTAT.

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Grid IDEAS associated with this test consists of 4 meshs of the type QUAD8.
These meshs have labels 1,2,3,4. But they are given in order 4,1,2,3.
The nodes of the grid have as labels: [1-5] and [7-22]. Node 12 is given in 1st.

NO18
NO13

NO12
MA1
\(X\)
NO1
NO3
NO4

\subsection*{3.2 Functionalities}
validated

\section*{LIRE_RESU}

FORMAT
"IDEAS"

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\section*{4}

Results of modeling \(A\)
4.1 Values
tested
Field
Node
CMP
Reference
Found value
Difference (\%)
DEPL:
NO12
DX
0.75
0.75
NET MA1 NO3
V10.50
0.50
0
NET MA1 NO4
V1
1.00
1.00
0
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5
Summary of the results

This case test makes it possible to check the correct operation of order LIRE_RESU to the format "IDEAS" when the numbers of entities IDEAS are different from the Aster numbers. Handbook of Validation
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Organization (S): EDF-R \& D /AMA, CS IF

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\section*{Summary:}

This test has as an aim the validation of the options of calculation of flow in thermics:
FLUX_ELGA_TEMP and
FLUX_ELNO_TEMP.
6 modelings tested are:
- A In PLANE 2D: triangles of degree 1, quadrangles of degree 1 and 2 and quadrangles with 9 nodes,
- B In 2D PLAN_DIAG: triangles of degree 1 and 2, quadrangles of degree 1 and quadrangles with 9 nodes,
- C In 2D AXIS: triangles of degree 1, quadrangles of degree 1 and 2 and quadrangles with 9 nodes, - D In 2D AXIS_DIAG: triangles of degree 1 and 2, quadrangles of degree 1 and quadrangles with 9 nodes,
- E In 3D: hexahedrons, pentahedrons, pyramids and tetrahedrons of degree 1 and 2, hexahedrons with 27 nodes,
- F In 3D_DIAG: hexahedrons, pentahedrons, pyramids and tetrahedrons of degree 1.

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

Each square is with a grid with a type of distinct element.

\section*{\(\boldsymbol{Y}\) \\ \(Y\)}

Each volume is included/understood
between \(Z=1.0\) and \(Z=1.1\)
7.5

C7
D7
C8
D8
C9
D9
5.0

J
K
\(L\)
A7
B7
A8
B8
A9
B9
5.5
5.0

G
H
I
C4
D4
C5
D5
C6
D6
3.0
2.5

A4
B4
\(A 5\)
\(B 5\)
\(A 6\)
\(B 6\)
\(D\)
\(E\)
\(F\)
3.0
2.5
\(C 1\)
\(D 1\)
\(C 2\)
\(D 2\)
\(C 3\)
\(D 3\)
\(W i t h\)
\(B\)
\(C\)
0.5
\(A 1\)
\(B 1\)
\(A 2\)
\(B 2\)
\(A 3\)
\(B 3\)
0.5
\(O\)
1.0
3.0
5.0
\(O 0.5\)
2.53 .0
5.05 .5
7.5
\(X\)
\(B\)
\(B\)
(Note:: in AXIS and AXIS_DIAG, the axis
(Note:: in 3D_DIAG, only them
of rotation is \(O Y\) and \(X\)-coordinate \(X\) corresponds
4
hexahedrons in bottom on the left are
thus with the ray).
with a grid).

\section*{1.2 \\ Material properties}

The thermal properties applied to the model are:
\[
\cdot=1.0 \mathrm{~W} / \mathrm{m}{ }^{\circ} \mathrm{C}
\]

\section*{1.3}

Boundary conditions and loadings

\section*{Nondefinite.}
1.4

Field of temperature
The field of temperature is directly affected with the model starting from a function. For each modeling, the calculation of flow is tested for a linear thermal field and a thermal field quadratic.

For modelings PLANE 2D and PLAN_DIAG (modelings A and B), the two fields successively affected are:
\(\cdot T(X, Y)=2 . X+3 . Y\)
\(\cdot T(X, Y)=2 . X 2+3 . Y 2\)
For modelings 2D AXIS and AXIS_DIAG (modelings C and D), the two fields successively affected are:
\(\cdot T(X, Y, Z)=2 . R+3 . Y(\) with \(R=\)
2
2
\(X+Z=X\) in the plan of grid (OXY))
\(\cdot T(X, Y, Z)=2 . R 2+3 . Y 2\)
For modelings 3D and 3D_DIAG (modelings E and F), the two fields successively affected are:
\(\cdot T(X, Y, Z)=2 . X+3 . Y+4 . Z\)
\(\cdot T(X, Y, Z)=2 . X 2+3 . Y 2+4 Z 2\)

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is analytical:
```

T
X= -
X
T
y=-
Y
T
Z = -.
only

```

Taking into account the value (=1), the flow obtained for each configuration is thus:
For modelings PLANE 2D and PLAN_DIAG (modelings A and B):
\(\cdot T(X, Y)=2 . X+3 . Y X(X, Y)=2\) and \(y(X, Y)=3\)
\(\cdot T(X, Y)=2 . X 2+3 . Y 2 X(X, Y)=4 . X\) and \(y(X, Y)=6 . Y\)
For modelings 2 D AXIS and AXIS_DIAG (modelings \(C\) and \(D\) ):
\(\cdot T(X, Y, Z)=2 . R+3 . Y(R=X\) in plan \((O X Y)) X(X, Y, Z)=2\) and \(y(X, Y, Z)=3\)
\(\cdot T(X, Y, Z)=2 . R 2+3 . Y 2(R=X\) in plan \((O X Y)) X(X, Y, Z)=4 . X\) and \(y(X, Y, Z)=6 . Y\)
For modelings 3D and 3D_DIAG (modelings \(E\) and \(F\) ):
\(\cdot T(X, Y, Z)=2 . X+3 . Y+4 . Z X(X, Y, Z)=2, y(X, Y, Z)=3\) and \(Z(X, Y, Z)=4\)
\(\cdot T(X, Y, Z)=2 . X 2+3 . Y 2+4 Z 2 X(X, Y, Z)=4 . X, y(X, Y, Z)=6 . Y\) and \(Z(X, Y, Z)=8 . Z\)

\section*{2.2 \\ Results of reference}

For modelings 2D (PLANE, PLAN_DIAG, AXIS and AXIS_DIAG), the values tested are:
- With the linear field of temperature: the temperature with the nodes \(A, C, J\) and \(L, f l o w\) according to \(X\) and \(Y\) by element with nodes \(A, C, J\) and \(L\) and flow at the first point of Gauss of same elements,
- With the quadratic field of temperature: the temperature with nodes \(A, C, J\) and \(L\), and flow according to \(X\) and \(Y\) by element with nodes \(A, C, J\) and \(L\).

For modelings 3D (3D and 3D_DIAG), the values tested are:
- With the linear field of temperature: the temperature with the Di nodes, flow following \(X\) and \(Y\) by element with the Di nodes and flow at the first point of Gauss of the same elements,
- With the quadratic field of temperature: the temperature with the Di nodes, and following flow
\(X\) and \(Y\) by element with the Di nodes.
With \(I=1\) to 9 for modeling \(3 D\) and \(I=1\) to 4 for modeling 3D_DIAG.
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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
Elements 2D modeling PLAN

\section*{3.2}

Characteristics of the grid
A number of nodes: 3382
A number of meshs and types: 2000 meshs including 400 QUAD8, 400 QUAD9, 400 QUAD4 and 800 TRIA3.

The grid includes/understands 4 squares SQU8, SQU9, SQU4 and STR3 with a grid respectively with elements QUAD8, QUAD9, QUAD4 and TRIA3. Each square is discretized with 20 elements according to \(X\) and following \(Y\).Name of
Type
SQU4
STR3
GROUP_MA
of element
G
H
I
3.0
SQU8 QUAD8
SQU9 QUAD9
2.5
D
E
F
SQU4 QUAD4
STR3 TRIA3
SQU8
SQU9
With
B
C
0.5
O
1.0
3.0
5.0
\(X\)

The nodes used for postprocessing are:

Name of

\section*{Co-ordinates}

Number of the node
Name of a mesh
Type of the mesh
node
containing this node
With
(1.0; 0.5)

N1
M1
QUAD8
C
(5.0; 0.5)

N1620
M420
QUAD9
\(J\)
(1.0; 5.0)

N3001
M1181
QUAD4
L
(5.0; 5.0)

N3440
M2000
TRIA3
The whole of the grid is affected by a thermal modeling PLAN.

\subsection*{3.3 Functionalities}
tested
Orders
"THERMAL" AFFE_MODELE "PLAN"
CREA_MAILLAGE MODI_MAILLAGE "QUAD8_9"

CREA_CHAMP "NOEU_TEMP_F"

\title{
CREA_RESU "EVOL_THER"
}

CALC_ELEM "FLUX_ELGA_TEMP"

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4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested
- With a linear field of temperature (constant flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG
NodeWith
T 3.53 .5
0\%
M1 (QUAD8)
Node A
X (ELNO)
-2.0
-2.0
0\%
M1 (QUAD8)
Node A
\(y\) (ELNO) -3.0
-3.0
0\%
M1 (QUAD8)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M1 (QUAD8)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
C
T \(11.511 .50 \%\)
M420 (QUAD9)
Node C
X (ELNO)
-2.0
-2.0
0\%
M420 (QUAD9)
Node C
\(y\) (ELNO) -3.0
-3.0
0\%

M420 (QUAD9)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M420 (QUAD9)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
J
T 17.017 .0 0\%
M1181 (QUAD4)
Node J
\(X\) (ELNO)
-2.0
-2.0
0\%
M1181 (QUAD4)
Node J
y (ELNO) -3.0
-3.0
0\%
M1181 (QUAD4)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M1181 (QUAD4)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
L
T 25.025 .0 0\%
M2000 (TRIA3)
Node L
X (ELNO)
-2.0

0\%
M2000 (TRIA3)
Node L
\(y\) (ELNO) -3.0
-3.0
0\%
M2000 (TRIA3)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M2000 (TRIA3)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
- With a quadratic field of temperature (linear flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG
Node
With
T 2.75 2.75 0\%
M1 (QUAD8)
Node A
X (ELNO)
-4.0
-4.0
\(0 \%\)
M1 (QUAD8)
Node A
y (ELNO) -3.0
-3.0
```

0%

```
Node
C
T 50.75
50.75 0\%
M420 (QUAD9)
Node C
\(X\) (ELNO)
-20.0
-20.0
0\%
M420 (QUAD9)
Node C
y (ELNO) -3.0
-3.0
0\%
Node
J
T 77.077 .0 0\%
M1181 (QUAD4)
Node J
X (ELNO)
-4.0
-4.2
5\%
M1181 (QUAD4)
Node J
\(y\) (ELNO) -30.0
-29.7
-1\%
Node
L
T 125.0
\(125.00 \%\)
M2000 (TRIA3)
Node L
X (ELNO)
-20.0
-19.8
-1\%
M2000 (TRIA3)
Node L
\(y\) (ELNO) -30.0

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of the orders HT-66/03/008/A

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\section*{5 Modeling \\ B}
5.1

Characteristics of modeling

\section*{Elements 2D modeling PLAN_DIAG}

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 3782.
A number of meshs and types: 2400 meshs including 800 TRIA6, 400 QUAD9, 400 QUAD4 and 800 TRIA3.

The grid includes/understands 4 squares STR6, SQU9, SQU4 and STR3 with a grid respectively with elements TRIA6, QUAD9, QUAD4 and TRIA3. Each square is discretized with 20 elements according to X and following \(Y\).


The nodes used for postprocessing are:
Name of
Co-ordinates
Number of the node
Name of a mesh
Type of the mesh
node
containing this node
With
(1.0;0.5)

N1
M1
TRIA6
C
(5.0; 0.5)

N2481
M820
QUAD9
\(J\)
(1.0; 5.0)

N3401
M1581
QUAD4
\(L\)
(5.0; 5.0)

N3840
M2400
TRIA3
The whole of the grid is affected by a thermal modeling PLAN_DIAG.

\subsection*{5.3 Functionalities}
tested
Orders
"THERMAL"AFFE_MODELE "PLAN_DIAG"

CREA_MAILLAGE MODI_MAILLAGE "QUAD8_9"

CREA_CHAMP "NOEU_TEMP_F"

CREA_RESU "EVOL_THER"

CALC_ELEM "FLUX_ELGA_TEMP"
"FLUX_ELNO_TEMP"
Handbook of Validation
V1.01 booklet: Tests of validity of the orders
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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6.4

Titrate:
ZZZZ166 - Calculation of flow in thermics

Date:
03/06/03
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J. PELLET, Key P. HERMAN
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6
Results of modeling \(B\)

\subsection*{6.1 Values}
tested
- With a linear field of temperature (constant flow):

\section*{Localization Size}

Reference
```

Aster %
difference
Net
Node or PG

```
Node
With
T 3.53 .5
\(0 \%\)
M1 (TRIA6)
Node A
X (ELNO)
-2.0
-2.0
\(0 \%\)
M1 (TRIA6)
Node A
\(y\) (ELNO) -3.0
-3.0
\(0 \%\)
M1 (TRIA6)
Point 1
\(X\) (ELGA)
-2.0
-2.0
\(0 \%\)
M1 (TRIA6)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
C
T11.511.50\%
M420 (QUAD9)
Node C
X (ELNO)
-2.0
-2.0
0\%
M420 (QUAD9)
```

Node C
y (ELNO) -3.0
-3.0
0%
M420 (QUAD9)
Point 1
X(ELGA)
-2.0
-2.0
0%
M420 (QUAD9)
Point 1
y (ELGA) -3.0
-3.0
0%
Node
J
T17.0 17.0 0%
M1181 (QUAD4)
Node J
X (ELNO)
-2.0
-2.0
0%
M1181 (QUAD4)
Node J
y (ELNO) -3.0
-3.0
0%
M1181 (QUAD4)
Point 1
X(ELGA)
-2.0
-2.0
0%
M1181 (QUAD4)

```
Point 1
\(y\) (ELGA) - 3.0
-3.0
0\%
Node
L
T25.0 25.0 0\%

\section*{M2000 (TRIA3)}

Node L
X (ELNO)
-2.0
-2.0
0\%
M2000 (TRIA3)
Node L
\(y\) (ELNO) -3.0
-3.0
\(0 \%\)
M2000 (TRIA3)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M2000 (TRIA3)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
- With a quadratic field of temperature (linear flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG
Node
With
T \(2.752 .750 \%\)
M1 (QUAD8)
Node A
X (ELNO)
-4.0
-4.2
\(5 \%\)

\section*{M1 (QUAD8)}

Node A
\(y\) (ELNO) -3.0
-3.3
10\%
Node
C
\(T 50.75\)
\(50.750 \%\)
M420 (QUAD9)
Node C
\(X(E L N O)\)
-20.0
-19.8
\(-1 \%\)
M420 (QUAD9)
Node C
\(y\) (ELNO) - 3.0
-3.3
10\%
Node
J
T \(77.077 .00 \%\)
M1181 (QUAD4)
Node J
\(X(E L N O)\)
-4.0
-4.2
5\%
M1181 (QUAD4)
Node J
y (ELNO) -30.0
-29.7
\(-1 \%\)
Node
\(L\)
T 125.0
125.0 0\%

M2000 (TRIA3)
Node L
X (ELNO)
-20.0
-19.8

\title{
M2000 (TRIA3)
}

Node L
\(y\) (ELNO) - 30.0
-29.7
\(-1 \%\)
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\section*{7 Modeling}

C

\section*{7.1}

Characteristics of modeling

\section*{Elements 2D modeling AXIS}

\section*{7.2}

\section*{Characteristics of the grid}

A number of nodes: 3382.
A number of meshs and types: 2000 meshs including 400 QUAD8, 400 QUAD9, 400 QUAD4 and 800 TRIA3.

The grid includes/understands 4 squares SQU8, SQU9, SQU4 and STR3 with a grid respectively with elements QUAD8, QUAD9, QUAD4 and TRIA3. Each square is discretized with 20 elements according
and following \(Y\).
```

$L$

```
SQU4
STR3

Name of
C
\(O\)
1.0
3.0
5.0

X

The nodes used for postprocessing are:

Name of
Co-ordinates
Number of the node
Name of a mesh
Type of the mesh
node
containing this node
With
(1.0;0.5)

N1
M1
QUAD8
C
(5.0; 0.5)

N1620
M420
QUAD9
\(J\)
(1.0;5.0)

N3001
M1181
QUAD4
\(L\)
(5.0;5.0)

N3440
M2000
TRIA3
The whole of the grid is affected by a thermal modeling AXIS.

\subsection*{7.3 Functionalities tested}

\section*{Orders}
"THERMAL" AFFE_MODELE "AXIS"
CREA_MAILLAGE MODI_MAILLAGE "QUAD8_9"

CREA_CHAMP "NOEU_TEMP_F"

CREA_RESU "EVOL_THER"

CALC_ELEM "FLUX_ELGA_TEMP"
"FLUX_ELNO_TEMP"
Handbook of Validation
V1.01 booklet: Tests of validity of the orders
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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8
Results of modeling \(C\)

\subsection*{8.1 Values}
tested
- With a linear field of temperature (constant flow):

\section*{Localization Size}

\section*{Reference}

\author{
Aster \%
}
difference
Net
Node or PG

\author{
Node \\ With \\ T 3.53 .5 \\ 0\% \\ M1 (QUAD8) \\ Node A \\ X (ELNO) \\ -2.0 \\ -2.0 \\ 0\% \\ M1 (QUAD8) \\ Node A \\ \(y\) (ELNO) -3.0 \\ -3.0 \\ 0\% \\ M1 (QUAD8) \\ Point 1 \\ \(X(E L G A)\) \\ -2.0 \\ -2.0 \\ 0\% \\ M1 (QUAD8) \\ Point 1 \\ \(y\) (ELGA) -3.0 \\ -3.0 \\ 0\% \\ Node \\ C
}
```

T11.511.5 0%
M420 (QUAD9)
Node C
X(ELNO)
-2.0
-2.0
0%
M420 (QUAD9)
Node C
y (ELNO) -3.0
-3.0
0%
M420 (QUAD9)
Point 1
X(ELGA)
-2.0
-2.0
0%
M420 (QUAD9)
Point 1
y(ELGA) -3.0
-3.0
0%
Node
J
T17.0 17.0 0%
M1181 (QUAD4)
Node J
X(ELNO)
-2.0
-2.0
0%
M1181 (QUAD4)
Node J
y (ELNO) -3.0
-3.0
0%
M1181 (QUAD4)
Point 1
X(ELGA)
-2.0
-2.0
0%

```
0\%
Node
\(L\)
T 25.025 .0 0\%
M2000 (TRIA3)
Node L
X (ELNO)
-2.0
-2.0
0\%
M2000 (TRIA3)
Node L
\(y\) (ELNO) -3.0
-3.0
0\%
M2000 (TRIA3)
Point 1
\(X(E L G A)\)
-2.0
-2.0
0\%
M2000 (TRIA3)
Point 1
\(y\) (ELGA) - 3.0
-3.0
0\%
- With a quadratic field of temperature (linear flow):

\section*{Localization Size}

Reference
Aster \%
difference
Net
Node or PG

Node

\section*{With}

T 2.752 .75 0\%
M1 (QUAD8)
Node A
\(X(E L N O)\)
-4.0
-4.0
0\%
M1 (QUAD8)
Node A
\(y\) (ELNO) -3.0
-3.0
0\%
Node
C
T 50.75
\(50.750 \%\)
M420 (QUAD9)
Node C
\(X(E L N O)\)
-20.0
-20.0
0\%
M420 (QUAD9)
Node C
y (ELNO) -3.0
-3.0
0\%
Node
J
T \(77.077 .00 \%\)
M1181 (QUAD4)
Node J
X (ELNO)
-4.0
-4.2
5\%
M1181 (QUAD4)
Node J
y (ELNO) -30.0
-29.7
-1\%
Node

Node L
X (ELNO)
-20.0
-19.8
\(-1 \%\)
M2000 (TRIA3)
Node L
y (ELNO) -30.0
-29.7
-1\%
Handbook of Validation
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HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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\section*{9 Modeling}

D

\section*{9.1}

Characteristics of modeling

\section*{Elements 2D modeling AXIS_DIAG}

\section*{9.2}

\section*{Characteristics of the grid}

A number of nodes: 3782
A number of meshs and types: 2400 meshs including 800 TRIA6, 400 QUAD9, 400 QUAD4 and 800 TRIA3.

The grid includes/understands 4 squares STR6, SQU9, SQU4 and STR3 with a grid respectively with elements TRIA6, QUAD9, QUAD4 and TRIA3. Each square is discretized with 20 elements according to X and following \(Y\).

\section*{Y}
5.0
\(J\)
K
\(L\)
SQU4
STR3
Name of
Type
GROUP_MA
of element
\(G\)
H
I
3.0

STR6 TRIA6
SQU9 QUAD9

\section*{2.5 \\ D \\ E \\ F \\ SQU4 QUAD4}

STR3 TRIA3

\section*{STR6}

SQU9
With
B
C
0.5
\(O\)
1.0
3.0
5.0
\(X\)
The nodes used for postprocessing are:

\author{
Name of \\ Co-ordinates \\ Number of the node \\ Name of a mesh \\ Type of the mesh \\ node \\ containing this node \\ With \\ (1.0;0.5) \\ N1 \\ M1 \\ TRIA6 \\ C \\ (5.0; 0.5) \\ N2481 \\ M820 \\ QUAD9 \\ \(J\) \\ (1.0; 5.0) \\ N3401 \\ M1581 \\ QUAD4 \\ \(L\) \\ (5.0;5.0) \\ N3840
}

\title{
The whole of the grid is affected by a thermal modeling AXIS_DIAG.
}

\subsection*{9.3 Functionalities}
tested

\section*{Orders}
"THERMAL" AFFE_MODELE "AXIS_DIAG"

CREA_MAILLAGE MODI_MAILLAGE "QUAD8_9"

CREA_CHAMP "NOEU_TEMP_F"

CREA_RESU "EVOL_THER"

CALC_ELEM "FLUX_ELGA_TEMP"
"FLUX_ELNO_TEMP"
Handbook of Validation
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Date:
03/06/03

\section*{Author (S):}

\author{
J. PELLET, Key P. HERMAN
}

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}

\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested
- With a linear field of temperature (constant flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG

\author{
Node \\ With \\ T 3.53 .5 \\ 0\% \\ M1 (TRIA6) \\ Node A \\ \(X\) (ELNO) \\ -2.0 \\ -2.0 \\ 0\% \\ M1 (TRIA6) \\ Node A \\ y (ELNO) -3.0 \\ -3.0 \\ 0\% \\ M1 (TRIA6) \\ Point 1 \\ \(X\) (ELGA) \\ -2.0 \\ -2.0 \\ 0\%
}

\section*{M1 (TRIA6)}

Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
C
T \(11.511 .50 \%\)
M420 (QUAD9)
Node C
\(X\) (ELNO)
-2.0
-2.0
0\%
M420 (QUAD9)
Node C
y (ELNO) -3.0
-3.0
0\%
M420 (QUAD9)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M420 (QUAD9)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
J
T 17.017 .0 0\%
M1181 (QUAD4)
Node J
X (ELNO)
-2.0
-2.0
0\%
M1181 (QUAD4)
Node J
y (ELNO) -3.0
-3.0
\(X(E L G A)\)
-2.0
-2.0
0\%
M1181 (QUAD4)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
Node
L
T 25.025 .0 0\%
M2000 (TRIA3)
Node L
X (ELNO)
-2.0
-2.0
0\%
M2000 (TRIA3)
Node L
\(y\) (ELNO) -3.0
-3.0
0\%
M2000 (TRIA3)
Point 1
\(X\) (ELGA)
-2.0
-2.0
0\%
M2000 (TRIA3)
Point 1
\(y\) (ELGA) -3.0
-3.0
0\%
- With a quadratic field of temperature (linear flow):

Localization Size
Reference
Aster \%

Node or PG

\author{
Node \\ With \\ T 2.752 .75 0\% \\ M1 (QUAD8) \\ Node A \\ X (ELNO) \\ -4.0 \\ -4.2 \\ 5\% \\ M1 (QUAD8) \\ Node A \\ \(y\) (ELNO) -3.0 \\ -3.3 \\ 10\% \\ Node \\ C \\ T 50.75 \\ 50.75 0\% \\ M420 (QUAD9) \\ Node C \\ X (ELNO) \\ -20.0 \\ -19.8 \\ -1\% \\ M420 (QUAD9) \\ Node C \\ y (ELNO) -3.0 \\ -3.3 \\ 10\% \\ Node \\ \(J\) \\ T 77.077 .0 0\% \\ M1181 (QUAD4) \\ Node J \\ X (ELNO) \\ -4.0 \\ -4.2
}
```

5%
M1181 (QUAD4)
Node J
y (ELNO) -30.0
-29.7
-1%
Node
L
T 125.0
125.0 0%
M2000 (TRIA3)
Node L
X (ELNO)
-20.0
-19.8
-1%
M2000 (TRIA3)
Node L
y (ELNO) -30.0
-29.7
-1%
Handbook of Validation
V1.01 booklet: Tests of validity of the orders
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Version
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Date:
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Author (S):
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```
11 Modeling

\subsection*{11.1 Characteristics of modeling}

Elements 3D modeling 3D

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 17281.
A number of meshs and types: 5268 meshs including 400 HEXA8, 400 HEXA20, 400 HEXA27, 800 PENTA6, 800 PENTA15, 600 PYRAM5, 600 PYRAM13, 634 TETRA4 and 634 TETRA10.

The grid includes/understands 9 hexahedrons VHE8, VHE20, VHE27, VPE6, VPE15, VPY5, VPY13, VTE4 and
VTE10 with a grid respectively with elements HEXA8, HEXA20, HEXA27, PENTA6, PENTA15, PYRAM5, PYRAM13, TETRA4 and TETRA10.

\section*{Y}

Each volume is included/understood
between \(Z=1.0\) and \(Z=1.1\)
Name of
Type
Discretization
GROUP_MA
of element
according to:
7.5

C7
D7

\section*{C8}

D8
C9
D9
X Y Z
VPY13
VHE20
VHE27
VHE8 HEXA8
20
20
```

l
A7
A9
B7
A8
B8
B9
5.5
VHE2O HEXA2O 20
20 1
5 . 0
VHE27 HEXA27 20
20 1
C4
D4
C5
D5
C6
D6
VPE6 PENTA6 }2
20
l
VPY5
VTE4
VTE10
VPE15 PENTA15 20 20 l
A4
B4
A5
B5
A6
B6
3.0
VPY5 PYRAM5
10
10
l
2.5
C1
DI
C2

```

D2
C3
D3
VPY13 PYRAM13 10101
VHE8
VPE6
VPE15
VTE4 TETRA4
10
10
1
A1
B1
A2
B2
A3
B3
VTE10 TETRA10 10101
0.5

O 0.5
2.53 .0
5.05 .5
7.5

X

The nodes used for postprocessing are:
Name of
Co-ordinates
Number of the node
Name of a mesh
Type of the mesh
node
containing this node
D1
(2.5; \(2.5 ; 1.0\) )

N440
M400
HEXA8
D2
(5.0; \(2.5 ; 1.0\) )

N1322

The whole of the grid is affected by a thermal modeling 3D.

\subsection*{11.3 Functionalities \\ tested}

\section*{Orders}
"THERMAL" AFFE_MODELE "3D"

CREA_CHAMP "NOEU_TEMP_F"

CREA_RESU "EVOL_THER"

CALC_ELEM "FLUX_ELGA_TEMP"

\author{
"FLUX_ELNO_TEMP" \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of the orders
}

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Author (S):
J. PELLET, Key P. HERMAN
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\section*{12 Results of modeling \(E\)}

\subsection*{12.1 Values}
tested

\section*{- With a linear field of temperature (constant flow):}

Localization Size
Reference
Aster \%
difference
Net
Node or PG
\begin{tabular}{l} 
Node \\
\(D 1\) \\
\(T\) \\
16.5 \\
\(16.50 \%\) \\
\(M 400\) (HEXA8) \\
\(D 1\) node \\
\(X\) (ELNO) \\
-2.0 \\
-2.0 \\
\(0 \%\) \\
\(M 400\) (HEXA8) \\
\(D 1\) node \\
\(y(E L N O)-3.0\) \\
-3.0 \\
\(0 \%\) \\
\(M 400\) (HEXA8) \\
\(D 1\) node \\
\(Z(E L N O)\) \\
-4.0 \\
-4.0 \\
\(0 \%\) \\
\(M 400(H E X A 8)\) \\
\(P o i n t 1\) \\
\(X(E L G A)-2.0\) \\
-2.0 \\
\(0 \%\) \\
Node \\
\(D 2\) \\
\(T\) \\
21.5 \\
\(21.50 \%\) \\
\\
\hline
\end{tabular}
\(21.50 \%\)

\section*{M1200 (PENTA6)}

D2 node
X (ELNO)
-2.0
-2.0
0\%
M1200 (PENTA6)
D2 node
y (ELNO) -3.0
-3.0
0\%
M1200 (PENTA6)
D2 node
Z (ELNO)
-4.0
-4.0
0\%
M1200 (PENTA6)
Point 1
\(X\) (ELGA) -2.0
-2.0
0\%
Node
D3
T
26.5
\(26.50 \%\)
M4034 (PENTA15)
D3 node
X (ELNO)
-2.0
-2.0
0\%
M4034 (PENTA15)
D3 node
y (ELNO) -3.0
-3.0
0\%
M4034 (PENTA15)
D3 node
Z (ELNO)
-4.0
-4.0
```

0%
M4034 (PENTA15)
Point 1
X (ELGA) -2.0
-2.0
0%
Node
D4
T
24.0
24.0 0%
M1795 (PYRAM5)
D4 node
X (ELNO)
-2.0
-2.0
0%
M1795 (PYRAM5)
D4 node
y (ELNO) -3.0
-3.0
0%
M1795 (PYRAM5)
D4 node
Z (ELNO)
-4.0
-4.0
0%
M1795 (PYRAM5)
Point 1
X (ELGA) -2.0
-2.0
0%
Node
D5
T
2 9 . 0
29.0 0%
M1838 (TETRA4)
D5 node
X (ELNO)
-2.0
-2.0

```
```

0%
M1838 (TETRA4)
D5 node
y (ELNO) -3.0
-3.0
0%
M1838 (TETRA4)
D5 node
Z (ELNO)
-4.0
-4.0
0%
M1838 (TETRA4)
Point 1
X (ELGA) -2.0
-2.0
0%
Node
D6
T
34.0
34.0 0%
M4672 (TETRA10)
D6 node
X (ELNO)
-2.0
-2.0
0%
M4672 (TETRA10)
D6 node
y (ELNO) -3.0
-3.0
0%
M4672 (TETRA10)
D6 node
Z (ELNO)
-4.0
-4.0
0%
M4672 (TETRA10)
Point 1
X (ELGA) -2.0
-2.0

```

Node
M4629 (PYRAM13)
D7 node
X (ELNO)
- 2.0
-2.0
0\%
M4629 (PYRAM13)
D7 node
y (ELNO) -3.0
-3.0
\(0 \%\)
M4629 (PYRAM13)
D7 node
Z (ELNO)
-4.0
-4.0
0\%
M4629 (PYRAM13)
Point 1
\(X\) (ELGA) -2.0
-2.0
0\%
Node
D8
\(T\)
36.5
\(36.50 \%\)
M2834 (HEXA20)
D8 node
X (ELNO)
-2.0
-2.0
0\%
M2834 (HEXA20)
D8 node
y (ELNO) -3.0
-3.0
```

0%

```
M2834 (HEXA20)
D8 node
Z (ELNO)
-4.0
-4.0
0\%
M2834 (HEXA20)
Point 1
\(X\) (ELGA) -2.0
-2.0
0\%
Node
D9
\(T\)
41.5
\(41.50 \%\)
M3234 (HEXA27)
D9 node
X (ELNO)
-2.0
-2.0
0\%
M3234 (HEXA27)
D9 node
\(y\) (ELNO) -3.0
-3.0
0\%
M3234 (HEXA27)
D9 node
Z (ELNO)
-4.0
-4.0
0\%
M3234 (HEXA27)
Point 1
\(X\) (ELGA) -2.0
-2.0
0\%
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}

\section*{:}

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- With a quadratic field of temperature (linear flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG
Node
\(D 1\)
\(T\)
35.25
\(35.250 \%\)
M400 (HEXA8)
D1 node
\(X\) (ELNO)
-10.0
-9.8
\(-2 \%\)
\(M 400\) (HEXA8)
\(D 1\) node
\(y(E L N O)-15.0\)
-14.7
\(-2 \%\)
```

M400 (HEXA8)
D1 node
Z (ELNO) -8.0
-8.4
5%
Node
D2
T
72.75
72.75 0%
M1200 (PENTA6)
D2 node
X (ELNO)
-20.0
-19.8
-1%
M1200 (PENTA6)
D2 node
y (ELNO) -15.0
-14.7
-2%
M1200 (PENTA6)
D2 node
Z (ELNO) -8.0
-8.4
5%
Node
D3
T
135.25
135.25 0%
M4034 (PENTA15)
D3 node
X (ELNO)
-30.0
-30.0
0%
M4034 (PENTA15)
D3 node
y (ELNO) -15.0
-15.0
0%
M4034 (PENTA15)

```
```

D3 node
Z (ELNO) -8.0
-8.0
0%
Node
D4
T
9 1 . 5 0
91.50 0%
M1795 (PYRAM5)
D4 node
X (ELNO)
-10.0
-9.6
-4%
M1795 (PYRAM5)
D4 node
y (ELNO) -30.0
-29.4
-2%
M1795 (PYRAM5)
D4 node
Z (ELNO) -8.0
-7.2
-10%
Node
D5
T
1 2 9 . 0 0
129.00 0%
M1838 (TETRA4)
D5 node
X (ELNO)
-20.0
-19.6
-2%
M1838 (TETRA4)
D5 node
y (ELNO) -30.0
-29.4
-2%
M1838 (TETRA4)
D5 node

```
```

Z (ELNO) -8.0

```
-8.4
5\%
Node
D6
\(T\)
191.5
\(191.50 \%\)
M4672 (TETRA10)
D6 node
X (ELNO)
-30.0
-30.0
0\%
M4672 (TETRA10)
D6 node
\(y\) (ELNO) -30.0
-30.0
0\%
M4672 (TETRA10)
D6 node
Z (ELNO) -8.0
-8.0
0\%
Node
D7
\(T\)
185.25
\(185.250 \%\)
M4629 (PYRAM13)
D7 node
X (ELNO)
-10.0
-10.0
0\%
M4629 (PYRAM13)
D7 node
\(\boldsymbol{y}\) (ELNO) -45.0
-45.0
0\%
M4629 (PYRAM13)
D7 node
Z (ELNO) -8.0

0\%
Node
D8
T
222.75
\(222.750 \%\)
M2834 (HEXA20)
D8 node
X (ELNO)
-20.0
-20.0
0\%
M2834 (HEXA20)
D8 node
\(y\) (ELNO) -45.0
-45.0
0\%
M2834 (HEXA20)
D8 node
Z (ELNO) -8.0
-8.0
0\%
Node
D9
T
285.25
\(285.250 \%\)
M3234 (HEXA27)
D9 node
X (ELNO)
-30.0
-30.0
0\%
M3234 (HEXA27)
D9 node
\(y\) (ELNO) -45.0
-45.0
0\%
M3234 (HEXA27)
D9 node
Z (ELNO) -8.0
-8.0

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\section*{13 Modeling}

F

\subsection*{13.1 Characteristics of modeling}

Elements 3D modeling 3D_DIAG
13.2 Characteristics of the grid

A number of nodes: 2356.
A number of meshs and types: 2434 meshs including 400 HEXA8, 800 PENTA6, 600 PYRAM5 and 634
TETRA4.
The grid includes/understands 4 hexahedrons VHE8, VPE6, VPY5 and VTE4 with a grid respectively with
elements HEXA8, PENTA6, PYRAM5 and TETRA4.

Y

Each volume is included/understood
between \(Z=1.0\) and \(Z=1.1\)

\author{
Name of \\ Type \\ Discretization \\ GROUP_MA \\ of element \\ according to:
}

X Y Z
5.0

C4
D4
VHE8 HEXA8
20
20
1
C5
D5
VPE6 PENTA6
20
20
1
VPY5
VTE4
A4
B4
A5
B5
VPY5 PYRAM5
10
10
1
3.0

VTE4 TETRA4
10
10

\author{
C1
}
\(X\)
The nodes used for postprocessing are:

\author{
Name of \\ Co-ordinates \\ Number of the node \\ Name of a mesh \\ Type of the mesh \\ node \\ containing this node \\ D1 \\ ( \(2.5 ; 2.5 ; 1.0\) ) \\ N440 \\ M400 \\ HEXA8 \\ D2 \\ (5.0; \(2.5 ; 1.0\) ) \\ N1322 \\ M1200 \\ PENTA6 \\ D3
}
( 7.5 ; 2.5 ; 1.0)
N11453
M4034
PENTA15
D4
(2.5; \(5.0 ; 1.0)\)

N1995
M1795
PYRAM5
The whole of the grid is affected by a thermal modeling 3D_DIAG.

\subsection*{13.3 Functionalities}
tested
Orders
"THERMAL" AFFE_MODELE "3D_DIAG"

CREA_CHAMP "NOEU_TEMP_F"

CREA_RESU "EVOL_THER"

CALC_ELEM "FLUX_ELGA_TEMP"
"FLUX_ELNO_TEMP"
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\section*{14 Results of modeling \(F\)}

\subsection*{14.1 Values}
tested
- With a linear field of temperature (constant flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG
Node
D1
T 16.5
\(16.50 \%\)
M400 (HEXA8)
D1 node
X (ELNO)
-2.0
-2.0
\(0 \%\)
M400 (HEXA8)
```

D1 node
y (ELNO) -3.0
-3.0
0%
M400 (HEXA8)
D1 node
Z (ELNO)
-4.0
-4.0
0%
M400 (HEXA8)
Point 1
X (ELGA) -2.0
-2.0
0%
Node
D2
T 21.5
21.5 0%
M1200 (PENTA6)
D2 node
X (ELNO)
-2.0
-2.0
0%
M1200 (PENTA6)
D2 node
y (ELNO) -3.0
-3.0
0%
M1200 (PENTA6)
D2 node
Z (ELNO)
-4.0
-4.0
0%
M1200 (PENTA6)
Point 1
X (ELGA) -2.0
-2.0
0%
Node
D3

```

T 24.0
24.0 0\%

M1795 (PYRAM5)
D3 node
X (ELNO)
-2.0
-2.0
0\%
M1795 (PYRAM5)
D3 node
y (ELNO) -3.0
-3.0
0\%
M1795 (PYRAM5)
D3 node
Z (ELNO)
-4.0
-4.0
0\%
M1795 (PYRAM5)
Point 1
\(X\) (ELGA) -2.0
-2.0
0\%
Node
D4
T 29.0
29.0 0\%

M1838 (TETRA4)
D4 node
X (ELNO)
-2.0
-2.0
0\%
M1838 (TETRA4)
D4 node
\(y\) (ELNO) -3.0
-3.0
0\%
M1838 (TETRA4)
D4 node
Z (ELNO)
-4.0
-4.0
0\%
M1838 (TETRA4)
Point 1
\(X\) (ELGA) -2.0
-2.0
0\%
- With a quadratic field of temperature (linear flow):

Localization Size
Reference
Aster \%
difference
Net
Node or PG
Node
D1
T 35.25
\(35.250 \%\)
M400 (HEXA8)
D1 node
\(X\) (ELNO)
-10.0
-9.8
\(-2 \%\)
M400 (HEXA8)
D1 node
\(y(E L N O)-15.0-14.7\)
\(-2 \%\)
\(M 400\) (HEXA8)
\(D 1\) node
\(Z(E L N O)-8.0\)
-8.4
\(5 \%\)
Node
\(D 2\)
\(T 72.75\)
\(72.750 \%\)
\(M 1200\) (PENTA6)

M1200 (PENTA6)
```

D2 node
X (ELNO)
-20.0
-19.8
-1%
M1200 (PENTA6)
D2 node
y (ELNO) -15.0-14.7
-2%
M1200 (PENTA6)
D2 node
Z (ELNO) -8.0
-8.4
5%
Node
D3
T }91.5
91.50 0%
M1795 (PYRAM5)
D3 node
X (ELNO)
-10.0
-9.6
-4%
M1795 (PYRAM5)
D3 node
y (ELNO) -30.0-29.4
-2%
M1795 (PYRAM5)
D3 node
Z (ELNO) -8.0
-7.2
-10%
Node
D4
T
1 2 9 . 0 0
129.00 0%
M1838 (TETRA4)
D4 node
X (ELNO)
-20.0
-19.6

```

\section*{-2\%}

M1838 (TETRA4)
D4 node
y (ELNO) -30.0-29.4
-2\%
M1838 (TETRA4)
D4 node
Z (ELNO) -8.0
-8.4
5\%
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15 Summary of the results
Summary of the results

\section*{modeling}

Flow with the nodes
Flow with the nodes with one
or with the PG with
quadratic temperature
a temperature
linear
Type
Discretization in relative Error
Relative error max \%
of element
\(X\) and \(Y\)
max \%
Modeling "PLANE" 2 D
QUAD4 10
el/m 0\%
5\%
TRIA3 10
el/m 0\%
1\%
QUAD8 10
el/m 0\%0\%
QUAD9 10
el/m 0\%
0\%
Modeling 2D "PLAN_DIAG" QUAD4 10
el/m 0\%
10\%
TRIA3 10
el/m 0\%
10\%
TRIA6 10
el/m 0\%
5\%
QUAD9 10
el/m 0\%
\(1 \%\)
Modeling 2D "AXIS"
QUAD4 10
el/m 0\%
5\%

\title{
TRIA3 10
}
el/m 0\%
1\%
QUAD8 10
el/m 0\%
0\%
QUAD9 10
el/m 0\%
\(0 \%\)
Modeling 2D "AXIS_DIAG"
QUAD4 10
el/m 0\%
10\%

TRIA3 10
el/m 0\%
10\%
TRIA6 10
el/m 0\%
5\%
QUAD9 10
el/m 0\%
1\%

Modeling "3D"
HEXA8 10
el/m 0\%
5\%
PENTA6 10
el/m
\(0 \%\)
5\%
PYRAM5 5
el/m
0\%
10\%
TETRA4 5
```

el/m
0%
5%
HEXA20 10
el/m
0%
0%
HEXA27 10
el/m
0%
0%
PENTA15 10
el/m
0%
0%
PYRAM13 5
el/m
0%
0%
TETRA10 5
el/m
0%
0%
Modeling "3D_DIAG"
HEXA8 10
el/m 0%
5%
PENTA6 10
el/m
0%
5%
PYRAM5 5
el/m
0%
10%

```

\section*{TETRA4 5}

With a linear field of temperature (and a constant flow): - All modelings give exact results.

With a quadratic field of temperature (and a linear flow):
- The results are exact with elements of order 2 and one modeling not DIAG. These elements having quadratic functions of form can represent the field exactly of temperature imposed. Flows are exact and would be it still with a discretization more coarse.
- The elements of order 2 with a modeling DIAG are treated like elements of order 1
for the calculation of flow. For these elements, flows are constant by element. The error is thus directly related to the size of the elements.
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Titrate:
SENSM06 - Sensitivity to the parameters materials in linear elasticity
Date:
02/06/03
Author (S):
NR. TARDIEU Key
V1.01.170-A Page:
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Organization (S): EDF-R \& D /AMA

\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ Document: V1.01.170
}

SENSM06 - Sensitivity to the parameters materials in isotropic linear elasticity

\section*{Summary:}

One tests the sensitivity to the parameters materials in isotropic linear elasticity on a tensile test uniaxial. One compares the results obtained by operators linear MECA_STATIQUE and nonlinear STAT_NON_LINE.

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}

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02/06/03
Author (S):
NR. TARDIEU Key

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1
Problem of reference

\subsection*{1.1 Geometry}
Traction of 100 MPa
1 mm
10 mm

\section*{1.2 \\ Properties of material}

The values of the parameters materials are as follows:
\(E=100000 . \mathrm{MPa}\)
\(=0.3\)

\section*{1.3 \\ Boundary conditions and loadings}

A homogeneous state of stresses is sought: one imposes only one pressure of 100 MPa .
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\section*{2 \\ Reference solution \\ 2.1 \\ Method of calculation}

The solution in displacement is given by:
\(U=. r\)
\(U=. Z\)
\(R\)
E
Z
E
For the derived fields compared to \(E\), one thus has:
\(U\)
\(U\)
\(R=-\)
.r
\(Z=-\)
. Z
E
E2
E

E2
For the derived fields compared to, one thus has:
```

U
=.
Z
R
= 0
E
2.2
Sizes and results of reference

```

One tests for each field derived the value from his components at the point higher right, that is to say:
ur
\(=-\)
\(U\)
5
1 E-8
\(Z=\).
\(1 E-8\)
E
E
\(U\)
U
.
5th-3
\(Z=0\)
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Axisymmetric modeling on the following grid:

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 4
Numbers and types of meshs: 4 SEG2, 1 QUAD4

\author{
3.3 Functionalities \\ tested \\ Orders
}

\author{
3.4 \\ Sizes tested and results
}

\section*{Sizes tested}

Values obtained
Analytical values
Variation (\%)
\(U\)
\(R\)
1.5E-8
1.5E-8

0
E
\(U\)

Z
1.E-8
1.E-8

0
E
\(U\)

\section*{R}
5.E-3
5.E-3

0

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\section*{Titrate:}

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\section*{4 Modeling \\ B}

\section*{4.1}

Characteristics of modeling
Axisymmetric modeling on the following grid:

\title{
4.2 \\ Characteristics of the grid
}

A number of nodes: 4
Numbers and types of meshs: 4 SEG2, 1 QUAD4

\subsection*{4.3 Functionalities}
tested
Orders

DEFI_PARA_SENSI

MECA_STATIQUE SENSITIVITY

\section*{4.4}

Sizes tested and results

\author{
Sizes tested \\ Values obtained \\ Analytical values \\ Variation (\%) \\ \(U\) \\ \(R\) \\ \(1.5 E-8\) \\ 1.5E-8 \\ 0 \\ E \\ \(U\)
}

Z
1.E-8
1.E-8

0
E
\(U\)

\section*{R}
5.E-3
5.E-3

0
\(U\)

Z
0
0
0

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Date:
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Author (S):
NR. TARDIEU Key
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\section*{5 \\ Summary of the results}

The results given by STAT_NON_LINE and MECA_STATIQUE are exactly those given by theory.
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\section*{Code_Aster \({ }^{\circledR}\)}

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Titrate:
SENSD04 - Sensitivity to the loading and C.L in dyna_line_tran
Date:
07/10/03
Author (S):
Key H. ANDRIAMBOLOLONA
:
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Organization (S): EDF-R \& D /AMA

\section*{Handbook of Validation}

V1.01 booklet: Tests of validity of the orders
Document: V1.01.171

\section*{SENSD04 - Sensitivity to the loading and C.L in dyna_line_tran}

\section*{Summary:}

This case test takes again the geometry and materials of the case test SENSD01"Sensitivity to the loading and C.L.
in dyna_line_harm " [V1.01.158]. Calculation carried out is the transitory response of a linear discrete system not deadened.

The functionality tested is the derivation of the fields of displacement, speed and acceleration compared to
boundary conditions and with the loadings.
The reference was obtained with Code_Aster version 7.1.3 per direct application of the "derived" loading on standard calculation.

\author{
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}

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Version
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Titrate:
SENSD04 - Sensitivity to the loading and C.L in dyna_line_tran
Date:
07/10/03
Author (S):
Key H. ANDRIAMBOLOLONA
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1
Problem of reference

\subsection*{1.1 Geometry}

U 1
U2
U3
U8
K
K
K
K
With
m
\(m\)
\(m\)
\(m\)
B
\(X, U\)
P 1
P2
P3
P8

\section*{1.2}

Properties of material

Stiffness in translation
\(K=105 \mathrm{~N} / \mathrm{m}\)
Specific mass
\(m=10 \mathrm{~kg}\)

\section*{1.3 \\ Boundary conditions and loadings}

Boundary conditions:
Points A and B (nodes 1 and 10): null displacements:
\(u P i=0 m\)

Not P1 (node 2):
imposed displacement:
\(u 1=D 1\)
\(D 1=1 \mathrm{~m}\)
Loading (tdébut = 0 S T tfin = 104 S):

Not P4 (node 5): nodal force according to \(X\) : \(F\)
\(P 4=F T\)
1
(TT
end
beginning)
1
\(F=2 N R\)

Not P5 (node 6): nodal force according to X: F
\(P 5=F T\)
2
(TT
end
beginning)
```

2
F=0 NR

```

\subsection*{1.4 Conditions \\ initial}

Initial displacement no one and null initial speed.
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The equilibrium equation of the standard problem can be written in the following matric form:
\(\boldsymbol{M}(p) \boldsymbol{X} \boldsymbol{\&}(T, p)+\boldsymbol{C}(p) \boldsymbol{X} \boldsymbol{\&}(T, p)+\boldsymbol{K}(T, p) \boldsymbol{X}(T, p)=\boldsymbol{F}(T, p)\)
where:
\(\boldsymbol{M}, \boldsymbol{C}\) and \(\boldsymbol{K}\) are respectively the matrices of mass, damping and stiffness of model,
\(X\) indicates the vector made up of \(N\) degrees of freedom of the model,
\(\boldsymbol{F}\) is the vector of the loadings.

The differential equation governing the derivative compared to the parameters of sensitivity \(p\) of the vector
\(X\)
displacement \(\boldsymbol{X}, \boldsymbol{Y}=\), are obtained by directly deriving the equilibrium equation compared to \(p\)
p. After rearrangement of the terms, one obtains:

\section*{\(M\) ( \\ F, \\ M \\ C \\ \(K\)}
p) \(\boldsymbol{Y} \boldsymbol{\&}(T, p)+(\)
\(\boldsymbol{C} p) \boldsymbol{Y} \boldsymbol{\&}(T, p)+\boldsymbol{K}(p) \boldsymbol{Y}(T, p)\)
(Tp)
(p)
=
-
\(\boldsymbol{X} \boldsymbol{\&}(T, p)\)
(p)
\(\boldsymbol{X} \boldsymbol{\&}(T, p)\)
(p)
-
\(\boldsymbol{X}(T, p)\)
p
\(p\)
\(p\)
\(p\)

One can note that the matrices (mass, damping and stiffness) of the system associated with calculation derivative are the same ones as those of the initial problem. Thus, we chose as solution
\(\boldsymbol{F}(T, p)\)
of reference, that obtained by direct application of the "derived" loading on the problem
\(p\)

\section*{standard.}

In our case, the derivative of the loading compared to 1
\(F\), respectively 2
\(F\), corresponds to one
unit force applied to node 5, respectively applied to node 6. The derivative of the loading compared to 1
\(D\) corresponds to a unit displacement applied to node 2.

\section*{2.2 \\ Results of reference}

The parameters of sensitivity considered are: 1
\(F, 2\)
\(F\) and 1
D.

At the moment tfin:
displacements \((X, y, Z)\) of node 5 ,
derived compared to 1
\(F\) of displacement and speed of node 5,
derived compared to 2
\(F\) of the displacement and acceleration of node 5,
derived compared to 1
\(D\) of the displacement of node 5 .

\subsection*{2.3 Uncertainties}

The results depend on the temporal discretization and the diagram of integration. Here, we have chosen as reference solution that obtained by the implicit scheme of Newmark (with \(=0.25\) and \(=0.5)\). We estimate uncertainty on the solution lower than \(0.5 \%\).
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Author (S):

\title{
Key H. ANDRIAMBOLOLONA
}

\section*{:}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Modeling identical to that of the SENSD01A

\section*{Characteristics of the elements:}

DISCRETE:
with nodal masses
\(M_{-} T \_D \_N\)
and matrices of rigidity
\(K_{-} T \_D \_L\)
Limiting conditions:
in all the nodes
DDL_IMPO =
( \(A L L=" Y E S " D Y=0 ., D Z=0\). )
with the nodes ends
\(\left(G R O U P \_N O=" A B " D X=0.\right)\)
Names of the nodes:
Not \(A=N 1\)
\(P 1=N 2\)
Not \(B=N 10\)
\(P 2=N 3\)
\(\qquad\)
\(P 8=N 9\)
Parameters of calculation:
Temporal incrementing
\(T=105 \mathrm{~S}\)
Parameters Newmark diagram
\(=0.25\)
\(=0.5\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and type: 9 SEG2

\subsection*{3.3 Functionalities}
tested
The functionality tested is the derivation of the fields of displacement, speed and acceleration by report/ratio in the boundary conditions and with the loadings.

\section*{Orders}

DYNA_LINE_TRAN SENSITIVITY

\title{
NEWMARK
}

DIFF_CENTRE

\author{
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}

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\subsection*{3.4 Values}
tested
The tests are carried out for the moment of calculation tfin.
Displacements of node 5:

\section*{Component Reference}

\section*{Aster Difference}
(\%)
DX 3.35
10-10 3.3499767946719
10-10 6.93
10-4
DY 0
0
0
DZ 0
0

\section*{Field Reference}

Aster Difference
(\%)
Displacement 1.67
10-10 1.6749824976405
10-10 0.298
Speed 5.00
10-6 4.9999150009176
10-6 0.002
Derived compared to 2
\(F\) of component DX of the displacement and the acceleration of node 5
(Newmark diagram)

\section*{Field Reference}

Aster Difference
(\%)
Displacement 8.75
10-16 8.7511563568538
10-16 0.013
Acceleration 1.67
10-6 1.6749649953512
10-6 0.297
Derived compared to 1
D of component DX of the displacement of node 5

\section*{Field Reference}

Aster Difference
(\%)
Displacement 1.18
10-15 1.179939099689
10-15 0.005
Derived compared to 1
\(F\) of component DX of the displacement and the speed of node 5 (diagram centered differences)

\section*{Field Reference}

Aster Difference
(\%)
Displacement 1.67
10-10 1.6664833417849
10-10 0.211
Speed 5.00
10-6 4.9999166758329
10-6 0.002
Derived compared to 2
\(F\) of component DX of the displacement and the acceleration of node 5 (diagram centered differences)

\section*{Field Reference}

Aster Difference
(\%)
Displacement 8.75
10-16 8.329087746405
10-16 4.81
Acceleration 1.67
10-6 1.6664666836292
10-6 0.212
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\section*{Summary of the results}

The precision of the results is coherent with the method of resolution used and the method of direct derivation "exact".

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Organization (S): EDF-R \& D /AMA

\section*{Handbook of Validation}

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\section*{Summary}

This case test takes again the geometry and the materials of test SENSD02 but calculation carried out here is the answer
transient of the system.
The functionality tested is the derivation of the fields of displacement, speed and acceleration compared to properties materials modulus Young and Poisson's ratio.

The reference is calculated by finished differences.

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1
Problem of reference
1.1 Geometry

\section*{CENTER}

BORD_GAU
3. \(m\)
10. \(m\)
\(1 m<X<11 m\)
\(1.5 m<Y<1.5 m\)
\(X\)
\(0 m<Z<1 m\)
Z

\section*{1.2}

Properties of material
\(E=105 \mathrm{~Pa}\)
\(=0.3\)
\(=2400 \mathrm{~kg} / \mathrm{m} 3\)
Viscous damping, matrix of damping: \(K+M,=0.002 S\) and \(=0.3 \mathrm{sl}\)

\section*{1.3}

Boundary conditions and loadings
Boundary conditions:
Line "BORD_GAU" is blocked according to \(X, y\) and \(Z\).
The node " \(I\) " is blocked according to \(X, y\) and \(Z\).
Loading (tdébut = 0 S T tfin = 104 S):

\section*{On the node "OPPOSES":}
nodal force according to \(X\) : \(F\)
\[
X=F T
\]
1
\(F=100\) NR,
nodal force according to \(y\) : \(F\)
\(Y=F T\)

2
(TT
end
beginning)
2
\(F=30 \mathrm{NR}\),
nodal force according to Z: F
\(Z=F T\)
\(F=10 \mathrm{NR}\),
3
(T T
end
beginning)
3

\subsection*{1.4 Conditions}
initial
Initial displacement no one and null initial speed.
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Code_Aster \({ }^{\circledR}\)

\section*{Titrate:}

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:

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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation}

The sensitivities of the fields displacement, speed and acceleration with the data materials are calculated by finished differences. These calculations by finished differences are carried out inside even case test by Code_Aster. The diagram of finished differences used is as follows (diagram of order 1 not centered):
\(F\) (,
\(X\) )
\(y\)
\(F(x+\),
\(y-F\),
\(X\) )
\(y\)

\section*{\(X\)}

The field of solution displacement of the standard problem (i.e nonderived) is subjected to a test of not regression. Tests of nonregression are also carried out on the sensitivities.

\section*{2.2 \\ Sizes and results of reference}

The parameters of sensitivity are the Young modulus E and the Poisson's ratio.

The values of reference are calculated at the moment tfin.
By finished differences:
derived compared to E from displacement following y to the node OPPOSES, derived compared to displacement following y to the node OPPOSES,

By nonregression:
displacement following \(Z\) of the node OPPOSES
derived compared to E speed following y to the node OPPOSES, derived compared to acceleration according to \(X\) with the node OPPOSES,

\subsection*{2.3 Uncertainties}

The results depend on the temporal discretization and the diagram of integration. Here, we have chosen as reference solution that obtained by the implicit scheme of Newmark (with \(=0.25\) and \(=0.5\) ). We estimate uncertainty on the solution lower than \(0.5 \%\).

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\section*{3 Modeling}

With

\section*{3.1}

\section*{Characteristics of modeling}

Modeling in voluminal element.

\section*{BORD_GAU}
```

F

```

Temporal incrementing
\(T=105 \mathrm{~S}\)
Parameters Newmark diagram
\(=0.25\)
\(=0.5\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 330
Numbers and type of mesh: 200 HEXA8

\subsection*{3.3 Functionalities \\ tested}

The functionality tested is the derivation of the fields of displacement, speed and acceleration by report/ratio with the properties materials (Young modulus and Poisson's ratio).

Orders

MEMO_NOM_SENSI NOM_UN

\author{
NOM_ZERO \\ NAME \\ NOM_SD \\ PARA_SENSI \\ NOM_COMPOSE \\ DYNA_LINE_TRAN SENSITIVITY
}

\section*{EXCIT}

VECT_ASSE
TEST_RESU SENSITIVITY

\section*{Handbook of Validation}

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Author (S):
Key H. ANDRIAMBOLOLONA

\section*{3.4 \\ Sizes tested and results}

The values of reference are calculated at the moment tfin.

By finished differences:
Derived compared to E from displacement in y, node OPPOSES:

\section*{Reference}

Aster Difference
(\%)
-9.0051091831840 10-20 -9.0051103022894
10-20 1.24
10-5

Derived compared to displacement in \(y\), node OPPOSES:

\section*{Reference}

Aster Difference
(\%)
-9.2846474265708 10-15 -9.2625025608931
10-15 0.239

By nonregression:
Displacement in Z with the node OPPOSES:

\section*{Reference}

Aster Difference
(\%)
1.93 10-9 1.9343489167908

10-9 0.225
Derived compared to E speed in y, node OPPOSES:

\section*{Reference}

Aster Difference
(\%)
-4.37 10-15 -4.3730681471811
10-15 0.07
Derived compared to acceleration in \(X\), node OPPOSES:

\section*{Reference}

Aster Difference
(\%)
7.65 10-6 7.6556719982727

10-6 0.074
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\section*{4 \\ Summary of the results}

The precision of the results is coherent with the method of resolution used and the method of direct derivation "exact".

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SENSD06 - Sensitivity to materials in transitory dynamics isotropic 2D Dates:
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Organization (S): EDF-R \& D /AMA

\author{
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}

SENSD06 - Sensitivity to materials in dynamics
isotropic transient 2D

\section*{Summary:}

This case test takes again the geometry and the materials of test SENSD03 [V1.01.163] but calculation carried out here is transitory response of the system.

The functionality tested is the derivation of the fields of displacement, speed and acceleration compared to properties materials modulus Young and Poisson's ratio. A simultaneous calculation of the sensitivities by report/ratio with the loading is also tested.

One also tests the derivation of the stress fields using operator CALC_ELEM.
The reference is calculated by finished differences.

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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{OPPOSE}

\author{
3. \(m\) \\ BORD_GAU \\ CENTER
}

ORIGIN
\(y\)
10. \(m\)
\(1 m<X<11 m\)
\(1.5 m<Y<1.5 m\)
X

Z

\section*{1.2 \\ Properties of material}
\(E=36.000 \mathrm{MPa}\)
\(=0.2\)
\(=2400 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

Modeling \(2 D\) (x0y plan) in plane constraint.
Boundary conditions:
Surface "BORD_GAU" is blocked according to X.
The line "ORIGIN" is blocked according to \(X\) and \(Y\).
Loading (tdébut = O S T tfin = 104 S):
On the line "OPPOSES":
nodal force according to \(X: F\)
\(X=F T\)

1
(TT
end
beginning)
1
\(F=10 N R\)
nodal force according to \(y: F\)
\(Y=F T\)
2
(T T
end
beginning)
2
\(F=3 N R\)

\subsection*{1.4 Conditions}
initial

\section*{Initial displacement no one and null initial speed.}

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Key H. ANDRIAMBOLOLONA
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2
Reference solution

\section*{2.1 \\ Method of calculation}

The sensitivities of the fields displacement, speed and acceleration with the data materials are calculated by finished differences. These calculations by finished differences are carried out inside even case test by Code_Aster. The diagram offinished differences used is as follows (diagram of order 1 not centered):

F (,
\(X)\)
\(y\)
\(F(x+\), \()\)
\(y-F(\),
\(X)\)
\(y\)
\(X\)

The field of solution displacement of the standard problem (i.e nonderived) is subjected to a test of not regression. Tests of nonregression are also carried out on the sensitivities.

\section*{2.2 \\ Sizes and results of reference}

The values of reference are calculated at the moment tfin.
By finished differences:
there derived compared to \(E\) of displacements following group from nodes OPPOSES, derived compared to E from constraints \(x x\) with the group from nodes OPPOSES M14 mesh, derived compared to displacements following group from nodes there OPPOSES, derived compared to constraints \(x x\) with the group from nodes OPPOSES M14 mesh, derived compared to 1
There F of displacements following group of nodes OPPOSES, derived compared to 1
F of constraints \(x x\) with the group of nodes OPPOSES M14 mesh, derived compared to 2
There F of displacements following group of nodes OPPOSES, derived compared to 2
F of constraints \(x x\) with the group of nodes OPPOSES M14 mesh.
By nonregression:
displacements according to \(X\) of the group of nodes OPPOSES
there derived compared to E speeds following group from nodes OPPOSES,
derived compared to accelerations according to \(X\) from the group from nodes OPPOSES,
derived compared to 1
\(F\) of displacements according to \(X\) of the group of nodes OPPOSES, derived compared to 2
There F of displacements following group of nodes OPPOSES, derived compared to 1
F of constraints \(x x\) with the group of nodes OPPOSES M14 mesh, derived compared to 2
F of the constraints yy to the group of nodes OPPOSES M14 mesh.

\subsection*{2.3 Uncertainties}

The results depend on the temporal discretization and the diagram of integration. Here, we have chosen as reference solution that obtained by the implicit scheme of Newmark (with \(=0.25\) and \(=0.5\) ). We estimate uncertainty on the solution lower than 0.5\%.

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}

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Modeling in plane constraints on the grid following 2D:

Surface BORD_GAU becomes a line, the lines ORIGIN and OPPOSES become points.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 55
Numbers and types of meshs: 28 SEG2, 80 TRIA3
Parameters of calculation:
Temporal incrementing
\(T=10-5 S\)
Parameters Newmark diagram
\(=0.25\)
\[
=0.5
\]

\subsection*{3.3 Functionalities \\ tested}

The functionality tested is the derivation of the fields of displacement, speed and acceleration by report/ratio with the properties materials (Young modulus and Poisson's ratio). A simultaneous calculation of
sensitivities compared to the loading is also tested.
One also tests the derivation of the stress fields using operator CALC_ELEM.

\section*{Orders}

\section*{MEMO_NOM_SENSI NOM_UN}

\author{
NOM_ZERO \\ NAME \\ NOM_SD \\ PARA_SENSI \\ NOM_COMPOSE \\ DYNA_LINE_TRAN SENSITIVITY
}

EXCIT
CHARGE
CALC_ELEM SENSITIVITY

\author{
TEST_RESU SENSITIVITY
}

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\section*{3.4 \\ Sizes tested and results}

The values of reference are calculated at the moment tfin.
By finished differences:
Derived compared to E from displacement following y, group of nodes OPPOSES:
Reference
Aster Difference
(\%)
-1.8694452655942 10-21-1.8702782343648
10-21 0.045
Derived compared to E from constraints xx with the group from nodes OPPOSES M14 mesh:
Reference
Aster Difference
(\%)
5.3289199061525 10-10 5.3317808348049

10-10 0.054
Derived compared to displacement following y, group of nodes OPPOSES:
Reference
Aster Difference
(\%)
-5.3083783409877 10-11-5.3069502892459
10-11 0.027
Derived compared to constraints \(x x\) with the group from nodes OPPOSES M14 mesh:

\section*{Reference}

Aster Difference
(\%)

Derived compared to 1
F of displacement following y, group of nodes OPPOSES:
Reference
Aster Difference
(\%)
-4.9326303397235 10-12 -4.9326303396751
10-12 9.82
10-10
Derived compared to 1
F of constraints xx with the group of nodes OPPOSES M14 mesh:
Reference
Aster Difference
(\%)
3.29540612386553 .29540612387081 .60

10-10
Derived compared to 2
F of displacement following y, group of nodes OPPOSES:
Reference
Aster Difference
(\%)
3.4334537389989 10-11 3.4334537389978

10-11 3.26
10-11
Derived compared to 2
F of constraints \(x x\) with the group of nodes OPPOSES M14 mesh:
Reference
Aster Difference
(\%)
3.6294380946345 10-1 3.6294380947267

10-1 2.54
10-9
By nonregression:
Displacement following X, group of nodes OPPOSES:
Reference
Aster Difference
(\%)

\subsection*{3.70 10-10 3.7000067178680}

10-10 1.82
10-4
Derived compared to E speed following y, group of nodes OPPOSES:

\section*{Reference}

Aster Difference
(\%)
-6.11 10-17-6.1127945905412
10-17 0.046
Derived compared to acceleration according to \(X\), group of nodes OPPOSES:

\section*{Reference}

Aster Difference
(\%)
3.35 10-2 3.3506762414860

10-2 0.02
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Derived compared to 1
\(F\) of displacement following X, group of nodes OPPOSES:
Reference
Aster Difference
(\%)
3.85 10-11 3.8479856280582

10-11 0.052
Derived compared to 2

\title{
\(F\) of displacement following y, group of nodes OPPOSES:
}

\section*{Reference}

Aster Difference
(\%)
3.43 10-11 3.43345373899978

10-11 0.101
Derived compared to 1
F of constraints \(x x\) with the group of nodes OPPOSES M14 mesh:
Reference
Aster Difference
(\%)
3.293 .2954061238708
0.164

Derived compared to 2
F of the constraints yy to the group of nodes OPPOSES M14 mesh:

\section*{Reference}

Aster Difference
(\%)
3.463 .4618790581068
0.054

\section*{4 \\ Summary of the results}

The precision of the results is coherent with the method of resolution used and the method of direct derivation "exact".

\section*{Handbook of Validation}

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Titrate:
SENSM08 - Sensitivity in non-linear mechanics (2D plane deformations) Date:
27/11/03
Author (S):

\section*{P. of Key BONNIERES}

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Document: V1.01.178

SENSM08 - Sensitivity to the parameters material in elastoplasticity (linear isotropic work hardening)

\section*{Summary}

One tests the calculation of sensitivity to the parameters material, in the case of the elastoplasticity of Von-settings with
linear isotropic work hardening in 2D plane deformations. The sensitivity is calculated by direct differentiation
discrete equations. The reference comes from an analytical solution (simple compactness).

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\section*{1}

\section*{Problem of reference}

\subsection*{1.1 Geometry}
\[
D Y=-0.2 T
\]

\section*{1.2}

\section*{Properties of material}
\(E=100.000 \mathrm{MPa}\)
\(=0.3\)
\(A N D=2000 \mathrm{MPa}\)
\(y=200 \mathrm{MPa}\)

\section*{1.3 \\ Boundary conditions and loadings}

Sides AC and data base are blocked according to \(X\).
Side \(A B\) is blocked according to \(Y\).
A displacement towards \(A B\) of standard \(0.2 * T\) is imposed on the side \(C D\) (the moment \(T\) belongs to [0,1])
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The reference solution comes from an analytical solution:
If \(y y<K(l+) / E\),
- 2

E
- \(2 k 1\)
( + )
\(p=\)
\(Y Y\)
\(3 r d+2 R 1\)
( + )
1
\(X X=-\)
\([(R P+K)+E p / 2]\)
1-2
\(=\)
\(-(R P+K)\)
YY
\(X X\)

\section*{With:}
p: cumulated plastic deformation
\(R=E A N D /(E-A N D)\)
\(K=y\)
That is to say:
2
2
2
p
1
(
\(2+3\)
) ( \(k E-6 k E E+K 1\)
(-2) \(E-2 n d E)\)
\(T\)
\(T\)
\(T Y Y\)
```

2
2
E
E 3
(E-1
(-2) E)
T
p
4
(+)(E
+K l
(+ ))
YY
=
2
E
3
(E-1
(-2) E)
T
T
p
-1
2+)
=
K
3rd +2R1
(+)
XX =
E
K
3rd+2R l
(+)

```
```

-2
YY=
E
K
3rd + 2R1
(+)

```

\section*{2.2 \\ Sizes and results of reference}

All the sizes are uniform on all the field. The point of calculation will thus not be clarified.
Plastic deformation cumulated at the moment \(T=1.0\) :
\(p(1.0)=0.129313\)
Sensitivity of the plastic deformation cumulated to the Young modulus \(E\) to the moment \(t=0.5\) :
\(d p / d E(0.5)=2.83475 E-8\)
Sensitivity of the plastic deformation cumulated to the Young modulus \(E\) to the moment \(t=1.0\) : \(d p / d E(1.0)=3.99649 E-8\)

Sensitivity of the plastic deformation cumulated to the module AND the moment \(t=0.5\) : \(d p / d E T(0.5)=5.65769 E-7\)
Sensitivity of the plastic deformation cumulated to the module AND the moment \(t=1.0\) : \(d p / d E T(1.0)=1.14664 E-6\)
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Sensitivity of the constraint according to \(x x\) to the \(t=1.0\) and elastic limit \(K\) to the moments \(t=0.5\) :
\(D x x / d k(0.5)=D x x / d k(1.0)=0.32754\)
Sensitivity of the constraint according to yy to the \(t=1.0\) and elastic limit \(K\) to the moments \(t=0.5\) :
\(D y y / d k(0.5)=D y y / d k(1.0)=0.65508\)
Sensitivity of the plastic deformation cumulated to the \(t=1.0\) and elastic limit \(K\) to the moments \(t=0.5\) : \(d p / d k(0.5)=d p / d k(1.0)=8.51604 E-6\)

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Modeling in plane deformations on a grid \(2 D\) consisted an element QUAD4

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 4
Numbers and types of meshs: 1 QUAD4, 2 SEG2, 1 POII

\subsection*{3.3 Functionalities tested \\ Orders}

DEFI_PARA_SENSI

\author{
STAT_NON_LINE COMP_INCR RELATION \\ VMIS_ISOT_LINE \\ SENSITIVITY \\ TEST_RESU SENSITIVITY
}

\section*{3.4 \\ Sizes tested and results}

\section*{Size tested}
Theory

Code_Aster
Difference (\%)
\(V 1\) with \(T=1.0\)
0.129313
0.1293128
1.28 E-4

Sensitivity of V1 to \(E\) to \(T=0.5\)
2.83475 E-8
2.834746 E-8
1.24 E-4

Sensitivity of V1 to \(E\) to \(T=1.0\)
3.99649 E-8
3.9964897 E-8
6.3 E-6

Sensitivity of V1 to AND to \(T=0.5\)
5.65769 E-7
5.6576897 E-7
5.87 E-6

Sensitivity of V1 to AND to \(T=1.0\)
1.14664 E-6
1.1466406 E-6
5.2 E-5

Sensitivity of V1 to \(K\) to \(T=0.5\)
8.51604 E-6
8.516043 E-6
3.27 E-5

Sensitivity of V1 to \(K\) to \(T=1.0\)
8.51604 E-6
8.516043 E-6
3.27 E-5

\title{
Sensitivity of SIXX to \(K\) to \(T=0.5\)
}
0.32754
0.3275401
3.27 E-5

Sensitivity of SIXX to \(K\) to \(T=1.0\)
\[
0.32754
\]
0.3275401
3.27 E-5

Sensitivity of SIYY to \(K\) to \(T=0.5\)
-0.65508
\(-0.6550802\)
3.27 E-5

Sensitivity of SIYY to \(K\) to \(T=1.0\)
-0.65508
-0.6550802
3.27 E-5

\section*{4 \\ Summary of the results}

\title{
The numerical results of sensitivity are in very good agreement with those of the theory. \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/03/008/A
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}

\section*{SENSM09 - Infinite elastoplastic cylinder under} pressure: sensitivity to the parameters material

\section*{Summary:}

One tests the calculation of sensitivity to the parameters material, in the case of the elastoplasticity of Von Mises with linear isotropic work hardening in axisymmetric 2D.

The sensitivity is calculated by direct differentiation of the discrete equations.
The reference comes from a calculation from derived by finished differences.
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Z}

3
9-27
With
D
p
I J
\(\cdots K\)
1
L
H
E
R
20
20

\section*{1.2}

Material properties
\(E=210.000 \mathrm{MPa}\)
\(=0.3\)
\(T\)
\(E=2000 \mathrm{MPa}\)

\section*{1.3}

Boundary conditions and loadings
On HE: uz = 0
On AD: uz uniform
100
Pressure \(p(T)\) distributed uniformly on AH: \(p(T)=\)
\(T\)
60
( \(p\) in MPa, \(T\) in seconds)
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation}

The solution comes from finished differences (approximation of derived with order 1) carried out with Aster.

\section*{2.2 \\ Sizes and results of reference}

2
23
The mesh 75, noted M75, corresponds to rectangle IJKL (side IJ has as a length
\(+\)
). All
5
135
the sizes are taken at the moment \(T=60\). The moment of calculation will thus not be specified.

Sensitivity of ur displacement to the point \(K\) compared to the Young modulus E:
\(u r=-, 228851 E-8\)
E
Sensitivity of uz displacement to the point K compared to the Young modulus E:
\(u z=, 4746 E-12\)
\(E\)

Sensitivity of the constraint rr to item 1 of M75 compared to the Young modulus E:
\(r r=7,4146 E-7\)
\(E\)

Sensitivity of the constraint to item 1 of M75 compared to the Young modulus E:
\(=08096\)

1
E-6
E

Sensitivity of the constraint zz to item 5 of M75 compared to the module \(T\)
E:

E-4
AND
Sensitivity of the cumulated plastic deformation p to item 5 of M75 compared to the module \(T\)

\section*{E:}
\(p=-78363\)

\section*{AND}

Sensitivity of the cumulated plastic deformation p to item 1 of M75 compared to the elastic limit
\(y\) :
\(p=-54865\)

Sensitivity of the cumulated plastic deformation p to item 5 of M75 compared to the elastic limit
\(y\) :
\(p=-50133\)
,
4
E-4
y
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}
```

Z
With
I J
B
CD
L
K
l
HG
FE
20 meshs
R
45 meshs
2020

```

Loading: pressure \(p(T)\) distributed on \(A H(p)\) defined in [\$1.3]).

Boundary conditions:
\(u z=0\) for the group of nodes NOEHGFE (side HE)
\(U\)
\(U\)
\(Z=Z(D)\) for each node located on side \(A D\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 381
A number of meshs and types: 94 elements QUAD8

\subsection*{3.3 Functionalities \\ tested}

Orders

\section*{3.4 \\ Sizes tested and results}

\section*{Size tested}

\section*{Reference}

\section*{Code_Aster}

Difference (\%)
Sensitivity of Ux to E to the point \(K\)
2,28851 E-8
2,28845 E-8
-0,002
Sensitivity of Uy to E to the point K
4,746 E-12
4,7425 E-12
-0,074
Sensitivity of SIXX to E to item 1 of M75
7,4146 E-7
7,41465 E-7
6,72 E-4
Sensitivity of SIZZ to E to item 1 of M75
1,08096 E-6
1,0809601 E-6
9,27 E-6
Sensitivity of SIYY to AND item 5 of M75
1,3384 E-4
1,33844 E-4
0,003
Sensitivity of V1 to AND item 5 of M75
1,78363 E-5
1,7836315 E-5
8,51 E-5
Sensitivity of V1 to y to item 1 of M75
4,54865 E-4

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\section*{4 \\ Summary of the results}

The numerical results of sensitivity are in concord with the reference consisted calculations by finished differences.
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Titrate:
SENSM10 - Elastoplastic elbow under pressure

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SENSM10-Elastoplastic elbow under pressure:
sensitivity compared to the loading and to material

\section*{Summary:}

One tests the calculation of sensitivity to the parameters loading and material, in the case of the elastoplasticity of Von
Settings with linear isotropic work hardening in 3D.

The sensitivity is calculated by direct differentiation of the discrete equations.
The reference comes from a calculation from derived by finished differences.
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SENSM10 - Elastoplastic elbow under pressure
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1
Problem of reference

\subsection*{1.1 Geometry}

The study relates to a piping including/understanding two right pipes and an elbow [Figure 1.1-a].
The geometrical data of the problem are as follows:
length \(L G\) of the two right pipes is \(\mathbf{3} \mathbf{m}\),
the Rc ray of the elbow is 0.6 m ,
the angle of the elbow is 90 degrees,
the thickness of the right pipes and the elbow is 0.02 m ,
and the ray external Re of the right pipes and the elbow is of 0.2 Mr .

\section*{\(L G\)}

D
B
section D
section \(B\)
RC
C
O
section C
Z
Y
E
L
Z
G
X
Re
X
With
section A

\section*{Appear 1.1-a \\ Note:}

The geometry of the problem has a symmetry compared to the plan \((A, X, Y)\).

\section*{1.2 \\ Material properties}
```

$E=210.000 \mathrm{MPa}$
$=0.3$
$T$
$E=2000 \mathrm{MPa}$
$y=200 \mathrm{MPa}$
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```

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\section*{1.3 \\ Boundary conditions and loadings}

On the level of section a: embedding.
On the level of section b: displacement imposed \(D(T)\) directed according to the axis \(Y\)
\(D(T)\)
0,1
0
\(0,40,6\)
1
\(T(S)\)

Internal pressure: \(p(T)=T\) with \(=5\)
2 MPa sl

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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The solution comes from finished differences (approximation of derived with order 1) carried out with Aster.

\section*{2.2 \\ Sizes and results of reference}

The position of the mesh 45, noted M45, is as follows (see [§3.1]):
- 5 th mesh starting from the segment \(d 2\), while turning towards [ \(p 1\),

I
p 1
\(E]\),
- 3rd mesh starting from section A towards Y positive.

The number of the quoted points is the number of the point of corresponding Gauss of M45.

\title{
, belongs to M45.
}

10510
Sensitivity of \(u x\) displacement to the N 3235 node by report/ratio (defined in [§1.3]) to \(T=0,6 \mathrm{~S}\) :
\(u x=-56007\)
1
E-3

Sensitivity of the constraint \(z z\) to item 27 of M45 compared to to \(T=0,6 \mathrm{~S}\) :
\(z z=36732\)
,

Sensitivity of the constraint \(y y\) to item 3 of M45 compared to to \(T=0,6 \mathrm{~S}\) :
\(y y=-0447\)
,
\(E+1\)

Sensitivity of the constraint \(x z\) to item 19 of M45 compared to to \(T=0,6 \mathrm{~S}\) :
\(x z=37787\)
3

Sensitivity of displacement \(U y\) to the N 3235 node compared to to \(T=1 \mathrm{~S}\) :
```

$U y=-71676$

```

2
E-3

Sensitivity of constraint \(x x\) to item 1 of M45 compared to to \(T=1 \mathrm{~S}\) :

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Sensitivity of the constraint yy to item 3 of M45 compared to to \(T=1 \mathrm{~S}\) :
\(y y=, 12685\)

Sensitivity of the constraint \(x y\) to item 27 of M 45 compared to to \(T=1 \mathrm{~S}\) :
\(x y=51774\)

Sensitivity of the cumulated plastic deformation \(p\) to item 27 of M45 compared to to \(T=1 \mathrm{~S}\) :
\(p=36402\)

Sensitivity of the constraint \(z z\) to item 7 of M45 compared to \(E\) to \(T=0,6 \mathrm{~S}\) :
```

zz=56151

```
6
E-5
E

Sensitivity of the cumulated plastic deformation \(p\) to item 1 of M45 compared to \(E\) to \(T=0,6 \mathrm{~S}\) :
\(p=07624\)

E-8
E
Sensitivity of constraint \(x x\) to item 25 of M45 compared to \(E\) to \(T=1 \mathrm{~S}\) :
\(x x=54847\)
,
2
E-5
E
Sensitivity of the constraint \(x z\) to item 7 of M45 compared to \(E\) to \(T=1 \mathrm{~S}\) :
\(x z=5494\)
3
E-6
E
Sensitivity of the constraint \(y z\) to item 19 of M45 compared to \(E\) to \(T=1 \mathrm{~S}\) :
\(y z=-00755\)
2
E-5
E

Sensitivity of \(u z\) displacement to the N3235 node compared to \(T\) \(E\) with \(T=1 \mathrm{~S}\) :
```

uz=53184
4
E-8

```
AND

Sensitivity of the constraint \(z z\) to item 27 of M45 compared to \(T\) \(E\) with \(T=1 \mathrm{~S}\) :
\(z z=-, 128275 E-2\)
AND
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Sensitivity of the constraint \(y y\) to item 25 of M45 compared to there to \(T=1 \mathrm{~S}\) :
\(y y=-52017\)
8
E-1
y

Sensitivity of the cumulated plastic deformation \(p\) to item 9 of M45 compared to there to \(T=1 \mathrm{~S}\) :

\author{
\(p=-, 141329 E-5\) \\ \(y\) \\ Handbook of Validation
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The right pipes and the elbow are modelled by quadratic isoparametric solid elements.
Piping presents a symmetry plane \(\mathrm{Z}=0\). Only one half volume is netted.
Y
X
With
d2 A
With D A
e2
i2
i1
1

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\section*{3.2}

Characteristics of the grid
4240 meshs HEXA20, 5913 nodes

\author{
3.3 Functionalities \\ tested \\ Orders \\ DEFI_PARA_SENSI \\ STAT_NON_LINE COMP_INCR \\ RELATION \\ VMIS_ISOT_LINE \\ SENSITIVITY
}

\section*{TEST_RESU SENSITIVITY}

\section*{3.4 \\ Sizes tested and results}

\section*{Size tested}

Localization \(T\) ( \(S\) )
Reference
Code_Aster
Difference (\%)
in M45
Sensitivity of Ux to
N3235

1,560071 E-3
3,93 E-5
Sensitivity of SIXX to point 27
0,6
2,36732
2,36731
3,37 E-4
Sensitivity of SIYY to
point 3
0,6 1,0447 E+1
\(1,04468 \mathrm{E}+1\)
-0,002
Sensitivity of SIXZ to
point 19
0,6
3,37787
3,377868
6,18 E-5
Sensitivity of Uy to
N3235
1
2,71676 E-3
2,71675 E-3
3,36 E-4
Sensitivity of SIXX to
point 1
1
9,9896
9,98963
3,52 E-4
Sensitivity of SIYY to point 3
1
1,2685
1,2689
0,033
Sensitivity of SIXY to
point 27
1
-4,51774
\(-4,51758\)
-0,004
Sensitivity of V1 to
point 27
1
5,36402 E-4
5,36399 E-4
5,71 E-4
Sensitivity of SIZZ to E
point 7
0,6
6,56151 E-5
6,55853 E-5
-0,045
Sensitivity of V1 to E
point 1
0,6
1,07624 E-8
1,076702 E-8
0,043
Sensitivity of SIXX to E
point 25
1
2,54847 E-5
2,55017 E-5
0,067
Sensitivity of SIXZ to E
point 7
1
3,5494 E-6
3,5606 E-6
0,315
Sensitivity of SIYZ to E
point 19
1
2,00755 E-5
2,00823 E-5
0,034
Sensitivity of Uz to AND
N3235
1
4,53184 E-8
4,53152 E-8
-0,007

\title{
Sensitivity of SIZZ to AND
}

\section*{point 27}

1
\(1,28275 \mathrm{E}-2\)
1,282695 E-2
-0,004
Sensitivity of SIYY to y
point 25
1
8,52017 E-1
8,52442 E-1
0,050
Sensitivity of V1 to y
point 9
1
1,41329 E-5
1,41175 E-5
-0,109

\section*{4 \\ Summary of the results}

The numerical results of sensitivity are in concord with the reference consisted calculations by finished differences.

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Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SENSM11 - Elastoplastic axisymmetric structure in inflection
Date:
26/04/04
Author (S):
P. of BONNIERES

Key: V1.01.182-A
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\author{
Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ V1.01.182 document
}

SENSM11-Elastoplastic axisymmetric structure in inflection: sensitivity compared to the loading and with material

\section*{Summary:}

One tests the calculation of sensitivity to the parameters loading and material, in the case of the elastoplasticity of
Von Mises with linear isotropic work hardening in axisymmetric 2D.
The sensitivity is calculated by direct differentiation of the discrete equations.
The reference comes from a calculation from derived by finished differences.
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Z}

1
With
G
B
1
H
C
O
\(\boldsymbol{R}\)
3

\section*{1.2}

Material properties
\[
E=210.000 \mathrm{MPa}
\]
\[
=0,3
\]
\(T\)
\(E=2000 \mathrm{MPa}\)
\(y=200 \mathrm{MPa}\)

\section*{1.3}

Boundary conditions and loadings
Side \(O A\) is blocked according to \(R\).
The point \(O\) is blocked according to \(Z\).
A specific force directed according to \(O Z\) towards \(Z<0\) is applied to the point \(B\). Its intensity depends time \(T\) and is worth:
\(F(T)=T\) with \(T\) in seconds and =1,6667
1
NS

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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation}

The solution comes from finished differences (approximation of derived with order 1) carried out with Aster.

\section*{2.2}

Sizes and results of reference

The M1 mesh corresponds to square AGHO on side 1 Misters.
All the sizes are taken at the moment \(T=60 \mathrm{~s}\). The moment of calculation will thus not be specified.
Sensitivity of ur displacement to the point \(G\) compared to the force \(F\) :
\(u r=, 422826 E-5\)
F
Sensitivity of the constraint rr to item 4 of M1 compared to the force F:
\(r r=1\)
- 28653

F
Sensitivity of the cumulated plastic deformation p to item 3 of M1 compared to the force F:
\(p=69781\)
,
1
E-3
F

Sensitivity of uz displacement to point A compared to the Young modulus E:
\[
u z=17304
\]

3
E-8
E
Sensitivity of the constraint \(r z\) to item 3 of M1 compared to the module \(T\)
E:
\(r z=, 80665 E-4\)
AND
Sensitivity of the cumulated plastic deformation p to item 1 of M1 compared to the elastic limit \(y\) :
\(p=-9766\)

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Z
With
G
B
1
O
H
C
R
3

Loading: specific force \(\boldsymbol{F}(\boldsymbol{T})\) to the bottom with the point \(\boldsymbol{B}\) (defined in [§1.3]) Boundary conditions:
\(u r=0\) for the node \(N 4\) (not A)
\(u r=u z=0\) for the node N1 (not O)

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes: 8
A number of meshs and types: 3 elements QUAD4

\subsection*{3.3 Functionalities}
tested
Orders

DEFI_PARA_SENSI

\author{
STAT_NON_LINE COMP_INCR \\ RELATION \\ VMIS_ISOT_LINE \\ SENSITIVITY
}

\section*{TEST_RESU SENSITIVITY}

\author{
3.4 \\ Sizes tested and results \\ Size tested \\ Reference \\ Code_Aster \\ Difference (\%) \\ Sensitivity of Ux to F to the point \(G\) \\ 4,22826 E-5 \\ 4,2282602 E-5 \\ 4,89 E-6 \\ Sensitivity of SIXX to F to item 4 of M1-1,28653 \\ -1,2865305 \\ 4,09 E-5 \\ Sensitivity of V1 to F to item 3 of M1 \\ 1,69781 E-3 \\ 1,6978105 E-3 \\ 2,94 E-5 \\ Sensitivity of Uy to E to point A \\ 3,17304 E-8
}

\section*{3,173036 E-8}
-1,16 E-4
Sensitivity of SIXY to AND item 3 of M1
8,0665 E-4
8,066447 E-4
-6,6 E-4
Sensitivity of V1 to y to item 1 of M1
-4,9766 E-4
-4,9765995 E-4
-1,06 E-5

\section*{4 \\ Summary of the results}

The numerical results of sensitivity are in concord with the reference consisted calculations by finished differences.
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Titrate:
SENSD07-Sensitivity in nonlinear dynamics 3D

\section*{Date:}

25/11/05
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Key S. CAMBIER
:
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Organization (S): EDF-R \& D /AMA

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}

SENSD07-Sensitivity in nonlinear dynamics
3D (VON-MISES ISOT)

\section*{Summary}

This case test takes again the geometry of test SENSD02 but calculation carried out here is the transitory answer not
linear of the system.
The functionality tested is the derivation of the fields of displacement and speed compared to the properties
materials (Young modulus, Poisson's ratio, slope of the traction diagram, elastic limit), and with loading (nodal force). Two DYNA_NON_LINE are connected to also test the functionality of recovery with sensitivity.

The reference solution of the sensitivities is calculated by finished differences, then one tests with a loading
such as the mode is linear, and one compares with the results of DYNA_LINE_TRAN.
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Key S. CAMBIER

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1
Problem of reference
1.1 Geometry

OPPOSE

\section*{CENTER}

BORD_GAU
3. \(m\)
```

y
10. m
$1 m<X<11 m$
$1.5 \mathrm{~m}<Y<1.5 \mathrm{~m}$
X

```

\section*{1.2}

\section*{Properties of material}
\(E=105 \mathrm{~Pa}\)
\(=0.3\)
\(A N D=2000 \mathrm{MPa}\)
\(y=200 M P a\)
\(=2400 \mathrm{~kg} / \mathrm{m} 3\)
The model integrates modal damping (amor_réduit \(=0.07\) ) and damping proportional \((K+M,=0 . S\) and \(=0.3 \mathrm{sl})\).

\section*{1.3}

\section*{Boundary conditions and loadings}

Boundary conditions:
Line "BORD_GAU" is blocked according to \(X, y\) and \(Z\).
The node " \(I\) " is blocked according to \(X, y\) and \(Z\).

Loading (tdébut = 0 S T tfin = 104 S):
On the node "OPPOSES":
nodal force according to \(X F\)
\(X=F T\)

1
(T T
end
beginning)
1
\(F=100 *\) coef_char,
nodal force according to \(y F\)
\(Y=F T\)

2
(T T
end
beginning)
2
\(F=30 *\) coef_char,
nodal force according to \(Z\) F
\(Z=F T\)
\(F=10 *\) coef_char.
3
( \(T\) T
end
beginning)
3
:
With coef_char \(=107\) for the first calculation (into nonlinear) and coef_char=1 for the second calculation (in linear).

\subsection*{1.4 Conditions}
initial
Initial displacement no one and null initial speed.
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25/11/05
Author (S):
Key S. CAMBIER

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The sensitivities of the fields displacement, speed and acceleration with the data materials are calculated by finished differences. These calculations by finished differences are carried out inside even case test by Code_Aster. The diagram of finished differences used is as follows (diagram of order 1 not centered):
\(F\) (,
\(X\) )
\(y\)
\(F(x+\),
\(y-F\) (,
\(X\) )
\(y\)
\(X\)

The field of solution displacement of the standard problem (i.e nonderived) is subjected to a test of not regression. Tests of nonregression are also carried out on the sensitivities.

\section*{2.2 \\ Sizes and results of reference}

The values of reference are calculated at the moment tfin.
By finished differences:
derived compared to \(E\) speed according to \(X, y\) and \(Z\) with the node OPPOSES, derived compared to speed according to \(X, y\) and \(Z\) with the node OPPOSES, derived compared to AND speed according to \(X, y\) and \(Z\) with the node OPPOSES, derived compared to there speed according to \(X, y\) and \(Z\) with the node OPPOSES, derived compared to 1
\(F\) speed according to \(X, y\) and \(Z\) with the node OPPOSES,

\section*{Note:}

In the command file, it is enough to replace nom_cham=' VITE' by nom_cham=' DEPL' or nom_cham='ACCE' to test (values of reference calculated automatically by finished differences) the derivative compared to these fields.

By nonregression:
displacement following \(X\) of the node OPPOSES
derived compared to \(E\) from displacement according to \(X\) to the node OPPOSES, derived compared to displacement following \(y\) to the node OPPOSES, derived compared to AND speed according to \(Z\) to the node OPPOSES, derived compared to speed according to \(X\) to the node OPPOSES there, derived compared to AND from acceleration following y to the node OPPOSES, derived compared to from acceleration according to \(Z\) to the node OPPOSES there,

By comparison with DYNA_LINE_TRAN:
displacement following \(X\) of the node OPPOSES
derived compared to \(E\) speed according to \(X\) with the node OPPOSES,
derived compared to speed following y to the node OPPOSES,
derived compared to 1
\(F\) speed following y to the node OPPOSES,
derived compared to 1
\(F\) speed according to \(Z\) with the node OPPOSES,

\subsection*{2.3 Uncertainties}

Results depending amongst other things on the convergence of calculations (what can disturb the values of reference calculated by difference-finished); uncertainty on the solution can be estimated order from \(0.5 \%\) for the values tested significant (not too small).
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Modeling in voluminal element.

\section*{BORD_GAU}

\section*{Characteristics of the grid}

A number of nodes: 330
Numbers and type of mesh: 200 HEXA8

\subsection*{3.3 Functionalities}

\section*{tested}

The functionality tested is the derivation of the fields of displacement and speed compared to properties materials (Young modulus, Poisson's ratio, slope of the traction diagram, limiting of elasticity), and with the loading (nodal force). Two DYNA_NON_LINE are connected to test also functionality of recovery with sensitivity.

\section*{Orders}

MEMO_NOM_SENSI NOM_UN

NOM_ZERO

NAME
NOM_SD

\author{
PARA_SENSI
}

\author{
DYNA_NON_LINE \\ COMP_INCR RELATION VMIS_ISOT_LINE \\ SENSITIVITY
}

\section*{EXCIT}

CHARGE

TEST_RESU SENSITIVITY

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\section*{3.4 \\ Sizes tested and results}

The values of reference are calculated at the moment tfin.
The results are obtained on the machine Compaq clayastr.
References obtained by finished differences:
derived compared to E speed to the node OPPOSES,
Reference
Aster v 8.01.16 Difference
Dx
-1.1166186341777D-08 1.1163137969561D-08
0.027
\%
Dy
1.3767938854149D-08-1.3768385542789D-08
-0.003
\(d z\)
1.4266008217134D-08 1.4266220205172D-08
-0.001
derived compared to speed with the node OPPOSES,

\section*{Reference}

Aster v 8.01.16 Difference
\(d x\)
-5.3322155999948D-03 5.3308516119917D-03
0.026
\%
Dy
-6.8047395954361D-03 -6.8049909411153D-03
-0.004
\%
\(d z\)
7.1532137250567D-03 7.1529269784399D-03
-0.004
\%
derived compared to AND speed to the node OPPOSES,
Reference
Aster v 8.01.16 Difference
\(d x\)
-7.9808160080574D-08 -7.9791920495776D-08
-0.020
\%
Dy
\(-2.5306690076832 D-08-2.5282262727618 D-08\)
-0.097
\%
\(d z\)
-5.9742433222709D-09 -5.9807380106367D-09
0.109
\%
derived compared to speed to the node OPPOSES there,

\section*{Reference}

Aster v 8.01.16 Difference
\(d x\)
\(-1.5106706996448 D-06-1.5118362142106 D-06\)
0.077
\%
Dy
-4.7919002099661D-07-4.7932548945879D-07
0.028
\(d z\)
-1.1976908353972D-07-1.1987342764877D-07
0.087
\%
derived compared to 1
\(F\) speed to the node OPPOSES,
Reference
Aster v 8.01.16 Difference
\(d x\)
5.7737287084819D-06 5.7737284500162D-06
-4.48E-06
\%
Dy
-7.5037860369775D-13
-2.7661560722007D-14
7.23E-13
\(d z\)
2.0658944777097D-12 \(2.3753534498295 D-12\)
14.979
\%

References obtained by nonregression:
displacement following \(X\) of the node OPPOSES
Reference
Aster Difference
1.9342000000000D-01 1.9342038655185D-01
2.00E-04
\%
derived compared to E from displacement according to \(X\) to the node OPPOSES,

\section*{Reference}

Aster Difference
-2.4739000000000D-13 -2.4738842664910D-13
-6.36E-04
\%
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\section*{Key S. CAMBIER}

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derived compared to displacement following \(y\) to the node OPPOSES, Reference
Aster Difference
-1.3969000000000D-07-1.3969162792960D-07
0.001
\%
derived compared to AND speed according to \(Z\) to the node OPPOSES,

\section*{Reference}

Aster Difference
-5.9807000000000D-09 -5.9807380106367D-09
6.36E-04
\%
derived compared to speed according to \(X\) to the node OPPOSES there,

\section*{Reference}
Aster Difference
-1.5118000000000D-06-1.5118362142106D-06
0.002
\%
derived compared to AND from acceleration following \(y\) to the node OPPOSES,
Reference
Aster Difference
-1.0214000000000D-03 -1.0214309235913D-03
0.003
\%
derived compared to from acceleration according to \(Z\) to the node OPPOSES there,

\section*{Reference}

Aster Difference
-1.7587000000000D-03 -1.7586721905067D-03
-0.002
\%

References by DYNA_LINE_TRAN:
Derived compared to E speed in y, node OPPOSES:

\section*{Reference}

Aster Difference
1.9342000000000D-08 1.9342020834004D-08
1.08E-04
\%

Derived compared to \(E\) speed in \(y\), node OPPOSES:

\section*{Reference}

Aster Difference
-1.0577000000000D-14-1.0577116503182D-14
0.001
\%

Derived compared to E speed in y, node OPPOSES:

\section*{Reference}

Aster Difference
-4.4996000000000D-10-4.4996345033970D-10
7.67E-04
\%
Derived compared to acceleration in \(X\), node OPPOSES:

\section*{Reference}

Aster Difference
-9.9810000000000D-13 -9.8309867263082D-13
-1.503
\%
Derived compared to acceleration in \(X\), node OPPOSES:

\section*{Reference}
```

Aster Difference
2.3704000000000D-12 2.3593190530811D-12
-0.467
%

```
```

4
Summary of the results

```

Precision evaluated by comparison of the results of the method of resolution by direct derivation "exact" with those obtained by finished differences and those obtained by another operator Code_Aster is completely satisfactory.
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ZZZZ185 - Validation MODI_MAILLAGE/PROJ_CHAMP
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J.L. Key FLÉJOU
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Organization (S): EDF-R \& D /AMA

\section*{Summary:}

The objective of this case test is to validate order MODI_MAILLAGE with the key word SYMMETRY and
order PROJ_CHAMP with key word DISTANCE_MAX.
order MODI_MAILLAGE, with the key word SYMMETRY makes it possible to take the symmetrical one of one
grid 2D or 3D,
order PROJ_CHAMP with key word DISTANCE_MAX makes it possible not to project the field on the nodes which do not answer the 2 following criteria:
- the nodes are not in an element of the 1st grid,
- the nodes are beyond DISTANCE_MAX.

The case test consists in realizing:
a thermal study on 1/8ème of structure,
the construction of the thermal field on 1/4 of structure by projection of the results obtained on 1/8ème of structure, with the taking into account of a symmetry plane,
a thermal study on 1/4 of structure,
the comparison of the field of temperature obtained by projection and a study on \(1 / 4\) of structure.

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

The structure is obtained by rotation around axis Z, of the section represented with [Figure 1.1-a].
\[
\begin{aligned}
& Z \\
& m \\
& 0,20 \\
& X \\
& 0,05 m \\
& R 1=5,00 \mathrm{~m} \\
& R 2=5,05 \mathrm{~m} \\
& R 3=5,30 \mathrm{~m} \\
& R 4=5,40 \mathrm{~m}
\end{aligned}
\]

Appear 1.1-a: Cut structure

\section*{1.2 \\ Properties of material}

The study is carried out in linear thermics, only the thermal characteristics are necessary to the definition of materials.

\title{
Isotropic thermal conductivity: LAMBDA \(=15.0 \mathrm{~W} / \mathrm{m} . \mathrm{k}\)
}

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\section*{1.3}

Boundary conditions and loadings
The study is carried out in linear thermics, the conditions on the imposed temperatures are represented with [Figure 1.3-a], the conditions of flow and heat exchange are indicated to [Figure 1.3-b].

\section*{Y}

Temp 1
Temp 2
\(X\)
Temp 1
Temp 2
Appear 1.3-a: Sight of top, with the conditions in imposed temperature

1
3
H2, Tf 2
h1, TF1
h3, Tf3
2
Appear 1.3-b: Cross-section, with the conditions of flow and heat exchange Handbook of Validation
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Conditions of flow:
\(1=10.0 \mathrm{~W} / \mathrm{m}^{2}\)
\(2=\)
\(0.0 \mathrm{~W} / \mathrm{m}^{2}\)
\(3=30.0 \mathrm{~W} / \mathrm{m}^{2}\)
Conditions of exchange by convection:
\(h 1=350.0 \mathrm{~W} / \mathrm{m}^{2} . k\)
\(T F 1=300.0^{\circ} \mathrm{C}\)
\(H 2=400.0 \mathrm{~W} / \mathrm{m}^{2} . k\)
\(t f 2=275.0^{\circ} \mathrm{C}\)
\(h 3=600.0 \mathrm{~W} / \mathrm{m}^{2} . k\)
\(t f 3=310.0^{\circ} \mathrm{C}\)
Conditions of imposed temperature:
Temp1 \(=250.0^{\circ} \mathrm{C}\)
Temp2 \(=160.0^{\circ} \mathrm{C}\)

\subsection*{1.4 Conditions}
initial
Without object.

\title{
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2
Reference solution

\section*{2.1 \\ Method of calculation}

For this case test, 2 studies are carried out.

The first study is carried out on 1/8ème structure. Calculation is a thermal analysis linear, with the boundary conditions described with [\$1.2].

The second study is carried out on \(1 / 4\) of structure. Calculation is a thermal analysis linear, with the boundary conditions described with [§1.2].

\section*{Z \\ Y \\ \(X\)}

Appear 2.1-a: Grid of 1/8ème of structure

\section*{2.2}

Sizes and results of reference

The result of reference is the field of temperature.
The result of the study on 1/8ème of structure and its grid are safeguarded in a file with format "MED". The projected fields will be then compared with those obtained by the study carried out on
1/4 of structure.
[Figure 2.2-a] the field of temperature gives obtained on 1/8ème of structure. Handbook of Validation
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Appear 2.2-a: Field of temperature calculated on 1/8ème of the structure

\section*{2.3 \\ Uncertainties on the solution}

No significance in this case.
The goal of the case test is to check that the symmetry of the grid and that projections of the field of temperature are correctly made.

\subsection*{2.4 References \\ bibliographical}

Without use.
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Date:
01/01/04

\section*{Author (S):}

\section*{J.L. Key FLÉJOU}

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\section*{3 Modeling}

\section*{With}

\section*{3.1 \\ Characteristics of modeling}

The goal of this modeling is to obtain the solution with the thermal problem starting from calculation carried out
on \(1 / 8\) ème of the structure.
The case test proceeds the following way:
reading of the grid, \(1 / 8\) ème of structure, starting from the file with the format "MED", order LIRE_MAILLAGE.
reading of the field of temperature starting from the file with format "MED", order LIRE_CHAMP.
creation of a result starting from the field previously read, order CREA_RESU.
creation of a model starting from the grid previously read, order AFFE_MODELE.
creation of a model for the groups of meshs which belong to the symmetry plane, order AFFE_MODELE (cf notices \(n^{\circ} 1\) ).
reading of the grid, \(1 / 8\) ème of structure, starting from the file with the format "MED" and modification of
grid by symmetry compared to the plan (sym) defined by:
AXE_1 \(=(1.0,1.0,0.0)\), AXE_2 \(=(0.0,0.0,-1.0)\), NOT \(=(0.0,0.0,0.0)\)
orders LIRE_MAILLAGE and MODI_MAILLAGE.
reading of the field of temperature starting from the file with format "MED", order LIRE_CHAMP.
creation of a result starting from the field previously read, order CREA_RESU.
creation of a model starting from the symmetrical grid, orders AFFE_MODELE.
reading of the grid accounting for \(1 / 4\) of the structure, orders LIRE_MAILLAGE.
creation of a model starting from the grid previously read, order AFFE_MODELE.
projection of the 3 results created starting from the solution calculated on 1/8ème of structure, order PROJ_CHAMP with key word DISTANCE_MAX.
extraction of the fields of temperature of the 3 results resulting from projection, orders CREA_CHAMP.
creation of a null field on the model built on \(1 / 4\) of the structure, orders
CREA_CHAMP (cf notices \(n^{\circ} 2\) ).
combination of all the fields, orders COMB_CHAM_NO.
creation of a result starting from the combination of the fields, CREA_RESU.

\section*{Notice \({ }^{\circ} 1\) :}

In this case test, nodes of the grid, [Figure 3.2-a], belong to the symmetry plane (sym). For these nodes, the projection of the field of temperature will thus be entered 2 times.
To avoid once cumulating the field of temperature of too, a solution is to create one model containing only these nodes and to carry out projection on the complete grid. This projected field will be then withdrawn using order COMB_CHAM_NO.

\section*{Notice \(\mathbf{n}^{\circ} \mathbf{2}\) :}

The projection of the fields using order PROJ_CHAMP and the key word DISTANCE_MAX makes it possible not to create a field on the nodes which are not contained in one of the elements of the initial grid and which are at a distance higher than DISTANCE_MAX of the element nearest. When one combines fields using order COMB_CHAM_NO, it is the first field with the nodes which is taken as reference. If the field is incomplete, as in our case, the combination does not go to give the anticipated result. The solution is thus to create a null field on all the model and to make use of it like reference field.
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Code_Aster \({ }^{\circledR}\)

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ZZZZ185 - Validation MODI_MAILLAGE/PROJ_CHAMP
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01/01/04
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\section*{3.2 \\ Characteristics of the grid}

The projection of the field of temperature calculated on 1/8ème of structure (grid of [Figure 1.1-a]) is projected on the model built starting from the grid of the \(1 / 4\) of structure [Figure 3.2-a]).

\section*{Appear 3.2-a: Grid of the \(1 / 4\) of the structure}

\author{
3.3 Functionalities \\ tested \\ Orders \\ LIRE_MAILLAGE \\ FORMAT= " MED " \\ IMPR_RESU \\ FORMAT= " MED "
}

\author{
LIRE_CHAMP \\ FORMAT= " MED " \\ CREA_RESU \\ OPERATION= " AFFE " TYPE_RESU= " EVOL_THER " \\ CREA_CHAMP \\ OPERATION = " EXTR " TYPE_CHAM= " NOEU_TEMP_R " \\ MODI_MAILLAGE \\ SYMMETRY \\ NOT, AXE_1, AXE_2 \\ PROJ_CHAMP \\ METHODE= " ELEM " \\ DISTANCE_MAX \\ COMB_CHAM_NO COMB_R \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/04/005/A
}

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\section*{3.4}

Sizes tested and results
The test is carried out on the field of temperature.
Result obtained by projection and combination on \(1 / 4\) of structure is extracted the field from temperature TEMP1. Result calculated on \(1 / 4\) of structure is extracted the 2 nd field from temperature TEMP2. These 2 fields are withdrawn using order COMB_CHAM_NO, and the test is carried out on the maximum and minimal value of the resulting field. In all points of the structure, one must have TEMP1 = TEMP2, the field resulting from order COMB_CHAM_NO must thus be null in all
points.
Values tested
Reference
Code_Aster
Precision
Max 0.0
\(2.694 \mathrm{E}-05\)
1.0E-4

Min 0.0
-1.808E-05
\(1.0 \mathrm{E}-4\)
[Figure 3.4-a] the chart of the differences between the two fields of temperature obtained gives at the time
this case test.

\title{
Appear 3.4-a: Chart of the differences in temperature \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/04/005/A
}

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\section*{4 \\ Summary of the results}

This case test makes it possible to validate:
order MODI_MAILLAGE associated with the key word SYMMETRY,
order PROJ_CHAMP associated with key word DISTANCE_MAX.
The validation is done on all the field of temperature and not only on some points.
When the 2 orders are associated, it should be held account owing to the fact that the fields supported by nodes which belong to the symmetry plane can be entered 2 times.

During the combination of the fields using the order COMB_CHAM_NO, it is the structure of first CHAM_NO which is used as structure of reference. If the structure is incomplete, it is thus necessary to create
a CHAM_NO of reference.

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Date:
14/10/04
Author (S):
Key H. ANDRIAMBOLOLONA

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Organization (S): EDF-R \& D /AMA

\title{
Handbook of Validation
}

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Document: V1.01.188

SENSD08 - Sensitivity of modes suitable for parameter material

\section*{Summary:}

This case test corresponds to a modal calculation of a beam with supported supported rectangular section. One calculates the derivative of the Eigen frequencies and the clean deformations compared to the Young modulus.

The functionality tested is the derivation of the Eigen frequencies and the clean deformations. One also tests
the derivative of the clean deformations following a modification of the standard of the modes.
The results of reference are obtained in an analytical way.
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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}
\(L\)

\section*{B}

Z
y
H

X

L: 0.4 m
b: 0.02 m
H: 0.01 m

\section*{1.2}

Properties of material
\[
E=2.11011 \mathrm{~Pa}
\]
\[
=0.3
\]
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)
Viscous damping, matrix of damping: \(K+M\), with \(=10-4\) S and \(=10-4 s-1\)

\section*{1.3 \\ Boundary conditions and loadings}

Displacements of the nodes located on the left edge of co-ordinates \(X=0 \mathrm{~m}, Z=0.005 \mathrm{~m}\) are blocked according to \(X, y\) and \(Z\).
Displacements of the nodes located on the flat rim of co-ordinates \(X=0.4 \mathrm{~m}, Z=0.005 \mathrm{~m}\) are
blocked according to \(X, y\) and \(Z\).

\subsection*{1.4 Conditions} initial

No initial condition is applied to the structure.
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2
Reference solution

\section*{2.1 \\ Method of calculation}

In the case of a supported supported beam, the Eigen frequency of the mode of inflection number I express yourself in the following way [bib1]:

I
2
I.E.(internal excitation)
\(F=\)
I
2L2
\(S\)

\section*{With:}
\(L\) : length of the beam
E: Young modulus
I: quadratic moment of inertia compared to the bending axis
: density
S: section of the beam
The clean deformation of mode I is given by the following relation:
ix
\(y=\sin\)

I
\(L\)
One can thus obtain in an analytical way the derivative of the Eigen frequency and clean deformation compared to a given parameter.

\section*{2.2 \\ Sizes and results of reference}

The values of reference are obtained analytically.
3
bh
For our case: \(I=\)
and: \(S=b h\)
12
The derivative of the Eigen frequency compared to the Young modulus \(E\), is thus given by the relation following:

And the clean deformation does not depend on the Young modulus.
\(y i=0\)
E

\subsection*{2.3 Uncertainties}

The reference solution is obtained analytically.

\subsection*{2.4 Reference \\ bibliographical}
[1] Formulated for natural frequency and shape mode. Robert D. Blevins Ph.D. Krieger.
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\section*{3 Modeling}

\section*{With}

\section*{3.1 \\ Characteristics of modeling}

One considers in this modeling modal calculation by simultaneous iterations, with and without catch in count matrix of damping. One calculates then the derivative of the clean modes by report/ratio with the Young modulus of material constituting the beam.

The beam was modelled in voluminal finite elements.

Boundary conditions: the central nodes \((Z=h / 2)\) of the faces left and right-hand side are blocked according to
\(X, y\) and \(Z\).
Parameters of calculation:
Modal calculation carried out by the operator: MODE_ITER_SIMULT
Standardization of the clean deformations by: NORM_MODE

\section*{3.2 \\ Characteristic of the grid}

Numbers and type of meshs: 10 elements of the type HEXA20.

\subsection*{3.3 Functionalities tested}

The functionalities tested are the Eigen frequencies resulting from a modal calculation by the method simultaneous iterations, the derivation of the Eigen frequencies and deformations clean by report/ratio with the Young modulus.

\section*{Orders}

\section*{DEFI_PARA_SENSI VALE}

\section*{MODE_ITER_SIMULT SENSITIVITY}

\author{
NORM_MODE SENSITIVITY
}

\author{
TEST_RESU SENSITIVITY
}

\section*{3.4 \\ Sizes tested and results}

The values of reference are calculated for the first and the second mode of the system.

\subsection*{3.4.1 Without taking into account of the matrix of damping (real modes)}

Eigen frequencies:
Number of the mode
Reference
Aster Difference
(\%)
1
147.05 Hz
147.31 Hz
0.17
2
588.21 Hz
592.35 Hz
0.70

Derived from the Eigen frequencies compared to \(E\) :

\section*{Number of the mode \\ Reference}

Aster Difference
(\%)
13.50
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(3.511010 \mathrm{~Hz} / \mathrm{Pa}\)
0.17
21.40
\(10-9 \mathrm{~Hz} / \mathrm{Pa}\)
1.41 \(109 \mathrm{~Hz} / \mathrm{Pa}\)
0.70

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\section*{Code_Aster \({ }^{\circledR}\)}

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Derived from the modal deformation of the central node (N6) compared to \(E\) :

\section*{Number of the mode}

\section*{Mode normalizes}

Reference
Aster
1 (*)
0
2.01

10-24
2 (*)
0
-9.34
10-24
1
MASS_GENE
03.60

10-24
2
MASS_GENE
0-1.67
10-23
(*): absolute value of the largest component not multiplier of Lagrange equalizes to 1.

\subsection*{3.4.2 Taking into account of the matrix of damping (complex modes)}

Eigen frequencies:

\section*{Number of the mode}

Reference
Aster Difference
(\%)
1
147.05 Hz
147.31 Hz
0.17

2
588.21 Hz
592.35 Hz
0.70

Derived from the Eigen frequencies compared to \(E\) :

\author{
Number of the mode \\ Reference \\ Aster Difference \\ (\%) \\ 13.50 \\ \(10-10 \mathrm{~Hz} / \mathrm{Pa}\) \\ \(3.511010 \mathrm{~Hz} / \mathrm{Pa}\) \\ 0.17 \\ 21.40 \\ \(10-9 \mathrm{~Hz} / \mathrm{Pa}\) \\ \(1.41109 \mathrm{~Hz} / \mathrm{Pa}\) \\ 0.70
}

Derived from the modal deformation of the central node (N6) compared to \(E\) :
Number of the mode
Mode normalizes
Reference

\author{
Aster
}

1 (*)
0
8.49

1024-I 5.70 10-27
2 (*)
0
-4.90
\(1023+I 5.4310-25\)
1
MASS_GENE
02.50

1025 I 1.68 10-28
2
MASS_GENE
0-7.19
\(1025+I 7.9710-27\)
(*): modulate larger component not multiplier of Lagrange equalizes to 1 .

\author{
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}

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\section*{4 Modeling \\ B}

\section*{4.1}

\section*{Characteristics of modeling}

One considers in this modeling a modal calculation by opposite iteration, with and without catch in count matrix of damping. One calculates then the derivative of the clean modes by report/ratio with the Young modulus of material constituting the beam.

The beam was modelled in voluminal finite elements.
Boundary conditions: the central nodes \((Z=h / 2)\) of the faces left and right-hand side are blocked according to
\(X, y\) and \(Z\).
Parameters of calculation:
Modal calculation carried out by the operator: MODE_ITER_INV
Standardization of the clean deformations by: NORM_MODE

\section*{4.2 \\ Characteristics of the grid}

Numbers and type of meshs: 10 elements of the type HEXA20.

\subsection*{4.3 Functionalities \\ tested}

\title{
The functionalities tested are the Eigen frequencies resulting from a modal calculation by the method
} iterations opposite, the derivation of the Eigen frequencies and deformations clean compared to Young modulus.

\section*{Orders}

\section*{DEFI_PARA_SENSI VALE}

MODE_ITER_INV SENSITIVITY

\section*{NORM_MODE SENSITIVITY}

TEST_RESU SENSITIVITY

\section*{4.4 \\ Sizes tested and results}

The values of reference are calculated for the first and the second mode of the system.

\subsection*{4.4.1 Without taking into account of the matrix of damping (real modes)}

Eigen frequencies:

\section*{Number of the mode \\ Reference \\ Aster Difference \\ (\%) \\ 1 \\ 147.05 Hz \\ 147.31 Hz \\ 0.17 \\ 2 \\ 588.21 Hz \\ 592.35 Hz \\ 0.70}

Derived from the Eigen frequencies compared to E:
Number of the mode
Reference
Aster Difference
(\%)
13.50
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(3.511010 \mathrm{~Hz} / \mathrm{Pa}\)
0.17
21.40
\(10-9 \mathrm{~Hz} / \mathrm{Pa}\)
1.41 \(109 \mathrm{~Hz} / \mathrm{Pa}\)
0.70
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Derived from the modal deformation of the central node (N6) compared to E:
Number of the mode
Mode normalizes
Reference
Aster ..... 1 (*) 0-2.26 ..... 10-16
2 (*) 0-5.66
10-15 ..... 1
\(\left.\mathbf{(}^{*}\right):\) absolute value of the largest component not multiplier of Lagrange equalizes to 1 .

\subsection*{4.4.2 Taking into account of the matrix of damping (complex modes)}

Eigen frequencies:
```

Number of the mode
Reference
Aster Difference
(\%)
1
147.05 Hz
147.31 Hz
0.17
2
588.21 Hz
592.35 Hz
0.70

```

Derived from the Eigen frequencies compared to \(E\) :

\section*{Number of the mode \\ Reference}

Aster Difference
(\%)
13.50
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(3.511010 \mathrm{~Hz} / \mathrm{Pa}\)
0.17
21.40
\(10-9 \mathrm{~Hz} / \mathrm{Pa}\)
\(1.41109 \mathrm{~Hz} / \mathrm{Pa}\)
0.70

Derived from the modal deformation of the central node (N6) compared to \(E\) :

Number of the mode
```

Mode normalizes
Reference
Aster
1 (*)0
3 . 2 0
1015 - I 6.76 10-16
2 (*)0
-2.38
1014-I 9.65 10-15
l
MASS_GENE
0.42
1017 I 1.99 10-17
2
MASS_GENE
0-3.50
1016 I 1.42 10-16
(*): modulate larger component not multiplier of Lagrange equalizes to 1.
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```
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\section*{5 \\ Summary of the results}

The precision relative of derived from each Eigen frequency compared to the Young modulus is lower than \(1 \%\) of the theoretical solution. The precision on the derivative of the clean deformation is
lower than 10-14 in absolute value.
The precision of the results depends obviously on the discretization in finite elements. Here, us modelled the beam in 10 parabolic elements. This discretization is reasonable and precision obtained on the first Eigen frequencies is acceptable.
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22/07/05
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NR. TARDIEU Key
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Organization (S): EDF-R \& D /AMA

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ZZZZ189 circumferential Orientation by one

\author{
buckle Python
}

\section*{Summary:}

In the case of the modeling of a hemisphere or a dome, it is delicate to direct circonférentiellement elements in AFFE_CARA_ELEM [U4.42.01]. One proposes a Python function which allows simply to carry out this action.
One treats the case of a concrete hemisphere with reinforcements under pressure in linear elasticity. One seeks with to direct the reinforcements circonférentiellement.

\author{
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}

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\section*{1}

Problem of reference
1.1 Geometry
```

B
B
y
With
With
X
X
Ray
R=10.m
Thickness
T=0.04
Co-ordinates of the points:
With
B
C
X 10.
0.
0.
y 0.
10.
0.
Z 0.
0.
10.

```

\section*{1.2}
```

Material properties

```

The hemisphere made up of concrete is reinforced by grids. As the objective is only of to test orders, one gives the same properties to the concrete and the grids.
\(E=200000 \mathrm{~Pa},=0.3\)
1.3

Boundary conditions and loadings
On a quarter of the hemisphere:
Not C
no displacement in \(Z\)
Side AC
symmetry compared to the xz plan
Side BC
symmetry compared to the yz plan
Side \(A B\)
free
Internal pressure: \(\mathbf{P = 1 0}\). Pa
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\subsection*{1.4 Problems \\ of orientation}

It is wanted that the grids of reinforcement are directed circonférentiellement. However, seen the form
hemispherical considered, one cannot impose this orientation by AFFE_CARA_ELEM in the state current of the code. One thus proposes a Python function allowing it.

The principle is as follows:
\(\cdot\) the axis of the hemisphere is \(Z\)
\(\cdot\) for each mesh, one calculates the vector indicating the ciconférentielle direction by \(V=Z^{\wedge} N\) where NR is the normal with the mesh. One notes \(V=[V x, V y, V z]\) and one defines his projection on plan ( XOY )
by \(W=[V x, V y, 0]\)
\(\cdot\) one calculates the values of \(A N G L \_R E P\) to be assigned to the current mesh by \(=\arctan (V y / V x)\) and

Function called LIST_CARA_CIRCONF buckles on all the meshs roasts, calculates and and creates key words:
```

_F (SECTION=20.0,
MAILLE=Nom_Maille_Courante,
EXCENTREMENT=0.0,
ANGL_REP=(,)),
GRILLE_NCOU=1,
COEF_RIGI_DRZ=1.E-10),
and it adds them in a list. One provides then this list to AFFE_CARA_ELEM. They are thus obtained
orders:
LIST_GRI=LIST_CARA_CIRCONF (GROUP_MA=' GRILLE',
AXE= (0. , 0. , 1.),
MODELE=MODEL,
GRILLE=_F (SECTION=20., EXCENTREMENT=0.,));
CARA_COQ=AFFE_CARA_ELEM (MODELE=MODEL,
COEF_RIGI_DRZ=0.,),
GRILLE=LIST_GRI,

```

\section*{);}

Let us note finally that, formally, LIST_CARA_CIRCONF obeys the following catalogue:

\section*{LIST_CARA_CIRCONF}
(
GROUP_MA
\(=\) SIMP (statut=' o' \(^{\prime}\), typ=grma, max=' ** '),

CENTER \(=\) SIMP (statut=' \(o^{\prime}\), typ=' R', max \(^{\prime}\), min \(=3\) ), MODEL
=SIMP (statut=' o', typ=modele_sdaster
),
ROAST
\(=\) FACT (statut=' \(f^{\prime}\), max=1,
SECTION =SIMP (statut=' \(o^{\prime}\), typ \(={ }^{\prime} R^{\prime}\) ), OFFSETTING
=SIMP (statut=' \(f^{\prime}\), typ=' \(R^{\prime}\) ),
COEF_RIGI_DRZ
\(=\) SIMP (statut=' \(f^{\prime}, t y p={ }^{\prime} R^{\prime}\) ), GRILLE_NCOU
\(=\) SIMP (statut \(={ }^{\prime} f^{\prime}\), typ \(=I^{\prime} I^{\prime}\) ),

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\section*{2 Modeling \\ With}

\author{
2.1 \\ Characteristics of modeling \\ 1373 elements of hull DKT \\ 1373 elements of grid GRILLE_MEMBRANE \\ Modeling of a quarter of the hemisphere in TRIA3.
}

\section*{2.2}

Characteristics of the grid
A number of nodes: 734
A number of meshs and types: 2746 TRIA3

\subsection*{2.3 Functionalities}
tested
Test of functionality Python within a command file.

\section*{3}

Results of modeling \(A\)

\subsection*{3.1 Values}
tested
One tests values of not-regression calculated with the V7.03.30 version.

\section*{Identification}

Aster
Node 30
0
displacement DX
Node 30
3.1392885337581E-05
displacement DY
Node 30
1.5356429239344E-05
displacement DZ

\author{
Node 700 \\ 4.4873398127688E-06 \\ displacement DX \\ Node 700 \\ 3.3210448836551E-05 \\ displacement DY \\ Node 700 \\ 1.5155123546576E-05 \\ displacement DZ \\ \section*{4} \\ Summary of the results
}

This test presents a advanced use of the Python language within Code_Aster. Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
8.1

\section*{Titrate:}

ZZZZ205 Calculation of the kinetic energy of a rectangular plate
Date:
02/11/05
Author (S):
X. DESROCHES, F. LEBOUVIER Key
:

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Organization (S): EDF-R \& D /AMA, DeltaCAD

\author{
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}

ZZZZ205 Calculation of the kinetic energy of one rectangular plate

\section*{Summary:}

This case test is intended to validate the calculation of the kinetic energy for modelings 2D solid masses.

Only one modeling is carried out:
Modeling A made up of meshs QUAD4 and TRIA3.

\author{
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```

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```
1

\section*{Problem of reference}

\subsection*{1.1 Geometry}

\section*{Y}
0.35
\(X\)
0.25
0.3
0.45

\section*{1.2}

Properties of material
- Acier

E \(=2 \times 1011 \mathrm{MPa}\)
\(-=0.3\)
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions}

\section*{1}

Calculation of the kinetic energy starting from speed \(E=V T M V\) :
C
2
- One imposes a uniform speed:
with \(t=1 \mathrm{~s}\) : according to \(X\) of \(1.5 \mathrm{~m} / \mathrm{s}\)
with \(t=2 \mathrm{~s}\) : according to \(X\) of \(1.5 \mathrm{~m} / \mathrm{s}\) and following \(Y\) of \(2.5 \mathrm{~m} / \mathrm{s}\)

\subsection*{1.4 Conditions \\ initial}

\author{
None. \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders \\ HT-66/05/005/A
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The analytical solution is presented below:
Calculation of the kinetic energy starting from speed
1
\(E=V T M V\)

C
2

\section*{2.2}

Sizes and results of reference

\author{
Sizes Values \\ Unit \\ Mass \\ \(2.0475 \times 103 \mathrm{~kg}\) \\ \(E(T=1 s)\) \\ C \\ 2.3034375x103 W \\ \(E(T=2 s)\) \\ C \\ 8.70187 x 103 W \\ Handbook of Validation \\ V1.01 booklet: Tests of validity of orders
}

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

\section*{3.2 \\ Characteristics of the grid}

A number of meshs: 541 (320 TRIA3, 221 QUAD4)
A number of nodes: 423

\author{
3.3 Functionalities \\ tested \\ Orders \\ Key word factor \\ Single-ended spanner word \\ Argument \\ AFFE_MODELE AFFE \\ PHENOMENON \\ "MECHANICAL" \\ MODELING \\ "D_PLAN" \\ POST_ELEM MASSE_TOT
}
3.4Sizes tested and results
Identification
Size Reference Aster
\% Difference
Mass
TOTAL 2.0475x103 2.0475x103-2.33
x10-13
Kinetic energy
TOTAL
\(2.30344 \times 1032.30344\)
x103-1.09
x10-4
( \(t=1\) )
Kinetic energy
TOTAL
8.70187 x 1038.70187
x103 5.75
x10-5
( \(t=2\) )
4
Summary of the results
This test makes it possible to validate the calculation of the kinetic energy for modeling \(D_{-} P L A N\).
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SENSD09 - Sensitivity of clean modes of a beam POU_D_E
Date:
01/09/05
Author (S):

\author{
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Document: V1.01.207

SENSD09 - Sensitivity of clean modes of one beam POU_D_E compared to the parameters material

\section*{Summary}

This case test validates the derivation of element \(P O U_{\_} D_{-} E\) compared to the parameters material (Young modulus and
Poisson's ratio). One calculates the clean modes, like their derivative compared to E and, of one beam with supported circular section "supported" in inflection and "embedded embedded" in torsion.

The results of reference are obtained in an analytical way.
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1
Problem of reference

\subsection*{1.1 Geometry}
```

R
Z
$X$

```
\(L: 0.4 m\)
\(R: 0.01 \mathrm{~m}\)

\section*{1.2}

Properties of material
\(E=2.11011 \mathrm{~Pa}\)

Viscous damping, matrix of damping: \(K+M\), with \(=10-6 S\) and \(=10-6 s-1\)

\section*{1.3 \\ Boundary conditions and loadings}

Displacements following y and rotations around \(Z\) are blocked.
Displacements according to \(X\) and \(Z\), as well as rotation around the axis of the beam (axis \(X\) ) of the nodes
located at the ends of the beam are blocked.

\subsection*{1.4 Conditions initial}

No initial condition is applied to the structure.
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2
Reference solution

\section*{2.1 \\ Method of calculation}

\title{
At the same time the modes of inflection and the modes of torsion are considered.
}

\subsection*{2.1.1 Mode of inflection}

In the case of a supported supported beam, the Eigen frequency of the mode of inflection number I express yourself in the following way [bib1]:

I
2
I.E.(internal excitation)
\(F=\)

I
\(2 L 2\)
\(S\)

\section*{With:}

L: length
beam
of

E: modulate
Young
I
: moment of
quadratiqu
inertia
by
\(E\)
with
report/ratio
inflection
of

In the case of a beam with circular section of ray \(R\), one obtains:
I
2 RE
\(F=\)
I
\(4 L 2\)

The clean deformation of the mode of inflection number I is given by the following relation:
I X
\(Z=\sin\)
\(L\)

\subsection*{2.1.2 Mode of torsion}

The Eigen frequency of the mode of torsion number I, an embedded fixed beam (in torsion), express yourself in the following way [bib1]:
\(I\)
\(C G\)
\(F=\)

With: G
: modulate
cisailleme
of

NT (21+)
C
:
torsion
of
constant

I
: moment of
quadratiqu
inertia
by

E
report/ratio
with
torsion
of
center
\(p\)

In the case of a beam with circular section of ray \(R\), one obtains:
\[
F=I
\]

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The clean deformation of the mode of torsion number I is given by the following relation:
I X
\(=\sin\)
\(I\)
\(L\)

One can thus obtain in an analytical way the derivative of the Eigen frequency and clean deformation compared to a given parameter.

\section*{2.2 \\ Sizes and results of reference}

With the geometrical characteristics and mechanics chosen, the first two modes of the beam are modes of inflection and the third clean mode corresponds to the first mode of torsion.

The derivative of the Eigen frequency of inflection are given by the following relations:
\(F\)
\(F\)
\(F\)
\(I\)
\(I\)
\(=\)
and
\(I=0\)
\(E\)
\(2 n d\)

The derivative of the Eigen frequency of torsion are given by the following relations:
F
\(F\)
\(F\)
\(F\)
\(I\)
\(I\)
\(=\)
and
\(I=\) -

The clean deformations depend neither on the Young modulus, nor of the Poisson's ratio.
\(Z\)
\(Z\)
\(I=0\)
\(I=0\) and \(I=0\)
\(I=0\)

\subsection*{2.3 Uncertainties}

The reference solution is obtained analytically.

\subsection*{2.4 Reference bibliographical}
[1] Formulated for natural frequency and shape mode. Robert D. Blevins Ph.D. Krieger (1984).

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

One considers in this modeling modal calculation by opposite iteration. One calculates then them derived from the clean modes compared to the Young modulus and the Poisson's ratio material constituting the beam.

\section*{3.2 \\ Characteristic of the grid}

Numbers and type of meshs: 10 elements of the type \(P O U_{-} D \_E\).

\subsection*{3.3 Functionalities \\ tested}

The functionalities tested are derivations of real clean modes resulting from a modal calculation by method of iteration opposite, compared to the Young modulus and the Poisson's ratio.

\section*{Orders}

DEFI_PARA_SENSI VALE

\author{
MODE_ITER_INV SENSITIVITY
}

\section*{TEST_RESU SENSITIVITY}

\section*{3.4 \\ Sizes tested and results}

The values of reference are calculated for the first three modes of the system.
Eigen frequencies:
Number of the mode
Reference
Aster Difference
(\%)
1
509.4034 Hz
509.4073 Hz
7.59 10-4

2
2037.614 Hz
2037.833 Hz

Derived from the Eigen frequencies compared to E:
Number of the mode

\section*{Reference}

Aster Difference
(\%)
11.21286

10-9
\(1.21287109 \mathrm{~Hz} / \mathrm{Pa}\)
7.59 10-4
\(\mathrm{Hz} / \mathrm{Pa}\)
24.85146

10-9
\(4.85198109 \mathrm{~Hz} / \mathrm{Pa}\)
0.011
\(\mathrm{Hz} / \mathrm{Pa}\)
39.57715

10-9
\(9.61658109 \mathrm{~Hz} / \mathrm{Pa}\)
0.412
\(\mathrm{Hz} / \mathrm{Pa}\)

Derived from the Eigen frequencies compared to:

\section*{Number of the mode}

\section*{Reference}

Aster Difference
(\%)
1
0.0 Hz
\(-2.711040 \mathrm{~Hz}\)
2
0.0 Hz
-1.24 1024 Hz
3-1.5471
103 Hz

Code_Aster \({ }^{\circledR}\)
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Derived from the modal deformation of the central node (GROUP_NO = MEDIUM) compared to E:
Number of the mode
Degree of freedom
Reference
Aster
1 DZ
0
6.069

10-21
3 DRX
0
-5.561
10-22

Derived from the modal deformation of the central node (GROUP_NO = MEDIUM) compared to:
Number of the mode
Degree of freedom
Reference
Aster
1 DZ
0

\title{
Code_Aster \({ }^{\circledR}\)
}

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\section*{4 Modeling \\ B}

\section*{4.1 \\ Characteristics of modeling}

One considers in this modeling a modal calculation by simultaneous iterations, with catch in count matrix of damping. One calculates then the derivative of the clean modes by report/ratio with the Young modulus and the Poisson's ratio of material constituting the beam.

\section*{4.2 \\ Characteristics of the grid}

Numbers and type of meshs: 10 elements of the type \(P O U \_D \_E\).

\subsection*{4.3 Functionalities \\ tested}

The functionalities tested are derivations of complex clean modes resulting from a modal calculation by the method of simultaneous iterations, compared to the Young modulus and the Poisson's ratio.

\section*{Orders}

\section*{DEFI_PARA_SENSI VALE}

\section*{MODE_ITER_SIMULT SENSITIVITY}

\section*{TEST_RESU SENSITIVITY}

\section*{4.4 \\ Sizes tested and results}

The values of reference are calculated for the first three modes of the beam.

Eigen frequencies:
Number of the mode

\section*{Reference}

Aster Difference
(\%)
1
509.4034 Hz
509.4066 Hz
6.31 10-4

2
2037.614 Hz
2037.792 Hz
0.009

3
4022.404 Hz
4038.640 Hz
0.404

Derived from the Eigen frequencies compared to \(E\) :

\author{
Number of the mode \\ Reference \\ Aster Difference \\ (\%) \\ 11.21286 \\ 10-9 \\ \(1.21287109 \mathrm{~Hz} / \mathrm{Pa}\) \\ 7.59 10-4 \\ \(\mathrm{Hz} / \mathrm{Pa}\) \\ 24.85146 \\ 10-9 \\ \(4.85198109 \mathrm{~Hz} / \mathrm{Pa}\) \\ 0.011 \\ \(\mathrm{Hz} / \mathrm{Pa}\) \\ 39.57715 \\ 10-9 \\ 9.61658 10-9 Hz/Pa \\ 0.412 \\ \(\mathrm{Hz} / \mathrm{Pa}\)
}

Derived from the Eigen frequencies compared to:
Number of the mode
Reference
Aster Difference
(\%)
1
0.0 Hz
\(-3.221036 \mathrm{~Hz}\)
2
0.0 Hz
\(-3.771031 \mathrm{~Hz}\)
3-1.5471
103 Hz
\(-1.5534103 \mathrm{~Hz}\)
0.412

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Derived from the modal deformation of the central node \(\left(G R O U P \_N O=M E D I U M\right)\) compared to \(E\) :
Number of the mode
Degree of freedom

\section*{Reference}

Aster
1
DZ
\(0+0 I\)
6.071021 - I 9.711024

3
DRX
\(0+0 I\)
\(-5.571022+I 7.091024\)

Derived from the modal deformation of the central node (GROUP_NO = MEDIUM) compared to:
Number of the mode
Degree of freedom
Reference
Aster
1
DZ
\(0+0 I\)
\(-5.801031-I 9.481032\)
3
DRX
\(0+0 I\)
\(9.001011-I 1.141012\)

\section*{5 \\ Summary of the results}

Precision relative of derived from each Eigen frequency compared to the Young modulus or to Poisson's ratio is lower than \(0.5 \%\) of the theoretical solution. Precision on the derivative of clean deformation is lower than 10-10 in absolute value.
The precision of the results depends obviously on the discretization in finite elements. Here, us modelled the beam in 10 elements POU_D_E. This discretization is reasonable and the precision obtained on the first Eigen frequencies is acceptable.
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Organization (S): EDF-R \& D /AMA

\title{
Handbook of Validation
}

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Document: V1.01.214

SENSD10 - Sensitivity of clean modes of one
plate simply supported compared to
parameters material

\section*{Summary}

This case test validates the derivation of the elements plates planes isotropic (DKT, DSQ, Q4G, COQUE_3D) by report/ratio with the parameters material (Young modulus and Poisson's ratio). One calculates the Eigen frequencies, thus that their derivative compared to E and, of a rectangular plate simply pressed on its edges.

The results of reference are obtained in an analytical way.
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{B}

X
has
y
Z

H

Width:
a: 0.2 m
Length:
b: 0.3 m
Thickness:
H: 0.001 m

\section*{1.2}

Properties of material
Young modulus:
\(E=2.11011 \mathrm{~Pa}\)
Poisson's ratio:
\(=0.3\)
Density:
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

Displacements according to \(X, y\) and \(Z\) on the edges of the plate are blocked.

\subsection*{1.4 Conditions}
initial
No initial condition is applied to the structure.
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

An isotropic homogeneous plate is considered and one calculates his first clean modes of inflection.

In the case of a plate simply pressed on its edges, the Eigen frequency of the mode of inflection number ij is expressed in the following way [bibl]:
```

=
H
fij
[2222
BI + has
]
E
J
2
2a B
12(
2
l-)

```

\section*{With:}

I: belly of vibration numbers according to dimension has
J: belly of vibration numbers according to the Directorate \(B\)
E: Young modulus
: Poisson's ratio
H: thickness of the plate
: density
In the case where, the a/b report/ratio is equal to \(2 / 3\), the order of arrangement of the first Eigen frequencies
of inflection is as follows:
\(f 11, f 12, f 21, f 13, f 22, f 23, \ldots\)

\section*{2.2 \\ Sizes and results of reference}

The derivative of the Eigen frequency of inflection are given by the following relations:
```

F
F
ij
ij
=
and:
ij=
ij
E
2nd
2
1-

```

\subsection*{2.3 Uncertainties}

The reference solution is obtained analytically.

\subsection*{2.4 Reference \\ bibliographical}

\section*{[1]}

Formulated for natural frequency and shape mode. Robert D. Blevins Ph.D. Krieger (1984).

\author{
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

One considers in this modeling a modal calculation of the plate modelled by elements DKT.
One calculates then the derivative of the clean modes compared to the Young modulus and compared to Poisson's ratio of material constituting the plate.

\section*{3.2 \\ Characteristic of the grid}

Numbers and type of meshs: 192 elements of the type TRIA3.

\subsection*{3.3 Functionalities \\ tested}

The functionalities tested are derivations of real clean modes of plate (DKT) resulting from one modal calculation by the method of simultaneous iterations, compared to the Young modulus and the coefficient
of Poisson.

\section*{Orders}

\section*{DEFI_PARA_SENSI VALE}

MODE_ITER_SIMULT SENSITIVITY

TEST_RESU SENSITIVITY

\section*{3.4}

\section*{Sizes tested and results}

The values of reference are calculated for the first three modes of the plate.
Eigen frequencies:
Number of the mode
Reference
Aster Difference
(\%)
1
89.0659 Hz
88.7199 Hz
0.388

2
171.280 Hz
170.004 Hz
0.745

3
274.049 Hz
272.288 Hz
0.643

Derived from the Eigen frequencies compared to E:
Number of the mode
Reference
Aster Difference
(\%)
12.1206
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(2.11241010 \mathrm{~Hz} / \mathrm{Pa}\)
0.388
24.0781
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(4.04771010 \mathrm{~Hz} / \mathrm{Pa}\)
0.745
36.5250
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(6.48301010 \mathrm{~Hz} / \mathrm{Pa}\)
0.643

Derived from the Eigen frequencies compared to:
Number of the mode

\section*{Reference}

Aster Difference
(\%)
1
29.3624 Hz
29.1626 Hz
0.680

2
56.4661 Hz
55.7189 Hz
1.323

3
90.3458 Hz
89.0907 Hz
1.389

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\section*{4 Modeling \\ B}

\section*{4.1 \\ Characteristics of modeling}

One considers a modal calculation of the plate modelled by elements DSQ. One calculates then them derived from the clean modes compared to the Young modulus and the Poisson's ratio
material constituting the plate.

\section*{4.2 \\ Characteristics of the grid}

Numbers and type of meshs: 96 elements of the type QUAD4.

\subsection*{4.3 Functionalities \\ tested}

The functionalities tested are derivations of real clean modes of plate (DSQ) resulting from one modal calculation by the method of simultaneous iterations, compared to the Young modulus and the coefficient
of Poisson.

\section*{Orders}

DEFI_PARA_SENSI VALE

\section*{MODE_ITER_SIMULT SENSITIVITY}

\section*{TEST_RESU SENSITIVITY}

\section*{4.4 \\ Sizes tested and results}

The values of reference are calculated for the first three modes of the beam.

\section*{Eigen frequencies: \\ Number of the mode \\ Reference \\ Aster Difference \\ (\%) \\ 1 \\ 89.0659 Hz \\ 88.4294 Hz \\ 0.715}

2
171.280 Hz
168.774 Hz
1.463

3
274.049 Hz
271.592 Hz
0.897

Derived from the Eigen frequencies compared to \(E\) :
Number of the mode
Reference
Aster Difference
(\%)
12.1206
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(2.10551010 \mathrm{~Hz} / \mathrm{Pa}\)
0.715
24.0781
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
4.0184 \(1010 \mathrm{~Hz} / \mathrm{Pa}\)
1.463
36.5250
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
6.4665 \(1010 \mathrm{~Hz} / \mathrm{Pa}\)
0.897

Derived from the Eigen frequencies compared to:

\section*{Number of the mode}

Reference
Aster Difference
(\%)
1
29.3624 Hz
29.0709 Hz
0.993

2
56.4661 Hz
55.3148 Hz
2.039

3
90.3458 Hz

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\section*{5 Modeling}

C

\section*{5.1}

\section*{Characteristics of modeling}

One considers a modal calculation of the plate modelled by elements Q4G. One calculates then them derived from the clean modes compared to the Young modulus and the Poisson's ratio material constituting the plate.

\section*{5.2 \\ Characteristic of the grid}

Numbers and type of meshs: 96 elements of the type QUAD4.

\subsection*{5.3 Functionalities \\ tested}

The functionalities tested are derivations of real clean modes of plate ( \(Q 4 G\) ) resulting from one modal calculation by the method of simultaneous iterations, compared to the Young modulus and the coefficient

\section*{Orders}

\section*{DEFI_PARA_SENSI VALE}

\section*{MODE_ITER_SIMULT SENSITIVITY}

\author{
TEST_RESU SENSITIVITY
}

\section*{5.4 \\ Sizes tested and results}

The values of reference are calculated for the first three modes of the plate.
Eigen frequencies:
Number of the mode
Reference
Aster Difference
(\%)
1
89.0659 Hz
88.7167 Hz
0.392

2
171.280 Hz
169.663 Hz
0.944

3
274.049 Hz
277.358 Hz
1.208

Derived from the Eigen frequencies compared to E:
Number of the mode
Reference
Aster Difference
(\%)
12.1206
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(2.11231010 \mathrm{~Hz} / \mathrm{Pa}\)
0.392
24.0781
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(4.03961010 \mathrm{~Hz} / \mathrm{Pa}\)
0.944
36.5250
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(6.60381010 \mathrm{~Hz} / \mathrm{Pa}\)
1.208

Derived from the Eigen frequencies compared to:
Number of the mode

\section*{Reference}

Aster Difference
(\%)
1
29.3624 Hz
29.1611 Hz
0.686

2
56.4661 Hz
55.5716 Hz
1.584

3
90.3458 Hz
91.0203 Hz
0.747

Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
SENSD10 - Sensitivity of clean modes of a plate

\section*{Date:}

\section*{6 Modeling \\ D}

\section*{6.1}

\section*{Characteristics of modeling}

One considers a modal calculation of the plate modelled by elements COQUE_3D. They are calculated derived from the first clean mode compared to the Young modulus and the coefficient from Poisson of material constituting the plate. For this modeling, we introduced one viscous damping proportional in the mechanical characteristics of constituent material the plate \(\left(A M O R \_A L P H A=10-6 S, A M O R \_B E T A=10-6 s-1\right)\).

\section*{6.2 \\ Characteristic of the grid}

Numbers and type of meshs: 24 elements of the type QUAD9.

\subsection*{6.3 Functionalities}
tested
The functionalities tested are derivations of clean mode complexes hull (COQUE_3D) resulting from a modal calculation by the method of simultaneous iterations, compared to the Young modulus and with
Poisson's ratio.

\section*{Orders}

DEFI_PARA_SENSI VALE

\author{
MODE_ITER_SIMULT SENSITIVITY
}

\section*{6.4 \\ Sizes tested and results}

The values of reference are calculated for the first mode of the plate.
Eigen frequency:
Number of the mode

\section*{Reference}

Aster Difference
(\%)
1
89.0659 Hz
88.33472 Hz
0.807

Derived from the first Eigen frequency compared to \(E\) :
Number of the mode
Reference
Aster Difference
(\%)
12.1206
\(10-10 \mathrm{~Hz} / \mathrm{Pa}\)
\(2.10351010 \mathrm{~Hz} / \mathrm{Pa}\)
0.807

Derived from the first Eigen frequency compared to:

\section*{Number of the mode}

\section*{Reference}

Aster Difference
(\%)
1
29.3624 Hz
30.3057 Hz
3.213

Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}
8.2

\section*{Titrate:}

SENSD10 - Sensitivity of clean modes of a plate
Date:
07/11/05
Author (S):
Key H. ANDRIAMBOLOLONA
V1.01.214-A Page:
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\section*{7 \\ Summary of the results}

The relative precision on the derivative of each Eigen frequency compared to the Young modulus is lower than \(2 \%\) of the theoretical solution for various modelings of the plate. For derivation compared to the Poisson's ratio, the precision is lower than 4\% of the solution theoretical.
The precision of the results depends obviously on the discretization in finite elements and on assumptions of plane constraint, small deformation and small displacement.
Here, we chose a discretization corresponding to 8 finite elements by wavelength of vibration. This discretization is certainly rather coarse, but it makes it possible nevertheless to obtain one
solution acceptable for the derivative of the first three Eigen frequencies of the plate.
Handbook of Validation
V1.01 booklet: Tests of validity of orders
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.1

Titrate:
FETIO01 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.101-A Page:

FETI001 Validation of the basic architecture of one calculation FETI

\section*{Summary:}

This case-test validates the not-regression of the basic commands of a calculation FETI in linear mechanics:
manual partitioning and sequential calculation multidomaine without rigid mode of body and Dirichlet or
loading being based on late meshs or nodes. It is a question of making sure that insertions of order DEFI_PART_OPS and key word SOLVEUR+METHODE=' FETI' in MECA_STATIQUE remains
operational.
One checks the algorithmic one thus (factorization symbolic system with the iterative resolution) and the management of
structures of encapsulated data (NUME_DDL, CHAM_NO, MATR_ASSE...) specific to FETI. For that one studies a calculation in linear elasticity on a plate 2D not aligned with the principal axes, subjected to a constant and not blocked pressure. The inversion of the local matrices of rigidity

Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETI001 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.101-A Page:
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\section*{1}

Problem of reference
The geometry on which one will affect materials and loadings does not correspond to any real case. It is here only about one functional, numerical validation and of data-processing notregression
of a basic calculation FETI in linear mechanics: manual partitioning and calculation multidomaine sequential without rigid mode of body and Dirichlet or loading being pressed on meshs or late nodes.

\subsection*{1.1 Geometry}

\section*{E}

\title{
Appear 1.1-a: Geometry and loading
}

\section*{Mesh-point}

The structural features are: \(A C=C D=E D=A E=5 \mathrm{~mm}, A B=B C, A F=F D=F E=F C\).

\section*{1.2 \\ Material properties}

The characteristics materials are homogeneous in all the geometry: \(E=180000\) Mpa and \(=0.3\).

\section*{1.3}

Boundary conditions and loadings
Imposed normal constant pressure:
\(P=1000\) Mpa on \([A, B]\)
\(P^{\prime}=2000\) Mpa on \([B, C]\)
Particular blockings of the substructures by addition of elementary matrices of rigidity to

Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETI001 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.101-A Page:
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2
Reference solutions

\section*{2.1}

Method of calculation used for the reference solutions
On such a case of figure, it is not possible to exhume an analytical solution. Calculations FETI are validated by comparison with the results of the multifrontale.

\section*{2.2}

Uncertainty on the solutions
Convergence of the grid and parameter of control of the test of stop of FETI (RESI_RELA).

\author{
Handbook of Validation
}

V1.04 booklet: -
HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETIOO1 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

N9
N15
SD2:
FETI2
NI
N8
N11
N14
N16
SD3:
FETI3
N2
N3
N10
N7
N12
SD1:
N17
FETII
N18
N4
N5
N13

\title{
Appear 3.1-a: Division of the square in four pennies fields
}

Mesh-point corresponding to finite elements DISCRETE
(modeling 2D SAY T)

\section*{Handbook of Validation}

V1.04 booklet: -
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.1

Titrate:
FETI001 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):

\section*{O. BOITEAU Key}

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Co-ordinates of some points:
\(N 1=\{5.5,5\}, N 2=\{7,3\}, N 3=\{3.5,3.5\}, N 4=\{5,1.5\}, N 5=\{1.5,2\}, N 6=\{3,0\}, N 7=\{0,4\}, N 8=\) \(\{2,5.5\}, N 9=\{4,7\} \ldots\)
The interface with direction FETI of the term thus relates to DDLs of the nodes: N1, N3, N4, N5, N8, N16 and
N17. These nodes are all multiplicity 2, except N3, of multiplicity 4.
The grid is composed of 24 TRIA3 and 4 SEG2 associated with usual modeling 2D_PLAN with MECHANICAL phenomenon (GROUP_MA=STRU). One there assistant 8 discrete finite elements (modeling
2D_DIS_T, GROUP_MA=RES) which will be able to contribute to the local matrices of rigidity to each under-fields and to make them invertible even if the substructures are not blocked. Via

AFFE_CARA_ELEM one thus adds elementary matrices of rigidity of the type:
E 0

0 E
Calculation FETI is sequential, it is pressed on the preconditionnor lumpé with scaling by nodal multiplicity and on a total reorthogonalisation of the directions of descent via a GSM. For to obtain a relative residue lower than 10-10, 18 iterations are necessary. The local solvor is multifrontale with its default settings.

\section*{3.2 \\ Characteristics of the grid}

19 points, 24 TRIA3, 4 SEG3.

\author{
3.3 Functionalities \\ tested \\ Orders
}

DEFI/AFFE_MATERIAU ELAS

AFFE_CHAR_MECA PRESS_REP

\section*{MECHANICAL AFFE_MODELE D_PLAN}

\section*{MECHANICS}

2D_DIS_PLAN

AFFE_CARA_ELEM DISCRET_2D

DEFI_PART_OPS

\title{
MECA_STATIQUE SOLVEUR \\ METHODE=' FETI'
}

\section*{Handbook of Validation}

V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.1

Titrate:
FETIO01 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.101-A Page:
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested
One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE=' MULT_FRONT'), and this, in any point of grid. The relative tolerance is thus severe: 1.10-8\%.

\author{
Values \\ Ux \\ Uy \\ tested \\ multi \\ FETI Variation \\ multi \\ FETI Variation \\ frontal
}
```

relative
frontal
relative
(in %)
(in %)
N1
4.063 10-3 4.063
10-3 1.2
10-123.512
10-3 3.512 10-3 1.
10-13
N2
1.958 10-3 1.958
10-3 9.
10-136.472
10-4 6.472 10-4 -9
10-13
N3
6.573 10-3 6.573
10-3 3
10-13 5.724
10-3 5.724 10-3 -1
10-13
N4
7.446 10-3 7.446
10-3 -4
10-14 2.484
10-3 2.484 10-3 6.
10-13
N5
1.579 10-2 1.579
10-2 2.0
10-12 1.175
10-2 1.175 10-2 1.4
10-12
N6
6.094 10-3 6.094
10-35
10-144.458
10-3 4.458 10-3 0
N7
3.443 10-3 3.443
10-3-3

```
2.928 10-3 2.928
10-3 9
10-13 3.626
10-3 3.626 10-3 -1.1
10-12
N12
5.0848 10-3 5.0848
10-3 8.
10-13 6.947
10-3 6.947 10-3 6.
10-14
N13
9.773 10-3 9.773
10-3 4.
10-13 4.893
10-3 4.893 10-3 6.
10-13
N14
2.630 10-3 2.630
10-3 4.
10-13 1.539
10-3 1.539 10-3-1.1.

N15
\(1.71010-31.710\)
10-3-2.5
10-12 2.179
10-3 2.179 10-3 -1.6.
10-12
N16
4.150 10-3 4.150

10-3 2.
10-13 5.393
10-3 5.393 10-3 -6.
10-14
N17
7.013 10-3 7.013

10-3-5.
10-13 4.056
10-3 4.056 10-3 -1.3
10-12
N18
7.099 10-3 7.099

10-3-5.
10-13 4.313
10-3 4.313 10-3 7.
10-13
N19
9.521 10-3 9.521

10-3 2.
10-13 7.817
10-3 7.817 10-3 8.
10-14

\section*{5 \\ Summary of the results}

One checks the good adequacy of the results, the variations are close to the precision machine.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version

FETIO02 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.102-A Page:
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Organization (S): EDF-R \& D /SINETICS

Handbook of Validation
V1.04 booklet: -
Document: V1.04.102

FETI002 Validation of the basic architecture of one calculation FETI with rigid modes

\section*{Summary:}

This case-test validates the not-regression of the basic commands of a calculation FETI in linear mechanics:
manual partitioning and sequential calculation multidomaine with rigid modes of body and without Dirichlet or
loading being based on late meshs or nodes. It is a question of making sure that insertions of order DEFI_PART_OPS and key word SOLVEUR+METHODE=' FETI' in MECA_STATIQUE remains
operational within this framework.
One checks the algorithmic one thus (factorization symbolic system with the iterative resolution) and the management of structures of encapsulated data (NUME_DDL, CHAM_NO, MATR_ASSE...) specific to FETI when some
under-fields are floating.
For that one studies a calculation in linear elasticity on a plate 2D not aligned with the principal axes, subjected to a constant and not blocked pressure. The inversion of the local matrices of rigidity remains possible
fact of the addition of ad hoc mesh-points. The initial field division in four parts is completely given in order to generate homogeneous under-fields sharing a node of junction of multiplicity 4 and to control their possible rigid modes.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETI002 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.102-A Page:
2/6

1
Problem of reference
One is based here on the geometry of case-test FETIO01. It does not correspond to any real case. It does not act
here that of a functional, numerical validation and of data-processing not-regression of a calculation FETI
basic in linear mechanics: manual partitioning and sequential calculation multidomaine with rigid modes of body and without Dirichlet or loading being based on meshs or nodes late.

To make floating of the under-fields initially blocked with FETIO01, it is enough to associate elementary matrices of null rigidity at the mesh-point correspondents.

\subsection*{1.1 Geometry}

\section*{E}

\title{
Appear 1.1-a: Geometry and loading
}

Mesh-point activates (nonfloating under-field)
Mesh-points inactive (floating under-field)
The structural features are: \(A C=C D=E D=A E=5 m m A B=B C, A F=F D=F E=F C\).

\section*{Material properties}

The characteristics materials are homogeneous in all the geometry: \(E=180000\) Mpa and \(=0.3\).
1.3

Boundary conditions and loadings
Imposed normal constant pressure: *
\(P=1000 \mathrm{Mpa}\) on \([\mathrm{A}, \mathrm{B}]\)
\(P^{\prime}=2000 \mathrm{Mpa}\) on \([B, C]\)
Particular blockings of the substructures by addition of elementary matrices of rigidity to mesh-points.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETI002 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key

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3/6

2
Reference solutions

\section*{2.1}

Method of calculation used for the reference solutions
On such a case of figure, it is not possible to exhume an analytical solution. Calculations FETI are validated by comparison with the results of the multifrontale.

\section*{2.2}

Uncertainty on the solutions

\title{
Convergence of the grid and parameter of control of the test of stop of FETI (RESI_RELA).
}

\section*{Handbook of Validation}

V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1
Titrate:
FETI002 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key:V1.04.102-A Page:4/6
3 ModelingWith
3.1
Characteristics of modeling
N9
N15SD2
floating:
FETI2N1
N8
N11
```

N14

```
N16
N2
N3
N10
N7
N12
N17
SD1
blocked:
FETII
N18
N4
N5
N13
SD3
SD4
blocked:
floating:
FETI3
FETI4

N19

N6

Appear 3.1-a: Division of the square in 4 pennies fields

Mesh-point corresponding to finite elements DISCRETE credits
(modeling 2D SAY T, GROUP MA=' POIACR')

Mesh-point corresponding to finite elements DISCRETE inactive
(modeling 2D SAY T, GROUP MA=' POISCR')

Co-ordinates of some points:
\(N 1=\{5.5,5\}, N 2=\{7,3\}, N 3=\{3.5,3.5\}, N 4=\{5,1.5\}, N 5=\{1.5,2\}, N 6=\{3,0\}, N 7=\{0,4\}, N 8=\{2,5.5\}\), \(N 9=\{4,7\} \ldots\)
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETI002 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.102-A Page:
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The interface with direction FETI of the term thus relates to DDLs of the nodes: N1, N3, N4, N5, N8, N16 and
N17. These nodes are all multiplicity 2, except N3, of multiplicity 4.

The grid is composed of 24 TRIA3 and 4 SEG2 associated with usual modeling 2D_PLAN with MECHANICAL phenomenon (GROUP_MA='STRU'). One there assistant 8 discrete finite elements (modeling
2D_DIS_T, GROUP_MA=' POIACR' and "POISCR") which will be able to contribute to the matrices of rigidity
local with each under-field.
Thus the \(n^{\circ} 1\) under-fields and \(n^{\circ} 3\) become blocked and their invertible local matrices of rigidity via AFFE_CARA_ELEM on the GROUP_MA=' POIACR' which adds elementary matrices of rigidity
type:
E 0

0 E
Contrary to the \(n^{\circ} 2\) under-fields and \(n^{\circ} 4\) which remains floating because the AFFE_CARA_ELEM on
GROUP_MA=' POISCR' adds null elementary matrices of rigidity. Calculation FETI comprises thus 6 rigid modes.

Calculation FETI is sequential, it is pressed on the preconditionnor lumpé with scaling by nodal multiplicity, a total reorthogonalisation of the directions of descent via a GSM and one seek automatic rigid modes by relative test on the value of the pivots. To obtain one relative residue lower than 10-10, 12 iterations are necessary. The local solvor is the multifrontale with its default settings.

\section*{3.2}

Characteristics of the grid
19 points, 24 TRIA3, 4 SEG3.

\subsection*{3.3 Functionalities \\ tested}

Orders

\title{
MECHANICAL AFFE_MODELE D_PLAN
}

\author{
MECHANICS \\ 2D_DIS_PLAN
}

\section*{AFFE_CARA_ELEM DISCRET_2D}

DEFI_PART_OPS

\section*{MECA_STATIQUE SOLVEUR METHODE=' FETI'}
Handbook of Validation
V1.04 booklet: -HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1
Titrate:
FETI002 - Validation of the basic architecture of a calculation FETI
Date:
02/11/05
Author (S):
O. BOITEAU Key ..... :
V1.04.102-A Page: ..... 6/6
4
Results of modeling \(A\)
4.1 Values
tested
One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE=' MULT_FRONT'), and this, in any point of grid. The relative tolerance is thus severe: 1.10-8\%.

Values
Ux
Uy
tested
Multi
FETI Variation
Multi
FETI Variation
frontal
relative
frontal
relative
(in \%)
(in \%)
N1
1.235 10-2 1.235

10-2 1.
10-13 2.379
10-2 2.379 10-2 1.7
10-12
N2
5.765 10-3 5.765

10-3 3.
10-13 4.487
10-3 4.487 10-3 -1
10-14
N3
1.504 10-2 1.504

10-2 2.7
10-12 2.915
10-3 2.915 10-3 -2.4
10-12
N4
2.683 10-2 2.683

10-2 -6
10-13 2.709
```

10-2 2.709 10-2 2

```
10-13
N5
3.494 10-2 3.494
10-2 1.0
10-12 2.320
10-2 2.320 10-2 -2.6
10-12
N6
5.918 10-2 5.918
10-2 2
10-13 3.058
10-2 3.058 10-2 -9
10-13
N7
5.480 10-3 5.4809
10-3-7
10-13 2.582
10-3 2.582 10-3 1.1
10-12
N8
5.489 10-3 5.489
10-3 1.2
10-12 1.982
10-2 1.982 10-2 -4.
10-13
N9
6.718 10-3 6.718
10-3 6.
10-13 2.430
10-2 2.430 10-2 -1.5
10-12
N10
1.284 10-2 1.284
10-2 1.0
10-12 2.342
10-2 2.342 10-2 -2.3
10-12
N11
9.127 10-3 9.127
10-3 -3.
10-13 2.531
10-2 2.531 10-2 -1.0
9.921 10-3 9.921
10-3 01.854
10-2 1.854 10-2 -1.2
10-12
N13
3.387 10-2 3.387
10-2 -4.
10-13 3.000
10-2 3.000 10-2 -1.
10-13
N14
8.438 10-3 8.438
10-3 8.
10-14 1.040
10-2 1.040 10-2 -4.1
10-12
N15
8.850 10-3 8.850
10-3-7.
10-13 2.446
10-2 2.446 10-2 -5
10-13
N16
9.156 10-3 9.156
10-3-1.
10-13 2.320
10-2 2.320 10-2 -2.9
10-12
N17
2.196 10-2 2.196
10-2 1.4
10-12 2.817
10-2 2.817 10-2 5.
10-13
N18
1.364 10-2 1.364
10-2 1.9
10-12 7.520
10-3 7.520 10-3 -7.
10-13
N19

\section*{5 \\ Summary of the results}

One checks the good adequacy of the results, the variations are close to the precision machine.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
FETI003 - Validation of a calculation FETI with simple Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.103-A Page:
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Organization (S): EDF-R \& D /SINETICS

FETI003 Validation of a calculation FETI with Dirichlet simple

\section*{Summary:}

This case-test validates the not-regression of a calculation FETI in linear mechanics: manual partitioning or automatic (MONGREL partitionneurs and SCOTCH TAPE) and calculation multidomaine sequential with Dirichlet simple,
homogeneous or not, divided between under-fields or not. It is a question of making sure that insertions of
orders DEFI_PART_OPS/FETI and of key word SOLVEUR+METHODE=' FETI' in MECA_STATIQUE
remain operational within this framework.
One thus checks the validity of automatic partitionings and that of the structures of data FETI and of the algorithmic associated one which manages the conditions of simple Dirichlet.
For that one studies a calculation in linear elasticity on a plate 2D not aligned with the principal axes, subjected to a pressure constant and blocked in 3 nodes and on a mesh 1D.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{1 \\ Problem of reference}

One is based here on the geometry of case-test FETIOO1 (without mesh-point). It does not correspond to any
real case. It is here only about one functional, numerical validation and of data-processing notregression
of a calculation FETI in linear mechanics: manual partitioning or automatic (partitionneurs MONGRELS and SCOTCH TAPE) and calculation multidomaine sequential with Dirichlet simple, homogeneous or not, divided between under-fields or not.

\subsection*{1.1 Geometry}

\section*{Appear 1.1-a: Geometry and loading}

Boundary conditions of the simple Dirichlet type (DDL_IMPO)
The structural features are: \(A C=C D=E D=A E=5, A B=B C, A J=J B, A F=F D=F E=F C\), \(A G=G F=F H=H D, E I=3 I D\).

\section*{1.2 \\ Material properties}

The characteristics materials are homogeneous on all the geometry: \(E=180000\) Mpa and \(=0.3\).

\section*{1.3}

Boundary conditions and loadings
Imposed normal constant pressure:
\(P^{\prime}=2000 \mathrm{Mpa}\) on \([B, C]\).

Simple Dirichlet:
\(\boldsymbol{U} \boldsymbol{y}=-1\) on \([A, J]\),
At the point \(G\) and \(H: \boldsymbol{U x}=\mathbf{2}\) and \(\boldsymbol{U} \boldsymbol{y}=\mathbf{- 3}\),
At item I: \(\boldsymbol{U x}=\mathbf{0}\).
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
FETIOO3 - Validation of a calculation FETI with simple Dirichlet
Date:
02/1 1/05
Author (S):
O. BOITEAU Key

V1.04.103-A Page:
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2
Reference solutions

\section*{2.1 \\ Method of calculation used for the reference solutions}

On such a case of figure, it is not possible to exhume an analytical solution. Calculations FETI are validated by comparison with the results of the multifrontale.

\section*{2.2 \\ Uncertainty on the solutions}

Convergence of the grid and parameter of control of the test of stop of FETI (RESI_RELA).
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)

\section*{Version}
8.2

Titrate:
FETIO03 - Validation of a calculation FETI with simple Dirichlet

\section*{Date:}

02/11/05
Author (S):
O. BOITEAU Key

V1.04.103-A Page:
4/8

\section*{3 Modeling}

With

\author{
3.1 \\ Characteristics of modeling N9 \\ N15 \\ SD2 \\ floating: \\ FETI2 \\ NI \\ N8 \\ N11 \\ N14 \\ N16 \\ N2 \\ N3 \\ N10 \\ N7 \\ N12 \\ N17 \\ SD1 \\ blocked: \\ FETII \\ N18 \\ N4 \\ N5 \\ N13 \\ SD3
}

SD4
blocked:
floating:
FETI3
FETI4
N19
N6
Appear 3.1-a: Division of square in four under-field
Co-ordinates of some points:
\(N 1=\{5.5,5\}, N 2=\{7,3\}, N 3=\{3.5,3.5\}, N 4=\{5,1.5\}, N 5=\{1.5,2\}, N 6=\{3,0\}, N 7=\{0,4\}, N 8=\) \(\{2,5.5\}, N 9=\{4,7\} \ldots\)

The interface with direction FETI of the term thus relates to DDLs of the nodes: N1, N3, N4, N5, N8, N16 and
N17. These nodes are all multiplicity 2, except N3, of multiplicity 4.
The grid is composed of 24 TRIA3 and 4 SEG 2 associated with usual modeling 2D_PLAN with MECHANICAL phenomenon (GROUP_MA='STRU').
The nodes blocked by conditions of simple Dirichlet are: N7 and N18 (FACE_IMPO on M25), N12 and N10 (DDL_IMPO on DIRI13) and N14 (DDL_IMPO on DIRI1). The LIGREL of load of the second
must thus be cut out because it relates to two under-fields.
Calculation FETI is sequential, it is pressed on the preconditionnor lumpé with scaling by nodal multiplicity, a total reorthogonalisation of the directions of descent via a GSM and one seek automatic rigid modes by relative test on the value of the pivots. To obtain one relative residue lower than 10-12, 12 iterations are necessary. The local solvor is the multifrontale with its default settings.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
FETIO03 - Validation of a calculation FETI with simple Dirichlet

Date:
02/11/05
Author (S):

\section*{O. BOITEAU Key}

V1.04.103-A Page:

\title{
3.2 \\ Characteristics of the grid \\ 19 points, 24 TRIA3, 4 SEG3.
}

\subsection*{3.3 Functionalities}
tested
Orders

DEFI/AFFE_MATERIAU ELAS

AFFE_CHAR_MECA PRESS_REP

FACE_IMPO

DDL_IMPO

MECHANICAL AFFE_MODELE D_PLAN

DEFI_PART_OPS

\author{
MECA_STATIQUE SOLVEUR \\ METHODE=' FETI'
}

\section*{Results of modeling \(A\)}

\subsection*{4.1 Values}

\section*{tested}

One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE=' MULT_FRONT'), and this, in any point of
grid. The relative tolerance is thus severe: 1.10-10\%.

\section*{Values}
\(U x\)
Uy
tested
Multi
FETI Variation
Multi
FETI Variation
frontal
relative
frontal
relative
(in \%)
(in \%)
N1
0.7640 .764
8.

10-13-2.451-2.451 6.
10-13
N2
1.8231 .823
-5.
10-13-2.055-2.055 8.
10-14
N3
2.0102 .010
3.

10-13-3.229-3.229-5.
10-13
N4
1.8641 .864
1.1

10-12-3.142-3.142 7.
10-13
2.

10-13-2.533-2.533-8.
10-13
N6
1.3281 .328
8.

10-13-2.775-2.775 1.6.
10-12
N7
2.0632 .063
-1.3
10-12-1.
-1.
0.

N8
1.8661 .866
-1.8
10-12-3.126-3.126-2.
10-13
N9
1.0281 .028
3.9

10-12 -2.604 -2.604 1.2
10-12
N10
2.
2. 0. -3. -3. 0 .

N11
1.426 1.426
3.2

10-12 -2.824-2.824 1.3
10-12
N12
2.
2. 0. \(-3 .-3.0\).

N13
1.6321 .632
8.

10-13-2.923-2.923 1.
10-13
N14
0. O. 0 .
-2.315
-2.315
-7.
10-13
N15
1.168 1.168
-2.
10-13-2.494-2.494 7.
10-13
N16
1.8791 .879 0. -3.126
-3.126
-4.
10-14
N17
1.9271 .927
-9.
10-13 -3.123-3.123 20.
10-12
N18
1.5311 .531
-1.6
10-12-1.
-1. 1.
10-14
N19
1.4511 .451
-1.1
10-12-2.644-2.644 9.
10-13

Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
FETIO03 - Validation of a calculation FETI with simple Dirichlet

Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.103-A Page:
\(6 / 8\)

\section*{5 Modeling}

B
5.1

Characteristics of modeling
It is same modeling as with A, by replacing manual partitioning by an automatic being pressed on the MONGREL tool.

\section*{5.2}

Characteristics of the grid
19 points, 24 TRIA3, 4 SEG3.

\subsection*{5.3 Functionalities}
tested
Orders

FACE_IMPO

\section*{MECA_STATIQUE SOLVEUR \\ METHODE = ' FETI'}

\section*{6 \\ Results of modeling \(\boldsymbol{B}\)}

\subsection*{6.1 Values}
tested
One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE='MULT_FRONT'), and this, in any point of
grid. The relative tolerance is thus severe: 1.10-10\%.

\section*{Values}
\(U x\)
Uy
tested
Multi
FETI
Variation
Multi
FETI
Variation
frontal
+ MONGREL
relative
frontal
+ MONGREL
relative
(in \%)
(in \%)
N1
0.7640 .764
-7.
1.8231 .823
7.
10-13-2.055-2.055 1.
10-13
N3
2.0102 .010
3.
10-13-3.229-3.229-5.
10-13
N4
1.8641 .864
1.1
10-12-3.142-3.142 7.
10-13
N5
1.2831 .283
2.
10-13-2.533-2.533-8.
10-13
N6
1.3281 .328
8.
10-13-2.775-2.775 1.8
10-12
N7
2.0632 .063
\(-1.2\)
10-12-1.
-1 .
0 .
N8
1.8661 .866
-1.7
10-12-3.126-3.126-2.
10-13
N9
1.0281 .028
4.2
10-12-2.604-2.604 1.3
10-12
10-12-2.824-2.824 1.3
10-12
N12
2.
2. 0. -3. -3. 0 .

N13
1.6321 .632
8.

10-13-2.923-2.923 1.
10-13
N14
0. 0. 0 .
-2.315
-2.315
-7.
10-13
N15
1.168 1.168
-2.
10-13-2.494-2.494 7.
10-13
N16
1.8791 .879 0. -3.126
-3.126
-4.
10-14
N17
1.9271 .927
-9.
10-13-3.123-3.123 20.
10-12
N18
1.5311 .531
-1.6
10-12-1.
-1. 1.
10-14

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
FETIO03 - Validation of a calculation FETI with simple Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.103-A Page:
7/8

\section*{7 Modeling}

C

\section*{7.1}

Characteristics of modeling
It is same modeling as with A, by replacing manual partitioning by an automatic being pressed on the tool SCOTCH TAPE.

\section*{7.2 \\ Characteristics of the grid}

19 points, 24 TRIA3, 4 SEG3.

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

DDL_IMPO

FACE_IMPO

\section*{MECHANICAL AFFE_MODELE D_PLAN}

\section*{DEFI_PART_FETI METHODE=' SCOTCH'}

\section*{MECA_STATIQUE SOLVEUR \\ METHODE=' FETI'}

\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested
One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE=' MULT_FRONT'), and this, in any point of
grid. The relative tolerance is thus severe: 1.10-10\%.

\section*{Values}
\(U x\)
Uy
tested
Multi

\title{
FETI
}

Variation
Multi
FETI
Variation
frontal
+ MONGREL
relative
frontal
+ MONGREL
relative
(in \%)
(in \%)
N1
0.7640 .764
-7.
10-13-2.451-2.451 6.
10-13
N2
1.8231 .823
7.

10-13-2.055-2.055 1 .
10-13
N3
2.0102 .010
3.

10-13-3.229-3.229-5.
10-13
N4
1.8641 .864
1.1

10-12-3.142-3.142 7.
10-13
N5
1.2831 .283
2.

10-13-2.533-2.533-8.
10-13
N6
1.3281 .328
8.

10-13-2.775-2.775 1.8
10-12

\section*{N7}
2.0632 .063
-1.2
10-12-1.
-1.
0.

N8
1.8661 .866
-1.7
10-12-3.126-3.126-2.
10-13
N9
1.0281 .028
4.2

10-12-2.604-2.604 1.3
10-12
N10
2.
2. 0. -3. -3. 0.

N11
1.4261 .426
3.3

10-12-2.824-2.824 1.3
10-12
N12
2.
2. 0. -3. -3. 0 .

N13
1.6321 .632
8.

10-13-2.923-2.923 1.
10-13
N14
0. O. 0 .
-2.315
-2.315
-7.
10-13
N15
1.168 1.168
-2.
10-13-2.494-2.494 7.
10-13

\section*{N16}
1.8791 .879 0. -3.126
-3.126
-4.
10-14
N17
1.9271 .927
-9.
10-13 -3.123-3.123 20.
10-12
N18
1.5311 .531
-1.6
10-12-1.
-1. 1 .
10-14
N19
1.4511 .451
-1.1
10-12-2.644-2.644 9.
10-13
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
FETIOO3 - Validation of a calculation FETI with simple Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.103-A Page:
8/8

9
Summary of the results

One checks the good adequacy of the results, the variations are close to the precision machine. Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETIO04 - Validation of a calculation FETI with generalized Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.104-A Page:
1/6
Organization (S): EDF-R \& D /SINETICS

\author{
Handbook of Validation \\ V1.04 booklet: - \\ Document: V1.04.104
}

\section*{FETI004 Validation of a calculation FETI with Dirichlet generalized}

\section*{Summary:}

\section*{This case-test validates the not-regression of a calculation FETI in linear mechanics: manual partitioning and calculation}
multidomaine sequential with Dirichlet simple and generalized, homogeneous or not, divided between under
fields or not. It is a question of making sure that insertions of orders DEFI_PART_OPS and the key word
SOLVEUR + METHODE \(=^{\prime}\) FETI' in MECA_STATIQUE remain operational within this framework.
One thus checks the validity of the structures of data FETI and algorithmic associated as for the catch in count condition of generalized Dirichlet.
For that one studies a calculation in linear elasticity on a plate 2D, not aligned with the principal axes, subjected to a constant pressure, whose 3 nodes are blocked and 2 are linearly dependent.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETIO04 - Validation of a calculation FETI with generalized Dirichlet
Date:
02/11/05
Author (S):

\section*{O. BOITEAU Key}

V1.04.104-A Page:
2/6

\section*{1 \\ Problem of reference}

One is based here on the geometry of case-test FETIOO1 (without mesh-point). It does not correspond to any
real case. It is here only about one functional, numerical validation and of data-processing notregression
of a basic calculation FETI in linear mechanics: manual partitioning and calculation multidomaine sequential with Dirichlet simple homogeneous and divided between under-fields and generalized Dirichlet
inhomogenous clean with a under-field.

\subsection*{1.1 Geometry}

\section*{Appear 1.1-a: Geometry and loading}

Boundary conditions of the simple Dirichlet type (DDL_IMPO).

Boundary conditions of generalized the Dirichlet type (LIAISON_DDL).
The structural features are
\[
\therefore A C=C D=E D=A E=5 \mathrm{~mm}, A B=B C, A F=F D=F E=F C \text {, }
\]
\(A G=G F=F H=H D, E J=J F=F I=I C\).

\section*{1.2 \\ Material properties}

The characteristics materials are homogeneous in all the geometry: \(E=180000\) Mpa and \(=0.3\).

\section*{1.3 \\ Boundary conditions and loadings}

Imposed normal constant pressure:
\(P^{\prime}=2000 \mathrm{Mpa}\) on \([B, C]\)
Simple Dirichlet:
At the points G, H and I: \(\boldsymbol{U x = 0}\) and \(\boldsymbol{U} \boldsymbol{y}=\mathbf{0}\)
Generalized Dirichlet:
Between the points E and J: 3rd
U-5 11
\(U=7\)
\(X\)
y
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.1

Titrate:
FETIO04 - Validation of a calculation FETI with generalized Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key

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3/6

\section*{Reference solutions}

\section*{2.1 \\ Method of calculation used for the reference solutions}

On such a case of figure, it is not possible to exhume an analytical solution. Calculations FETI are validated by comparison with the results of the multifrontale.

\section*{2.2 \\ Uncertainty on the solutions}

Convergence of the grid and parameter of control of the test of stop of FETI (RESI_RELA).
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETIO04 - Validation of a calculation FETI with generalized Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key

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4/6

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling \\ N9}

N15
SD2
floating:
FETI2
N1
N8
N11
N14
N16
N2
N3
N10
N7
N12
N17
SD1
floating:
FETII
N18
N4
N5
N13
SD4

\title{
Appear 3.1-a: Division of square in four under-field
}

Co-ordinates of some points:
\(N 1=\{5.5,5\}, N 2=\{7,3\}, N 3=\{3.5,3.5\}, N 4=\{5,1.5\}, N 5=\{1.5,2\}, N 6=\{3,0\}, N 7=\{0,4\}, N 8=\) \(\{2,5.5\}, N 9=\{4,7\} \ldots\)

The interface with direction FETI of the term thus relates to DDLs of the nodes: N1, N3, N4, N5, N8, N16 and
N17. These nodes are all multiplicity 2, except N3, of multiplicity 4.
The grid is composed of 24 TRIA3 and 4 SEG 2 associated with usual modeling 2D_PLAN with MECHANICAL phenomenon (GROUP_MA='STRU').
The nodes blocked by conditions of simple Dirichlet are: N10, N12 and N13 (DDL_IMPO). Their
LIGREL of load must thus be cut out because it relates to three under-fields. Those blocked by one condition of Dirichlet generalized are: N9 and N11.
Calculation FETI is sequential, it is pressed on the preconditionnor lumpé with scaling by nodal multiplicity, a total reorthogonalisation of the directions of descent via a GSM and one seek automatic rigid modes by relative test on the value of the pivots. To obtain one relative residue lower than 10-12, 13 iterations are necessary. The local solvor is the multifrontale with its default settings.

\section*{Handbook of Validation}

V1.04 booklet: -
HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
FETI004 - Validation of a calculation FETI with generalized Dirichlet
Date:
02/11/05
Author (S):

\section*{O. BOITEAU Key}

V1.04.104-A Page:
5/6

\title{
3.2 \\ Characteristics of the grid \\ 19 points, 24 TRIA3, 4 SEG3.
}

\author{
3.3 Functionalities \\ tested \\ Orders \\ DEFI/AFFE_MATERIAU ELAS \\ AFFE_CHAR_MECA PRESS_REP \\ DDL_IMPO \\ LIAISON_DDL
}

MECHANICAL AFFE_MODELE D_PLAN

DEFI_PART_OPS

\author{
MECA_STATIQUE SOLVEUR \\ METHODE=' FETI'
}

Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version

Titrate:
FETIO04 - Validation of a calculation FETI with generalized Dirichlet
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.104-A Page:
6/6

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values \\ tested}

One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE=' MULT_FRONT'), and this, in any point of
grid. The relative tolerance is thus severe: 1.10-10\%.

\section*{Values}

Ux
Uy
tested
Multi
FETI Variation
Multi
FETI Variation
frontal
relative
frontal
relative
(in \%)
(in \%)
N1
\(-0.2340 .234\)
-2.1.
10-12
-0.162-0.162 3.7
10-12

\section*{N2}
1.61 10-3 1.61

10-3-2.4
10-12 2.53
10-2 2.53
10-2 -1.1
10-11
N3
9.90 10-2 9.90

10-2 1.0
10-12 -7.69
10-3 -7.69
10-3 1.0
10-11
N4
-6.04 10-3 -6.04
10-3 1.0
10-11 1.88
10-2 1.88
10-2 5.
10-14
N5
8.19 10-3 8.19

10-3 -7.8
10-12 -4.93
10-2 -4.93
10-2 4.
10-13
N6
5.82 10-2 5.82

10-2 -1.2
10-12 -6.30
10-3-6.30
10-3 1.9
10-12
N7
3.01 10-2 3.01

10-2 9.
10-13-1.73
10-1-1.73
10-1 8.
10-13
N8
0
N11
\(-0.493-0.493\)
-2.7
10-13-0.102-0.102 1.8
10-12
N12
0. 0.0
0.
0.
0
N13
O. 0.0
0.
0.
0
N14
-4.98 10-2 -4.98
10-2 -3.0
10-12 1.90
10-2 1.90
10-2 -1.4
10-11
N15
2.15 10-1 2.15
10-1-8

10-13-4.44
10-1 -4.44
10-1 2.
10-13
N16
2.66 10-1 2.66

10-1-1.4
10-12 3.81
10-2 3.81
10-2 -2.1
10-12
N17
-2.36 10-2 -2.36
10-2-1
10-13 1.72
10-2 1.72
10-2-1.2
10-12
N18
2.66 10-2 2.66

10-2 -9
10-13-8.58
10-2 -8.58
10-2-2.
10-13
N19
3.16 10-2 3.16

10-2-7
10-13-2.64
10-2-2.64
10-2 5.
10-13

\section*{5 \\ Summary of the results}

One checks the good adequacy of the results, the variations are close to the precision machine.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
FETIO05 - Validation of a calculation FETI with simple Dirichlet with the interface Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.105-A Page:
1/6
Organization (S): EDF-R \& D /SINETICS

Handbook of Validation
V1.04 booklet: -
Document: V1.04.105

FETI005 Validation of a calculation FETI with Dirichlet simple with the interface

\section*{Summary:}

This case-test validates the not-regression of a calculation FETI in linear mechanics: manual partitioning and calculation
multidomaine sequential with simple Dirichlet with the interface. It is a question of making sure that insertions of
orders DEFI_PART_OPS and of key word SOLVEUR+METHODE=' FETI' in MECA_STATIQUE remain
operational within this framework.
One thus checks the validity of the structures of data FETI and algorithmic associated as for the catch in
count conditions of Dirichlet on nodes of the interface.
For that one studies a calculation in linear elasticity on a plate 2D, not aligned with the principal axes,
subjected to a pressure constant and blocked in 4 nodes (including one on the interface) and 2 meshs
1D (including one
concerning the interface).
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
FETI005 - Validation of a calculation FETI with simple Dirichlet with the interface
Date:
02/11/05
Author (S):
O. BOITEAU Key

V1.04.105-A Page:
2/6

\section*{1}

Problem of reference
One is based here on the geometry of case-test FETI001 (without mesh-point). It does not correspond to any
real case. It is here only about one functional, numerical validation and of data-processing notregression
of a calculation FETI in linear mechanics: manual partitioning and sequential calculation multidomaine
with Dirichlet simple with the interface

\subsection*{1.1 Geometry}
```

E

```
D

\section*{With}

\section*{B}

Boundary conditions of the simple Dirichlet type (DDL_IMPO)
The structural features are:
\(A C=C D=E D=A E=5 \mathrm{~mm}, A B=B C, A F=F D=F E=F C, A G=G F=F H=H D, E I=3 I D\).
1.2

Material properties
The characteristics materials are homogeneous on all the geometry: \(E=180000\) Mpa and \(=0.3\).
1.3

Boundary conditions and loadings
Imposed normal constant pressure:
\(P^{\prime}=2000 \mathrm{Mpa}\) on \([B, C]\)
Simple Dirichlet:
\(U y=-1\) on \([A, B]\),
At the point \(G, F\) and \(H: U x=2\) and \(U y=-3\),
At item I: Ux=0.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
FETI005 - Validation of a calculation FETI with simple Dirichlet with the interface
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.105-A Page:
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\section*{2}

Reference solutions

\section*{2.1 \\ Method of calculation used for the reference solutions}

On such a case of figure, it is not possible to exhume an analytical solution. Calculations FETI are validated by comparison with the results of the multifrontale.

\section*{2.2 \\ Uncertainty on the solutions}

Convergence of the grid and parameter of control of the test of stop of FETI (RESI_RELA).
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
FETI005 - Validation of a calculation FETI with simple Dirichlet with the interface Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.105-A Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ N9}

N15
SD2
floating:
FETI2
N1
N8
N11
N14
N16
N2
N3
Appear 3.1-a: Division of square in four under-field
Co-ordinates of some points:
\(N 1=\{5.5,5\}, N 2=\{7,3\}, N 3=\{3.5,3.5\}, N 4=\{5,1.5\}, N 5=\{1.5,2\}, N 6=\{3,0\}, N 7=\{0,4\}, N 8=\{2,5.5\}\),
\(N 9=\{4,7\} \ldots\)

The interface with direction FETI of the term thus relates to DDLs of the nodes: N1, N3, N4, N5, N8, N16 and
N17. These nodes are all multiplicity 2, except N3, of multiplicity 4.
The grid is composed of 24 TRIA3 and 4 SEG2 associated with usual modeling 2D_PLAN with MECHANICAL phenomenon (GROUP_MA=' STRU').
The nodes blocked by conditions of simple Dirichlet are: N7, N18 and N5 (FACE_IMPO on M25 and M26), N12, N3 and N10 (DDL_IMPO on DIRI13) and N14 (DDL_IMPO on DIRII). LIGRELs of charge of the two first must thus be cut out because they relate to several under-fields. Calculation FETI is sequential, it is pressed on the preconditionnor lumpé with scaling by nodal multiplicity, a total reorthogonalisation of the directions of descent via a GSM and one seek automatic rigid modes by relative test on the value of the pivots. To obtain one relative residue lower than 10-12, 10 iterations are necessary. The local solvor is the multifrontale with
its default settings.
Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
FETI005 - Validation of a calculation FETI with simple Dirichlet with the interface
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.105-A Page:
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\section*{3.2}

Characteristics of the grid
19 points, 24 TRIA3, 4 SEG3.

\subsection*{3.3 Functionalities}
tested
Orders

AFFE_CHAR_MECA PRESS_REP

\author{
FACE_IMPO
}

DDL_IMPO

\section*{MECA_STATIQUE SOLVEUR \\ METHODE=' FETI'}

\section*{Handbook of Validation}

V1.04 booklet: -
HT-66/05/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version

\section*{8.2}

Titrate:
FETI005 - Validation of a calculation FETI with simple Dirichlet with the interface
Date:
02/11/05
Author (S):
O. BOITEAU Key
:
V1.04.105-A Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
One tests the data-processing not-regression of the values of the field of displacement compared to results resulting from a calculation with multifrontale (METHODE=' MULT_FRONT'), and this, in any point of
grid. The relative tolerance is thus severe: 1.10-8\%.
Values
Ux
Uy
tested
Multi
FETI Variation
```

Multi
FETI Variation
frontal
relative
frontal
relative
(in %)
(in %)
N1
0.8190.819
2.
10-13 -2.446 -2.446 1.5
10-12
N2
1.751 1.751
-2.0
10-12 -2.273-2.273 3.
10-13
N3
2.2.0.
-3.
-3.
0.
N4
1.501 1.501
1.4
10-12 -3.158 -3.158 8.
10-13
N5
1.1941.194
1.
10-13-1. -1.
0.
N6
0.301 0.301
-1.2
10-12-2.160-2.160 5.
10-13
N7
2.032 2.032
2 . 3
10-12-1.
-1.

```
```

0. 

N8
1.8351.835
-1.1
10-12 -3.071 -3.071 -8.
10-14
N9
1.059 1.059
-1.1
10-12 -2.499 -2.499 -2.2
10-12
N10
2.2.0.
-3.
-3.
0.
N11
1.445 1.445
-2.0
10-12 -2.693 -2.693 -1.1
10-12
N12
2.2.0.
-3.
-3.
0.
N13
1.296 1.296
2 . 0
10-12-2.453-2.453 1.2
10-12
N14
0.0.0.
-2.406
-2.406
-1.2
10-12
N15
1.208 1.208
-2.
10-13 -2.426 -2.426 -4.
10-13
N16

```

\subsection*{1.8551 .855}
-4.
10-13-3.008-3.008 1.0
10-12
N17
1.7851 .785
-3.
10-14-2.957-2.957-1.1
10-12
N18
1.1911 .191
2.

10-13-1. -1.
0.

N19
0.7960 .796
6.

10-13 -1.633-1.633-8.
10-13

\section*{5 \\ Summary of the results}

One checks the good adequacy of the results, the variations are close to the precision machine. Handbook of Validation
V1.04 booklet: -
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
YYYY102 Eigen frequency of a paddle of ventilator
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.102-A
Page:
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Organization (S): EDF/IMA/MMN
Handbook of Validation

\section*{V1.10 booklet: Benchmarks}

V1.10.102 document
YYYY102-Eigen frequency of a paddle
of ventilator
Summary:
This case test of calculation of Eigen frequencies results from an industrial problem. It makes it possible to be ensured of not regression of the calculation algorithm of clean modes. The values of reference result from a calculation carried out on the grid with code PERMAS.
In the preceding versions, this case test was named SDLV100.
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY102 Eigen frequency of a paddle of ventilator
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.102-A
Page:
2/6
1
Problem of reference
1.1 Geometry

B
With
\(A^{\prime}\)
C
G
K
blocked part
D
EB
\(=742.5 \mathrm{~mm}\)
E
F
EC.
\(=151.5 \mathrm{~mm}\)
ED
\(=56 \mathrm{~mm}\)
\(\mathrm{AA}^{\prime}\)
\(=10 \mathrm{~mm}\)
AK
\(=208 \mathrm{~mm}\)
BEA
\(=67.5^{\circ}\)
1.2

Material properties
\(\mathrm{E}=210.000 \mathrm{Mpa}\)
\(=0.3\)
\(=7.8106 \mathrm{~kg} / \mathrm{mm} 3\)

\section*{1.3}

\section*{Boundary conditions and loadings}

Embedding of the end of the higher veil in the hub (shaded zone).
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
YYYY102 Eigen frequency of a paddle of ventilator
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.102-A
Page:
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2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that obtained on the same grid with code PERMAS (Version 3.12) on CRAY XMP.
2.2

Results of reference
Mode
Frequency
1
0.31401786

2
0.77987974

3
2.5495422

4
2.7474513

5
3.7961008

6
6.6220969

7
10.122032

8

\title{
Numerical solution.
}

Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster ® \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY102 Eigen frequency of a paddle of ventilator
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.102-A

\section*{Page:}

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3 Modeling

\section*{With}

\section*{3.1}

\section*{Characteristics of modeling}

Algorithm of eigenvalues: simultaneous iterations on a subspace.
57 nodes blocked on the elements hexaedric with 20 nodes ( 6 elements of the end with exception of the nodes located at the interface with the remainder of the structure).

\section*{3.2}

Characteristics of the grid
A number of nodes: 841
A number of meshs and types: 96 HEXA20, 15 PENTA15.

\subsection*{3.3 Functionalities}

\section*{tested}

\section*{Orders}

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]

\author{
AFFE_MATERIAU \\ ALL \\ [U4.23.01] \\ AFFE_MODELE \\ "MECHANICAL" \\ "3D" \\ ALL \\ [U4.22.01] \\ CALC_MATR_ELEM \\ "MASS_MECA" \\ "RIGI_MECA" \\ [U4.41.01] \\ NUME_DDL \\ MATR_RIGI \\ [U4.42.01] \\ ASSE_MATRICE \\ [U4.42.02] \\ MODE_ITER_SIMULT \\ [U4.52.01] \\ Handbook of Validation \\ V1.10 booklet: Benchmarks \\ HI-75/97/002 - Ind A
}

\section*{Code_Aster ®}

Version
4.0

Titrate:
YYYY102 Eigen frequency of a paddle of ventilator
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.102-A
Page:
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4
Results of modeling A
4.1 Values
tested

\section*{Identification}

Reference
Aster
\% difference

\section*{(number of mode)}

1
0.3140
0.3140

0
2
0.7799
0.7799

0
3
2.5495
2.5496

0
4
2.7475
2.7475

0
5
3.7961
3.7961

0
6
6.6221
6.6221

0
7
10.122
10.122

0
8
12.562
12.562

0
9
18.980
18.981

0
10
21.923
21.854
0.3
4.2 Parameters
of execution

Version: 3.06
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
16 MW
Time CPU To use:
20 seconds
4.3

Time CPU to use of the last obsvervation
Values tested in conformity with the reference solution.
Order
Time To use
BEGINNING
1.0022

PRE_IDEAS
0.4370

LIRE_MAILLAGE
0.7115

AFFE_MODELE
0.0981

DEFI_MATERIAU
0.4856

AFFE_MATERIAU
0.0553

AFFE_CHAR_MECA
0.6626

CALC_MATR_ELEM
0.5441

CALC_MATR_ELEM
0.2601

NUME_DDL
0.2123

ASSE_MATRICE
0.4764

ASSE_MATRICE
0.4495

MODE_ITER_SIMULT
9.3649

IMPR_RESU
2.9920

TEST_RESU
0.1095

\section*{END}
0.0583

Total time:
19.6095

\section*{4.4}

\section*{History of the performances}

\section*{Code}

Version
Date
Memory
To use total CPU
System
PERMAS
3.12

23/01/91
?
27.61 S

Unicos 5.1
ASTER
1.03
0.7799

16 MW
21.29 S

Unicos 5.1
ASTER
2.0.8
2.5496

16 MW
26.12 S

Unicos 6.0
ASTER
2.2.18
2.7475

16 MW
19.41 S

Unicos 6.1
ASTER
3.6.0
3.7961

16 MW
19.89 S

Unicos 8.0
Handbook of Validation

V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY102 Eigen frequency of a paddle of ventilator
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.102-A
Page:
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5
Summaries of the results
One notes few modifications in the values of time CPU with the various versions of code.
A more consequent grid would be certainly more significant to measure the performances of the associated operator.
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
Page:
1/16
Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V1.10 booklet: Benchmarks

\section*{V1.10.104 document}

YYYY104 - Prismatic test-tube fissured in 3D
Summary:

This case test of elastic design results from an industrial problem. It makes it possible to compare the performances of
various solveurs established in Code_Aster, as well as the two methods of treatment of the conditions with
limits of the Dirichlet type.
The values of reference result from a calculation carried out on the grid with code PERMAS.
The test comprises 11 modelings on the same problem of reference: modeling 3D of a quarter of prismatic test-tube fissured in 4784 pentahedrons with 15 nodes and 598 hexahedrons with 20 nodes, are 16565
nodes.
In the preceding versions, this case test was named SSLV103.
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
Page:
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1
Problem of reference
1.1 Geometry
fissure
\(1 / 4\) of circle
\(\mathrm{U}=0\)
in the xy plan
Z
\(\mathrm{U}=-1 \mathrm{~mm}\)
15 mm
X
\(7,5 \mathrm{~mm}\)
With
B
X
C
```

y
D
E
Z
F
72,5 mm
G
H
U = 0
y
I
120 mm
J
in the xz plan
K
support
225 mm
725 mm
Dimension in X:
A: }127.
D: }105.0
G: }60.0
J: 15.00
(mm)
B: }123.7
E: }90.0
H: 45.00
K: 0.00
C: 120.00
F: 75.00
I: 30.00
1.2
Material properties
$\mathrm{E}=210.000 \mathrm{Mpa}$
$=0.3$
1.3

```

\section*{Boundary conditions and loadings}
```

$\mathrm{Uz}=0$
in the plan:
Z = 0
$\mathrm{Ux}=0$

```
on the segment:
\(\mathrm{X}=0\)
\(\mathrm{Y}=225 \mathrm{~mm}\)
\(\mathrm{Uy}=0\)
in the plan:
\(\mathrm{Y}=0\)
out of the crack
\(\mathrm{Ux}=-1 \mathrm{~mm}\)
on the segment:
\(\mathrm{X}=120 \mathrm{~mm}\)
\(\mathrm{Y}=725 \mathrm{~mm}\)
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster ©}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.104-A
Page:
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2
Modeling of reference
2.1

Modeling common to all the tests
With
NO13943 G NO15862
B NO13944 H NO15863
C NO15854 I NO15868
D NO15855 J NO15869
E NO15858 K NO15866
F NO15860

\subsection*{2.1.1 Grid}

A number of nodes: 16565
A number of meshs and types: 598 HEXA20, 4784 PENTA15.

\subsection*{2.1.2 Boundary conditions}
in all the nodes of the \(\mathrm{Y}=0\) plan except crack DDL_IMPO:
(GROUP_NO: Supy DY: 0.)
in all the nodes of the segment \(\mathrm{X}=0 \mathrm{~mm} \mathrm{Y}=225 \mathrm{~mm}\)
(GROUP_NO: Support DX: 0.)
in all the nodes of the \(\mathrm{Z}=0\) plan
(GROUP_NO: Supz DZ: 0.)
in all the nodes of the segment \(\mathrm{X}=120 \mathrm{~mm} \mathrm{Y}=725 \mathrm{~mm}\)
(GROUP_NO: Charge DX: -1.)

\section*{2.2}

\section*{Functionalities tested common to all modelings}

\section*{Orders}

Keys
AFFE_MODELE
[U4.22.01]
DEFI_MATERIAU
[U4.23.01]
AFFE_MATERIAU
[U4.23.02]
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
Page:
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3
Reference solution
3.1

\subsection*{3.1 Method of calculation used for the reference solution}

The reference solution is that obtained on the same grid with code PERMAS (Version 3.12) on CRAY XMP.

\section*{3.2 \\ 3.2 Result of reference: values tested Identification}

\section*{Reference}

\author{
Aster \\ \% difference \\ WITH DX \\ 0.65855 E 03 \\ 0.65855 E 03 \\ 0 \\ WITH DY \\ 0.64428 E 01 \\ 0.64428 E 01 \\ 0 \\ WITH DZ \\ 0.63926 E 02 \\ 0.63926 E 02 \\ 0 \\ B DX \\ 0.86832E03 \\ 0.86831 E 03 \\ 0 \\ B DY \\ 0.61357 E 01 \\ 0.61357 E 01 \\ 0 \\ B DZ \\ 0.62244 E 02 \\ 0.62244 E 02 \\ 0 \\ C DX \\ 0.11315 E 02 \\ 0.11315 E 02 \\ 0 \\ C DY \\ 0.58735 E 01 \\ 0.58735 E 01 \\ 0 \\ C DZ \\ 0.60645 E 02 \\ 0.60645 E 02 \\ 0 \\ D DX \\ 0.23537 E 02 \\ 0.23537 E 02 \\ 0 \\ D DY
}

\section*{H DX}
0.35835 E 02
0.35835 E 02

0
H DY
0.18953E01
0.18953 E 01

0
H DZ
0.21508 E 02
0.21508 E 02

0
I DX
0.29512 E 02
0.29512 E 02

0
I DY
0.34851 E 01
0.34851 E 01

0
J DZ
0.37679E02
0.37679 E 02

0
J DX
0.17340 E 02
0.17340 E 02

0
J DY
0.50995E01
0.50995 E 01

0
J DZ
0.49749 E 02
0.49749 E 02

0
K DX
\(0.00000 \mathrm{E}+00\)
0.30011 E 14

0
K DY
0.67226 E 01
0.67226 E 01

K DZ
0.58204 E 02
0.58204 E 02

0
C SIXX
\(0.35204 \mathrm{E}+01\)
\(0.35546 \mathrm{E}+01\)
0.97

C SIXY
\(0.18101 \mathrm{E}+01\)
\(0.17856 \mathrm{E}+01\)
1.35

C SIYY
\(0.53914 \mathrm{E}+02\)
\(0.53884 \mathrm{E}+02\)
0.06

C SIXZ
\(0.71500 \mathrm{E}+00\)
\(0.69591 \mathrm{E}+00\)
2.67

C SIYZ
\(0.10373 \mathrm{E}+01\)
\(0.10348 \mathrm{E}+01\)
+0.24
C SIZZ
\(0.10672 \mathrm{E}+01\)
\(0.11319 \mathrm{E}+01\)
+6.06
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.104-A

\section*{Page:}

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4 Modeling
With
4.1

Functionalities tested by modeling
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
MECA_STATIQUE
SOLVEUR
METHOD
"LDLT"
[U4.31.01]
RENUM
"WITHOUT"
CALC_ELEM
[U4.61.02]
4.2

Parameters of executions of the last observation
Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
50 MW
Time CPU To use:
474 seconds
4.3

Time CPU to use by order at the time of the last observation
Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.65

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU

\subsection*{0.01}

AFFE_CHAR_MECA
10.06

MECA_STATIQUE
435.84

\section*{CALC_ELEM}
5.33

TEST_RESU
0.26

TEST_RESU
0.07

END
0.06

TOTAL_JOB
473.87
4.4

History of the performances

\section*{Code}

Version
Machine/System
Date
Time CPU to use total
PERMAS
3.12

YMP 264/Unicos 5.1
23/01/91
1036
PERMAS
4.0

YMP 264/Unicos 6.0
02/07/91
1032
Code_Aster
1.03

YMP 264/Unicos 5.1
16/01/91
1069
Code_Aster
2.0.02

YMP 264/Unicos 6.0
24/07/91
1096
Code_Aster
2.00.08

YMP 264/Unicos 6.0
23/09/91
1063
Code_Aster
2.02.18

YMP 264/Unicos 6.1
09/06/92
1071
Code_Aster
2.05.24

YMP 264/Unicos 7.0
27/09/93
944
Code_Aster
3.05.24

C98/Unicos 8.0
27/11/95
474
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
Page:
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5 Modeling
B
5.1

Functionalities tested by modeling
Orders
Keys
AFFE_CHAR_CINE
MECA_IMPO
[U4.25.01]
NUME_DDL
[U4.42.01]
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]
AFFE_CHAM_NO
[U4.26.01]
CALC_CHAR_CINE
[U4.41.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM
[U4.61.02]
5.2

Parameters of executions of the last observation
Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z

Obstruction memory:
45 MW
Time CPU To use:
457 seconds
5.3

Time CPU to use by order at the time of the last observation
Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.66

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_CINE
0.33

CALC_MATR_ELEM
8.91

NUME_DDL
2.08

ASSE_MATRICE
36.15

AFFE_CHAM_NO
1.42

CALC_CHAR_CINE
0.01

FACT_LDLT
378.34

RESO_LDLT
1.02

CALC_CHAM_ELEM
5.31

TEST_RESU
0.26

TEST_RESU
0.08

END
0.04

TOTAL_JOB
456.21

Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
YYYY104 prismatic Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
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6 Modeling
C
6.1

Functionalities tested by modeling
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
MECA_STATIQUE
SOLVEUR
METHOD
"LDLT"
[U4.31.01]
RENUM
"RCMK"
CALC_ELEM
[U4.61.02]
6.2

Parameters of executions of the last observation
Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
50 MW

Time CPU To use:
588 seconds
6.3

Time CPU to use by order at the time of the last observation Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.66

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_MECA
10.08

MECA_STATIQUE
549.10

CALC_ELEM
5.34

TEST_RESU
0.25

TEST_RESU
0.07

END
0.06

TOTAL_JOB
587.17

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HI-75/97/002 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

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7 Modeling
D
7.1

Functionalities tested by modeling Orders
Keys
AFFE_CHAR_CINE
MECA_IMPO
[U4.25.01]
NUME_DDL
RENUM
"RCMK"
[U4.42.01]
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]
AFFE_CHAM_NO
[U4.26.01]
CALC_CHAR_CINE
[U4.41.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM
[U4.61.02]
7.2

Parameters of executions of the last observation
Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
45 MW
Time CPU To use:
472 seconds
7.3

Time CPU to use by order at the time of the last observation Orders

\title{
Time CPU to use
}

BEGINNING
1.00

LIRE_MAILLAGE
18.66

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_CINE
0.33

CALC_MATR_ELEM
8.89

NUME_DDL
6.41

ASSE_MATRICE
35.48

AFFE_CHAM_NO
1.41

CALC_CHAR_CINE
0.01

FACT_LDLT
390.18

RESO_LDLT
1.00

CALC_CHAM_ELEM
5.35

TEST_RESU
0.08

TEST_RESU
0.04

END
0.07

TOTAL_JOB
471.70

Handbook of Validation
V1.10 booklet: Benchmarks
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\section*{Code_Aster \({ }^{\circledR}\)}

Version

\section*{4.0}

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

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8 Modeling
E
8.1

Functionalities tested by modeling
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]
CALC_VECT_ELEM
[U4.41.02]
ASSE_VECTEUR
[U4.42.03]
FACT_GRAD
[U4.51.03]
RESO_GRAD
[U4.51.04]
CALC_CHAM_ELEM
[U4.61.02]
8.2

Parameters of executions of the last observation
Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z

Obstruction memory:
45 MW
Time CPU To use:
163 seconds
8.3

Time CPU to use by order at the time of the last observation
Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.66

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_MECA
10.06

CALC_MATR_ELEM
9.41

NUME_DDL
9.16

ASSE_MATRICE
15.04

CALC_VECT_ELEM
0.57

ASSE_VECTEUR
0.08

FACT_GRAD
5.30

RESO_GRAD
85.02

CALC_CHAM_ELEM
5.30

TEST_RESU
0.08

TEST_RESU
0.04

END
0.07

TOTAL_JOB

\subsection*{162.57}

Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster ©}

Version
4.0

Titrate:
YYYY104 prismatic Test-tube fissured in 3D

\section*{Date:}

05/01/98
Author (S):
J.P. LEFEBVRE

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9 Modeling
F
9.1

Functionalities tested by modeling
Orders
Keys
AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]
AFFE_CHAM_NO
[U4.26.01]
CALC_CHAR_CINE
[U4.41.03]
FACT_GRAD
[U4.51.03]
RESO_GRAD
CHAM_CINE
[U4.51.04]

\section*{CALC_CHAM_ELEM}
[U4.61.02]

\section*{9.2}

Parameters of executions of the last observation
Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
45 MW
Time CPU To use:
152 seconds
9.3

Time CPU to use by order at the time of the last observation
Orders
Time CPU to use
BEGINNING
0.83

LIRE_MAILLAGE
18.07

AFFE_MODELE
0.21

DEFI_MATERIAU
0.21

AFFE_MATERIAU
0.01

AFFE_CHAR_CINE
0.32

CALC_MATR_ELEM
12.77

NUME_DDL
8.85

ASSE_MATRICE
16.54

AFFE_CHAM_NO
0.61

CALC_CHAR_CINE
0.01

FACT_GRAD
5.30

RESO_GRAD
73.86

CALC_CHAM_ELEM

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

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10 Modeling
G
10.1 Functionalities tested by modeling

Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"RCMK"
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]

\section*{CALC_VECT_ELEM}

\section*{[U4.41.02]}

ASSE_VECTEUR
[U4.42.03]
FACT_GRAD
[U4.51.03]
RESO_GRAD
CHAM_NO
[U4.51.04]
CALC_CHAM_ELEM
[U4.61.02]

\subsection*{10.2 Parameters of executions of the last observation}

Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
45 MW
Time CPU To use:
150 seconds
10.3 Time CPU to use by order at the time of the last observation

Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.64

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_MECA
10.04

CALC_MATR_ELEM
9.38

NUME_DDL
13.53

ASSE_MATRICE
14.99

CALC_VECT_ELEM
0.57

\author{
ASSE_VECTEUR \\ 0.08 \\ FACT_GRAD \\ 5.35 \\ RESO_GRAD \\ 67.89 \\ CALC_CHAM_ELEM \\ 5.33 \\ TEST_RESU \\ 0.08 \\ TEST_RESU \\ 0.04 \\ END \\ 0.07 \\ TOTAL_JOB \\ 149.76 \\ Handbook of Validation \\ V1.10 booklet: Benchmarks \\ HI-75/97/002 - Ind A
}

\section*{Code_Aster ®}

Version
4.0

Titrate:
YYYY104 prismatic Test-tube fissured in 3D
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.104-A
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H
11.1 Functionalities tested by modeling

Orders
Keys
AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"

\title{
11.2 Parameters of executions of the last observation
}

\author{
Version: 3.5.24
}

Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
45 MW
Time CPU To use:
150 seconds
11.3 Time CPU to use by order at the time of the last observation

Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.67

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_CINE
0.33

CALC_MATR_ELEM

NUME_DDL
12.71

ASSE_MATRICE
16.60

AFFE_CHAM_NO
1.42

CALC_CHAR_CINE
0.01

FACT_GRAD
5.32

RESO_GRAD
76.21

CALC_CHAM_ELEM
5.31

TEST_RESU
0.08

TEST_RESU
0.04

END
0.06

TOTAL_JOB
149.47

Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster ©}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):

\section*{J.P. LEFEBVRE}

Key: V1.10.104-A
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I
12.1 Functionalities tested by modeling Orders

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"MANDELEVIUM"
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]
CALC_VECT_ELEM
[U4.41.02]
ASSE_VECTEUR
[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM
[U4.61.02]

\subsection*{12.2 Parameters of executions of the last observation}

Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
45 MW
Time CPU To use:
223 seconds
12.3 Time CPU to use by order at the time of the last observation Orders
Time CPU to use
BEGINNING
1.00

LIRE_MAILLAGE
18.65

AFFE_MODELE
0.24

DEFI_MATERIAU

AFFE_MATERIAU
0.01

AFFE_CHAR_MECA
10.07

CALC_MATR_ELEM
9.41

NUME_DDL
31.30

ASSE_MATRICE
15.08

CALC_VECT_ELEM
0.57

ASSE_VECTEUR
0.08

FACT_LDLT
128.21

RESO_LDLT
0.51

CALC_CHAM_ELEM
5.33

TEST_RESU
0.08

TEST_RESU
0.04

END
0.07

TOTAL_JOB
223.18

Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY104 prismatic Test-tube fissured in 3D
Date:
05/01/98
Author (S):

\author{
J.P. LEFEBVRE
}

Key: V1.10.104-A

\section*{Page:}

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13 Modeling
J
13.1 Functionalities tested by modeling

\section*{Orders}

\section*{Keys}

AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"MANDELEVIUM"
CALC_MATR_ELEM
[U4.41.01]
ASSE_MATRICE
[U4.42.02]
AFFE_CHAM_NO
[U4.26.01]
CALC_CHAR_CINE
[U4.41.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
CHAM_CINE
[U4.51.02]
CALC_CHAM_ELEM
[U4.61.02]

\subsection*{13.2 Parameters of executions of the last observation}

Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
45 MW
Time CPU To use:
206 seconds
13.3 Time CPU to use by order at the time of the last observation

Orders
Time CPU to use

\section*{BEGINNING}
1.00

LIRE_MAILLAGE
18.66

AFFE_MODELE
0.24

DEFI_MATERIAU
0.45

AFFE_MATERIAU
0.01

AFFE_CHAR_CINE
0.33

CALC_MATR_ELEM
8.90

NUME_DDL
30.81

ASSE_MATRICE
16.73

AFFE_CHAM_NO
1.42

CALC_CHAR_CINE
0.01

FACT_LDLT
119.12

RESO_LDLT
0.47

CALC_CHAM_ELEM
5.31

TEST_RESU
0.08

TEST_RESU
0.04

END
0.06

TOTAL_JOB
205.75

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
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K
14.1 Functionalities tested by modeling

Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
MECA_STATIQUE
SOLVEUR
METHOD
"GCPC"
[U4.31.01]
RENUM
"WITHOUT"
PRE_COND
"LDLT_INC"
JAGG_DIAG
"SCAN"
CALC_ELEM
[U4.61.02]

\subsection*{14.2 Parameters of executions of the last observation}

Version: 3.5.24
Machine: CRAY C98
System:
UNICOS 8.0 Mode Z
Obstruction memory:
50 MW
Time CPU To use:
170 seconds
14.3 Time CPU to use by order at the time of the last observation Orders
Time CPU to use
BEGINNING

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Prismatic YYYY104 Test-tube fissured in 3D
Date:
05/01/98
Author (S):
J.P. LEFEBVRE

Key: V1.10.104-A
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15
Summary of the results
15.1 Classification per time growing for the last observation

Code_Aster
3.05.24

C98/Unicos 8.0
27/11/95
Key
Solvor
Time To use
3.05.24

H
AFFE_CHAR_CINE GCPC RENUM "RCMK"
149.47

G
AFFE_CHAR_MECA GCPC RENUM "RCMK"
149.76

F
AFFE_CHAR_CINE GCPC without RENUM
151.28

E
AFFE_CHAR_MECA GCPC without RENUM 162.57

K
AFFE_CHAR_MECA GCPC - SCAN without RENUM 169.60

J
AFFE_CHAR_CINE MULT_FRONT 205.75

I
AFFE_CHAR_MECA MULT_FRONT

AFFE_CHAR_CINE LDLT without RENUM
AFFE_CHAR_CINE LDLT RENUM "RCMK"
471.70
With
AFFE_CHAR_MECA LDLT without RENUM
473.87
C
AFFE_CHAR_MECA LDLT RENUM "RCMK"
587.17
15.2 Comparison enters the two last observations
Key
Solvor
Time To use
Time To use
2.05.24
3.05.24
With
AFFE_CHAR_MECA LDLT without RENUM
943.97
473.87
B
AFFE_CHAR_CINE LDLT without RENUM
892.13
456.21
C
AFFE_CHAR_MECA LDLT RENUM "RCMK"
1006.80
587.17
D
AFFE_CHAR_CINE LDLT RENUM "RCMK"
959.50
471.70
E
AFFE_CHAR_MECA GCPC without RENUM
290.16
162.57
F
AFFE_CHAR_CINE GCPC without RENUM
257.25
AFFE_CHAR_CINE GCPC RENUM "RCMK"

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V1.10 booklet: Benchmarks
V1.10.108 document
YYYY108 - Test-tube CTJ25
Summary:

This test has two objectives:
- to ensure itself of nonthe regression of the various methods of resolution of the linear problem of elasticity
with various solveurs or methods of treatments of the boundary conditions kinematics (dualisation or elimination),
- to ensure a follow-up of the performances for a 3D problem of about 10000 ddl .

The test comprises 14 modelings on the same problem of reference: modeling 3D of a quarter of test-tube CTJ25 in 630 hexahedrons for a loading of imposed displacement. Only method of resolution is different with each modeling.

\section*{In the preceding versions this test was named SSLV101.}

Handbook of Validation
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\section*{Code_Aster ®}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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1
Problem of reference
1.1 Geometry

Of
D
\(\mathrm{E}^{\prime}\)
\(F^{\prime}\)
X
F
It
Y
B \(5^{\prime}\)
C
X
Z
B5
B0'
B4
X
B3
B2
X
\(A^{\prime}\)
B 0
B1
With
The geometry represents only one quarter of test-tube CTJ25
symmetry planes: (X B0 y) and (X B0 Z)
Thickness: \(\mathrm{DD}^{\prime}=12.5 \mathrm{~mm}\)
Face 1: (A, B0, B1, B2, B3, B4, B5, C, D, E)
Face2: (A, B0, B0', A')
Co-ordinates of the points (mm):
min
max
B0
\(F^{\prime}\)
B5'
X
-20.
42.5
0 .
30.
30.
y
0.
30.
0 .
20.25
3.5
Z
0.
12.5
0 .
12.5
12.5
1.2
Material properties
\(\mathrm{E}=2.0270271011 \mathrm{~Pa}\)
\(=0.3\)
1.3

\section*{Boundary conditions and loadings}

All nodes of the face 1:
\(\mathrm{W}=0\).
All nodes of the face2:
\(\mathrm{v}=0\).
All nodes of line \(\mathrm{FF}^{\prime}\) :
\(\mathrm{U}=0\).
\(\mathrm{v}=0.01 \mathrm{~mm}\)
Handbook of Validation
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\section*{Code_Aster ©}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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2
Modeling of reference
2.1

Modeling common to all the tests
Grid: A number of nodes: 3323 a Number of meshs: 630 HEXA20
Cutting:
Face1 (A, B1,..., B5, C, D, E)
428 nodes
Face2 (A, B0, B0', A')
198 nodes
Segment FF'
11 nodes
Name of the nodes:
Not \(\mathrm{F}^{\prime}=\) NO2958
Not B5' = NO2974
Boundary conditions:
in all the nodes of Face1 DDL_IMPO:
(GROUP_NO: Grno1 DZ: 0.)
in all the nodes of Face2

\section*{Keys}

AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
3
Reference solution

\section*{3.1}

Method of calculation used for the reference solution
The reference solution is that obtained on the same grid with code PERMAS (Version 3.12) on CRAY XMP 25/01/91.

\section*{3.2}

Results of reference: values tested

\section*{Reference}

Aster
\% Difference

\section*{Localization}
(mm)
(mm)

Not \(\mathrm{F}^{\prime} \mathrm{vF}^{\prime}\)
1. 102
1. 102
0.

W
0.
\(\mathrm{F}^{\prime}\)
1.0296104
1.0296104

Not B5' uB5'
4.3006103
4.3006103

0 .
V
0.

B5'
9.2890103
9.2890103

W
2.9173105
2.9173105

0 .
B5'
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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4 Modeling
With

\section*{4.1}

Functionalities tested by modeling
Dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Solveur LDLT by defect without renumerotation.

\section*{Orders}

Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
MECA_STATIQUE
SOLVEUR
METHOD
"LDLT"
[U4.31.01]
RENUM
"WITHOUT"

\section*{4.2}

Parameters of execution of the two last observations
Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\section*{4.3}

Time CPU to use of the two last observations

\section*{Values tested in conformity with the reference solution.}

\section*{Order}

Time To use
Time To use
3.05.24
3.02.11

BEGINNING
0.9954
0.7486

LIRE_MAILLAGE
3.0474
3.4488

AFFE_MODELE
0.0761
0.0753

DEFI_MATERIAU
0.4799
0.3783

AFFE_MATERIAU
0.0115
0.0118

AFFE_CHAR_MECA
2.2759
1.9005

MECA_STATIQUE
24.9792
25.7438

TEST_RESU
0.0459
0.0474

END
0.0537
0.0451

TOTAL_JOB 3.05.24

\title{
4.4
}

\section*{History of the performances}

\section*{Code}

Version

\section*{Date}

Memory
To use Total CPU
Remarks
PERMAS
3.12

6/02/90
47.3
without dualisation
SIV A/88
6/02/90
157.2
without dualisation
ASTER
1.00.23

6/02/90
138.7

YMP
ASTER
1.00.55

22/05/90
16 MW
139.76

YMP
ASTER
1.01.07

6/08/90
8 MW
70.27

YMP
ASTER
1.02.15

\title{
72.2
}

YMP
ASTER
2.02.02

14/04/92
8 MW
76.82

YMP
ASTER
2.02.25

17/07/92
8 MW
51.29

YMP
ASTER
2.05.25

13/10/93
16 MW
57.61

YMP
ASTER
3.02.11

11/10/94
16 MW
34.37

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
33.97

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
5/18
5 Modeling

\section*{B}
5.1

Functionalities tested by modeling
No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE).
Solveur LDLT by defect without renumerotation.

\section*{Orders}

\section*{Keys}

AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
RENUM
"WITHOUT"
[U4.42.01]
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC_CHAR_CINE
CHAR_CINE
[U4.41.03]
RESO_LDLT
[U4.51.02]
5.2

Parameters of execution of the two last observations
Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\section*{5.3}
Time CPU to use of the two last observations
Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15
BEGINNING
0.9956
0.7619
LIRE_MAILLAGE
3.0437
3.5535
AFFE_MODELE
0.0761
0.0747
DEFI_MATERIAU
0.4792
0.3838
AFFE_MATERIAU
0.0115
0.0116
AFFE_CHAR_CINE
0.0421
0.0430
CALC_MATR_ELEM
1.5854
1.5696
NUME_DDL
0.4535
0.4522
ASSE_MATRICE
4.1539
4.1030
AFFE_CHAM_NO
0.2946
0.1848
CALC_CHAR_CINE
0.0109
0.0111
FACT_LDLT
18.2386

\section*{Code}
Version
Date
Memory
To use Total CPU
Remarks
PERMAS
3.12
6/02/90
47.3
without dualisation
SIVA/88
6/02/90
157.2
without dualisation
ASTER
2.02.25
17/07/92
8 MW
47.9
YMP
ASTER
2.05.25

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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6 Modeling
C
6.1

Functionalities tested by modeling
No the dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Solveur LDLT by defect with renumerotation (Reverse Kuthil Mac Kee).

\section*{Orders}

Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
MECA_STATIQUE
SOLVEUR
METHOD
"LDLT"
[U4.31.01]
RENUM
"RCMK"
6.2
Parameters of execution of the two last observations
Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW
6.3
Time CPU to use of the two last observations
Values tested in conformity with the reference solution.OrderTime To use
Time To use3.05.24
3.02.11
BEGINNING
0.99390.7484LIRE_MAILLAGE
3.0415
3.4450
AFFE_MODELE
0.0761
0.0756
DEFI_MATERIAU
0.4794
0.3778
AFFE_MATERIAU
0.0115
0.0117
AFFE_CHAR_MECA
2.2751
1.8991
MECA_STATIQUE
24.0927
19.6053

\section*{TEST_RESU}
0.0459
0.0474

END
0.0537
0.0451

TOTAL_JOB 3.05.24
32.1968
0.5955
32.7923

TOTAL_JOB 3.02.11
27.2516
0.7576
28.0092
6.4

History of the performances

\section*{Code}

Version

\author{
Date
}

Memory
To use Total CPU
Remarks
ASTER
2.05.25

13/10/93
16 MW
45.70

YMP
ASTER
3.02.11

11/10/94
16 MW
28.00

C90
ASTER
3.05.24

27/11/95
16 MW
32.79

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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7 Modeling
D

\section*{7.1}

\section*{Functionalities tested by modeling}

No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE).
Solveur LDLT by defect with renumerotation (Reverse Kuthil Mac Kee).

\section*{Orders}

\section*{Keys}

AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
RENUM
"RCMK"
[U4.42.01]
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC_CHAR_CINE
CHAR_CINE
[U4.41.03]
RESO_LDLT
[U4.51.02]
7.2

Parameters of execution of the two last observations
Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\section*{7.3}

Time CPU to use of the two last observations
Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9952
0.7592

LIRE_MAILLAGE
3.0445
3.5527

AFFE_MODELE
0.0763
0.0750

DEFI_MATERIAU
0.4810
0.3837

AFFE_MATERIAU
0.0115
0.0116

AFFE_CHAR_CINE
0.0423
0.0429

CALC_MATR_ELEM
1.5824
1.5685

NUME_DDL
1.2654
1.2577

ASSE_MATRICE
3.7553
3.7108

AFFE_CHAM_NO
0.2948
0.1845

CALC_CHAR_CINE
0.0109
0.0111

FACT_LDLT
12.0916

\subsection*{12.0831}

RESO_LDLT
0.0754
0.0767

TEST_RESU
0.0194
0.0197

END
0.0537
0.0427

TOTAL_JOB 3.05.24
25.3171
0.9215
26.2386

TOTAL_JOB 3.02.15
25.0749
0.2375
25.3124
7.4

History of the performances
Code
Version
Date
Memory
To use Total CPU

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
42.82

YMP
ASTER
3.02.15

07/11/94
16 MW
25.31

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
26.24

\section*{C90 Mode Z}

Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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8 Modeling
E
8.1

Functionalities tested by modeling
Dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Solvor gradient combined (Solveur GCPC) without renumerotation.
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"WITHOUT"
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
8.2

Parameters of execution of the two last observations
Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98

System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\section*{8.3}

Time CPU to use of the two last observations
Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9567
0.7181

LIRE_MAILLAGE
3.0336
3.5353

AFFE_MODELE
0.0561
0.0547

DEFI_MATERIAU
0.4658
0.3724

AFFE_MATERIAU
0.0075
0.0075

AFFE_CHAR_MECA
2.2138
1.8889

CALC_MATR_ELEM
1.7262
1.7065

NUME_DDL
1.4979
1.4652

ASSE_MATRICE
2.7750
2.7599

CALC_VECT_ELEM
0.1904
0.1939

ASSE_VECTEUR
0.0287
0.0284

\section*{FACT_GRAD}
1.3066
1.3093

RESO_GRAD
5.4980
5.3648

TEST_RESU
0.0158
0.0159

END
0.0465
0.0434

TOTAL_JOB 3.05.24
21.2454
0.8778
22.1232

TOTAL_JOB 3.02.15
20.6797
0.1453
20.8250

\section*{8.4}

History of the performances

\section*{Code}

Version

\section*{Date}

Memory
To use Total CPU
Remarks
ASTER
2.08.03

14/11/94
16 MW
19.88

C90 Mode Y
ASTER
3.02.15

07/11/94
16 MW
20.82

C90 Mode Y
ASTER
3.05.24

27/11/95

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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9 Modeling
F
9.1

Functionalities tested by modeling
No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE).
Solvor gradient combined (Solveur GCPC) without renumerotation.
Orders
Keys
AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"WITHOUT"
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC_CHAR_CINE
CHAR_CINE
[U4.41.03]

\section*{RESO_GRAD}

\section*{METHOD}
"GCPC"
[U4.51.04]
9.2

Parameters of execution of the two last observations
Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW
9.3

Time CPU to use of the two last observations
Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9583
0.7197

LIRE_MAILLAGE
3.0326
3.5394

AFFE_MODELE
0.0561
0.0549

DEFI_MATERIAU
0.4655
0.3730

AFFE_MATERIAU
0.0075
0.0075

AFFE_CHAR_CINE
0.0374
0.0382

CALC_MATR_ELEM
1.5646
1.5510

NUME_DDL
1.4038
1.3754

\section*{ASSE_MATRICE}
3.1365
3.1324

AFFE_CHAM_NO
0.2908
0.1802

CALC_CHAR_CINE
0.0070
0.0072

FACT_GRAD
1.2990
1.3031

RESO_GRAD
5.1014
5.1417

TEST_RESU
0.0154
0.0154

END
0.0451
0.0418

TOTAL_JOB 3.05.24
18.8650
0.3580
19.2231

TOTAL_JOB 3.02.15
18.7115
0.1380
18.8495
9.4

History of the performances
Code
Version
Date
Memory
To use Total CPU

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
31.5553

YMP

\author{
ASTER \\ 3.02.15 \\ 07/11/94 \\ 16 MW \\ 18.85 \\ C90 Mode Y \\ ASTER \\ 3.05.24 \\ 27/11/95 \\ 16 MW \\ 19.22 \\ C90 Mode Z \\ Handbook of Validation \\ V1.10 booklet: Benchmarks \\ HI-75/97/002 - Ind A
}

Code_Aster ®
Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
10/18
10 Modeling
G
10.1 Functionalities tested by modeling

Dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Solvor gradient combined (Solveur GCPC) with renumerotation (Reverse Kuthil Mac Kee)
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"RCMK"
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
10.2 Parameters of execution of the two last observations

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW
10.3 Time CPU to use of the two last observations

Values tested in conformity with the reference solution.
Order
Time To use
Time To use

\subsection*{3.05.24}
3.02.15

BEGINNING
0.9965
0.7764

LIRE_MAILLAGE
3.0426
3.3226

AFFE_MODELE
0.0760
0.0758

DEFI_MATERIAU
0.4791
0.3788

AFFE_MATERIAU
0.0115
0.0117

AFFE_CHAR_MECA
2.2744
2.3866

CALC_MATR_ELEM
1.7402
1.7182

NUME_DDL
2.4128
2.4004

ASSE_MATRICE
2.8799
2.8937

CALC_VECT_ELEM
0.2107
0.2154

ASSE_VECTEUR
0.0326
0.0328

FACT_GRAD
1.2709
1.2873

RESO_GRAD
4.5478
4.5395

TEST_RESU
0.0198

\title{
10.4 History of the performances
}

\author{
Code
}

Version

\section*{Date}

Memory
To use Total CPU

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
31.32

YMP
ASTER
3.03.15

07/02/95
16 MW
22.58

C90 Mode Z
ASTER
3.05.24

27/11/95
16 MW
22.30

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version

\section*{4.0}

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
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11 Modeling
H

\subsection*{11.1 Functionalities tested by modeling}

No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE). Solvor gradient combined (Solveur GCPC) with renumerotation (RCMK).

\section*{Orders}

\section*{Keys}

AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"WITHOUT"
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC CHAR CINE
CHAR_CINE
[U4.41.03]
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
11.2 Parameters of execution of the two last observations

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\subsection*{11.3 Time CPU to use of the two last observations}

Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9947
0.7602

LIRE_MAILLAGE
3.0449
3.5489

AFFE_MODELE
0.0761
0.0750

DEFI_MATERIAU
0.4798
0.3836

AFFE_MATERIAU
0.0115
0.0116

AFFE_CHAR_CINE
0.0422
0.0428

CALC_MATR_ELEM
1.5818
1.5637

NUME_DDL
2.4403
2.3971

ASSE_MATRICE
3.2501
3.2363

AFFE_CHAM_NO
0.2949
0.1843

CALC_CHAR_CINE
0.0108
0.0110

FACT_GRAD
1.2575
1.2628

\section*{RESO_GRAD}
3.4183
3.4349

TEST_RESU
0.0195
0.0197

END
0.0532
0.0424

TOTAL_JOB 3.05.24
18.5089
0.8430
19.3519

TOTAL_JOB 3.02.15
18.3448
0.3324
18.6772
11.4 History of the performances

Code
Version
Date
Memory
To use Total CPU

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
28.67

YMP
ASTER
3.02.15

07/11/94
16 MW
18.68

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
19.35

C90 Mode Z
Handbook of Validation

V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
12/18
12 Modeling
I
12.1 Functionalities tested by modeling

Dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Frontal Solvor multi with renumerotation degree minimum.
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"MANDELEVIUM"
RESO_LDLT
[U4.51.02]

\subsection*{12.2 Parameters of execution of the two last observations}

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\subsection*{12.3 Time CPU to use of the two last observations}

Values tested in conformity with the reference solution.
Order
Time To use

\section*{Time To use}
3.05.24
3.02.15

BEGINNING
0.9963
0.7603

LIRE_MAILLAGE
3.0429
3.5500

AFFE_MODELE
0.0760
0.0748

DEFI_MATERIAU
0.4790
0.3836

AFFE_MATERIAU
0.0115
0.0116

AFFE_CHAR_MECA
2.2738
1.9087

CALC_MATR_ELEM
1.7427
1.7246

NUME_DDL
6.0912
6.0898

ASSE_MATRICE
2.7848
2.7653

CALC_VECT_ELEM
4.9629
5.0035

ASSE_VECTEUR
0.2109
0.2143

FACT_LDLT
0.0328
0.0327

RESO_LDLT
0.0810
0.0811

TEST_RESU

\title{
Code
}

Version
Date
Memory
To use Total CPU
Remarks
ASTER
2.05.25

13/10/93
16 MW
39.64

YMP
ASTER
3.02.15

07/11/94
16 MW
24.43

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
25.31

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
13/18
13 Modeling
J
13.1 Functionalities tested by modeling

No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE).
Frontal Solvor multi with renumerotation degree minimum.
Orders
Keys
AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"MANDELEVIUM"
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC_CHAR_CINE
CHAR_CINE
[U4.41.03]
RESO_LDLT
[U4.51.02]

\subsection*{13.2 Parameters of execution of the two last observations}

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\subsection*{13.3 Time CPU to use of the two last observations}
Values tested in conformity with the reference solution.Order
Time To use
Time To use
3.05.24
3.02.15
BEGINNING
0.9971
0.7594
LIRE_MAILLAGE
3.0496
3.5502
AFFE_MODELE
0.0764
0.0749
DEFI_MATERIAU
0.4807
0.3835
AFFE_MATERIAU
0.0115
0.0116
AFFE_CHAR_CINE
0.0423
0.0428
CALC_MATR_ELEM
1.5895
1.5661
NUME_DDL
5.9534
5.9512
ASSE_MATRICE
3.1384
3.1357
AFFE_CHAM_NO
4.4731
4.4979
CALC_CHAR_CINE
0.2958
0.1854
FACT_LDLT
0.0115
0.0117
RESO_LDLT

\subsection*{13.4 History of the performances}

\section*{Code}

Version
Date
Memory

\section*{To use Total CPU}

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
35.64

YMP
ASTER
3.02.15

07/11/94
16 MW
21.92

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
22.40

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks

\section*{Code_Aster ® \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
14/18
14 Modeling
K
14.1 Functionalities tested by modeling

Dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Solvor gradient combined (Solveur GCPC SCAN) without renumerotation.
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"WITHOUT"
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
JAGG_DIAG
"SCAN"

\subsection*{14.2 Parameters of execution of the two last observations}

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\subsection*{14.3 Time CPU to use of the two last observations}

\section*{Values tested in conformity with the reference solution.}

Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9570
0.7181

LIRE_MAILLAGE
3.0341
3.5346

AFFE_MODELE
0.0562
0.0547

DEFI_MATERIAU
0.4656
0.3724

AFFE_MATERIAU
0.0075
0.0075

AFFE_CHAR_MECA
2.2139
1.8901

CALC_MATR_ELEM
1.7216
1.7085

NUME_DDL
1.4976
1.4655

ASSE_MATRICE
2.7756
2.7673

CALC_VECT_ELEM
0.1901
0.1936

ASSE_VECTEUR
0.0287
0.0284

FACT_GRAD
1.3071
1.3092

RESO_GRAD
4.6705

TEST_RESU
0.0158
0.0159

END
0.0465
0.0435

TOTAL_JOB 3.05.24
20.3528
0.6655
21.0183

TOTAL_JOB 3.02.15
19.9953
0.1146
20.1099
14.4 History of the performances

Code
Version
Date
Memory

\section*{To use Total CPU}

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
31.61

YMP
ASTER
3.02.15

07/11/94
16 MW
20.11

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
21.02

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
15/18
15 Modeling
L
15.1 Functionalities tested by modeling

No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE).

\section*{Solvor gradient combined (Solveur GCPC SCAN) without renumerotation.}

\section*{Orders}

\section*{Keys}

AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"WITHOUT"
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC_CHAR_CINE
CHAR_CINE
[U4.41.03]
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
JAGG_DIAG
"SCAN"

\subsection*{15.2 Parameters of execution of the two last observations}

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW
15.3 Time CPU to use of the two last observations

Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9563
0.7179

LIRE_MAILLAGE
3.0335
3.5346

AFFE_MODELE
0.0562
0.0547

DEFI_MATERIAU
0.4656
0.3724

AFFE_MATERIAU
0.0075
0.0075

AFFE_CHAR_CINE
0.0374
0.0381

CALC_MATR_ELEM
1.5660
1.5469

NUME_DDL
1.4040
1.3728

ASSE_MATRICE
3.1339
3.1244

AFFE_CHAM_NO
0.2907
0.1806

\section*{CALC_CHAR_CINE}
0.0070
0.0072

FACT_GRAD
1.2969
1.3075

RESO_GRAD
4.2371
4.2871

TEST_RESU
0.0153
0.0154

END
0.0450
0.0419

TOTAL_JOB 3.05.24
17.9955
0.7874
18.7829

TOTAL_JOB 3.02.15
17.8389
0.1930
18.0319
15.4 History of the performances

Code
Version
Date
Memory
To use Total CPU
Remarks
ASTER
2.05.25

13/10/93
16 MW
27.57

YMP
ASTER
3.02.15

07/11/94
16 MW
18.03

C90 Mode Y
ASTER

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
16/18
16 Modeling
M
16.1 Functionalities tested by modeling

Dualisation of the boundary conditions kinematics (AFFE_CHAR_MECA).
Solvor gradient combined (Solveur GCPC SCAN) with renumerotation (RCMK).

\section*{Orders}

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"WITHOUT"
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
JAGG_DIAG
"SCAN"

\subsection*{16.2 Parameters of execution of the two last observations}

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW

\subsection*{16.3 Time CPU to use of the two last observations}

Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9573
0.7182

LIRE_MAILLAGE
3.0392
3.5388

AFFE_MODELE
0.0561
0.0547

DEFI_MATERIAU
0.4660
0.3727

AFFE_MATERIAU
0.0075
0.0075

AFFE_CHAR_MECA
2.2184
1.8911

CALC_MATR_ELEM
1.7240
1.7083

NUME_DDL
2.3929
2.4952

ASSE_MATRICE
2.8752
2.8713

CALC_VECT_ELEM
0.1901
0.1940
ASSE_VECTEUR
0.0286
0.0285
FACT_GRAD
1.2689
1.2684
RESO_GRAD
3.3547
2.9505
TEST_RESU
0.0158
0.0159
END
0.0466
0.0436
TOTAL_JOB 3.05.24
20.0669
0.7356
20.8025
TOTAL_JOB 3.02.15
19.3740
0.1921
19.5661
16.4 History of the performances
Code
Version
Date
Memory
To use Total CPU
Remarks
ASTER
2.05.25
13/10/93
16 MW
28.73
YMP
ASTER
3.02.15
07/11/94
16 MW
19.57
C90 Mode Y
ASTER

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
17/18
17 Modeling
NR
17.1 Functionalities tested by modeling

No the dualisation of the boundary conditions kinematics (AFFE_CHAR_CINE).
Solvor gradient combined (Solveur GCPC SCAN) with renumerotation (RCMK).

\section*{Orders}

\section*{Keys}

AFFE_CHAR_CINE
MECA_IMPO
[U4.25.05]
NUME_DDL
STORAGE
"MORSE"
[U4.42.01]
RENUM
"RCMK"
AFFE_CHAM_NO
SIZE
"DEPL_R"
[U4.26.01]
CALC_CHAR_CINE
CHAR_CINE
[U4.41.03]
RESO_GRAD
METHOD
"GCPC"
[U4.51.04]
JAGG_DIAG
"SCAN"
17.2 Parameters of execution of the two last observations

Version: 3.05.24
Date: 27/11/95
Machine: CRAY C98
System: Unicos 8.0 Mode Z
Obstruction memory: 16 MW
17.3 Time CPU to use of the two last observations

Values tested in conformity with the reference solution.
Order
Time To use
Time To use
3.05.24
3.02.15

BEGINNING
0.9555
0.7187

LIRE_MAILLAGE
3.0361
3.5396

AFFE_MODELE
0.0561
0.0547

DEFI_MATERIAU
0.4657
0.3725

AFFE_MATERIAU
0.0075
0.0075

AFFE_CHAR_CINE
0.0374
0.0382

CALC_MATR_ELEM
1.5660
1.5497

NUME_DDL
2.4135
2.3755

ASSE_MATRICE
3.2384
3.2293

AFFE_CHAM_NO
0.2906
0.1800

CALC_CHAR_CINE
0.0070
0.0072

FACT_GRAD
1.2541
1.2580

RESO_GRAD
2.6638
2.7046

TEST_RESU
0.0153
0.0154

END
0.0450
0.0418

TOTAL_JOB 3.05.24
17.4953
0.4958
17.9911

TOTAL_JOB 3.02.15
17.3232
0.1282
17.4514
17.4 History of the performances

Code
Version
Date
Memory
To use Total CPU

\section*{Remarks}

ASTER
2.05.25

13/10/93
16 MW
26.13

YMP

\section*{ASTER}
3.02.15

07/11/94
16 MW
17.45

C90 Mode Y
ASTER
3.05.24

27/11/95
16 MW
17.99

C90 Mode Z
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/97/002 - Ind A

\section*{Code_Aster ® \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY108 - Test-tube CTJ25
Date:
05/01/98
Author (S):
X. DESROCHES, J.R. LÉVESQUE

Key:
V1.10.108-D Page:
18/18
18
Summaries of the results
18.1 Classification in increasing weather

Code_Aster
3.05.24

27/11/95
16 MW
C90 Mode Z
Key
Solvor
Time To use
Time To use
Evolution
3.05.24
3.02.11/15
```

NR
AFFE_CHAR_CINE GCPC - SCAN RENUM
17.495
17.323
1,01
"RCMK"
L
AFFE_CHAR_CINE GCPC - SCAN without
17.995
17.838
1,01
RENUM
H
AFFE_CHAR_CINE GCPC RENUM "RCMK"
18.509
18.345
1,01
F
AFFE_CHAR_CINE GCPC without RENUM
18.865
1 8 . 7 1 1
1,01
M
AFFE_CHAR_MECA GCPC - SCAN RENUM
20.067
19.374
1,04
"RCMK"
K
AFFE_CHAR_MECA GCPC - SCAN without
20.353
19.995
1,02
RENUM
E
AFFE_CHAR_MECA GCPC without RENUM
21.245
20.680
1,03
G
AFFE_CHAR_MECA GCPC RENUM "RCMK"
21.582
21.559

```
AFFE_CHAR_CINE LDLT without RENUM
31.06430.946
AFFE_CHAR_MECA LDLT RENUM "RCMK"

32.197
27.252

1,18
With
AFFE_CHAR_MECA LDLT without RENUM
33.077
33.396

0,99

\subsection*{18.2 Evolutions between the two last observations}

One notes a small overall degradation ( \(<1 \%\) ) of the total performances.
By observing two modelings implementing MECA_STATIQUE with RENUM: "RCMK", one
important degradation appears:
MECA_STATIQUE
MECA_STATIQUE

\section*{Evolution}
3.02.11
3.05.24

C
AFFE_CHAR_MECA LDLT RENUM
24.09

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY110 Reactor vessel. Modal analysis
Date:
26/08/99
Author (S):
B. QUINNEZ, G. ROUSSEAU

Key:
V1.10.110-B Page:
1/6
Organization (S): EDF/IMA/MMN, EP/AMV

\section*{Handbook of Validation}

\section*{V1.10 booklet: Benchmarks}

\section*{Document: V1.10.110}

YYYY110 - Reactor vessel. Modal analysis

\section*{Summary:}

This three-dimensional problem consists in seeking modes of vibration of a reactor vessel modelled by elements of hull or beams. This test of mechanics of the structures corresponds to an analysis dynamics of an assembled structure having a linear behavior. It includes/understands two modelings (two solveurs of resolution of system linear are tested).
Via this problem, one tests the element of hull DKT, the element of beam of Timoshenko, it calculation of the clean modes by the method of Lanczos.
There are no results of reference. They were only compared with modelings by "beams equivalent " and they were in concord.
This test also makes it possible to ensure a follow-up of the performances in particular of order MODE_ITER_SIMULT
and of the methods of resolution of linear systems LDLT and MULT_FRONT.
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
YYYY110 Reactor vessel. Modal analysis
Date:
26/08/99
Author (S):

\section*{B. QUINNEZ, G. ROUSSEAU}

Key:
V1.10.110-B Page:
2/6
1
Problem of reference
1.1 Geometry
encass 1
supp_cuv
1.2

\section*{Material properties}

For the part modelled by hulls (steel):
\(\mathrm{E}=2.11011 \mathrm{~Pa}\)
\(=0.3\)
\(=7.85103 \mathrm{~kg} / \mathrm{m} 3\)
For the part modelled by beams:
\(\mathrm{E}=3.151011 \mathrm{~Pa}\)
\(=0.3\)
\(=7.85103 \mathrm{~kg} / \mathrm{m} 3\)
1.3

\section*{Boundary conditions}

The groups of nodes SUPP_CUV and ENCASS1 are embedded (three displacements and the three rotations are blocked).
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
YYYY110 Reactor vessel. Modal analysis
Date:
26/08/99
Author (S):

\section*{B. QUINNEZ, G. ROUSSEAU}

Key:
V1.10.110-B Page:
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2
Modeling of reference
2.1

Characteristics of modeling
The elements of hull are modelled by DKT and the elements of beam by POU_D_T.

\subsection*{2.2 Functionalities}
tested
Orders

\section*{Keys}

AFFE_MODELE
MODELING
"POUT_D_T"
[U4.22.01]
"DKT"
AFFE_CARA_ELEM
HULL
THICK
[U4.24.01]
BEAM
SECTION
"GENERAL"
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
2.3

Characteristics of the grid
A number of nodes: 5876
A number of meshs and types: 425 POU_D_T, 11032 DKT (TRIA3)
2.4

Boundary conditions
For the groups of nodes SUPP_CUV and ENCASS1 one has (embedding):
\(\mathrm{DX}=\mathrm{DY}=\mathrm{DZ}=0\).
\(\mathrm{DRX}=\mathrm{DRY}=\mathrm{DRZ}=0\).
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version

\section*{4.0}

Titrate:
YYYY110 Reactor vessel. Modal analysis

\section*{Date:}

26/08/99
Author (S):
B. QUINNEZ, G. ROUSSEAU

Key:
V1.10.110-B Page:
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3
Reference solution
3.1

Method of calculation used for the reference solution
The reference solution is that obtained on the same grid with Code_Aster (version 3.06.6) on CRAY C98 18/03/96.
3.2

Results of reference: values tested

\section*{Eigen frequencies}

\section*{Reference}

Aster
Difference
3.06.06

Mode 1
46.77
46.775
0.012

Mode 2
46.95
46.954
0.009

Mode 3
85.89
85.894
0.005

Mode 4
85.94
85.947
0.009

\subsection*{3.3 Remarks}

The structure being slightly nonsymmetrical, one does not obtain perfect double modes.
Handbook of Validation
V1.10 booklet: Benchmarks

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
YYYY110 Reactor vessel. Modal analysis
Date:
26/08/99
Author (S):
B. QUINNEZ, G. ROUSSEAU

Key:
V1.10.110-B Page:
5/6
4 Modeling
With
4.1

Functionalities tested by modeling
SOLVEUR LDLT by defect with renumerotation RCMK. Method of Lanczos for calculation of frequencies of vibration.
Orders
Keys
NUME_DDL
RENUM
"RCMK"
[U4.42.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"BAND"

\section*{4.2}

Parameters of execution (last observation)
Version: 4.02.09
Date: 28/10/97
Machine: CRAY C98
System:
UNICOS 8.0
Obstruction memory:
50 MW

\section*{4.3}

\section*{Time CPU to use of the last observation}

Values tested in conformity with the reference solution.

\section*{Order}
3.7.0
4.3.0

BEGINNING
0.89
1.06

LIRE_MAILLAGE
19.97
21.19

AFFE_MODELE
0.14
0.15

DEFI_MATERIAU
0.46
0.62

AFFE_MATERIAU
0.03
0.03

DEFI_VALEUR
0.03
0.04

AFFE_CARA_ELEM
16.29
15.59

AFFE_CHAR_MECA
7.53
7.51

CALC_MATR_ELEM (rigidity)
6.10
5.97

CALC_MATR_ELEM (mass)
4.81
4.59

NUME_DDL
0.87
0.86

ASSE_MATRICE (rigidity)
5.44
5.83

ASSE_MATRICE (mass)
5.32
5.67
MODE_ITER_SIMULT
311.95
311.91
TEST_RESU
0.03
0.02
END
0.07
0.06
TOTAL
381.96
383.28
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
YYYY110 Reactor vessel. Modal analysis
Date:
26/08/99
Author (S):
B. QUINNEZ, G. ROUSSEAU
Key:
V1.10.110-B Page:
6/6
5 Modeling
B
5.1
Functionalities tested by modeling
SOLVEUR MULT_FRONT by defect with renumerotation. Method of Lanczos for calculation offrequencies of vibration.
Orders
Keys
NUME_DDL
METHOD
"MULT_FRONT"
[U4.42.01]
RENUM
"MDA"

\title{
MODE_ITER_SIMULT
}

METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"BAND"
5.2

Parameters of execution (last observation)
Version: 4.02.09
Date: 28/10/97
Machine: CRAY C98
System:
UNICOS 8.0
Obstruction memory:
50 MW
5.3

Time CPU to use of the last observation
Values tested in conformity with the reference solution.
Order
4.03.00

BEGINNING
1.06

LIRE_MAILLAGE
21.18

AFFE_MODELE
0.15

DEFI_MATERIAU
0.62

AFFE_MATERIAU
0.027

DEFI_VALEUR
0.04

AFFE_CARA_ELEM
15.575

AFFE_CHAR_MECA
7.51

CALC_MATR_ELEM (rigidity)
5.98

CALC_MATR_ELEM (mass)
4.6

NUME_DDL
5.73

\title{
ASSE_MATRICE (rigidity)
}
5.48

ASSE_MATRICE (mass)

\section*{5.3}

MODE_ITER_SIMULT
42.62

TEST_RESU
0.02

END
0.06

TOTAL
118.12

6
Summary of the results
Nothing to announce for the moment.
It is noticed that the change of solvor (passage of method LDLT to the method
MULT_FRONT) made it possible to gain approximately a factor 8 on the resolution of the modal problem.
Handbook of Validation
V1.10 booklet: Benchmarks
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
ELSA01 - Nonlinear seismic analysis of piping
Date:
22/11/01
Author (S):
E. CHAMPAIN Key

V1.10.119-A Page:
1/22
Organization (S): EDF/RNE/AMV

\author{
Handbook of Validation \\ V1.10 booklet: Benchmarks \\ Document: V1.10.119
}

\section*{ELSA01-Nonlinear seismic analysis of a line of piping}

\section*{Summary:}

This test validates calculations of pipes right and bent in dynamics of the nonlinear structures (DYNA_NON_LINE).
A seismic excitation is imposed on the line. This one involves a partial plasticization ( \(1 \%\) ) elbows only. The values tested are displacements. The values of reference are displacements resulting tests ELSA realized at the ECA.
Four modelings are tested: a modeling in elements beams of Timoshenko POU_D_T with plasticity total (linear isotropic work hardening), modeling A, a modeling in elements PIPE with 3 nodes with taking into account of local plasticity (linear kinematic work hardening), modeling B, a modeling in elements PIPE with 4 nodes, modeling \(C\) and a modeling D made up of elements DKQ in the elbows and of PIPE with 3 nodes in the right parts.
Calculations carried out let appear that modeling \(P O U_{-} D_{-} T\) gives correct results for first cycles ( 0,6 to \(21 \%\) of variation compared to the values measured in displacements), is far from expensive
in computing times but takes on the other hand a long time to implement because of necessary identification of one
a significant number of parameters of the total criterion of plasticity. This modeling does not make it possible to reach
with the constraints.
Modeling PIPE with 3 nodes gives good results ( \(0,75 \%\) to \(12,67 \%\) of variation compared to measured displacements). It is more expensive in computing times than modeling POU_D_T (7435 dryness
CPU 71 dryness) but much faster to implement against since it has the advantage of not to depend on parameters to identify as a preliminary.
The use of the element PIPE to 4 nodes allows a significant profit in terms of a number of elements and thus
of ddl. For a computing time divided by 2 compared to modeling B, the results remain excellent \((3,1\) to 5\% of variation on displacements). Analysis of the test by modelings PIPE on a number of cycles more important shows than the behavior of material is represented better by a kinematic work hardening
linear that a linear isotropic work hardening.
Lastly, calculation by a mixed modeling (DKT/PIPE) is much more expensive in computing times (12874 dryness CPU) for a less precision ( \(0,6 \%\) to \(21 \%\) of variation).
Very realistic results obtained by modelings PIPE over the total duration of the test comparatively with modelings POU_D_T as their fast time of implementation show, in spite of times of calculation more important, interest of this type of modeling for the nonlinear seismic analysis of pipings.
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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}

The system considered is the line of piping ELSA used during tests of seismic characterization of piping at laboratory EMSI of the ECA. The line is a simplification of a segment of line of piping REFERENCE MARK RRA05 of diameter 6 austenitic steel " 316L. Piping forms a quadrant in
plan OYZ. It is made up of five right sections and four elbows with \(90^{\circ}\). On the right section in top of this quadrant is welded a rigid mass which simulates a valve.

\section*{Characteristics of the line:}

Tubular section of external ray Rext \(=81,775 \mathrm{~mm}\) and thickness \(e p=7,345 \mathrm{~mm}\)
Radius of curvature of the elbows: \(R c=228,6 \mathrm{~mm}\)
Characteristic of the rod:
Tubular section of external ray Rext \(=38,05 \mathrm{~mm}\) and thickness \(\mathrm{ep}=4,5 \mathrm{~mm}\)
Characteristic of the valve:
\(L x=343 \mathrm{~mm}\)
\(L y=408 \mathrm{~mm}\)
\(L z=200 \mathrm{~mm}\)
Mass: \(M=275 \mathrm{~kg}\)

\section*{1.2}

Material properties
Density of the right parts: \(=13027 \mathrm{~kg} / \mathrm{m} 3\) Poisson's ratio: \(=0,3\)
Density of the elbows: \(=14737 \mathrm{~kg} / \mathrm{m} 3\)
Density of the rod: \(=6860 \mathrm{~kg} / \mathrm{m} 3\)
Young modulus of the line: \(E=1,9.105 \mathrm{MPa}\)
Young modulus of the rod: \(E=1,81011\) Pa
Elastic limit:
\(y=242,4 M P a\)
Linear slope of work hardening: \(H=7670 \mathrm{MPa}\)
Constant of Prager: \(C=5,33109 \mathrm{MPa}\)

\section*{N.B.:}

The line being filled with water at rest, densities of the elbows and the parts right-hand sides were modified in order to take account of the mass of the fluid.
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\section*{1.3 \\ Boundary conditions, loadings and initial conditions}

Embedding in \(A\) and \(B\) and blocking of the vertical movements out of \(C\).
Pressure interns \(P=12 \mathrm{MPa}\).
Loading: seism according to identical direction OX for the 2 points of supports A and B (structure mono-supported). It corresponds to the acceleration measured on the level of the mobile plate (see accélérogramme on the following figure). Computing times over the total duration of the test being very important [bib2], calculation presented was carried out until \(T=1,1 S\) (approximately 2 periods).

The structure is initially at rest (null speeds and displacements).

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is given by experimental displacements and the results of calculations of each modeling obtained at the time of the study to test nonthe regression.

\section*{2.2}

\section*{Results of reference}

The comparison relates to displacements of the valve in the direction of the excitation for different moments. The results of various modelings are compared initially with measurements and in the second time, to test nonthe regression, with the results obtained with Code_Aster at the time of the study.

Experimental results:

\section*{Moment (S) Displacement (m)}

0,66-20,1
10-3
\(0,75+53,0\)
10-3
0,86-28,9
10-3
1,06 +67,5
10-3
Results of the calculation of modeling A to test nonthe regression:

\section*{Moment (S) Displacement (m)}

0,66-20,3
10-3
\(0,75+52,6\)
10-3
0,86-27,0
10-3
\(1,06+81,6\)
10-3
Results of the calculation of modeling \(B\) to test nonthe regression:

\section*{Moment (S) Displacement (m)}

0,66-20,2
10-3
\(0,75+52,2\)
10-3
0,86-28,5
10-3
1,06 +76,0
10-3
Results of the calculation of modeling \(C\) to test nonthe regression:

\section*{Moment (S) Displacement (m)}

0,66-20,0
10-3
\(0,75+52,0\)
10-3
0,86-29,2
10-3
\(1,06+71,8\)
10-3
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Results of the calculation of modeling \(D\) to test nonthe regression:

\section*{Moment (S) Displacement (m)}

0,66-20,4
10-3
\(0,75+52,7\)
10-3
0,86-25,0
10-3
1,06 +81,7
10-3
2.3

\section*{Uncertainties on the solution}

On the level of the precision of displacements, the report/ratio of test [bib1] made state of a precision of the order of 1/10 Misters.

\subsection*{2.4 References bibliographical}
[1]
BULAND P., CARRIES J., CANDA Mr., MAHE Mr., "seismic Tests of a section of line of piping REFERENCE MARK RRA05 ", Report ECA DMT/CEA/95/545.

CHAMPAIN E., "Project CACIP: Validation of the development relating to the taking into account of plasticity in the pipes ", Notes HP-52/99/029.
[3]
MASSIN P., PROIX J.M., "Finite elements of right pipe and curve with ovalization, swelling and warping in elastoplasticity ", Note HI-74/99/024/A.
[4]
RCC-M - Edition June 1988.
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The line is modelled in beams of Timoshenko ( \(P O U \_D \_T\) ). They are circular and hollow for pipes and full for the rod. The valve is modelled by a discrete element in translation/rotation (DIS_TR). The rod is modelled in POU_D_T like by a discrete element in translation (DIS_T) in its top.

Inertias of the bent parts are divided by the lawful factors of flexibility [bib4] so to take account of the effects of ovalization. The factors of flexibility are given by:
moy: average radius
R 2
K

Rc: radius of curvature
moy
E rmoy
\(P\) : internal pressure
E: Young modulus
Plasticity during the tests having remained localised on the level of the elbows (1\% of deformation plastic approximately), one admits that only the elements in the bent parts have a behavior elastoplastic (linear isotropic work hardening) and that the right parts have a behavior rubber band. The parameters of the total criterion of plasticity were identified on the calculation of an elbow in
monotonous inflection (while comparing with the results of reference obtained by the elements PIPE), them
values selected are:
\[
y=Z=0,84
\]
\(y=Z=4,010-3\)
Limiting normal effort: \(N p=9,081105 N R\)
Moment of the first plasticization of the elbows: \(\mathrm{Me}=8,48103\) N.m
Plastic bending moment limits elbows: \(M p=25.103\) N.m
Limiting torque: \(M p x=7,067104\) N.m
One calculates the seismic response until \(T=1,1 S\) with a step of calculation of \(10-2\) seconds. The diagram
of integration in time used is that of NEWMARK (alpha \(=0,25\) delta \(=0,5\) ). The problem incremental nonlinear is solved by the method of NEWTON for which the matrix used in the total iterations is the elastic matrix at the time of the phase of the prediction then the matrix tangent reactualized with each iteration for the following iterations.
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3.2

Characteristics of the grid

Grid of an elbow: 30 elements \(P O U \_D \_T\)
Ddl numbers: 2512

A number of nodes: 342
A number of meshs and types: 340 SEG2 including 331 for piping, 3 for the valve and 6 for the rod.
2 POIl (1 for the valve and 1 for the top of the rod)
A sight of the grid is given on the following figure.

\subsection*{3.3 Functionalities \\ tested}

Orders

\author{
AFFE_MODELE AFFE \\ MODELING \\ "POU_D_T" \\ AFFE_CARA_ELEM BEAM \\ SECTION \\ "CIRCLE" \\ DEFI_MATERIAU ECRO_LINE
}

VMIS_POUTRE

ELAS

AFFE_CHAR_MECA DDL_IMPO

LIAISON_UNIF

LIAISON_SOLIDE
MACRO_MATR_ASSE CHARGES

\section*{CALC_CHAR_SEISME MONO_APPUI}
"YES"

\author{
DYNA_NON_LINE COMP_INCR \\ RELATION \\ "VMIS_POU_LINE" \\ "ELAS" \\ RECU_FONCTION NOM_CHAM \\ DEPL
}

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Values of displacements (m) of the valve in the direction of the excitation (direction \(X\) ) to different moments.

Moment (S)
Displacement
Aster (m)

\section*{\% difference}
measured (m)
0,66-20,1
10-3-20,3
10-3 1,24
\(0,75+53,0\)
\(10-3+52,6\)
10-3-0,68
0,86-28,9
10-3-27,0
10-3-6,61
\(1,06+67,5\)
\(10-3+81,6\)
10-3 20,95
Evolution of displacements of the valve calculated and measured according to time:
0,08
0,06
reference
modeling A
0,04
winnow [m]
has
0,02
acement of \(L\)
pl
Die
0
-0,02
-0,04
0
0,2
0,4
0,6
0,8
1
Time [S]

\section*{4.2}

Parameters of execution of the last observation
Version: 5.03.18

Machine: SGI Origin 2000
Obstruction memory: 30 Mo
Time CPU To use: 71 seconds
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Code_Aster \({ }^{\circledR}\)
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22/1 1/01
Author (S):
E. CHAMPAIN Key
:
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\section*{4.3}

Time CPU to use by order at the time of the last observation

\section*{Order}

Time CPU To use
5.03.18

AT THE BEGINNING OF 0.26
LIRE_MAILLAGE 0.23
DEFI_GROUP 0.03
AFFE_MODELE 0.03
AFFE_CARA_ELEM 0.92
DEFI_MATERIAU 0.11
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.11
DEFI_MATERIAU 0.11
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.11
AFFE_MATERIAU 0.02

\section*{Handbook of Validation}

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
ELSA01 - Nonlinear seismic analysis of piping
Date:
22/11/01
Author (S):
E. CHAMPAIN Key

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\section*{5 Modeling}

\section*{B}

\section*{5.1}

\section*{Characteristics of modeling}

The line is modelled in elements PIPE in the right parts and the bent parts of piping. The rod is on the other hand always modelled in beams and the valve by a discrete element in translation/rotation.
The element PIPE used is the element with 3 nodes comprising 2 layers and 16 sectors. Three modes of Fourier allow to describe ovalization, the swelling and the warping of the sections.

As previously, it is admitted that only the elements in the bent parts have one elastoplastic behavior (kinematic work hardening) and that the right parts have one elastic behavior.

One uses a step of times of 102 S for temporal integration. The seismic answer is calculated until \(T=1,1\) S. the diagram of integration in time used is that of NEWMARK (alpha \(=0,25\) delta \(=0,5\) ). The nonlinear incremental problem is solved by the method of NEWTON for which stamp used in the total iterations is the elastic matrix at the time of the phase of the prediction then the tangent matrix reactualized with each iteration for the following iterations.

\section*{5.2 \\ Characteristics of the grid}

Grid of an elbow: 30 elements PIPE with 3 nodes
Ddl numbers: 14827
A number of nodes: 673
A number of meshs and types: 211 SEG3 (left right and bent piping) for the elements
PIPE
9 SEG2 (3 for the valve and 6 for the rod) for elements \(P O U_{-} D_{-} T\)
2 POIl (1 for the valve and 1 for the top of the rod)
A sight of the grid is given on the following figure.

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\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
AFFE_MODELE AFFE \\ MODELING \\ "PIPE" \\ "POU_D_T" \\ AFFE_CARA_ELEM BEAM
}

\author{
ORIENTATION \\ CARA \\ "GENE_TUYAU" \\ DEFI_MATERIAU ECRO_LINE \\ PRAGER \\ AFFE_CHAR_MECA DDL_IMPO \\ LIAISON_UNIF \\ LIAISON_SOLIDE \\ FORCE_TUYAU \\ MACRO_MATR_ASSE CHARGES
}

CALC_CHAR_SEISME MONO_APPUI
"YES"
AFFE_CHAR_MECA VECT_ASSE

\title{
DYNA_NON_LINE COMP_INCR RELATION
}
"VMIS_ECMI_LINE"
"ELAS"
TUYAU_NCOU

\section*{TUYAU_NSEC}

\section*{RECU_FONCTION NOM_CHAM}

DEPL
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\section*{Titrate:}

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Author (S):
E. CHAMPAIN Key

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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested
Values of displacements (m) of the valve in the direction of the excitation (direction \(X\) ) to different moments.

\section*{Moment (S)}

\author{
Displacement \\ Aster (m) \\ \% difference \\ measured (m) \\ 0,66-20,1 \\ 10-3-20,2 \\ 10-3 0,75 \\ \(0,75+53,0\) \\ \(10-3+52,2\) \\ 10-3-1,55 \\ 0,86-28,9 \\ 10-3-28,5 \\ 10-3-1,25 \\ \(1,06+67,5\) \\ \(10-3+76,0\) \\ 10-3 12,67
}

Evolution of displacements of the valve calculated and measured according to time:
0,08
0,06
reference
modeling \(B\)
0,04
winnow [m]
has
0,02
acement of \(L\)
pl
Die
0
-0,02
-0,04
0
0,2
0,4
0,6
0,8
1
Time [S]

\section*{6.2}

Parameters of execution of the last observation

Version: 5.03.18
Machine: SGI Origin 2000
Obstruction memory: 300 Mo
Time CPU To use: 7436 seconds
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Author (S):
E. CHAMPAIN Key
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\section*{6.3}

Time CPU to use by order at the time of the last observation

\author{
Order
}

Time CPU To use
5.03.18

AT THE BEGINNING OF 0.27
LIRE_MAILLAGE 0.29
DEFI_GROUP 0.03
AFFE_MODELE 0.03
AFFE_CARA_ELEM 1.06
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.11
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.11

DEFI_MATERIAU 0.12
AFFE_MATERIAU 0.02
AFFE_CHAR_MECA 0.96
AFFE_CHAR_MECA 0.04
DEFI_FONCTION 0.00
DEFI_FONCTION 0.00
CALC_MATR_ELEM 18.28
NUME_DDL 1.51
ASSE_MATRICE 0.42
CALC_MATR_ELEM 108.15
ASSE_MATRICE 0.46
CALC_CHAR_SEISME 0.08
AFFE_CHAR_MECA 0.02
DEFI_VALEUR 0.00
DEFI_VALEUR 0.00
DEFI_LIST_REEL 0.01
DYNA_NON_LINE 7300.69
RECU_FONCTION 0.12
TEST_FONCTION 0.01
TEST_FONCTION 0.01
AT THE END OF 0.50
TOTAL_JOB 7435.77
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Author (S):
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:
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\section*{7 Modeling}

\section*{C}

\section*{7.1}

\section*{Characteristics of modeling}

The line is modelled in elements PIPE with 4 nodes in the right parts and the bent parts piping. The rod is on the other hand always modelled in beams and the valve by an element discrete in translation/rotation.
The element PIPE used is the element comprising 2 layers and 16 sectors. Three modes of Fourier allow to describe ovalization, the swelling and the warping of the sections.
As previously, it is admitted that only the elements in the bent parts have one elastoplastic behavior (kinematic work hardening) and that the right parts have one elastic behavior.

One uses a step of times of 102 S for temporal integration. The seismic answer is calculated until \(T=1,1 S\). the diagram of integration in time used is that of NEWMARK (alpha \(=0,25\) delta \(=0,5\) ). The nonlinear incremental problem is solved by the method of NEWTON for which stamp used in the total iterations is the elastic matrix at the time of the phase of the prediction then the tangent matrix reactualized with each iteration for the following iterations.

\section*{7.2}

\section*{Characteristics of the grid}

Grid of an elbow: 5 elements PIPE with 4 nodes
Ddl numbers: 5026
A number of nodes: 161
A number of meshs and types: 75 SEG4 (left right and bent piping) for the elements

\section*{PIPE}

9 SEG2 (3 for the valve and 6 for the rod) for elements \(P O U_{-} D_{-} T\)
2 POII (1 for the valve and 1 for the top of the rod)
A sight of the grid is given on the following figure.

\section*{Handbook of Validation}

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\section*{:}

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\subsection*{7.3 Functionalities}
tested
Orders

CREA_MAILLAGE MODI_MAILLE OPTION
"SEG3_4"
AFFE_MODELE AFFE
MODELING
"PIPE"
"POU_D_T"
AFFE_CARA_ELEM BEAM

ORIENTATION
CARA
"GENE_TUYAU"
DEFI_MATERIAU ECRO_LINE

\section*{PRAGER}

AFFE_CHAR_MECA DDL_IMPO

LIAISON_UNIF

LIAISON_SOLIDE

\author{
Handbook of Validation
}

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E. CHAMPAIN Key

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\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested
Values of displacements (m) of the valve in the direction of the excitation (direction \(X\) ) to different moments.

\section*{Moment (S)}

Displacement
Aster (m)
\% difference
measured (m)
0,66-20,1
10-3-20,0
10-3-0,4
\(0,75+53,0\)
\(10-3+52,0\)
10-3-1,9
0,86-28,9
10-3-29,2
10-3 1,2
\(1,06+67,5\)
\(10-3+71,8\)
10-3 6,4
Evolution of displacements of the valve calculated and measured according to time:

\section*{8.2 \\ Parameters of execution of the last observation}

Version: 5.04.01

Machine: SGI Origin 2000
Obstruction memory: 300 Mo
Time CPU To use: 3387 seconds
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Version
5.0

Titrate:
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\section*{Date:}

22/11/01
Author (S):
E. CHAMPAIN Key

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\section*{8.3}

Time CPU to use by order at the time of the last observation
Order
Time CPU To use
5.04.01

AT THE BEGINNING OF 0.37
LIRE_MAILLAGE 0.11
DEFI_GROUP 0.03
CREA_MAILLAGE 0.05
AFFE_MODELE 0.03
AFFE_CARA_ELEM 0.34
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.13
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.12
DEFI_MATERIAU 0.12
AFFE_MATERIAU 0.02
AFFE_CHAR_MECA 0.31
AFFE_CHAR_MECA 0.03
DEFI_FONCTION 0.01
DEFI_FONCTION 0.01
CALC_MATR_ELEM 10.62
NUME_DDL 0.47
ASSE_MATRICE 0.18
CALC_MATR_ELEM 62.11
ASSE_MATRICE 0.16
CALC_CHAR_SEISME 0.04
AFFE_CHAR_MECA 0.02
DEFI_VALEUR 0.00
DEFI_VALEUR 0.00
DEFI_LIST_REEL 0.00
DYNA_NON_LINE 3305.68

\title{
Handbook of Validation
}

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E. CHAMPAIN Key

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\section*{9 Modeling}

D

\section*{9.1}

\section*{Characteristics of modeling}

The line is modelled in elements PIPE with 3 nodes in the right parts and in elements DKQ in the bent parts of piping. The elements pipes make it possible not to model them right parts contiguous to the elbows in elements DKQ. The rod is on the other hand always modelled in beams and the valve by a discrete element in translation/rotation.
The element PIPE used is the element comprising 2 layers and 16 sectors. Three modes of Fourier allow to describe ovalization, the swelling and the warping of the sections.
As previously, it is admitted that only the elements in the bent parts have one elastoplastic behavior (kinematic work hardening) and that the right parts have one elastic behavior.

The pressure applied in the elbows is modified to hold account owing to the fact that in the hulls pressure is applied to the average surface of the element in not in skin interns ( \(P=110,2\) bars with place of 120 bars).
One uses a step of times of 102 S for temporal integration. The seismic answer is calculated until \(T=1,1 S\). the diagram of integration in time used is that of NEWMARK (alpha \(=0,25\) delta \(=0,5\) ). The nonlinear incremental problem is solved by the method of NEWTON for which stamp used in the total iterations is the elastic matrix at the time of the phase of the prediction then the tangent matrix reactualized with each iteration for the following iterations.

\section*{9.2}

\section*{Characteristics of the grid}

Grid of an elbow: 1200 elements DKQ (50 in the section and 24 in the curve)
Ddl numbers: 39027
A number of nodes: 5437
A number of meshs and types: 4800 DKQ (left bent piping)
75 SEG3 (left right piping) for the elements PIPE
9 SEG2 (3 for the valve and 6 for the rod) for elements \(P O U_{-} D_{-} T\)
2 POII (1 for the valve and 1 for the top of the rod)
A sight of the grid is given on the following figure.

\section*{Handbook of Validation}

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\section*{Titrate:}

ELSA01 - Nonlinear seismic analysis of piping
Date:
22/11/01
Author (S):
E. CHAMPAIN Key

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\subsection*{9.3 Functionalities}
tested

Orders

\author{
AFFE_MODELE AFFE MODELING
}

\section*{"DKT"}
"POU_D_T"
AFFE_CARA_ELEM BEAM

ORIENTATION
CARA
"GENE_TUYAU"
HULL

DEFI_MATERIAU ECRO_LINE

PRAGER

AFFE_CHAR_MECA DDL_IMPO

LIAISON_UNIF

LIAISON_SOLIDE

FORCE_COQUE

FORCE_TUYAU

MACRO_MATR_ASSE CHARGES

\title{
DYNA_NON_LINE COMP_INCR RELATION
}
"VMIS_ECMI_LINE"
"ELAS"
COQUE_NCOU

TUYAU_NCOU

TUYAU_NSEC
RECU_FONCTION NOM_CHAM
DEPL
Handbook of Validation
V1.10 booklet: Benchmarks
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested
Values of displacements ( \(m\) ) of the valve in the direction of the excitation (direction \(X\) ) to different moments.

\section*{Moment (S)}

\section*{Displacement}

Aster (m)
\% difference
measured (m)
0,66-20,1
10-3-20,4
10-3 1,5
0,75 +53,0
\(10-3+52,7\)
10-3 -0,6
0,86-28,9
10-3-25,0
10-3 -13,4
\(1,06+67,5\)
\(10-3+81,7\)
10-3 21,0
Evolution of displacements of the valve calculated and measured according to time:
```

0,1
0,08
reference
modeling D
0,06
0,04
winnow [m]
has
0,02
acement of L
pl
Die
O
-0,02
-0,04
0
0,2
0,4

```

\subsection*{10.2 Parameters of execution of the last observation}

Version: 5.05
Machine: SGI Origin 2000
Obstruction memory: 900 Mo
Time CPU To use: 12314 seconds
Handbook of Validation
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10.3 Time CPU to use by order at the time of the last observation Order
Time CPU To use
5.05
at THE BEGINNING OF 0.31
LIRE_MAILLAGE 4.29
DEFI_GROUP 0.09
AFFE_MODELE 0.07
AFFE_CARA_ELEM 2.75

DEFI_MATERIAU 0.13
DEFI_MATERIAU 0.14
DEFI_MATERIAU 0.13
DEFI_MATERIAU 0.13
DEFI_MATERIAU 0.13
DEFI_MATERIAU 0.13
AFFE_MATERIAU 0.02
AFFE_CHAR_MECA 29.61
AFFE_CHAR_MECA 3.18
AFFE_CHAR_MECA 0.06
DEFI_FONCTION 0.00
DEFI_FONCTION 0.01
CALC_MATR_ELEM 13.22
NUME_DDL 30.14
ASSE_MATRICE 2.26
CALC_MATR_ELEM 69.66
ASSE_MATRICE 1.26
CALC_CHAR_SEISME 0.33
AFFE_CHAR_MECA 0.02
DEFI_VALEUR 0.00
DEFI_VALEUR 0.00
DEFI_LIST_REEL 0.00
DYNA_NON_LINE 12144.95
RECU_FONCTION 0.38
TEST_FONCTION 0.01
TEST_FONCTION 0.01
DEFI_FONCTION 0.03
DEFI_FONCTION 0.04
DEFI_FONCTION 0.03
DEFI_FONCTION 0.03
DEFI_FONCTION 0.03
DEFI_FONCTION 0.03
DEFI_FONCTION 0.04
DEFI_FONCTION 0.03
DEFI_FONCTION 0.03
DEFI_FONCTION 0.04
DEFI_FONCTION 0.03
DEFI_FONCTION 0.03
DEFI_FONCTION 0.04
DEFI_FONCTION 0.03
DEFI_FONCTION 0.03
IMPR_COURBE 0.23
AT THE END OF 0.88

\title{
TOTAL_JOB 12314.26
}

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11 Summary of the results
Modeling A, starting from POU_D_T, gives correct results (0,6 to 21\% of variation by report/ratio with the measured values) and is far from expensive in computing times ( 71 dryness CPU). It requires, by
against, a rather important time of modeling since the criterion of total plasticity depends on one certain number of parameters (coefficients numerical of the criterion of plasticity, limiting moments, factors of flexibility) which must be identified. Moreover, calculation over one longer duration shows that the linear isotropic work hardening of the beams is not representative of the behavior of material under seismic loading (the use of a kinematic work hardening in total plasticity redévelopper a new element beam would require). Lastly, one cannot reach the fields of constraints.

Modeling B, containing elements PIPE with 3 nodes with linear kinematic work hardening, give good results \((0,75 \%\) to \(12,7 \%\) of variation compared to measured displacements). Because of numbers more important of degrees of freedom ( 14827 ddl against 2512 for modeling \(A\) ), it is more expensive in computing times than modeling POU_D_T ( 7435 dryness CPU against 71 dryness). However, its implementation is much faster to since it has the advantage of not to depend on parameters to identify as a preliminary.

The use of the element PIPE to 4 nodes in modeling C allows a significant profit in terms of a number of elements and thus of ddl ( 5026 ddl\()\). For a computing time divided by 2 by report/ratio with modeling B, the results are excellent (0,4 to 6,4\% of variation). Analysis of the test by

\footnotetext{
file:///Z|/process/valid/p430.htm (11 of 16)9/28/2006 4:30:50 PM
}
modelings PIPE on a more significant number of cycles shows than the behavior of material is represented better by a linear kinematic work hardening than an isotropic work hardening linear.

Modeling D, made up of elements DKT in the elbows and PIPE with 3 nodes in right parts is much more expensive in computing times than preceding modelings ( 12314 dryness CPU). It gives results correct but all the same less precise than the 3 others modelings ( \(0,6 \%\) to \(21 \%\) of variation compared to measured displacements).

Very realistic results obtained by modelings PIPE over the total duration of the test compared to modelings POU_D_T like their fast time of implementation show, in spite of the times computings more important, interest of this type of modeling for nonlinear seismic analysis of pipings.

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Version
4.0

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SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):
B. QUINNEZ

Key:
V2.01.002-E Page:
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Organization (S): EDF/IMA/MMN
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
Document: V2.01.002
SDLD02 - System mass-arises with 8 degrees
of freedom

\section*{Summary:}

This two-dimensional problem consists in seeking the frequencies and the modes of vibration of a structure
mechanics made up of masses and springs. This case-test of Mechanics of the Structures corresponds to one
analyze dynamic of a discrete model having a linear behavior.
This test allows a complete validation of the options of modeling of discrete rigidity and mass (without

\footnotetext{
file:///Z|/process/valid/p430.htm (12 of 16)9/28/2006 4:30:50 PM
}
finite elements) offered by order AFFE_CARA_ELEM [U4.24.01]. Four different modelings are proposed: two modelings for the discrete elements in translation and two others for the elements discrete in translation/rotation. In addition, various functionalities of orders MODE_ITER_INV [U4.52.01] (calculation of values and clean vectors per iteration reverses), MODE_ITER_SIMULT [U4.52.02]
(calculation of the values and vectors clean by the method of Lanczos) and NORME_MODE [U4.64.02] (definition of
normalizes of a clean vector) are tested.
This test refers to a test VPCS, but it was modified. Indeed, the Code_Aster test directs the system mechanics on an axis \(3 y=4 x\), which makes it possible to validate the entry of the data in local reference mark.

\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A
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1
Problem of reference
1.1 Geometry
\(U\)
\(U\)
1
2

\section*{m}

B
\(\boldsymbol{X}, \boldsymbol{U}\)
\(K\)
\(K\)
\(K\)

\section*{P1}
\(P 2\)
P3
P8
Specific masses: \(m P=m=m=\ldots \ldots=m=m\)
        1

P2
P3

\section*{P8}

Stiffnesses of connection: \(k A P 1=k P 1 P 2=k P 2 P 3=\ldots \ldots=k P 8 B=K\)

\section*{1.2}

Material properties
Comes out from linear elastic translation
\(K=105 \mathrm{~N} / \mathrm{m}\)
Specific mass
\(m=10 \mathrm{~kg}\)
1.3

Boundary conditions and loadings
Embedded points \(A\) and \(b:(U=0)\).
1.4 Conditions
initial
Without object for the modal analysis.
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card SDLD02/89 of the guide VPCS which presents method of calculation in the following way:
The problem led to seek the eigenvalues and clean vectors of:
(Km
I)
\(I=0\)
K
\(K\)
0

\section*{K 2k K}
m
\(K=\)
\(M=\)

K
\(2 k\)
K
m
\(K\)

\section*{0}
from where:
1
K
\(N+1-I\)
\(\boldsymbol{F}=\)
I
cos
m
(n+) 12
\(I=1,2, \ldots \ldots, N\)
\(N=a\) number of masses
Ti calculated by resolution of the linear system.
2.2

Results of reference
the first 8 Eigen frequencies and the first and eighth clean vectors normalized such as:
Test V.P.C.S. provides modes normalized to \(M=10\). Normalized modes are presented:
\(T\)
with the unit generalized mass: \(M=1\); the components of reference are divided by 10,
with the generalized stiffness what amounts dividing the preceding components by I, - with the largest component of displacement.
2.3

Uncertainty on the solution
Analytical solution.
2.4 References
bibliographical
[1]
Mr. LALANNE, P. BERTHIER, J. DERHAGOPIAN. Mechanics of the linear vibrations. Paris:
MASSON, \(2^{\circ}\) edition, chapter 3, p. 100-101 (1986)
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3 Modeling
With
3.1

Characteristics of modeling
Discrete element of rigidity in translation DIS_T
Y
0.4
0.3

P
P
P
1
P
3
P
6
P
X
2
4
P5
7
P8
X
Characteristics of the elements:
ORIENTATION:
in all the nodes
with an angle \(=53.130102^{\circ}\)
DISCRETE:
with nodal masses all nodes
M_T_D_N
in absolute reference mark
( \(\mathrm{m}=10\).)
matrices of rigidity all meshs
K_T_D_L
in local reference mark
( \(\mathrm{Kx}=1.105\) )
with the nodes ends
K_T_D_N
in local reference mark
( \(\mathrm{Kx}=1.105\) )
Limiting conditions:
DDL_IMPO: (ALL: "YES" DZ: 0. )
LIAISON_DDL: (such as 3Dy=4Dx in all the nodes)
Names of the nodes: P1, P2,...., P8
Not A = N1
= N 2
3.2

Characteristics of the grid
A number of nodes: 8
A number of meshs and types: 7 SEG2

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_T_D_L"
[U4.24.01]
NODE
"K_T_D_N"
"M_T_D_N"
ORIENTATION
GROUP_NO
"ANG_NAUT"
AFFE_CHAR_MECA
DDL_IMPO
ALL
[U4.25.01]
LIAISON_DDL
NODE
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
ALL
"MECHANICAL"
"DIS_T"
[U4.22.01]
GROUP_NO
"DIS_T"
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_INV
CALC_FREQ
OPTION
"ADJUSTS"
[U4.52.01]
FREQ
CALC_MODE
OPTION
"DIRECT"
RECU_CHAMP
"DEPL"
NUME_ORDRE
[U4.62.01]
FREQ
NORM_MODE
"MASS_GENE"
[U4.64.02]
NORM_MODE
"RIGI_GENE"
[U4.64.02]
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
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Date:
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4
Results of modeling A
4.1 Values
tested

\section*{Identification}

Reference
Aster
\% Difference
tolerance
Number of the mode
valley. relative
clean
1
5.5274
5.5274

0 .
104
2
10.8868
10.8868

3
15.9155
15.9155

4
20.4606
20.4606

5
24.3840
24.3840

6
27.5664

\section*{Normalized mode with 1 with the largest component}

Nature of the clean mode

\author{
Not
}

\section*{Reference}

\section*{Aster}

P1
0.3473
0.34729

P2
0.6527
0.65270

Translation 1
P3
0.8793
0.87938
(Dy)
P4
-1.
-1.
P5
-1.
-1.
1
P6
0.8793
0.87938

P7
0.6527
0.65270

P8
0.3473
0.34729

P1
0.3473
0.34729

P2
0.6527
0.65270

Translation 8

\section*{P3}
0.8793
0.87938
(Dy)
P4
-1.
-1.
P5
1.
1.

8
P6
0.8793
0.87938

P7
0.6527
0.65270

\section*{P8}
0.3473
0.34729

Maximum error lower than: \(0.03 \%\).
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Titrate:
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Date:
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\section*{Author (S):}

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
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Mode normalized with the unit generalized mass
Nature of the clean mode
Not
Reference
Aster
P1
4.0781E2
4.0788E2

P2
7.6654E2
7.6657E2

Translation 1
P3
1.0327 E 1
1.0327 E 1
(Dy)
P4
1.1743E1
1.1744 E 1

\section*{P5}
1.1743E1
1.1744 E 1

1
P6
1.0327 E 1
1.0327 E 1

P7
7.6654E2
7.6657E2

P8
4.0781E2
4.0788E2

P1
4.0781E2
4.0788 E 2

P2
7.6654E2

\subsection*{7.6657E2}

Translation 8
P3
1.0327 E 1
1.0328 E 1
(Dy)
P4
1.1743E1
1.1744 E 1

P5
1.1743E1
1.1744E1

8
P6
1.0327 E 1
1.0328E1

P7
7.6654E2
7.6657E2

P8
4.0781 E 2
4.0788 E 2

Maximum error lower than: 0.03\%.
Mode normalized with the unit generalized stiffness
Nature of the clean mode
Not
Reference
Aster
P1
1.1742 E 3
1.1744 E 3

P2
2.2072E3
2.2072E3

Translation 1
P3
2.9735E3
2.9738E3
(Dy)
P4
3.3813E3
3.3817E3
```

P5
3.3813E3
3.3817E3
1
P6
2.9735E3
2.9738E3
P7
2.2072E3
2.2072E3
P8
1.1742E3
1.1744E3
P1
2.0705E4
2.0709E4
P2
3.8918E4
3.8920E4
Translation }
P3
5.2432E4
5.2436E4
(Dy)
P4
5.9621E4
5.9628E4
P5
5.9621E4
5.9628E4
8
P6
5.2432E4
5.2436E4
P7
3.8918E4
3.8920E4
P8
2.0705E4
2.0709E4
Maximum error lower than: 0.03%.

```

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\subsection*{4.2 Remarks}

Calculations carried out by:
MODE_ITER_INV
OPTION: "ADJUSTS"
LIST_FREQ: (5. , 10., 15. , 20. , 24. , 27. , 30. , 32.)
CALC_MODE: (OPTION: "DIRECT")

\section*{Contents of the file results:}
the first 8 Eigen frequencies, clean vectors and modal parameters

\subsection*{4.3 Parameters}

\section*{of execution}

Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
6 seconds
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\section*{5 Modeling}

\section*{B}
5.1

Characteristics of modeling
Discrete element of rigidity in translation DIS_T
Y
0.4
0.3

P
P
P
1
P
3
P
6
P
X
2
4
P5
7
P8
X
Characteristics of the elements:
ORIENTATION:
in all the nodes
with an angle \(=53.130102^{\circ}\)
DISCRETE:
nodal masses all nodes
M_T_N
in absolute reference mark
( \(\mathrm{m}=10\).)
matrices of rigidity all meshs
K_T_L
in local reference mark
\((\mathrm{Kx}=1.105)\)
with the nodes ends
K_T_N
in local reference mark
( \(\mathrm{Kx}=1.105\) )
Limiting conditions:
DDL_IMPO: (ALL: "YES" DZ: 0. )
LIAISON_DDL: (such as 3Dy=4Dx in all the nodes)
Names of the nodes: : P1, P2,...., P8
5.2

Characteristics of the grid
A number of nodes: 8
A number of meshs and types: 7 SEG2
5.3 Functionalities
tested

\section*{Orders}

\section*{Keys}

AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_T_L"
[U4.24.01]
NODE
"K_T_N"
"M_T_N"
ORIENTATION
GROUP_NO
"ANGL_NAUT"
AFFE_MODELE
ALL
"MECHANICAL"
"DIS_T"
[U4.22.01]
GROUP_NO
"DIS_T"
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.01]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ

IMPR_STURM
FREQ_MIN
[U4.73.01]
FREQ_MAX
NORM_MODE
"MASS_GENE"
or
"RIGI_GENE"
[U4.64.02]
NORM_MODE
"AVEC_CMP"
or
"SANS_CMP"
[U4.64.02]
NORM_MODE
NODE
CMP
[U4.64.02]
NORM_MODE
"STANDARD"
"TRAN" or "EUCL"
or "EUCL_TRAN"
[U4.64.02]
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\section*{Code_Aster ©}

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6
Results of modeling B
6.1 Values
tested

\section*{Identification}

\section*{Reference}

Aster
\% Difference
tolerance
Number of the mode
valley. relative

\section*{clean}

1
5.5274
5.5274
0.

104
2
10.8868
10.8868

3
15.9155
15.9155

4
20.4606
20.4606

5
24.3840
24.3840

6
27.5664
27.5664

\section*{8}
31.3474
31.3474

\section*{Normalized mode with 1 with the largest component Nature of the clean mode}

\author{
Not
}

\section*{Reference}

Aster
P1
0.3473
0.34729

P2
0.6527
0.65270

Translation 1
P3
0.8793
0.87938
(Dy)
P4
-1.
-1.

P5
-1.
-1.
1
P6
0.8793
0.87938

P7
0.6527
0.65270

P8
0.3473
0.34729

P1
0.3473
0.34729

P2
0.6527

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Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
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Mode normalized with the unit generalized mass
Nature of the clean mode
Not

\section*{Reference}

Aster

\author{
P1
}
4.0781E2
4.0788E2

P2
7.6654E2
7.6657E2

Translation 1
P3
1.0327 E 1
1.0327 E 1
(Dy)
P4
1.1743E1
1.1744E1

P5
1.1743E1
1.1744E1

1
P6
1.0327 E 1
1.0327 E 1

P7
7.6654E2
7.6657E2

P8
4.0781E2
4.0788E2

P1
4.0781E2
4.0788E2

P2
7.6654 E 2
7.6657E2

Translation 8
P3
1.0327 E 1
1.0328E1
(Dy)
P4
1.1743E1

\section*{P5}
1.1743E1
1.1744E1

8
P6
1.0327 E 1
1.0328 E 1

P7
7.6654E2
7.6657E2

P8
4.0781E2
4.0788 E 2

Maximum error lower than: \(0.03 \%\).
Mode normalized with the unit generalized stiffness
Nature of the clean mode
Not
Reference
Aster
P1
1.1742E3
1.1744 E 3

P2
2.2072E3
2.2072E3

Translation 1
P3
2.9735E3
2.9738E3
(Dy)
P4
3.3813E3
3.3817 E 3

P5
3.3813E3
3.3817 E 3

1
P6
2.9735E3
2.9738 E 3
```

P7
2.2072E3
2.2072E3
P8
1.1742E3
1.1744E3
P1
2.0705E4
2.0709E4
P2
3.8918E4
3.8920E4
Translation }
P3
5.2432E4
5.2436E4
(Dy)
P4
5.9621E4
5.9628E4
P5
5.9621E4
5.9628E4
8
P6
5.2432E4
5.2436E4
P7
3.8918E4
3.8920E4
P8
2.0705E4
2.0709E4
Maximum error lower than: 0.03%.

```

\subsection*{6.2 Remarks}
```

Calculations carried out by:
MODE_ITER_SIMULT
METHOD: "TRI_DIAG"
CALC_FREQ:
OPTION = "PLUS_PETITE"
NMAX_FREQ = 8

```

\section*{Contents of the file results:}
the first 8 Eigen frequencies, clean vectors and modal parameters
6.3 Parameters
of execution
Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
16.686 seconds

Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
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7 Modeling
C
7.1

\section*{Characteristics of modeling}

Transposition of the test of reference to the case of the degrees of freedom of rotation (from torsion + inertia comes out)
by using the discrete element of rigidity in translation/rotation DIS_TR.
Y
0.4
0.3

P
P
P
1
P
3

Characteristics of the elements:
ORIENTATION:
in all the nodes
with an angle \(=53.130102^{\circ}\)
DISCRETE:
with nodal masses all nodes
M_TR_D_N
in absolute reference mark
( \(\mathrm{Ixx}=10\).)
matrices of rigidity all meshs
K_TR_D_L
in local reference mark
\((\mathrm{KRx}=1.105)\)
with the nodes
K_TR_D_N
in local reference mark
\((\mathrm{KRx}=1.105)\)
ends
Limiting conditions:
DDL_IMPO: (ALL: "YES" DX: 0. , DZ: 0. , DRZ: 0. )
LIAISON_DDL: (such as 3DRY=4DRY in all the nodes)
Names of the nodes: : P1, P2,...., P8
7.2

Characteristics of the grid
A number of nodes: 8
A number of meshs and types: 7 SEG2
7.3 Functionalities

\section*{tested}

Orders
Keys
AFFE_CARA_ELEM
DISCRETE
GROUP_MA
```

"K_TR_D_L"
[U4.24.01]
NODE
"K_TR_D_N"
"M_TR_D_N"
ORIENTATION
GROUP_NO
"ANGL_NAUT"
AFFE_MODELE
ALL
"MECHANICAL"
"DIS_TR"
[U4.22.01]
GROUP_NO
"DIS_TR"
MODE_ITER_INV
CALC_FREQ
OPTION
"ADJUSTS"
[U4.52.01]
FREQ
CALC_MODE
OPTION
"DIRECT"
NORM_MODE
"MASS_GENE"
or "RIGI_GENE"
[U4.64.02]
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:

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8
Results of modeling C
8.1 Values
tested

\section*{Identification}

\section*{Reference}

\section*{Aster}
\% Difference
tolerance
Number of the mode
valley. relative clean
1
5.5274
5.5274

0 .
104
2
10.8868
10.8868

3
15.9155
15.9155

4
20.4606
20.4606

5
24.3840
24.3840

6
27.5664
27.5664

\section*{Normalized mode with 1 with the largest component \\ Nature of the clean mode}

\section*{Not}

\section*{Reference}

Aster
P1
0.3473
0.34729

P2
0.6527
0.65270

\section*{Rotation 1}

P3
0.8793
0.87938
(DRY)
P4
-1.
-1.
P5
-1.
-1.
1
P6
0.8793
0.87938

P7
0.6527
0.65270

P8
0.3473
0.34729
P1
\[
0.3473
\]
0.34729P2
\[
0.6527
\]
0.65270
Rotation 8

P3
0.8793
0.87938
(DRY)
P4
-1.
-1.
P5
1.
1.
8
P6
0.8793
0.87938
P7
0.6527
0.65270
P8
0.3473
0.34729
Maximum error lower than: 0.03\%.
Mode normalized with the unit generalized mass
Nature of the clean mode
Not
Reference
Aster
P1
4.0781E2
4.0788E2
P2
7.6654E2
7.6657E2
Rotation 1
P3
1.0327E1

\subsection*{7.6657E2}

P8
4.0781 E 2
4.0788E2

Maximum error lower than: \(0.03 \%\).
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
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\section*{Mode normalized with the unit generalized stiffness}

Nature of the clean mode
Not
Reference
Aster
P1
1.1742 E 3
1.1744 E 3

P2
2.2072E3
2.2072E3

Rotation 1
P3
2.9735 E 3
2.9738 E 3
(DRY)
P4
3.3813E3
3.3817E3

P5
3.3813E3

\subsection*{3.3817 E 3}

1
P6
2.9735E3
2.9738E3

P7
2.2072E3
2.2072E3

P8
1.1742 E 3
1.1744 E 3

P1
2.0705E4
2.0709E4

P2
3.8918E4
3.8920E4

Rotation 8
P3
5.2432E4
5.2436E4
(DRY)
P4
5.9621 E 4
5.9628E4

P5
5.9621E4
5.9628E4

8
P6
5.2432E4
5.2436E4

P7
3.8918E4
3.8920E4

P8
2.0705E4
2.0709 E 4

Maximum error lower than: \(0.03 \%\).
8.2 Remarks

Calculations carried out by: MODE_ITER_INV

LIST_FREQ: (5. , 10. , 15. , 20. , 24. , 27. , 30. , 32.)
CALC_MODE: (OPTION: "DIRECT")

\section*{Contents of the file results:}
the first 8 Eigen frequencies, clean vectors and modal parameters

\subsection*{8.3 Parameters}

\section*{of execution}

Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5.8 seconds

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HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
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9 Modeling
D
9.1

\section*{Characteristics of modeling}

Transposition of the test of reference to the case of the degrees of freedom of rotation (from torsion + inertia comes out)
by using the discrete element of rigidity in translation/rotation: DIS_TR.
Y
0.4
0.3

P
P
P
1
P
3
P
6
P
X
2
4
P5
7
P8
X
Characteristics of the elements:
ORIENTATION:
in all the nodes
with an angle \(=53.130102^{\circ}\)
DISCRETE:
with nodal masses all nodes
M_TR_N
in absolute reference mark
( \(\operatorname{Ixx}=10\).)
matrices of rigidity all meshs
K_TR_L
in local reference mark
\((\mathrm{KRx}=1.105)\)
with the nodes
K_TR_N
in local reference mark
\((\mathrm{KRx}=1.105)\)
ends
Limiting conditions:
DDL_IMPO: (ALL: "YES" DX: 0. , DY: 0. , DZ: 0. , DRZ: 0. )
LIAISON_DDL: (such as 3DRY=4DRY in all the nodes)
Names of the nodes: P1, P2,...., P8

\section*{9.2}

\section*{Characteristics of the grid}

A number of nodes: 8
A number of meshs and types: 7 SEG2

\subsection*{9.3 Functionalities}

\section*{tested}

\section*{Orders}

\section*{Keys}

AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_TR_L"
[U4.24.01]
NODE
"K_TR_N"
"M_TR_N"
ORIENTATION
GROUP_NO
"ANGL_NAUT"
AFFE_MODELE
ALL
"MECHANICAL"
"DIS_TR"
[U4.22.01]
GROUP_NO
"DIS_TR"

\title{
MODE_ITER_INV
}

CALC_FREQ
OPTION
"ADJUSTS"
[U4.52.01]
FREQ
CALC_MODE
OPTION
"DIRECT"
RECU_CHAMP
"DEPL"
NUME_ORDRE
[U4.62.01]
FREQ
NORM_MODE
"MASS_GENE"
Or
"RIGI_GENE"
[U4.64.02]
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):
B. QUINNEZ

Key:
V2.01.002-E Page:
15/24
10
Results of modeling \(D\)
10.1 Values
tested
Identification

\section*{Reference}

\section*{Aster}
\% Difference

\section*{tolerance}

Number of the mode
valley. relative

\section*{clean}

1
5.5274
5.5274

0 .
104
2
10.8868
10.8868

3
15.9155
15.9155

4
20.4606
20.4606

5
24.3840
24.3840

6
27.5664
27.5664
29.9113
29.9113

\section*{8}
31.3474
31.3474

\section*{Normalized mode with 1 with the largest component}

\section*{Nature of the clean mode}

Not
Reference
Aster
P1
0.3473
0.34729

P2
0.6527
0.65270

Rotation 1
P3
0.8793
0.87938
(DRY)

\section*{P4}
-1.
-1.
P5
-1.
-1.
1
P6
0.8793
0.87938

P7
0.6527
0.65270
P8
0.3473
0.34729

P1
0.3473
0.34729

P2
0.6527
0.65270

Rotation 8
P3
0.8793

\section*{Nature of the clean mode}

\section*{Not}

Reference
Aster
P1
4.0781 E 2
4.0788E2

P2
7.6654E2
7.6657E2

Rotation 1
P3
1.0327 E 1
1.0327 E 1
(DRY)
P4
1.1743E1
1.1744E1

P5
1.1743E1
1.1744 E 1
1 P6
1.0327 E 1
1.0327 E 1

P7
7.6654 E 2
7.6657E2

P8
4.0781E2
4.0788E2

P1
4.0781E2
4.0788 E 2

P2
7.6654E2
7.6657E2

Rotation 8
P3
1.0327 E 1
1.0328 E 1
(DRY)
P4
1.1743E1
1.1744E1

\section*{P5}
1.1743E1
1.1744E1

8
P6
1.0327 E 1
1.0328 E 1

P7
7.6654E2
7.6657E2

P8
4.0781E2
4.0788 E 2

Maximum error lower than: \(0.03 \%\).
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
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\section*{Mode normalized with the unit generalized stiffness}

Nature of the clean mode
Not
Reference
Aster
P1
1.1742E3
1.1744 E 3

P2
2.2072E3
2.2072E3

Rotation 1
P3
2.9735E3
2.9738E3
(DRY)
P4
3.3813E3
3.3817E3

P5
3.3813E3
3.3817E3

1
P6
2 9735E3
2.9738E3

P7
2.2072E3
2.2072E3

P8
1.1742E3
1.1744E3
P1
2.0705E4
2.0709 E 4
P2
3.8918E4
3.8920E4
Rotation 8
P3
5.2432E4
5.2436E4
(DRY)
P4
5.9621 E 4
5.9628E4
P5
5.9621E4
5.9628 E 4
8
P6
5.2432E4
5.2436 E 4
P7
3.8918E4
3.8920E4
P8
2.0705E4
2.0709 E 4
Maximum error lower than: \(0.03 \%\).
10.2 Remarks
Calculations carried out by:
MODE_ITER_INV
OPTION: "ADJUSTS"
LIST_FREQ: (5. , 10. , 15. , 20. , 24. , 27. , 30. , 32.)
CALC_MODE: (OPTION: "DIRECT")
Contents of the file results:
the first 8 Eigen frequencies, clean vectors and modal parameters
10.3 Parameters
of execution
Version: 3.02.21
Machine: CRAY C90
```

System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5.6 seconds
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

```

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
17/24
11 Modeling
E
11.1 Characteristics of modeling

Discrete element of rigidity in translation 2D_DIS_T
Y
0.4
0.3

P
P
P
1
P
3
P
6
P
X
2
4
P5
7

\section*{P8}

X
Characteristics of the elements:
ORIENTATION:
in all the nodes
with an angle \(=53.130102^{\circ}\)
DISCRETE:
with nodal masses all nodes
M_T_D_N
in absolute reference mark
( \(\mathrm{m}=10\).)
matrices of rigidity all meshs
K_T_D_L
in local reference mark
( \(\mathrm{Kx}=1.105\) )
with the nodes ends
K_T_D_N
in local reference mark
( \(\mathrm{Kx}=1.105\) )
Limiting conditions:
LIAISON_DDL: (such as 3Dy=4Dx in all the nodes)
Names of the nodes: P1, P2,...., P8
\(\operatorname{Not} \mathrm{A}=\mathrm{N} 1\)
= N2

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 8
A number of meshs and types: 7 SEG2
11.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_T_D_L"
[U4.24.01]
NODE
"K_T_D_N"
"M_T_D_N"
ORIENTATION
GROUP_NO
"ANG_NAUT"
AFFE_CHAR_MECA

\section*{ALL}
[U4.23.02]

\section*{AFFE_MODELE}

ALL
"MECHANICAL" "2D_DIS_T"
[U4.22.01]
GROUP_NO
"2D_DIS_T"
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_INV
CALC_FREQ
OPTION
"ADJUSTS"
[U4.52.01]
FREQ
CALC_MODE
OPTION
"DIRECT"
RECU_CHAMP
"DEPL"
NUME_ORDRE
[U4.62.01]
FREQ
NORM_MODE
"MASS_GENE"
[U4.64.02]
NORM_MODE
"RIGI_GENE"
[U4.64.02]
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version \\ 4.0}

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
18/24
12
Results of modeling \(\mathbf{E}\)
12.1 Values
tested
Identification
Reference
Aster
\% Difference
tolerance
Number of the mode
valley. relative
clean
1
5.5274
5.5274
0.

104
2
10.8868
10.8868

3
15.9155
15.9155

4
20.4606
20.4606

\section*{Normalized mode with 1 with the largest component}

Nature of the clean mode

\section*{Not}

\section*{Reference}

Aster
P1
0.3473
0.34729

P2
0.6527
0.65270

Translation 1
P3
0.8793
0.87938
(Dy)
P4
-1.
-1.

P5
-1.
-1.
1.
1.

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
19/24
Mode normalized with the unit generalized mass
Nature of the clean mode
Not
Reference
Aster
P1
4.0781 E 2
4.0788E2

P2
7.6654E2
7.6657E2

Translation 1
P3
1.0327 E 1
1.0327 E 1
(Dy)
P4
1.1743E1
1.1744 E 1

P5
1.1743E1
1.1744 E 1

1
P6
1.0327 E 1
1.0327 E 1

P7
7.6654 E 2
7.6657E2

P8

\subsection*{4.0781 E 2}
4.0788E2

P1
4.0781E2
4.0788E2

P2
7.6654E2
7.6657E2

Translation 8
P3
1.0327 E 1
1.0328E1
(Dy)
P4
1.1743E1
1.1744E1

P5
1.1743E1
1.1744E1

8
P6
1.0327 E 1
1.0328 E 1

P7
7.6654E2
7.6657E2

P8
4.0781E2
4.0788E2

Maximum error lower than: \(0.03 \%\).
Mode normalized with the unit generalized stiffness

\section*{Nature of the clean mode}

Not
Reference

\section*{Aster}

P1
1.1742 E 3
1.1744 E 3

P2
2.2072E3
2.2072 E 3

Translation 1

\author{
P3 \\ 2.9735E3 \\ 2.9738E3 \\ (Dy) \\ P4 \\ 3.3813E3 \\ 3.3817E3 \\ P5 \\ 3.3813E3 \\ 3.3817E3 \\ 1 \\ P6 \\ 2.9735E3 \\ 2.9738E3 \\ P7 \\ 2.2072E3 \\ 2.2072E3 \\ P8 \\ 1.1742E3 \\ 1.1744 E 3 \\ P1 \\ 2.0705E4 \\ 2.0709E4 \\ P2 \\ 3.8918E4 \\ 3.8920E4
}

Translation 8
P3
5.2432E4
5.2436E4
(Dy)
P4
5.9621E4
5.9628E4

\section*{P5}
5.9621E4
5.9628 E 4

8
P6
5.2432E4
5.2436E4

\section*{P7}
3.8918E4
3.8920 E 4

P8
2.0705 E 4
2.0709 E 4

Maximum error lower than: \(0.03 \%\).

\subsection*{12.2 Remarks}

Calculations carried out by:
MODE_ITER_INV
OPTION: "ADJUSTS"
LIST_FREQ: (5. , 10. , 15. , 20. , 24. , 27. , 30. , 32.)
CALC_MODE: (OPTION: "DIRECT")
Contents of the file results:
the first 8 Eigen frequencies, clean vectors and modal parameters

\subsection*{12.3 Parameters}

\section*{of execution}

Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
6 seconds
Handbook of Validation
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\section*{Code_Aster © \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
20/24
13 Modeling
F

\subsection*{13.1 Characteristics of modeling}

Discrete element of rigidity in translation/rotation: 2D_DIS_TR
Y
0.4
0.3

Characteristics of the elements:
ORIENTATION:
in all the nodes
with an angle \(=53.130102^{\circ}\)
DISCRETE:
with nodal masses all nodes
M_T_D_N
in absolute reference mark
( \(\mathrm{m}=10\).)
matrices of rigidity all meshs
K_T_D_L
in local reference mark
( \(\mathrm{Kx}=1.105\) )
with the nodes ends
K_T_D_N
in local reference mark
( \(\mathrm{Kx}=1.105\) )
Limiting conditions:
DDL_IMPO: (ALL: "YES" DRZ: 0. )
LIAISON_DDL: (such as 3Dy=4Dx in all the nodes)
Names of the nodes: P1, P2,...., P8
Not A = N1

\section*{\(=\mathrm{N} 2\)}

\subsection*{13.2 Characteristics of the grid}

A number of nodes: 8
A number of meshs and types: 7 SEG2

\subsection*{13.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_T_D_L"
[U4.24.01]
NODE
"K_T_D_N"
"M_T_D_N"
ORIENTATION
GROUP_NO
"ANG_NAUT"
AFFE_CHAR_MECA
DDL_IMPO
ALL
[U4.25.01]
LIAISON_DDL
NODE
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
ALL
"MECHANICAL"
"2D_DIS_TR"
[U4.22.01]
GROUP_NO
"2D_DIS_TR"
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_INV
CALC_FREQ
OPTION
"ADJUSTS"
[U4.52.01]
```

FREQ
CALC_MODE
OPTION
"DIRECT"
RECU_CHAMP
"DEPL"
NUME_ORDRE
[U4.62.01]
FREQ
NORM_MODE
"MASS_GENE"
[U4.64.02]
NORM_MODE
"RIGI_GENE"
[U4.64.02]
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```
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
SDLD02 Système mass-arises with 8 degrees of freedom
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.01.002-E Page:
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14
Results of modeling \(\mathbf{F}\)
14.1 Values
tested
Identification
Reference
Aster
\% Difference
tolerance
Number of the mode
valley. relative
clean

Normalized mode with 1 with the largest component Nature of the clean mode

\section*{Reference}

\section*{Aster}

P1
0.3473
0.34729

P2
0.6527
0.65270

Translation 1
P3
0.8793
0.87938
(Dy)
P4
-1.
-1.
P5
-1.
-1.
1
P6
0.8793
0.87938

P7
0.6527
0.65270

P8
0.3473
0.34729

P1
0.3473
0.34729

P2
0.6527
0.65270

Translation 8
P3
0.8793
0.87938
(Dy)
P4
-1.

\section*{Code_Aster \({ }^{\circledR}\)}

Version
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Titrate:
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Date:
01/12/98
Author (S):

\section*{B. QUINNEZ}

Key:
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Mode normalized with the unit generalized mass
Nature of the clean mode
Not
Reference
Aster
P1
4.0781E2
4.0788E2

P2
7.6654E2
7.6657 E 2

\section*{Translation 1}

P3
1.0327 E 1
1.0327 E 1
(Dy)
P4
1.1743E1
1.1744E1

\section*{P5}
1.1743E1
1.1744 E 1

1
P6
1.0327 E 1
1.0327 E 1

P7
7.6654E2
7.6657E2

P8
4.0781E2
4.0788 E 2

P1
4.0781E2
4.0788 E 2

P2
7.6654 E 2
7.6657E2

Translation 8
P3
1.0327 E 1
1.0328 E 1
(Dy)
P4
1.1743E1
1.1744E1

P5
1.1743E1
1.1744 E 1

8
P6
1.0327 E 1

\author{
Aster
}

P1
1.1742E3
1.1744 E 3
P2
2.2072E3
2.2072E3
Translation 1
P3
2.9735E3
2.9738E3
(Dy)
P4
3.3813E3
3.3817 E 3
P5
3.3813E3
3.3817E3

1

P6

2.9735E3

2.9738E3

P7

2.2072E3

2.2072E3

P8

    1.1742E3

    1.1744 E 3

P1

2.0705E4

\subsection*{2.0709 E 4}

P2
3.8918E4
3.8920E4

Translation 8
P3
5.2432E4
5.2436E4
(Dy)
P4
5.9621E4
5.9628 E 4

P5
5.9621E4
5.9628E4

8
P6
5.2432E4
5.2436E4

P7
3.8918E4
3.8920E4

P8
2.0705E4
2.0709E4

Maximum error lower than: \(0.03 \%\).

\subsection*{14.2 Remarks}

Calculations carried out by:
MODE_ITER_INV
OPTION: "ADJUSTS"
LIST_FREQ: (5. , 10. , 15. , 20. , 24. , 27. , 30. , 32.)
CALC_MODE: (OPTION: "DIRECT")

\section*{Contents of the file results:}
the first 8 Eigen frequencies, clean vectors and modal parameters

\subsection*{14.3 Parameters}
of execution
Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords

\title{
Time CPU To use:
}

6 seconds
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\section*{Code_Aster ®}

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Author (S):

\section*{B. QUINNEZ}

Key:
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15
Summary of the results
For all the options of modeling of the discrete elements of rigidity and mass offered by
AFFE_CARA_ELEM the solutions obtained are those of the reference solution (frequencies and modes clean with various standardizations).
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
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Titrate:
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\section*{B. QUINNEZ}

Key:
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Transitory SDLD04 Response of a system mass-springs
Date:
30/08/01
Author (S):
Fe WAECKEL, L. VIVAN Key
:
V2.01.004-C Page:
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Organization (S): EDF/RNE/AMV, CS IF

\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
Document: V2.01.004

SDLD04-Transitory response of a system mass-springs subjected to an acceleration imposed

\section*{Summary}

This test consists in calculating the not deadened transitory response of a linear system mass-springs embedded
free subjected to an imposed acceleration.
One tests the discrete element in traction and compression, the calculation of the clean modes, the static modes and it
calculation of the transitory response of a system subjected to an imposed acceleration. Direct calculation is compared
response to its calculation by modal recombination.
This case test is from guide VPCS. The reference solution is an analytical calculation. Errors on results obtained are normal taking into account the step of time chosen for numerical integration. Handbook of Validation
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Author (S):
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:
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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}

One calculates the response of a linear system composed of three masses and three springs to one acceleration imposed on the level of its point of anchoring (A):

\author{
\(y\) \\ kI \\ k2 \\ k3 \\ X \\ (T) \\ m1 \\ m \\ m \\ 1 \\ 2 \\ m3 \\ With \\ B \\ C \\ D
}
1.2

Properties of materials
stiffnesses of connection: \(K=k 1=k 2=k 3=1000 \mathrm{~N} / \mathrm{m}\);
specific masses: \(m=m 1=m 2=m 3=1 \mathrm{~kg}\).

\section*{1.3}

Boundary conditions and loadings
Boundary conditions
Only authorized displacements are the translations according to axis \(X\).
Point \(A\) is embedded: \(d x=D y=d z=d r x=d r y=d r z=0\).

\section*{Loading}

The point of anchoring \(A\) is subjected to an acceleration, function increasing of time, according to direction \(X:(T)=2.105 . t 2\) ( \(T\) vary from 0 to \(0,1 S\) ).

\subsection*{1.4 Conditions \\ initial}

The system is initially at rest: with \(T=0, d x(0)=0\) and \(d x / d t(0)=0\) in any point.
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\section*{:}

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\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
One initially calculates the Eigen frequencies fi and the clean vectors Nor associated standardized compared to the matrix of mass. One calculates then the generalized response of the system
mono-excited by solving analytically the integral of Duhamel [bib1]. Lastly, one restores on the basis physique displacement relating to point \(D\).

\section*{Calculation of the Eigen frequencies}

The matrices of mass and stiffness are as follows:
```

m00
2-1
0
M=0m0,K
= K-1
2-
1
0
m
O-
1
1
The Eigen frequencies are solution of the equation
[
det K-2M]=0, is
3
K
-2
5
+
6-1=0 where = and =

```
m
0

In the absolute reference mark, the fundamental equation of the dynamics of the system mass-springs not
deadened is written: MR. \(X\)
\(\&+K X\)
has
\(=0\) have.
Absolute displacement Xa breaks up into a uniform displacement of drive in translation \(X\) and in a relative displacement \(X\)
E
R: \(X\)
\(X\)
\(X\)
has \(=\)
\(+\)
R
E.

The equation of the movement in the relative reference mark is written then: MR. \(X\) \(\&+K X=-M R . X\)
\(\boldsymbol{R}\)
\(R\)
\(\&=Q\)
\(S\)

1
1
2
with \& \(X\)
(T) \(A\).
\(S=\)
=T2 and =
1 and thus \(Q=\) a.t m
1 .
1
1
The equation of the movement projected on the basis of dynamic mode standardized compared to stamp of mass is written:
\(T\)
. Mr.
\(\&(T)+2(T)\)
\(I\)
\(=\)
\((T)=-p(T)(T)\)
I
\(I\)
\(I\)
\(T\)
. Mr.
I
\(I\)
I
The response of this linear system, to one moment \(T\) is given by the integral of Duhamel:
```

T
I
p(T)T
\& (T) =
-p(T)(T).\operatorname{sin}(T-)
I
D = -
a.t 2 sin}(T

```
I
\(I\)
\(I\)
\(I\)
\(I\)
\(D\).
\(I\)
\(I\)
0
0

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Code_Aster \({ }^{\circledR}\)

\author{
Version \\ 5.0
}

\section*{Titrate:}

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\(T\)

However, according to [bib1], a.t \(2 \sin (T-) D=\)
\(T 2\)
2
\(+\)
( \(\cos T\)
I
I-)
1
\(\stackrel{\dot{I}}{I}\)
a. \(p\) (T).

Thus X
I
I
=. = -
2
2
1
2
\(T+\)
\((\cos T\)
R
\(I\)
I
I-).
I
I
I
2.2

Results of reference
One takes for results of reference the three Eigen frequencies of the system and relative displacement xr at the point \(D\), for various moments ranging between 0 and 0,1 S.

\section*{2.3}

Uncertainty on the solution

No if one calculates the integral of Duhamel analytically [bib1], [bib2].

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}
J.S. PRZEMIENIECKI: Theory of matrix structural analysis. New York, Mac Graw-Hill, 1968, p. 351-357
[2]
S.P. TIMOSHENKO, D.H. YOUNG and W. WEAVER: Vibrations problems in engineering 4th edition, New York, Wiley \& Sounds, 1974, p. 284-321
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Version
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\title{
Author (S):
}

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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
The springs and specific masses are modelled by discrete elements with 3 degrees of freedom "DIS_T":

Y
\(K\)
\(K\)
\(\boldsymbol{K}\)
1
2
3
\(X\)
NO1
NO2
NO3
NO4

Node NO1 is embedded and subjected to an imposed acceleration (T). Displacement is calculated relative of node NO4.

Calculations by modal synthesis
One considers the complete base of the clean modes. Temporal integration is carried out with algorithms of Newmark, Euler and Devogelaere with a step of times of 0,001 S. calculations all the steps of time are filed.
One considers a damping reduces no one for the whole of the calculated modes.
I
The loading is taken into account in the form of vector projected on the modal basis
EXCIT: (VECT_GENE) or in the form of modal component EXCIT: (NUME_MODE) or both with the time.

\section*{Direct calculations}

Temporal integration is carried out either with the algorithm of Newmark or with the explicit algorithm
differences centered with a step in times of 0,001 S. calculations are filed all the ten steps time.

\section*{Note:}

As the diagram of the centered differences can be used only with one matrix of mass diagonal, one calculates the elementary matrices with option MASS_MECA_DIAG in the operator CALC_MATR_ELEM.

Taking into account of an initial state
In the two types of calculation, one checks that the relative displacement obtained of a calculation carried out in one
time is identical to that obtained in several times, i.e. while regarding as initial state, it result of the last step of calculated time:

ETAT_INIT: (RESU_GENE: ...) for a calculation by modal synthesis;
ETAT_INIT: (DYNA_TRAN) or
ETAT_INIT: (DEPL_INIT: ...
VITE_INIT: .) for a direct calculation.
Taking into account of the modes neglected by static correction:
One considers a modal base made up of the first two clean modes and one has supplements it by a mode corresponding to the static response of the system studied to a unit loading of type force imposed in the direction X (key words MODE_CORR and CORR_STAT in L ooperator DYNA_TRAN_MODAL).

\section*{3.2}

Characteristics of the grid
A number of nodes: 4
A number of meshs and types: 3 DIS_T
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\section*{Author (S):}

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:

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\subsection*{3.3 Functionalities}
tested
Orders

\author{
Keys Doc. V5 \\ AFFE_MODELE GROUP_MA "MECHANICAL" \\ "DIS_T" \\ [U4.41.01] \\ DISCRETE AFFE_CARA_ELEM \\ NODE \\ \(M_{-} T_{-} D_{-} N[U 4.42 .01]\) \\ GROUP_MA \\ \(K_{-} T_{-} D_{-} L\) \\ CALC_MATR_ELEM OPTION \\ MASS_MECA_DIAG
}
[U4.61.01]
MODE_ITER_SIMULT PLUS_PETITE
MAX_FREQ
[U4.52.03]
CALC_CHAR_SEISME MONO_APPUI
[U4.63.01]
MACRO_PROJ_BASE
[U4.63.11]
AFFE_CHAR_MECA FORCE_NODALE
```

[U4.44.01]
MACRO_ELAS_MULT CHAR_MECA_GLOBAL

```
[U4.51.02]
CAS_CHARGE
CHAR_MECA

DYNA_TRAN_MODAL METHOD
NEWMARK
[U4.53.21]
DEVOGE

\section*{EULER}

\author{
ETAT_INIT \\ RESU_GENE
}

\author{
MODE_CORR
}

\section*{EXCIT}

CORR_STAT

\section*{DYNA_LINE_TRAN METHOD \\ NEWMARK}
[U4.53.02]
DIFF_CENTRE

ETAT_INIT
DEPL_INIT
VITE_INIT

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4
Results of modeling \(A\)

\section*{4.1}

Values tested of modeling \(A\)
Eigen frequencies (in Hz ) of the system:
Number of the mode
Analytical
Code_Aster
relative error \%
1 2,239
2,240
0,038
26,275
6,276
0,015
3 9,069
9,069
0

Values of the relative displacement of node NO4 for various moments:
Transitory calculation by modal synthesis
One tests the taking into account of a loading in the form of vector projected on the modal basis, under
form of modal component, in the form of projected vector and of modal component at the same time as well as the taking into account of the neglected modes.

Code_Aster
Code_Aster
Time (S)
Reference
Loading of the type
Error
Loading of the type
Error
generalized vector
relative \% modal component
relative
Algorithm of
Algorithm of Euler
\%
Newmark
0,02 2,700E03 2,680E03
0,741 2,660E03
-1,481
0,04 4,260E02 4,272E02
0,279 4,264E02 0,091
0,05 1,041E01 1,042E01
0,134 1,041E01 0,015
0,06 2,158E01 2,161E01
0,121 2,159E01 0,038
0,08 6,813E01 6,819E01
0,094 6,816E01 0,049
0,10 1,658E+00 1,659E+00 0,082
1,659E+00
0,055
Type of loadingTime (S)
Reference
Code_Aster
relative error \%
0,02
5,400E03
5,320E03
-1,482
Generalized vector
0,04
8,520E02
8,528E02
0,091
and 0,05
2,082E01
2,082E01
0,015
modal component
0,06 4,316E01
4,318E01
0,038
simultanéement 0,08
\(1,363 E+001,363 E+00\)
0,049
(Euler) 0,10
3,316E+00 3,318E+00
0,055
0,02
4,000E03
3,985E03
-0,373
Generalized vector
0,04
4,640E02
4,640E020,01
Devogelaere 0,05
1,085E01 1,086E01
0,084
(more correction0,06 2,203E01

\section*{2,204E01}

0,039
statics) 0,08
6,842E01 6,843E01
0,021
0,10
\(1,659 E+00\)
1,659E+00
0,026

The results with incomplete modal base without static correction are not tested. One illustrates below interest of the static correction:
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Displacement of node NO4 (in meters) according to time

Complete base
Incomplete base
Incomplete base with
without correction
static correction
statics

Direct transitory calculation
One compares displacements calculated with the N04 node according to various diagrams of integration:

Time (S)
Reference
Code_Aster
Error
Code_Aster
Error
Diagram of
relative \%
Diagram of
relative \%
Newmark
centered differences
0,02 2,700E03 2,680E03 0,741 2,660E03 1,482
0,04 4,260E02 4,272E02
0,279 4,264E02 0,091
0,05 1,041E01 1,042E01
0,134 1,041E01 0,015
0,06 2,158E01 2,161E01
0,121 2,159E01 0,038
0,08 6,813E01 6,819E01
0,094 6,745E01
-1,004
0,10 1,658E+00 1,659E+00 0,082 1,645E+00 0,803
Taking into account of an initial state:
As waited, the relative displacements calculated in once are strictly identical to those obtained by regarding as initial state the result of the last step of calculated time.

\subsection*{4.2 Parameters}
of execution
Version: STA 5.02
Machine: SGI Origin 2000
Time CPU to use: 5,9 seconds

\section*{5 \\ Summary of the results}

The reference solution is an analytical calculation. The errors on the results obtained are normal taking into account the step of time chosen for numerical integration.
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Titrate:
SDLD21-System mass-arises to 8 ddl with viscous shock absorber
Date:
23/06/03
Author (S):
O. NICOLAS

Key: V2.01.021-C Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
V2.01.021 document

SDLD21-System mass-arises to 8 ddl with viscous shock absorber

\section*{Summary:}

This one-way problem consists in carrying out a harmonic analysis of a mechanical structure composed of a whole of mass-springs with shock absorbers viscous and subjected to an excitation sinusoidal. This test of mechanics of the structures corresponds to a dynamic analysis of a discrete model
having a linear behavior. It includes/understands three modelings.
Via this problem, one tests the discrete elements in translation (mass, arises, shock absorber), the definition of a force of specific excitation harmonic, the operator of calculation modal (MODE_ITER_SIMULT
[U4.52.03]) into quadratic and the operator of harmonic calculation of answer (DYNA_LINE_HARM [U4.54.02]).
In addition, several operators of postprocessing are tested: RECU_FONCTION [U4.62.03], TEST_FONCTION
[U4.72.02], RECU_CHAMP [U4.62.01].
Results obtained (field of displacement, speed and acceleration for various frequencies of excitation) are in concord with the results of guide VPCS.

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Date:
23/06/03
Author (S):
O. NICOLAS

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

U1
U2
U3
U8
K
K
K
K
With
m
m
m
m
B
X, U
P1
P2
P3
P8
C
C
C
C
Specific masses:
\(\mathrm{mP}=\mathrm{m}=\mathrm{m}=\)

\(\qquad\)

\[
=\mathrm{m}=\mathrm{m}
\]
\[
1
\]
P2
P3
P8
Stiffnesses of connection:
\[
\mathrm{kAP} 1=\mathrm{kP} 1 \mathrm{P} 2=\mathrm{kP} 2 \mathrm{P} 3=\ldots \ldots . .=\mathrm{kP} 8 \mathrm{~B}=\mathrm{K}
\]
Viscous damping:
\(\mathrm{cAP} 1=\mathrm{cP} 1 \mathrm{P} 2=\mathrm{cP} 2 \mathrm{P} 3=\ldots \ldots .=\mathrm{cP} 8 \mathrm{~B}=\mathrm{C}\)

\section*{1.2 \\ Material properties}
Comes out from linear elastic translation
K =
105 N/m
Specific mass
\(\mathrm{m}=\)
10 kg
One-way viscous damping
\(\mathrm{C}=\)
\(50 \mathrm{~N}(\mathrm{~m} / \mathrm{s})\)

\section*{1.3 \\ Boundary conditions and loadings}

Boundary conditions:
Embedded points A and b : \((\mathrm{U}=0)\).
Loading: Force concentrated sinusoidal of variable frequency at the P 4 point

\section*{Not P}

\subsection*{1.4 Conditions}

\section*{initial}

Without object for the study of the permanent harmonic mode.
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Date:
23/06/03
Author (S):
O. NICOLAS

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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The system of differential equations of the second order coupled is form:
\(M u \&+C u \&+K U=F\)
2-1
```

-12
-1
10
with M
1
2
=
C=50

```
\([-2 \mathbf{I}+J] \mathbf{Q}=T \mathbf{F 0}\)
\(\mathbf{I}\) is the identity, here is diagonal =
[]
(
)
II bus damping is proportional \(\mathbf{C}=\mathbf{K}\).
\(N\)
\(T\)
The answer is written: \(U\)
I
I
\(i=1 I-2+J I I\)
One obtains the exact solution by taking all the clean modes.
One deduces some: \(\& \mathrm{u}=J U\)
and
\&u
2
\(=-U\)
0
0
0
0

\section*{2.2}

Results of reference
Displacement according to X of the P 4 point for certain frequencies.

\section*{2.3 \\ Uncertainty on the solution}

Semi-analytical solution.

\subsection*{2.4 Reference bibliographical}

\section*{[1]}
J. PIRANDA: Note of use of the software of modal analysis MODAN - Version 0.2 (1990).

Laboratory of Mechanics Applied - University of Frank County - Besancon (France).
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.0

\section*{Titrate:}

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23/06/03
Author (S):
O. NICOLAS

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Discrete element of rigidity in translation}
y
With
P
P
B
X
1
2
P3
P4

\section*{P5}

Characteristics of the elements
DISCRETE:
with nodal masses
M_T_D_N
and matrices of rigidity
K_T_D_L
and matrices of damping
A_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. ) with the nodes ends
(GROUP_NO: AB DX: 0. )
Names of the nodes:
Not A = N1
\(\mathrm{P} 1=\mathrm{N} 2\)
Not B = N10
\(\mathrm{P} 2=\mathrm{N} 3\)
\(\mathrm{P} 8=\mathrm{N} 9\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and types: 9 SEG2

\subsection*{3.3 Functionalities \\ tested}

\section*{Orders}
GROUP_NO
"DIS_T"
AFFE_CHAR_MECA DDL_IMPO GROUP_NO
FORCE_NODALE
NODE
DYNA_LINE_HARM MATR_AMOR
DEFI_LIST_REEL BEGINNING
INTERVAL
RECU_FONCTION LIST_FREQ
TEST_FONCTION
TEST_RESU
IMPR_RESU
LIRE_RESU
Handbook of ValidationV2.01 booklet: Linear dynamics of the discrete systemsHT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version7.0
Titrate:

SDLD21 - System mass-arises to 8 ddl with viscous shock absorber
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Author (S):
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\section*{3.4 \\ Results of modeling A}

Parts real and imaginary of component DX of the displacement of the P 4 point.

\section*{Frequency Reference}

Aster \%
Difference
5.00
1.0237 E 4
1.02369 E 4
0.0004
8.5187 E6
8.51874 E6
5.50
4.5066 E 4
4.50662 E 4
0.0004
7.7914 E4
7.79143 E4
6.00
9.4101 E5
9.41096 E5
0.0002
1.0585 E5
1.05851 E 5
10.00
8.4143 E7
8.41427 E7
0.0024
1.0335 E6
1.03346 E6
15.00
1.2656 E5

\author{
1.26556 E5 \\ 0.0032 \\ 5.6652 E6 \\ 5.66517 E6 \\ 20.00 \\ 2.9784 E6 \\ 2.97844 E6 \\ 0.0003 \\ 6.6970 E6 \\ 6.69700 E6 \\ 25.00 \\ 1.2536 E6 \\ 1.25362 E 6 \\ 0.0008 \\ 5.2703 E6 \\ 5.27033 E6 \\ 30.00 \\ 2.0904 E6 \\ 2.09042 E6 \\ 0.0009 \\ 5.4821 E6 \\ 5.48215 E6 \\ 35.00 \\ 4.5447 E6 \\ 4.54473 E6 \\ 0.0011 \\ 1.1190 E6 \\ 1.11903 E6 \\ 39.50 \\ 2.6895 E6 \\ 2.68949 E6 \\ 0.0003 \\ 3.0505 E 7 \\ 3.05048 E7
}

Parts real and imaginary of component DX the speed of the P4 point.

\section*{Frequency Reference}

Aster \%
Difference
5.00
2.6762 E 4
2.6762 E4

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.0

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Parts real and imaginary of component DX of the acceleration of the P4 point.

\section*{Frequency Reference}

Aster \%
Difference
5.00
1.0103 E1
1.01035 E 1
0.000
8.4076 E3
8.40766 E3
```

5.50
5.3819 E1
5.38190 E1
0.000
9.3047 E1
9.30470 E1
6 . 0 0
1.3374 E1
1.33738 E1
0.000
1.5044 E2
1.50439 E2
10.00
3.3218 E3
3.32182 E3
0.002
4.0801 E3
4.07996 E3
15.00
1.1242 E1
1.12415 E1
0.003
5.0322 E2
5.03217 E2
2 0 . 0 0
4.7033 E2
4.70337 E2
0.001
1.0575 E1
1.05755 E1
2 5 . 0 0
3.0931 E2
3.09320 E2
0.001
1.3004 E1
1.30040 E1
30.00
7.4273 E2
7.42739 E2
0.001
1.9478 E1
1.94780 E1
35.00

```

\author{
2.1979 E1 \\ 2.19788 E1 \\ 0.001 \\ 5.4116 E 2 \\ 5.41178 E2 \\ 39.50 \\ 1.6566 E1 \\ 1.65662 E1 \\ 0.000 \\ 1.8789 E 2 \\ 1.87898 E 2
}

\subsection*{3.5 Remarks}

\section*{Contents of the file results:}

Values of the displacement of component DX of the P4 point for all the frequencies from 5 to 40 Hz by step of 0.5 (Case initial test of VPCS).

Values the speed and the acceleration of component DX of the P4 point for some frequencies of vibration.

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\section*{4 Modeling \\ B}

\section*{4.1 \\ Characteristics of modeling}

\section*{Discrete element of rigidity in translation}
y
With
P
P
B
X
1
2
P3
P4
P5
P6
P7
P8

Characteristics of the elements
DISCRETE:
with nodal masses
M_T_D_N
and matrices of rigidity
K_T_D_L
and matrices of damping
A_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the nodes ends
(GROUP_NO: AB DX: 0. )
Names of the nodes:
Not A = N1
P1 \(=\mathrm{N} 2\)
Not B = N10
\(\mathrm{P} 2=\mathrm{N} 3\)
\(\mathrm{P} 8=\mathrm{N} 9\)

\section*{4.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and types: 9 SEG2

\subsection*{4.3 Functionalities}
tested

\section*{Orders}

DISCRETE AFFE_CARA_ELEM GROUP_MA "K_T_D_L"
GROUP_MA
"A_T_D_L"
GROUP_MA
"M_T_D_N"
"MECHANICAL" AFFE_MODELE VERY "DIS_T"
GROUP_NO
"DIS_T"
AFFE_CHAR_MECA DDL_IMPO GROUP_NO

\author{
FORCE_NODALE \\ NODE \\ MODE_ITER_SIMULT \\ MACRO_PROJ_BASE \\ DYNA_LINE_HARM MATR_AMOR
}

REST_BASE_PHY
DEFI_LIST_REEL BEGINNING

INTERVAL

\section*{RECU_FONCTION LIST_FREQ}

\section*{TEST_FONCTION}

TEST_RESU
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\section*{4.4}

\section*{Results of modeling B}

Parts real and imaginary of component DX of the displacement of the P 4 point.

\section*{Frequency Reference}

\section*{Aster \%}

Difference
5.00
1.0237 E 4
1.02369 E 4
0.0004
8.5187 E6
8.51874 E6
5.50
4.5066 E 4
4.50662 E 4
0.0004

\author{
7.7914 E 4 \\ 7.79143 E4 \\ 6.00 \\ 9.4101 E 5 \\ 9.41096 E5 \\ 0.0002 \\ 1.0585 E 5 \\ 1.05851 E 5 \\ 10.00 \\ 8.4143 E7 \\ 8.41427 E7 \\ 0.0024 \\ 1.0335 E6 \\ 1.03346 E 6 \\ 15.00 \\ 1.2656 E 5 \\ 1.26556 E5 \\ 0.0032 \\ 5.6652 E6 \\ 5.66517 E6 \\ 20.00 \\ 2.9784 E6 \\ 2.97844 E6 \\ 0.0003 \\ 6.6970 E6 \\ 6.69700 E6 \\ 25.00 \\ 1.2536 E6 \\ 1.25362 E6 \\ 0.0008 \\ 5.2703 E 6 \\ 5.27033 E6 \\ 30.00 \\ 2.0904 E6 \\ 2.09042 E6 \\ 0.0009 \\ 5.4821 E6 \\ 5.48215 E6 \\ 35.00 \\ 4.5447 E6 \\ 4.54473 E6 \\ 0.0011 \\ 1.1190 E6
}
1.11903 E6
39.50
2.6895 E6
2.68949 E6
0.0003
3.0505 E 7
3.05048 E7

\subsection*{4.5 Remarks}

\section*{Contents of the file results:}

Values of the displacement of component DX of the P4 point for all the frequencies from 5 to 40 Hz by step of 0.5 (Case initial test of VPCS).

Values the speed and the acceleration of component DX of the P4 point for some frequencies of vibration.

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\section*{5 Modeling}

C
5.1

Characteristics of modeling

\section*{Discrete element of rigidity in translation}
y
With
P
P
B
X
1
2
P3
P4
P5
P6
P7
P8

Characteristics of the elements
DISCRETE:
with nodal masses
M_T_D_N
and matrices of rigidity
K_T_D_L
and matrices of damping
A_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the nodes ends
(GROUP_NO: AB DX: 0. )
Names of the nodes:
Not A = N1
P1 = N2
Not B = N10
\(\mathrm{P} 2=\mathrm{N} 3\)

P8 = N9

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and types: 9 SEG2

\author{
5.3 Functionalities \\ tested \\ Orders \\ DISCRETE AFFE_CARA_ELEM GROUP_MA "K_T_D_L" \\ GROUP_MA \\ "A_T_D_L" \\ GROUP_MA \\ "M_T_D_N" \\ "MECHANICAL" AFFE_MODELE VERY "DIS_T" \\ GROUP_NO \\ "DIS_T" \\ AFFE_CHAR_MECA DDL_IMPO GROUP_NO \\ FORCE_NODALE \\ NODE \\ MODE_ITER_SIMULT MATR_AMOR \\ DYNA_LINE_HARM AMOR_REDUIT
}

\section*{DEFI_LIST_REEL BEGINNING}

INTERVAL
RECU_FONCTION LIST_FREQ

\section*{Handbook of Validation}

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\section*{5.4 \\ Results of modeling \(\mathbf{C}\)}

Eigen frequencies of the structure for the sequence numbers from 1 to 5 .

\section*{Sequence number}

\section*{Reference}

Aster \%
Difference

\section*{1}
5.52715 .5271848238694
0.002

2
10.88681 .088524727521
-0.014
3
15.91551 .5910519939851
-0.031
4
20.460620 .449995091940
-0.052
5
24.38424 .366059022201
-0.074

Damping reduce structure for the sequence numbers from 1 to 5 .
```

Sequence number
Reference
Aster %
Difference
1
0.00868241 8.6824088833463D-03
1.29E-05
2
0.017101 1.7101007166284D-02
4.19E-05
3
0.025 2.5000000000002D-02
9.19E-12
4
0.0321394 3.2139380484326D-02
6.07E-05
5
0.0383022 3.8302222155950D-02
5.78E-05

```

\section*{6 \\ Summary of the results}

The results obtained are excellent, which is normal for a direct integration.
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SDLD22 - Transient of a system mass-arises to 8 ddl
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E. BOYERE Key

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\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
Document: V2.01.022

SDLD22-Transient of a system mass-arises
to 8 ddl with viscous shock absorber

\section*{Summary:}

The mechanical structure considered is made up of a linear one-way whole of mass-springs with shock absorbers viscous and subjected to a transitory excitation of crenel type.

Two modelings are developed. The first retains only the degree of freedom in axial translation of masses, the second considers the axial translation and rotation.

This problem makes it possible to test:
discrete elements (masses, springs, shock absorbers) in translation-rotation, the definition of a force of specific excitation transitory,
[U4.53.21]), as well as the recovery with initial conditions (modeling A),
the operator of calculation of direct transitory response with the diagram to step of adaptive time (DYNA_LINE_TRAN [U4.53.02]) (modeling B).

In addition, several operators of postprocessing are tested: RECU_FONCTION [U4.32.03], TEST_FONCTION
[U4.92.02], RECU_CHAMP [U4.71.01].
The results obtained (field of displacements, speeds) are in concord with the results of the guide VPCS, taken for reference solution.
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1
Problem of reference

\subsection*{1.1 Geometry}

U1
U2
U3
U8
K
K
K
K
With

Specific masses:
\(m P=m=m=\ldots \ldots .=m=m\)
1
P2
P3
P8
Stiffnesses of connection:
\(k A P 1=k P 1 P 2=k P 2 P 3=\ldots \ldots . .=k P 8 B=K\)
Viscous damping:
\(c A P 1=c P 1 P 2=c P 2 P 3=\ldots \ldots .=c P 8 B=C\)

\section*{1.2}

Material properties
Comes out from linear elastic translation
\(K=\)
\(105 \mathrm{~N} / \mathrm{m}\)
Specific mass
\(m=\)
10 kg
One-way viscous damping
\(C=\)
\(50 N(\mathrm{~m} / \mathrm{s})\)

\section*{1.3 \\ Boundary conditions and loadings}

Boundary conditions: embedded points \(A\) and \(B(U=0)\).
Loading: force concentrated at the P4 point in the shape of crenel:
Not \(P\)
\(=\)
\(=\)
4
\(F\)
\(F(T)\)

\subsection*{1.4 Conditions} initial

For \(T=0\), in any point, \(U=0\) and
\(=0\).
\(d t\)
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2
Reference solution
The reference solution is from guide VPCS.

\section*{2.1 \\ Method of calculation used for the reference solution} finished differences, of the method type - Newmark improved, with step of time of 0.001s [bib2].
```

l
l
1
l
M+

```
\(C+\)
Kun+2 = \((N\)
\(\boldsymbol{F}+2+N\)
F \(1+\)
\(+\)
\(N\)
\(\boldsymbol{F}\) )
2
\(T\)
\(2 T\)
3
2
1
1
1
1
\(+\)
\(\boldsymbol{K} \boldsymbol{U}\)
\begin{tabular}{l}
+ \\
+ \\
- \\
\(\mathbf{C}\) \\
\hline
\end{tabular}
M +
K
2
\(T\)

The displacement of item 4 according to time takes the following form:

\section*{Appear 2.1-a: Point 4: displacement according to time}

\section*{2.2 \\ Results of reference}

Displacement according to \(X\) of the \(P 4\) point.

\section*{2.3 \\ Uncertainty on the solution}

Precision of the diagram of Newmark.

\subsection*{2.4 References \\ bibliographical}
[1]
Card-index SDLD22/90 of commission VPCS.
[2]
NEWMARK NR. Mr.: "A method of computation for structural dynamics", proceeding ASCE J. Eng. Mech. Div E-3, July 1959, pp 67-94.

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Version
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Titrate:

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

This modeling allows the validation of integration by modal recombination.

\section*{Discrete element of rigidity in translation}
\(y\)
With

\section*{\(P\) \\ \(P\) \\ B \\ X \\ 1 \\ 2 \\ P3 \\ P4 \\ P5 \\ P6 \\ P7 \\ P8}

Characteristics of the elements
DISCRETE with
nodal masses
\(M_{-} T_{-} D_{-} N\)
\(M_{-} T \_N\)
matrices of rigidity
\(K \_T \_D \_L\)
\(K_{-} T \_L\)
matrices
of damping
A_T_D_LA_T_L
Blocking of the DDL in \(Y\) and \(Z\) of all the nodes

DDL_IMPO: (ALL: "YES" DY: 0. , DZ: 0. )
Boundary conditions with the extreme nodes
(GROUP_NO: AB DX: 0. )
Names of the nodes:
Not \(A=N 1\)
\(P 1=N 2\)
Not \(B=N 10\)
\(P 2=N 3\)
\(P 8=N 9\)
Modal recombination with all the modes (either 8),
diagram of EULER, resumption of the first calculation with \(T=0.455 \mathrm{~S}\)
no time used: \(T=1\). E3 \(S\).

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and types: 9 SEG2

\subsection*{3.3 Functionalities \\ tested}

Orders

\title{
DISCRETE AFFE_CARA_ELEM GROUP_MA " \(K_{-} T \_D \_L\) "
}

\author{
GROUP_MA \\ "A_T_D_L" \\ GROUP_NO \\ " \(M_{-} T_{-} D_{-} N\) " \\ "MECHANICAL" AFFE_MODELE VERY \\ "DIS_T" \\ GROUP_NO \\ "DIS_T" \\ AFFE_CHAR_MECA DDL_IMPO GROUP_NO \\ FORCE_NODALE \\ NODE \\ MODE_ITER_INV ADJUSTS \\ DYNA_TRAN_MODAL MATR_AMOR EULER \\ \section*{NOT} \\ 0.001 \\ DEFI_LIST_REEL BEGINNING
}

INTERVAL

\section*{RECU_FONCTION LIST_INST}

\author{
Handbook of Validation
}

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Code_Aster \({ }^{\circledR}\)
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

Time Reference Aster \%
Difference
0.09
4.02 E5
4.022 E5
0.05
0.18
4.22 E6
3.973 E6
-5.8
0.27
3.89 E5
3.902 E5
0.32
0.37
5.98 E6
5.750 E6
2.791 E5
0.31
1.45
2.65 E5
2.652 E5
0.09

\subsection*{4.2 Remarks}

Relative minima ( \(T=0.18,0.54, \ldots\) ) do not have a very good precision during the phase of excitation with a step \(T=0.001\).

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\section*{5 Modeling}

\section*{B}

\section*{5.1}

\section*{Characteristics of modeling}

This modeling allows, in addition to a new use of the modal recombination, the validation of direct integration with adaptive step.

\section*{Discrete element of rigidity in translation and rotation}

\section*{Characteristics of the elements:}

DISCRETE:
with nodal masses
\(M \_T R \_D \_N\)
\(M \_T R \_N\)
and matrices of rigidity
K_TR_D_L
\(K \_T R \_L\)
and matrices of damping
A_TR_D_LA_TR_L

Boundary conditions and directions blocked:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
(ALL: "YES" DRX: 0. DRY: 0 DRZ: 0)
with the nodes ends
(GROUP_NO: AB DX: 0. )
Direct integration by DYNA_LINE_TRAN, algorithm ADAPT, not of time max 103 S .
Integration by modal recombination on all the modes, diagram of Euler.

\section*{5.2 \\ Characteristics of the grid \\ A number of nodes: 10 \\ A number of meshs and types: 9 SEG2}

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

DISCRETE AFFE_CARA_ELEM GROUP_MA
" \(K_{-} T R_{-} D_{-} L\) "

\author{
GROUP_MA \\ "A_TR_D_L" \\ GROUP_MA \\ "M_TR_D_N" \\ "MECHANICAL" AFFE_MODELE VERY \\ "DIS_T" \\ GROUP_NO \\ "DIS_T" \\ AFFE_CHAR_MECA DDL_IMPO GROUP_NO
}

NODE

CALC_MATR_ELEM OPTION
"MASS_MECA_DIAG"

MODE_ITER_INV ADJUSTS

NOT
0.001

RECU_FONCTION LIST_INST

REST_BASE_PHYS INTERPOL "FLAX"

DYNA_LINE_TRAN ADAPT

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6
Results of modeling B
6.1 Values
tested

\section*{Transient by modal recombination}

\author{
Time Reference Aster \% Difference \\ 0.09 \\ 4.02 E5 \\ 4.022 E 5 \\ 0.05 \\ 0.18 \\ 4.22 E6 \\ 3.973 E6 \\ -5.8 \\ 0.27 \\ 3.89 E5 \\ 3.902 E5 \\ 0.32 \\ 0.37 \\ 5.98 E6 \\ 5.750 E6 \\ -3.83 \\ 0.46 \\ 3.73 E5 \\ 3.746 E5 \\ 0.43 \\ 0.54 \\ 7.14 E6 \\ 6.977 E6 \\ -2.27 \\ 0.63 \\ 3.64 E5 \\ 3.646 E5 \\ 0.16 \\ 0.72 \\ 8.07 E6 \\ 7.923 E6 \\ -1.81 \\ 0.81 \\ 3.58 E5 \\ 3.586 E5 \\ 0.18 \\ 0.9 \\ 8.76 E6 \\ 8.861 E6 \\ -1.12
}

\section*{Direct transient}

\section*{Time Reference Aster \%}

Difference
0.09
4.02 E5
4.022 E5
0.06
0.18
4.22 E6
4.000 E6
\(-5.19\)
0.27
\(3.89 E 5\)
3.900 E5
0.27
0.37
5.98 E6
```

5.764 E5

```
-3.60
0.46
3.73 E5
3.743 E5
0.36
0.54
7.14 E6
6.990 E6
-2.10
0.63
3.64 E5
3.645 E5
0.14
0.72
8.07 E6
7.936 E6
-1.64
0.81
3.58 E5
\(3.586 E 5\)
0.17
0.9
8.76 E6
8.663 E6
-1.09
0.99
3.52 E5
3.531 E5
0.32
1.08
3.08 E5
3.078 E5
-0.04
1.18
3.02 E5
\(3.023 E 5\)
0.11
1.27
2.88 E5
2.884 E5
0.15
1.36

\subsection*{6.2 Remarks}

Modelings A and B lead to the same results.
Relative minima ( \(T=0.18,0.54, \ldots\) ) do not have a very good precision during the phase of excitation with a step \(T=0.001\).

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V2.01 booklet: Linear dynamics of the discrete systems
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.0

\section*{Titrate:}

SDLD22-Transient of a system mass-arises to 8 ddl
Date:
17/02/04
Author (S):
E. BOYERE Key

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\section*{7 \\ Summary of the results}

This test is to be supplemented while using:
a step of time \(T=1 . E 4\), other diagrams of integration.
Handbook of Validation

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLD23 System of masses and springs under random excitation
Date:
30/08/01
Author (S):
J. PIGAT

Key: V2.01.023-B Page:
1/6

Organization (S): EDF/RNE/AMV

Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
V2.01.023 document

SDLD23 - System of masses and springs
under random excitation

\section*{Summary:}

This test is in the course of validation within the framework of the VPCS.
It comprises a whole of eight specific masses and nine springs excited by an imposed random force on one of the masses.

The excitation is of white vibration type. It is given by the spectral concentration of power of the exiting force.

The movement of the excited mass is calculated by a stochastic approach according to different frequential discretizations for the answer.

One also calculates in postprocessing the spectral moments of the answer.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
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Code_Aster \({ }^{\circledR}\)
Version
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Titrate:
SDLD23 System of masses and springs under random excitation
Date:
30/08/01
Author (S):
J. PIGAT

Key: V2.01.023-B Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(d x\)
\(\boldsymbol{K}\)
P1
K
P2
K
\(P\)

The excitation is a seismic movement of type forces imposed applied to the P4 point in the direction \(d x\).

One is interested in the DSP of displacement of the P4 node.
1.2

Material properties
Specific masses:
\(m=10 \mathrm{~kg}\)
Elastic springs:
\(K=105 \mathrm{~N} / \mathrm{m}\)
Shock absorbers:
\(C=50 \mathrm{~N}(\mathrm{~m} / \mathrm{s})\)

\section*{1.3}

Boundary conditions and loadings
The problem is unidimensional in direction \(X\) ( 1 ddl by mass).
The excitation is a DSP of constant force of level 1, between 3 and 13 Hz .

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Code_Aster \({ }^{\circledR}\)
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5.0

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SDLD23 System of masses and springs under random excitation
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Author (S):
J. PIGAT

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2
Reference solution
2.1

Method of calculation used for the reference solution

The solution taken for reference results from test SDLD23 of guide VPCS [bib1].
> 2.2

> Results of reference
> Peak of the response to the first Eigen frequency.
> Values of the first spectral moments for various discretizations.
2.3 Reference

\author{
Guide VPCS. \\ Handbook of Validation
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLD23 System of masses and springs under random excitation
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Author (S):
J. PIGAT

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Discrete element in translation of the type DIS_T}

Modeling respects the geometry.
Characteristics of the elements:
With the P1 nodes in P8: matrices of masses of the type M_T_D_N with \(m=10 \mathrm{~kg}\). Elements of spring: a matrix of stiffness of the type K_T_D_L with Kx = 105 N/m Elements of damping: a matrix of damping of the type A_T_D_L with cx \(=50 \mathrm{~N} / \mathrm{m}\)

Boundary conditions:
All the DDL are blocked except the DDL dx.
Modal damping is calculated by the operator of modal calculation, it is reinjected like modal damping in random dynamic calculation.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 10
A number of meshs and types: 9 SEG2, 10 POI1

\subsection*{3.3 Functionalities tested}

\section*{Orders}

\section*{DEFI_INTE_SPEC PAR_FONCTION}

\author{
DYNA_ALEA_MODAL BASE_MODALE AMOR
}

\section*{EXCIT}

SIZE
"EFFO"
ANSWER

REST_SPEC_PHYS

\section*{POST_DYNA_ALEA MOMENT}

\section*{Handbook of Validation}

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Author (S):
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\section*{4 \\ Results of modeling A}

\subsection*{4.1 Values tested}

\section*{DSP of displacement to the P4 node}

Identification Reference Aster \%
Difference
ABSOLUTE: \(\mathrm{F}=5.5259 \mathrm{~Hz}\)
0.1059 E 5
0.1059 E 5

0\%

\section*{Spectral moments for the discretization by defect (50 points per peak)}

\section*{Identification Reference Aster \%}

Difference
Spectral moment \(\mathrm{n}^{\circ} 0\)
1.585 10-7 1.618

10-7 2.11\%
Spectral moment \(\mathrm{n}^{\circ} 2\)
1.902 10-4 1.942

10-4 2.12\%
Spectral moment \(\mathrm{n}^{\circ} 4\)
2.322 10-1 2.370

10-1 2.09\%
Spectral moment \(\mathrm{n}^{\circ} 6\)
2.9411023 .001

102 2.05\%
Spectral moment \(\mathrm{n}^{\circ} 8\)
4.1431054 .226

105 2.00\%
Spectral moments for the discretization with regular step 0.25 Hz ( 40 steps)
Identification Reference Aster \%
Difference
Spectral moment \(\mathrm{n}^{\circ} 0\)
1.585 10-7 2.339

10-7 47.61\%
Spectral moment \(\mathrm{n}^{\circ} 2\)
1.902 10-4 2.790

10-4 46.73\%
Spectral moment \(\mathrm{n}^{\circ} 4\)
2.322 10-1 3.368

10-1 45.05\%
Spectral moment \(\mathrm{n}^{\circ} 6\)
2.9411024 .173

102 41.91\%
Spectral moment \(\mathrm{n}^{\circ} 8\)
4.1431055 .600

105 35.17\%

\section*{Spectral moments for the discretization with regular step 0.05 Hz ( 200 steps)}

\section*{Identification Reference Aster \%}

Difference
Spectral moment \(\mathrm{n}^{\circ} 0\)
1.585 10-7 1.577

10-7-0.44\%
Spectral moment \(n^{\circ} 2\)
1.902 10-4 1.893

10-4 -0.46\%
Spectral moment \(\mathrm{n}^{\circ} 4\)
2.322 10-1 2.311

10-1 -0.48\%
Spectral moment n \({ }^{\circ} 6\)
2.9411022 .928

102-0.43\%
Spectral moment \(\mathrm{n}^{\circ} 8\)
4.1431054 .130

105-0.30\%
Spectral moments for the discretization with regular step 0.025 Hz ( 400 steps)
Identification Reference Aster \%
Difference
Spectral moment \(\mathrm{n}^{\circ} 0\)
1.585 10-7 1.585

10-7 0.02\%
Spectral moment \(\mathrm{n}^{\circ} 2\)
1.902 10-4 1.902

10-4 0.01\%
Spectral moment \(\mathrm{n}^{\circ} 4\)
2.322 10-1 2.322

10-1 -0.02\%
Spectral moment \(n^{\circ} 6\)
2.9411022 .941

Spectral moment \(\mathrm{n}^{\circ} 8\)
4.1431054 .144

105 0.03\%

\subsection*{4.2 Parameters \\ of execution}

Version: 5.02

Machine: SGI ORIGIN 2000
System:
Obstruction memory: 8 megawords, time CPU To use:
3.60 seconds

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\section*{Titrate:}

SDLD23 System of masses and springs under random excitation
Date:
30/08/01
Author (S):

\section*{J. PIGAT}

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\section*{5 \\ Summary of the results}

The preceding tables highlight the importance of the smoothness of the discretization frequential of the DSP response for the calculation of the spectral moments.

The user can choose the step: the frequency band is then discretized in a uniform way and it or the peaks can be badly represented: it is the case with 40 steps of 0.25 Hz , which involves an error
of more than \(40 \%\) at the spectral time.
More one refines the discretization, better is the result.
To avoid refining unnecessarily far from the peaks, one proposes a discretization by rather broad defect supplemented by a refinement of 50 points of discretization around each peak.

In the case of this test which includes/understands one peak, this discretization by defect makes it possible to estimate
spectral moments with a precision of about \(2 \%\).

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\(V 2.01\) booklet: Linear dynamics of the discrete systems
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\section*{Code_Aster ®}

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4.0

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SDLD25 Système mass-arises with viscous shock absorber
Date:
05/01/98
Author (S):

\section*{P. GUIHOT}

Key:
V2.01.025-B Page:
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Organization (S): EDF/EP/AMV

\section*{Handbook of Validation}

\section*{V2.01 booklet: Linear dynamics of the discrete systems}

\section*{V2.01.025 document}

\section*{SDLD25 - System mass-arises with shock absorber}
viscous proportional (spectral answer)

\section*{Summary}

This one-way problem consists in carrying out a spectral seismic analysis of a mechanical structure composed of a whole of mass-springs with viscous shock absorbers subjected to a seismic request provided in the shape of a spectrum of response of pseudo oscillators in acceleration.
Via this problem, one tests modal combination SRSS of operator COMB_SISM_MODAL
[U4.54.04]. In addition, several operators of preprocessing are tested; DEFI_FONCTION and DEFI_NAPPE.
The results obtained are in concord with the results of guide VPCS.
This test is also a test of resorption of LICE. There are no differences between the ASTER results and them
LICE results.

\section*{Handbook of Validation}

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\section*{Code_Aster \({ }^{\circledR}\)}

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SDLD25 Système mass-arises with viscous shock absorber
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05/01/98
Author (S):
P. GUIHOT

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1
Problem of reference

\subsection*{1.1 Geometry}

U1
U2
U3
U8
K
K
K
K
With
m
m
m
m
B
X, U
P1
P2
P3
P8
C
C
C
C
Specific masses:
```

mP=m=m=_···..= m=m

```
1
P2
P3
P8

Stiffnesses of connection:
\(\mathrm{kAP} 1=\mathrm{kP} 1 \mathrm{P} 2=\mathrm{kP} 2 \mathrm{P} 3=\ldots \ldots .=\mathrm{kP} 8 \mathrm{~B}=\mathrm{K}\)
Viscous depreciation:
\(\mathrm{cAP} 1=\mathrm{cP} 1 \mathrm{P} 2=\mathrm{cP} 2 \mathrm{P} 3=\ldots \ldots .=\mathrm{cP} 8 \mathrm{~B}=\mathrm{C}\)

\section*{1.2}

Material properties
Comes out from linear elastic translation
K =
105 N/m
Specific mass
m =
10 kg
One-way viscous damping
C =
\(50 \mathrm{~N}(\mathrm{~m} / \mathrm{s})\)
1.3

\section*{Boundary conditions and loadings}

Not A and b: embedded \((\mathrm{U}=0)\)
Spectrum of acceleration to supports Ý
Ý
U (F, has) normalized with 1. ms2
Points A and b: Ý
Ý
\(\mathrm{U}=\hat{Y}\)
Ý
U ( F, has)
ms2
25
\(0.5 \%\)
5\%
10

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Author (S):
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2
Reference solution
2.1

Method of calculation used for the reference solution
Comparison with other codes.
Guide VPCS: test in preparation.
2.2

Results of reference
Absolute acceleration according to X at the points \(\mathrm{A}, \mathrm{P} 1, \mathrm{P} 2, \mathrm{P} 3, \mathrm{P} 4\).

\subsection*{2.3 References}

\section*{bibliographical}

\section*{[1]}

Guide VPCS: Second edition to be appeared.
[2]
J. PIRANDA: Note of use of the software of modal analysis MODAN - Version 0.2.

Laboratory of Mechanics Applied - University of Frank County Besancon (1990).
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HP-52/96/042 - Ind A

\section*{Code_Aster ®}

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4.0

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SDLD25 Système mass-arises with viscous shock absorber
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Author (S):

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3 Modeling
With
3.1

Characteristics of modeling
y
With
P
P
P
P
P
P
P
B
X

Characteristics of the elements:
DISCRETE:
with nodal masses
M_T_D_N
and matrices of rigidity
K_T_D_L
and matrices of damping
A_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the nodes ends
(GROUP_NO: AB DX: 0. )
Names of the nodes:

Not \(\mathrm{A}=\mathrm{N} 1\)
P1 = N2
Not \(\mathrm{B}=\mathrm{N} 10\)
\(\mathrm{P} 2=\mathrm{N} 3\)
\(\mathrm{P} 8=\mathrm{N} 9\)
3.2

Characteristics of the grid
A number of nodes: 10
A number of meshs and types: 9 SEG2

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_MODELE
ALL
"MECHANICAL"
"DIS_T"
[U4.22.01]
GROUP_NO
"DIS_T"
AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_T_D_L"
[U4.24.01]
GROUP_MA
"A_T_D_L"
GROUP_NO
"M_T_D_N"
COMB_SISM_MODAL
[U4.54.04]
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HP-52/96/042 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLD25 Système mass-arises with viscous shock absorber
Date:
05/01/98

\section*{Author (S): \\ P. GUIHOT}

Key:
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4
Results of modeling A

\subsection*{4.1 Values}
tested
Identification

\section*{Reference}

Aster
\% difference
Eigen frequencies
1
5.53
5.527
-0.04
2
10.89
10.886
-0.02
3
15.92
15.915
-0.02
4
20.46
20.460
0.00

5
24.38
24.383
0.01

6
27.57
27.566
-0.01
7
29.91
29.911
0.00

8
31.35
31.347
-0.01
Size localization
ACCE_ABSOLU A DX
1.0
1.0
1.0

P1 DX
10.69
10.45
2.24

P2 DX
19.26
19.03
1.19

P3 DX
25.46
25.32
0.55

P4 DX
29.16
28.95
0.73
4.2 Remarks

Mode
1
2
3
4
5
6
7
8
Damping (in \%)
0.868
1.710
2.500
3.213
3.830
4.331
4.698
4.924

\title{
4.3 Parameters
}

\author{
of execution
}

Version: 3.05.02
Machine: CRAY
System: UNICOS 8.0
Obstruction memory: 8 megawords
Time CPU To use: 5.75 seconds
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

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SDLD25 Système mass-arises with viscous shock absorber
Date:
05/01/98
Author (S):

\section*{P. GUIHOT}

Key:
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5
Summary of the results
One obtains a good agreement between the Aster solution and the reference solution which comes from other codes.
The Aster results are identical to the LICE results until the second decimal.
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Code_Aster \({ }^{\circledR}\)
Version

\section*{Titrate:}

SDLD27-System mass-arises with 8 degrees of freedom
Date:
17/02/04
Author (S):
E. BOYERE Key

V2.01.027-C Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
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V2.01.027 document

SDLD27-System mass-arises with 8 degrees
of freedom with viscous shock absorber not
proportional (analyzes modal)

\section*{Summary:}

This two-dimensional problem consists in seeking the frequencies, the modes of vibration and them depreciation of a mechanical structure made up of masses, springs and shock absorbers viscous. It
case-test of Mechanics of the Structures corresponds to a dynamic analysis of a discrete model having one
linear behavior.
This test allows a complete validation of the options of discrete modeling of rigidity, of damping viscous and of mass (without finite elements) offered by order AFFE_CARA_ELEM [U4.42.01]. Five different modelings are proposed: the modeling of the discrete elements is either in translation, or in translation/rotation and is written either in total reference mark, or in local reference mark. In addition, different
functionalities of orders MODE_ITER_INV [U4.52.04] (search for eigenvalues per iteration opposite), MODE_ITER_SIMULT [U4.52.03] (search for eigenvalues by the method of Lanczos) and NORME_MODE [U4.52.11] (definition of the standard of a clean vector) are tested for this problem quadratic.

This test refers to a test VPCS, but it was modified. Indeed, the Aster test directs the mechanical system
on an axis \(3 y=4 x\), which makes it possible to validate the entry of the data in local reference mark. The results obtained are in concord with the results of reference.

\section*{Handbook of Validation}

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SDLD27-System mass-arises with 8 degrees of freedom
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Author (S):
E. BOYERE Key

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1
Problem of reference

\subsection*{1.1 Geometry}

U1
U2
U3
U8
K
K
K
K
With
\(X, U\)
p1
p2
p3
p8
DC
C
C
\(C d\)
```

Specific masses:
mP=m=m= ..... = m=m
l
P2
P3
P8
Stiffnesses of connection:
$k A P 1=k P 1 P 2=k P 2 P 3=\ldots \ldots . .=k P 8 B=K$
Viscous damping:
$c P 1 P 2=c P 2 P 3=\ldots \ldots=C P 7 P 8=C$
$c A P 1=D C$
$C P 8 B=C d$

```

\section*{1.2 \\ Material properties}
```

Comes out from linear elastic translation
$K=$
$105 \mathrm{~N} / \mathrm{m}$
Specific mass

```
```

m=

```
m=
10 kg
10 kg
One-way viscous shock absorbers
\(C=\)
\(50 N(\mathrm{~m} / \mathrm{s})\)
\(D C=\)
\(250 \mathrm{~N}(\mathrm{~m} / \mathrm{s})\)
\(C d=\)
\(25 N(\mathrm{~m} / \mathrm{s})\)
```


## 1.3

```
Boundary conditions and loadings
```

Embedded points A and B: $(U=0)$.

### 1.4 Conditions

## initial

Without object for the modal analysis.
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## 2

## Reference solution

## 2.1 <br> Method of calculation used for the reference solution

The reference solution is that given in card SDLD27 of guide VPCS.
The problem led to seek the eigenvalues and clean vectors of the dissipative system according to:
$\boldsymbol{M} \& \boldsymbol{u}+\boldsymbol{C} \& u+\boldsymbol{K} \boldsymbol{u}=0$
with $\boldsymbol{M}$ stamps of mass, $\boldsymbol{C}$ stamps damping, $\boldsymbol{K}$ stamps rigidity.
One associates this dissipative problem, the conservative problem: $\mathbf{K u}+\mathbf{M u \&}=0$. In form harmonic, it is written $\boldsymbol{K}-2 M=0$.

Are $=[2 v]$ the spectral diagonal matrix of the eigenvalues of this conservative system and $=[v]$ the corresponding matrix of the clean vectors.

The solutions of the dissipative system are form:
$U U$ is
from where $(m s+C s+K)) u o=0$.
One breaks up uo in the base of the $v$. There is then $U$
$Q$
$O=$, from where:
$(I s 2+s+) Q=0$ with $=T$
$C$ (full matrix)
This problem with the eigenvalues is solved by a method of power reverses while taking for initial estimate $S=J$
$v$
$v$.

## 2.2

Results of reference
8 depreciation and Eigen frequencies of the system, as well as 1st and the 8th mode (complexes).

## 2.3 <br> Uncertainty on the solution

Semi-analytical solution.

### 2.4 References <br> bibliographical

[1]
J. PIRANDA - Note of use of the software of modal analysis MODAN - Version 0.2 (1990)

# Guide VPCS. Complement Groups Dynamic. September 94 

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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

Discrete element of rigidity in translation DIS_T

## Characteristics of the elements

## DISCRETE:

with nodal masses

in all the nodes
$M \_T \_D \_N$
in absolute reference mark
( $m=10$.)
matrices of rigidity
in all the meshs
$M_{-} T \_D \_L$
in absolute reference mark
( $K x=1.105$ )
matrices of damping
internal meshs
$A \_T \_D \_L$
in absolute reference mark
( $C x=50$.)
initial mesh
A_T_D_L
in absolute reference mark
( $C x=250$.)
net final
A_T_D_L
in absolute reference mark
( $C x=25$.)
Limiting conditions:
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the nodes ends
(NODE: (A B) DX: 0. )
Names of the nodes: With, P1, P2,...., P8, B

## 3.2 <br> Characteristics of the grid

A number of nodes:
10
A number of meshs and types:
9 SEG2 and 8 POII

3.3 Functionalities<br>tested<br>Orders<br>DISCRETE AFFE_CARA_ELEM<br>GROUP_MA<br>"K_T_D_L"<br>" $A_{-} T_{-} D_{-} L$ "<br>NODE<br>" $M_{-} T_{-} D_{-} N$ "<br>AFFE_CHAR_MECA DDL_IMPO<br>ALL<br>LIAISON_DDL<br>NODE<br>AFFE_MATERIAU ALL

AFFE_MODELE ALL<br>"MECHANICAL"<br>"DIS_T"<br>GROUP_NO<br>"DIS_T"<br>DEFI_MATERIAU ELAS

MODE_ITER_SIMULT METHOD
"TRI_DIAG"
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
NORM_MODE NORMALIZES MASS_GENE
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
6.4
Titrate:
SDLD27-System mass-arises with 8 degrees of freedom
Date:
17/02/04
Author (S):

## E. BOYERE Key

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## 4

Results of modeling $A$

### 4.1 Values

tested

## Frequency Reference

Aster \%
Difference
Order of the clean mode 1
5.53
5.529

## -0.015

Order of the clean mode 2
10.90
10.896
-0.037
Order of the clean mode 3
15.93
15.927
-0.019
Order of the clean mode 4
20.45
20.452
-0.011
Order of the clean mode 5
24.34
24.336
-0.019
Order of the clean mode 6
27.49
27.486
-0.019
Order of the clean mode 7
29.84
29.835
-0.01
Order of the clean mode 8
31.29
31.295
0.015

## Damping Reference

Aster \%
Difference
Order of the clean mode 1
1.521e-2
$1.5209 e-2$
-0.007
Order of the clean mode 2
$2.877 e-2$
$2.8757 e-2$
-0.043
Order of the clean mode 3
3.960e-2

```
3.9565e-2
```

-0.09
Order of the clean mode 4 $4.709 e-2$
$4.7034 e-2$
-0.119
Order of the clean mode 5
$5.098 e-2$
$5.0917 e-2$
-0.1240
Order of the clean mode 6
$5.183 e-2$
$5.1770 e-2$
-0.126
Order of the clean mode 7
$5.115 e-2$
$5.1084 e-2$
-0.128
Order of the clean mode 8
$5.036 e-2$
$5.0296 e-2$
-0.126

## Nature of the mode

Not Mode
proper clean Mode Aster
\% Difference

clean

Reference in 103
Real part
Real part
Imaginary part
Imaginary part

## Pl

4.07, -4.56
4.0735, -4.5552
0.096

P2
7.97, -8.28
7.9652, -8.2846
0.058

Translation 1

```
P3
10.9, -11.0
10.882,-11.026
0.205
(Dy)
P4
12.5,-12.5
12.468, -12.541
0.315
l
P5
12.5, -12.4
12.594,-12.398
0.168
P6
11.1, -10.9
11.058,-10.865
0.349
P7
8.24,-8.04
8.2355,-8.0376
0.045
P8
4.41,-4.25
4.4055,-4.2529
0.088
P1
2.23,-1.14
2.2336,-1.1391
0 . 1 5
P2
-3.71, 2.98
-3.7107, 2.9759
0.087
Translation 8
P3
4.75, -4.41
4.7547, -4.4145
0.101
(Dy)
P4
```

-5.25, 5.27
-5.2487, 5.2688
0.024

8
P5
5.14, -5.43
5.1389, -5.4291
0.019

P6
-4.44, 4.88
-4.4401, 4.8766
0.052

P7
3.23, -3.69
3.2340, -3.6852
0.128

P8
-1.66, 2.01
-1.6596, 2.0121
0.081

Clean mode normalized with the unit modal mass: $T$
$T$
$I C I+2 I I M I=1$
: is the eigenvalue associated damping and the Eigen frequency.

## 4.2 <br> Contents of the file results

8 depreciation and Eigen frequencies, as well as the associated clean vectors.
Handbook of Validation
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HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
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## Titrate:

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## 5 Modeling <br> B <br> 5.1 <br> Characteristics of modeling

Discrete element of rigidity in translation DIS_T
in all the nodes
$M \_T \_D \_N$
in absolute reference mark
( $m=10$.)
matrices of rigidity
in all the meshs
$K_{-} T \_D \_L$
in local reference mark
( $K x=1.105$ )
with the nodes ends
$K_{-} T \_D \_N$
in local reference mark
( $K x=1.105$ )
matrices of damping
internal meshs
$A_{-} T \_D \_L$
in local reference mark
( $C x=50$.)
initial mesh
$A_{-} T_{-} D_{-} N$
in local reference mark
( $C x=250$.)
net final
$A_{-} T_{-} D \_N$
in local reference mark
( $C x=25$.)
Limiting conditions:
DDL_IMPO:
(ALL: "YES" DZ: 0. )
LIAISON_DDL:
(such as 3Dy=4Dx in all the nodes)
Names of the nodes: P1, P2,...., P8
5.2

Characteristics of the grid

## A number of nodes:

8
A number of meshs and types:
7 SEG2
The points P1 and P8 are connected to a fixed fictitious point by nodal springs ( $K_{-} T_{-} D_{-} N, A_{-} T_{-} D_{-} N$ ) it who allows not to model the nodes $A$ and $B$.

5.3 Functionalities<br>tested<br>\section*{Orders}<br>DISCRETE AFFE_CARA_ELEM GROUP_MA<br>" $K_{-} T \_D \_L$ "<br>$$
\text { " } A_{-} T_{-} D_{-} L \text { " }
$$<br>\section*{NODE}<br>"K_T_D_N"<br>> "A_T_D_N"<br>GROUP_NO<br>" $M_{-} T_{-} D_{-} N$ "<br>AFFE_CHAR_MECA DDL_IMPO ALL<br>LIAISON_DDL<br>NODE<br>AFFE_MATERIAU ALL

"MECHANICAL" AFFE_MODELE VERY
"DIS_T"
GROUP_NO
"DIS_T"
DEFI_MATERIAU ELAS

## MODE_ITER_INV CALC_FREQ

OPTION

## 6

## Results of modeling B

### 6.1 Values

tested

## Frequency Reference

Aster \%
Difference
Order of the clean mode 1
5.53
5.529
0.015

Order of the clean mode 2
10.90
10.897
0.025

Order of the clean mode 3

## Damping Reference

Aster \%

Difference
Order of the clean mode 1
1.521e2
$1.5208 e 2$
-0.007
Order of the clean mode 2
2.877e2
$2.8743 e 2$
-0.093
Order of the clean mode 3
3.960e2
$3.9497 e 2$
-0.259
Order of the clean mode 4 $4.709 e 2$
$4.6910 e 2$
-0.381

Order of the clean mode 5
5.098e2
$5.0792 e 2$
-0.369
Order of the clean mode 6
5.183e2
$5.1689 e 2$
-0.270
Order of the clean mode 7
5.115e2
$5.1056 e 2$
-0.184
Order of the clean mode 8
5.036e2
$5.0290 e 2$
-0.139
Nature of the mode
Not Mode
proper clean Mode Aster
\% Difference
clean
Reference in 103
Real part
Real part
Imaginary part
Imaginary part
P1
-2.442, 2.736
-2.4440, 2.7331
0.0963

P2
-4.782, 4.968
-4.7791, 4.9707
0.0577

Translation 1
P3
-6.54, 6.6
-6.5293 , 6.6158
0.2048
(Dy)
P4
P6
-6.66 , 6.54
-6.6351, 6.5189
0.3491
P7
-4.944, 4.824
-4.9413, 4.8226
0.0445
P8
-2.646, 2.55
-2.6433, 2.5518
0.0877
P1
-1.338, 0.684
$-1.2892,0.7585$
5.9257
P2
-2.226, 1.788
-2.2318, 1.7719
0.5976
Translation 8
P3
-2.85, 2.646
-2.8521, 2.6475
0.0659
(Dy)
P4
-3.15, 3.162
$-3.1509,3.1548$
0.1615
8
P5
-3.084, 3.258
0.1064

P6
-2.664, 2.928
-2.6657, 2.9208
0.1855

P7
-1.938, 2.214
-1.9401, 2.2089
0.1851

P8
-0.996, 1.206
$-1.0061,1.1985$
0.8025

Clean mode normalized with the unit modal mass: $T$
T
$I C I+2 I I M I=1$
: is the eigenvalue associated damping and the Eigen frequency.

## 6.2

## Contents of the file results

8 depreciation and Eigen frequencies, as well as the associated clean vectors.

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## Code_Aster ${ }^{\circledR}$

Version
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## Titrate:

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Date:
17/02/04
Author (S):
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:
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## 7 Modeling <br> C <br> 7.1 <br> Characteristics of modeling

Discrete element of rigidity in translation DIS_T
in all the meshs
K_T_L
in local reference mark ( $K x=1.105$ )
with the nodes ends
$K_{-} T \_N$
in local reference mark
( $K x=1.105$ )
matrices of damping
in all the meshs
$A_{-} T \_L$
in local reference mark
( $C x=50$.)
with the initial node
$A_{-} T \_N$
in local reference mark
( $C x=250$.)
with the final node
A_T_N
in local reference mark
( $C x=25$.)
Limiting conditions:
DDL_IMPO:
(ALL: "YES" DZ: 0. )
LIAISON_DDL:
(such as 3Dy=4Dx in all the nodes)
Names of the nodes: P1, P2,...., P8

## 7.2

## Characteristics of the grid

A number of nodes:
8
A number of meshs and types:
7 SEG2

The points P1 and P8 are connected to a fixed fictitious node by nodal springs ( $K_{-} T \_N, A_{-} T \_N$ ).
7.3 Functionalitiestested
Orders
DISCRETE AFFE_CARA_ELEM GROUP_MA
" $K_{-} T L_{-}$"
"A_T_L"
NODE
"K_T_N"
"A_T_N"
GROUP_NO
"M_T_N"
AFFE_CHAR_MECA DDL_IMPO ALL
LIAISON_DDL
NODE
AFFE_MATERIAU ALL
"MECHANICAL" AFFE_MODELE VERY
"DIS_T"
GROUP_NO
"DIS_T"
DEFI_MATERIAU ELAS
MODE_ITER_INV CALC_FREQ OPTION ..... "SEPARATE"
FREQ
NORM_MODE NORMALIZES ..... MASS_GENE

Handbook of Validation

## Code_Aster ${ }^{\circledR}$

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## 8 <br> Results of modeling $C$

### 8.1 Values

tested

## Frequency Reference

Aster \%
Difference
Order of the clean mode 1
5.53
5.529
-0.015
Order of the clean mode 2
10.90
10.897
-0.025
Order of the clean mode 3
15.93
15.934
0.026

Order of the clean mode 4
20.45
20.469
0.0931

Order of the clean mode 5
24.34
24.360
0.082

Order of the clean mode 6
27.49
27.510
0.074

Order of the clean mode 7
29.84
29.849
0.032

Order of the clean mode 8
31.29
31.298
0.027

## Damping Reference

Aster \%
Difference
Order of the clean mode 1
1.521e2
$1.5208 e 2$
-0.007
Order of the clean mode 2
2.877e2
2.8743e2
-0.093
Order of the clean mode 3
3.960e2
$3.9497 e 2$
-0.26
Order of the clean mode 4
4.709e2
4.6911e2
-0.379
Order of the clean mode 5
5.098e2
$5.0790 e 2$
-0.373
Order of the clean mode 6
5.183e2
5.1691e2

Order of the clean mode 7
5.115e2
$5.1062 e 2$
-0.171
Order of the clean mode 8
5.036e2
$5.0283 e 2$
-0.151
Nature of the mode
Not Mode
proper clean Mode Aster
\% Difference
clean
Reference in 103
Real part
Real part
Imaginary part
Imaginary part
P1
$-2.442,2.736$
-2.4441, 2.7331
0.096

P2
-4.782, 4.968
-4.7791, 4.9707
0.058

Translation 1
P3
-6.54 , 6.6
-6.5293, 6.6158
0.205
(Dy)
P4
-7.5, 7.5
-7.5481, 7.4725
0.315

1
P5
$-7.5,7.44$
-7.5177, 7.4388

$$
-6.66,6.54
$$

$$
-6.6351,6.5189
$$

$$
0.349
$$

$$
P 7
$$

$$
-4.944,4.824
$$

$$
-4.9413,4.8226
$$

$$
0.044
$$

$$
P 8
$$

$$
-2.646,2.55
$$

$$
-2.6433,2.5518
$$

$$
0.088
$$

P1
$-1.338,0.684$
-1.2889, 0.7533
5.651

P2
-2.226, 1.788
-2.2326, 1.7638
0.879

Translation 8
P3
-2.85 , 2.646
-2.8538, 2.6402
0.178
(Dy)
P4
$-3.15,3.162$
-3.1525, 3.1519
0.233

8
P5
-3.084, 3.258
-3.0846, 3.2560
0.046

P6
-2.664, 2.928
-2.6650, 2.9280
0.027

P7

## 8.2 <br> Contents of the file results

8 depreciation and Eigen frequencies, as well as the associated clean vectors.
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## Code_Aster ${ }^{\circledR}$

Version
6.4

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Date:
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Author (S):
E. BOYERE Key
:
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## 9 Modeling <br> D

## 9.1

Characteristics of modeling

Transposition of the test of reference to the case of the degrees of freedom of rotation (comes out from torsion + inertia) by using the discrete element of rigidity in translation/rotation.
in all the nodes
with an angle $=53.130102^{\circ}$
DISCRETE:

with nodal masses

in all the nodes
$M \_T R \_D \_N$
in local reference mark
( $m=10$.)
matrices of rigidity
in all the meshs
K_TR_D_L
in local reference mark
$(K R x=1.105)$
with the nodes ends
K_TR_D_N
in local reference mark
( $K R x=1.105$ )
matrices of damping
in all the meshs
A_TR_D_L
in local reference mark
( $C R x=50$.)
with the initial node
A_TR_D_N
in local reference mark
( $C R x=250$.)
with the final node
A_TR_D_N
in local reference mark
( $C R x=25$.)
Limiting conditions:
DDL_IMPO:
(ALL: "YES" DX: 0. , DY: 0. , DZ: 0. , DRZ: 0. )
LIAISON_DDL:
(such as $3 D R y=4 D R x$ in all the nodes)
Names of the nodes: P1, P2,...., P8

## 9.2

## Characteristics of the grid

A number of nodes:
8
A number of meshs and types:
7 SEG2
The nodes P1 and P8 are connected to a fixed fictitious node by nodal springs ( $K \_T R \_N, A \_T R \_N$ ).

### 9.3 Functionalities

tested

## Orders

"A_TR_D_N"
GROUP_NO"M_TR_D_N"
AFFE_CHAR_MECA DDL_IMPO
ALL
LIAISON_DDL
NODE
AFFE_MATERIAU ALL
AFFE_MODELE ALL
"MECHANICAL"
"DIS_TR"
GROUP_NO
"DIS_TR"
DEFI_MATERIAU ELAS
"CLOSE" MODE_ITER_INV CALC_FREQ OPTION
FREQ
NORM_MODE NORMALIZES MASS_GENE
9.4
Contents of the file results
Results obtained with:
CALC_FREQ: (LIST_FREQ: (6. , 10. , 15. , 19. , 24. , 29. , 29. , 31.))
CALC_MODE: (NMAX_MODE: 75)
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems

Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD27-System mass-arises with 8 degrees of freedom

Date:
17/02/04
Author (S):
E. BOYERE Key

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## 10 Results of modeling $D$

### 10.1 Values

tested
Frequency Reference
Aster \%
Difference
Order of the clean mode 1
5.53
5.529
-0.015
Order of the clean mode 2
10.90
10.897
-0.025
Order of the clean mode 3
15.93
15.934
0.027

Order of the clean mode 4
20.45
20.469
0.095

Order of the clean mode 5
24.34
24.359
0.079

Order of the clean mode 6
27.49
27.512
0.082

Order of the clean mode 7
29.84
29.853
0.042

Order of the clean mode 8
31.29
31.293
0.013

## Damping Reference

Aster \%
Difference
Order of the clean mode 1
1.521e2
$1.5210 e 2$
0.003

Order of the clean mode 2
2.877e2
2.8747e2
-0.079
Order of the clean mode 3
3.960e2
$3.9525 e 2$
-0.189
Order of the clean mode 4 4.709 e 2
$4.6924 e 2$
-0.352
Order of the clean mode 5
5.098e2
5.0899e2
-0.159
Order of the clean mode 6
5.183e2
5.1550e2
-0.539
Order of the clean mode 7

5.115e2<br>5.1180e2<br>-0.059<br>Order of the clean mode 8<br>5.036e2<br>5.0160e2<br>-0.396<br>Nature of the mode<br>Not Mode<br>proper clean Mode Aster<br>\% Difference<br>clean<br>Reference in 103<br>Real part<br>Real part<br>Imaginary part<br>Imaginary part

P1
-2.442, 2.736
-2.4438, 2.7338
0.076

P2
-4.782, 4.968
-4.7789, 4.9712
0.064

Rotation 1
P3
-6.54, 6.6
-6.5293, 6.6160
0.207
(DRx)
P4
-7.5, 7.5
-7.4810, 7.4724
0.316

1
P5
-7.5, 7.44
-7.5177, 7.4387
0.169

P6

```
-6.66 , 6.54
-6.6352, 6.5187
0.350
P7
-4.944, 4.824
-4.9414, 4.8227
0.045
P8
-2.646, 2.55
-2.6434, 2.5515
0.082
P1
-1.338, 0.684
-1.2674, 0.6692
4 . 7 9 9
P2
-2.226, 1.788
-2.2225, 1.6307
5 . 5 0 9
Rotation }
P3
-2.85, 2.646
-2.8626, 2.5198
3.261
(DRx)
P4
-3.15, 3.162
-3.1703, 3.1001
1.458
8
P5
-3.084, 3.258
-3.1022, 3.2965
0.949
P6
-2.664, 2.928
-2.6763, 3.0414
2.883
P7
-1.938, 2.214
-1.9423, 2.3489
```

4.590

P8
-0.996, 1.206
-1.0038, 1.2906
5.437

Clean mode normalized with the unit modal mass: $T$
$T$
$I C I+2 I I M I=1$
is the eigenvalue associated damping and the Eigen frequency.
10.2 Contents of the file results

8 depreciation and Eigen frequencies, as well as the associated clean vectors. Handbook of Validation
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Code_Aster ${ }^{\circledR}$
Version
6.4

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Date:
17/02/04
Author (S):
E. BOYERE Key

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## 11 Modeling

E

### 11.1 Characteristics of modeling

Transposition of the test of reference to the case of the degrees of freedom of rotation (comes out from torsion + inertia) by using the discrete element of rigidity in translation/rotation: DIS_TR
with an angle $=53.130102^{\circ}$
DISCRETE:
with nodal masses
in all the nodes
$M_{-} T R \_N$
in local reference mark
( $\operatorname{Ixx}=10$.)
matrices of rigidity
in all the meshs
K_TR_L
in local reference mark
( $K R x=1.105$ )
with the nodes ends
K_TR_N
in local reference mark
( $K R x=1.105$ )
matrices of damping
in all the meshs
$A_{-} T R_{-} L$
in local reference mark
( $C R x=50$.)
with the initial node
A_TR_N
in local reference mark
( $C R x=250$.)
with the final node
$A_{-} T R_{-} N$
in local reference mark
( $C R x=25$.)
Limiting conditions:
DDL_IMPO:
(ALL: "YES"DX: 0. , DY: 0. , DZ: 0. , DRZ: 0. )

## LIAISON_DDL:

(such as 3DRy=4DRx in all the nodes)
Names of the nodes: P1, P2,...., P8
11.2 Characteristics of the grid

A number of nodes:
8
A number of meshs and types:
7 SEG2
The nodes P1 and P8 are connected to a fixed fictitious node by nodal springs ( $K_{-} T R_{-} N, A_{-} T R_{-} N$ ).

### 11.3 Functionalities

tested

## Orders

DISCRETE AFFE_CARA_ELEM GROUP_MA
"K_TR_L"

```
"A_TR_L"
NODE
"K_TR_N"
"A_TR_N"
GROUP_NO
"M_TR_N"
AFFE_CHAR_MECA DDL_IMPO ALL
LIAISON_DDL
NODE
AFFE_MATERIAU ALL
"MECHANICAL"AFFE_MODELE VERY
"DIS_TR"
GROUP_NO
"DIS_TR"
DEFI_MATERIAU ELAS
```

MODE_ITER_SIMULT METHOD
"TRI_DIAG"
CALC_FREQ
OPTION
"CENTER"
FREQ
NMAX_FREQ
NORM_MODE NORMALIZES ..... MASS_GENE
Handbook of ValidationV2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
6.4
Titrate:
SDLD27-System mass-arises with 8 degrees of freedom
Date:
17/02/04
Author (S):
E. BOYERE Key
:
V2.01.027-C Page:
13/14
12 Results of modeling $E$
12.1 Values
tested
Frequency Reference
Aster \%
DifferenceOrder of the clean mode 1
5.53
5.529
0.0154

Order of the clean mode 2
10.90
10.896
0.0374

Order of the clean mode 3
15.93
15.927
0.0190

Order of the clean mode 4
20.45
20.452
0.0113

Order of the clean mode 5
24.34
24.336
0.0185

Order of the clean mode 6
27.49
27.487
0.0105

Order of the clean mode 7
29.84
29.835
0.0163

Order of the clean mode 8
31.29
31.295
0.0154

Damping Reference
Aster \%
Difference
Order of the clean mode 1
1.521e2
$1.5209 e 2$
-0.007
Order of the clean mode 2
$2.877 e 2$
$2.8757 e 2$
-0.043

## Order of the clean mode 3

3.960e2
$3.9565 e 2$
-0.09
Order of the clean mode 4
4.709 e2
$4.7034 e 2$
-0.119
Order of the clean mode 5
5.098e2
$5.0917 e 2$
-0.124
Order of the clean mode 6
5.183e2
$5.1764 e 2$
-0.126
Order of the clean mode 7
5.115e2
5.1084e2
-0.128
Order of the clean mode 8
5.036e2
5.0296e2
-0.126
Nature of the mode
Not Mode
proper clean Mode Aster
\% Difference
clean
Reference in 103
Real part
Real part
Imaginary part
Imaginary part
P1
-2.442, 2.736
-2.4440, 2.7331
0.096

P2
-4.782, 4.968
-4.7791, 4.9707

### 0.058

## Rotation 1

P3
$-6.54,6.6$
-6.5293, 6.6158
0.205
(DRx)
P4
-7.5, 7.5
-7.5481, 7.4725
0.315

1
P5
-7.5, 7.44
-7.5177, 7.4388
0.168

P6
-6.66, 6.54
-6.6351, 6.5189
0.349

P7
-4.944, 4.824
-4.9413, 4.8226
0.045

P8
-2.646, 2.55
-2.6433, 2.5518
0.088

P1
-1.338, 0.684
-1.3401, 0.6834
0.150

P2
-2.226, 1.788
-2.2264, 1.7855
0.087

Rotation 8
P3
-2.85, 2.646
-2.8528, 2.6488
0.101

## P8

-0.996, 1.206
-0.9958, 1.2072
0.081

Clean mode normalized with the unit modal mass: $T$
T
$I C I+2 I I M I=1$
: is the eigenvalue associated damping and the Eigen frequency.

### 12.2 Contents of the file results

8 depreciation and Eigen frequencies, as well as the associated clean vectors.

## Handbook of Validation

V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
6.4

## Titrate:

SDLD27-System mass-arises with 8 degrees of freedom

## Date:

17/02/04
Author (S):
E. BOYERE Key

V2.01.027-C Page:
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## 13 Summary of the results

For all the options of modeling of the discrete elements of rigidity, of mass and of damping offered by $A F F E$ _CARA_ELEM the solutions obtained are those of the solution of reference (frequencies and clean modes).

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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD29 Transitoire masses spring with 8 ddl
Date:
01/12/98
Author (S):
A.C. LIGHT

Key:
V2.01.029-C Page:
1/6
Organization (S): EDF/EP/AMV
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
Document: V2.01.029
SDLD29 - Transient masses spring with 8 ddl
and viscous damping nonproportional

## Summary:

This problem corresponds to a transitory analysis by modal recombination of a linear discrete system constituted of 8 degrees of freedom. This system has a non-proportional damping. A force transient of the crenel type is applied into 1 degree of freedom.
In this problem the elements DISCRETE with modal masses are tested ( $M_{-} T_{-} D_{-} N$ ), matrices of

## rigidity

( $K_{-} T_{-} D_{-} L$ ) and matrices of damping $\left(A_{-} T_{-} D_{-} L\right.$ ) in a modeling.
The problem has a reference solution suggested by commission VPCS. Variations with Code_Aster do not exceed 1,8\%.
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V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD29 Transitoire masses spring with 8 ddl
Date:
01/12/98
Author (S):
A.C. LIGHT

## Key:

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1
Problem of reference
1.1 Geometry

U
$U$
$U$
2
U
1
3
8
B
With
m
m
m
m
$\boldsymbol{X}, \boldsymbol{U}$

K
$K$
$K$
P1
$P$
$P$
$P$
2
3
8
DC
C
C
Cd
Specific masses:
$m P=m=m=\ldots \ldots . .=m=m$
1
P2
P3
P8
Stiffnesses of connection:
$k A P 1=k P 1 P 2=k P 2 P 3=\ldots \ldots .=k P 8 B=K$

Viscous damping:
$c P 1 P 2=c P 2 P 3=\ldots . .=C P 7 P 8=C$
$c A P 1=D C$
$C P 8 B=C d$
1.2

Material properties
Comes out from linear elastic translation
$K=$
$105 \mathrm{~N} / \mathrm{m}$
Specific mass
$m=$
10 kg
Damping of connection
$C=$
$50 \mathrm{~N}(\mathrm{~m} / \mathrm{s})$
$D C=$
$250 \mathrm{~N}(\mathrm{~m} / \mathrm{s})$
$C d=$
$25 \mathrm{~N}(\mathrm{~m} / \mathrm{s})$
1.3

Boundary conditions and loadings
Embedded points A and B: $U=0$
Loading: Force concentrated not periodical at the P4 point
Fx
Not P4

```
Fx
F(T)=1N= constant
4
4=F(T) 0 2 T T }1\textrm{l
T> 1s
F(T) = 0
l
O
l
T
1.4 Conditions
initial
```

For $T=0$, in any point $P$
=
$I: U=0$,
0 .
$d t$
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HI-75/98/040 - Ind A

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Version
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Titrate:
SDLD29 Transitoire masses spring with 8 ddl
Date:
01/12/98
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## A.C. LIGHT

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2
Reference solution
2.1

Method of calculation used for the reference solution
Numerical integration (approximate) by the direct method using a diagram of integration numerical by finished differences, the step of time used must be sufficiently small to obtain one sufficiently precise solution. With one of the diagrams used (method - Newmark improved), the step appointed time was of $0.001 s$.
Method of - Newmark improved (NEWMARK NR. Mr., "A method of computation for structural
dynamics " proceeding ASCE J. Eng. Mech. Div E-3, July 1959, pp 67-94) use the diagram of integration according to:
1 [
M]
1
$+$
[] 1
$\boldsymbol{C}+[\boldsymbol{K}] \boldsymbol{U}$
2
$(n+2)$
$T$
$2 t$
3

1
$=$
$[(\boldsymbol{P}++++$
$+$
N2]
[Pn 1] [Pn]) 2 [ $\mathbf{M}] 1[\boldsymbol{K}] \boldsymbol{U}$
M
C
$K$
U
2
$(n+1)$
1[] [ [] 1[]
2
(N)

3
$T$
3
$+$
$+$

Indices $N, N+1, N+2$ respectively indicate the calculations carried out at time $t n$,
$=+$
= +
[ ] [ ] [ ]
$N+1$
tn
$T$ and $t n+2$ tn $2 t$, where $T$ is the increment of appointed time. $\boldsymbol{M}, \boldsymbol{C}$ and $\boldsymbol{K}$ are respectively the matrices masses, damping and stiffness, $\boldsymbol{U}$
() is the vector displacement and $\boldsymbol{P}$
()
the vector forces associated.

## Point 4: displacement according to time

2.2

## Results of reference

Displacement at the P4 point according to time, cf graph above.
2.3

Uncertainty on the solution

- position of the extremas: $T<0.015$
- maximum amplitude: $U / U<0.5 \%$


### 2.4 References

bibliographical
[1]
Card-index SDLD29/90 of commission VPCS
Handbook of Validation
$V 2.01$ booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD29 Transitoire masses spring with 8 ddl
Date:
01/12/98
Author (S):
A.C. LIGHT

Key:
V2.01.029-C Page:
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3 Modeling
With

## 3.1

Characteristics of modeling
Discrete element of rigidity in translation
$y$
With
$P$
$P$
$P$
$P$
$P$
$P$
$P$
B
$X$
$P$
1
2

Characteristics of the elements
DISCRETE:
with nodal masses
$M_{-} T \_D \_N$
and matrices of rigidity
$K \_T \_D \_L$
and matrices of damping
A_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the nodes ends
(GROUP_NO: AB DX: 0. )
Names of the nodes:

Not $A=N 1$
$P 1=N 2$
Not $B=N 10$
$P 2=N 3$
$P 8=N 9$
Modal recombination with all the modes (8)
no time used
$d t=1 . E 3 S$
diagram of EULER
3.2

Characteristics of the grid
A number of nodes:
10
A number of meshs and types:
9 SEG2
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
DISCRETE
GROUP_MA
"K_T_D_L"
[U4.24.01]
GROUP_NO
" $M_{-} T \_D \_N$ "
$A F F E \_C H A R \_M E C A$
DDL_IMPO
ALL
[U4.25.01]
GROUP_NO
AFFE_MODELE
ALL
"MECHANICAL"
"DIS_T"
[U4.22.01]
GROUP_NO
"DIS_T"
MODE_ITER_INV
"ADJUSTS"
[U4.52.01]
DYNA_TRAN_MODAL
[U4.54.03]

REST_BASE_PHYS<br>[U4.64.01]<br>Handbook of Validation<br>V2.01 booklet: Linear dynamics of the discrete systems<br>HI-75/98/040 - Ind A

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Version
4.0

Titrate:
SDLD29 Transitoire masses spring with 8 ddl
Date:
01/12/98
Author (S):

## A.C. LIGHT

Key:
V2.01.029-C Page:
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4
Results of modeling $A$
4.1 Values
tested
Time (S)
Reference
Aster
\% Difference
0.09
3.97 E-5
3.95 E-5
0.503
0.18
5.10 E-6
5.03 E-5
1.38
0.27
3.77 E-5
3.77 E-5

0
0.36
7.30 E-6
7.28 E-6
0.293
0.45
$3.59 E-5$
3.59 E-5

0
0.54
8.81 E-6
8.77 E-6
0.486
0.63
3.47 E-5
3.47E-5
0.034
0.72
1.01 E-5
1.00 E-5
0.514
0.81
$3.36 E-5$
$3.36 E-5$
0
0.91
$1.11 E-5$
1.14E-5
2.36
0.99
3.27 E-5
3.26 E-5
0.171

### 4.2 Remarks

Contents of the file results: displacements.
4.3 Parameters
of execution
Version: NEW3.03
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
200 seconds
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HI-75/98/040 - Ind A

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Key:
V2.01.029-C Page:
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5

## Summary of the results

One obtains a relatively good agreement between the calculated solution and solution VPCS ( $<0.7 \%$ ) except with
moment 0.91 (2.4\%). The differences are primarily due to the fact that the moments of test are not given that with 2 significant figures, which does not make it possible to seize the moment sufficiently well of the extremum.

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Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
05/03/04
Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

V2.01.030-A Page:
1/16
Organization (S): EDF-R \& D /AMA, (*) CS IF

# Handbook of Validation <br> V2.01 booklet: Linear dynamics of the discrete systems <br> V2.01.030 document 

SDLD30 - Spectral seismic response of one system 2 masses and 3 springs multimedia

## Summary:

The problem consists in calculating the spectral response of a system 2 masses - $\mathbf{3}$ springs subjected to one multiple seismic excitation.

One tests the discrete element in traction, the calculation of the clean modes, the static modes and the answer
spectral by modal superposition via operator COMB_SISM_MODAL. Various office pluralities are tested at the time of calculation of the answers of supports.

The results obtained are in very good agreement with the analytical results of reference.
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V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A

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6.4

Titrate:
SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates: 05/03/04
Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

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1
Problem of reference

### 1.1 Geometry

The structure is modelled by a unit of 3 springs and 2 specific masses.

## k1

k2
k3
$m 2$
m3
$X$
NO1
NO2
NO3
NO4

## 1.2 <br> Material properties

Stiffness of connection: $k 1=k 2=K=1000 \mathrm{~N} / \mathrm{m} ; k 3=10 K=10000 \mathrm{~N} / \mathrm{m}$ specific mass: $m 2=m 3=m=10 \mathrm{~kg}$.

## 1.3

Boundary conditions and loadings

## - boundary conditions

Only authorized displacements are the translations according to axis $X$.
Points NO1 and NO4 are embedded: $D X=D Y=D Z=D R X=D R Y=D R Z=0$.
The other points are free in translation according to direction $X: D Y=D Z=D R X=D R Y=D R Z=0$.

- loading

The structure is subjected to a multiple spectral seismic excitation and displacements differentials.
The spectra of answers of oscillator in pseudo acceleration are simplified. Only them values corresponding to the 2 Eigen frequencies of the system are mentioned. They do not depend on damping:
with node NO1:
$\operatorname{SRONO1}(f 1)=A 11=7 \mathrm{~m} / \mathrm{s} 2$
SRONO1 (f2) $=A 12=5 \mathrm{~m} / \mathrm{s} 2$
DDSNO1 = D1 $=-0.04 \mathrm{~m}$
with node NO4:
SRONO4 (f1) $=A 21=12 \mathrm{~m} / \mathrm{s} 2$
SRONO4 (f2) $=A 22=6 \mathrm{~m} / \mathrm{s} 2$
DDSNO4 = D2 = 0.06 m

### 1.4 Conditions

initial
The system is initially at rest.
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V2.01 booklet: Linear dynamics of the discrete systems

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Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

## 2

Reference solution

## 2.1

Method of calculation used for the reference solution
One calculates the spectral response by modal superposition of a system masses spring subjected to two
distinct excitations. One determines the displacement of the masses and the reactions of support to the nodes
NO1 and NO4 following axis X.
One calculates analytically:
-
Eigen frequencies fi,
associated clean vectors Nor standardized compared to the modal mass,
static modes of supports J of the system,
factors of modal participation Pij relating to the supports,
Rmij the maximum of the response of each mode starting from the spectra of excitation,
Rej the contribution of the movement of drive of each support starting from displacements differentials,

Rcj the static term of correction,
primary and secondary components of the response according to the régles of office plurality adopted.

## 2.2

Results of reference

- matrix of rigidity $K$

K

- K $2 k$
- K

0
$K=$
0

- K
$K$
11
-10k

0
0
-10k
10k

## - matrix of mass $M$

0000

0 m 00
$M=$
00 m 0

0000

- modal calculation in embedded base

$$
\begin{aligned}
& (K-i M)=0 \\
& I \\
& 2 \\
& I=I \\
& K \\
& K \\
& 1 \\
& = \\
& (13-85) 2=(13+85)
\end{aligned}
$$

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Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD30-Spectral seismic response of a system 2 masses 3 springs Dates: 05/03/04

## Author (S):

Y. PONS, D. NUNEZ*, L. VIVAN* Key
:
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- not normalized clean modes:

```
-1
```


## - generalized modal masses

$T$
$\mu i=i M i$ :
$1=m$
$\mu$
$(170-1885) 2=m$
$\mu$
(170+18 85)
4
4

- clean modes normalized with generalized modal mass unit Ni:

```
1
2
1=
NR
=
\mu
N2
1
\mu2
```

- modal reactions the IMF:
-1
$K$
0
$K$
0
$F m=$
K
$=$
1
$F m=K$
=
$\mu$
0
2
N2

```
\mu
0
1
2
(
59-85)
-(
59+85)
- factors of modal participation
T
ij
P = I M J:
```

- contribution of the dynamic mode 1 to the movement imposed on node NO1:
$T$
m
11
$P=1$
M $1=$
$(13+85)$
$42 \mu 1$
- contribution of the dynamic mode 1 to the movement imposed on node NO4:
$T$
$10 m$
12
$P=1$
M2 =
$(-8+85)$
$21 \mu 1$
- contribution of the dynamic mode 2 to the movement imposed on node NO1:
$T$
21
$P=$
m
2
M 1 =
$(-13+85)$
$42 \mu 2$
- contribution of the dynamic mode 2 to the movement imposed on node NO4:
2
M2 =
(8+85)
$21 \mu 2$
- factor of participation of the dynamic mode 1 in direction $X$ :
1
$P X=11$
$P+12$
$P$
- factor of participation of the dynamic mode 2 in direction $X$ :
2
$P X=21$
$P+22$
$P$
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Code_Aster ${ }^{\circledR}$
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Titrate:
SDLD30-Spectral seismic response of a system 2 masses 3 springs Dates:
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Author (S):
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- static modes of supports $J$
- static solution with a unit displacement of node NO1:

21

- static modes $U$ J solution of $K u J=M$ J:

0
-122

## m 122

m 231
displacements: $U=$
1
nodal reactions: $\boldsymbol{F u}=$
441k 13
1

441k 50

2

## 441420

0

- 500
- static correction relating to the movement of the support J if mode 2 is not retained:
Pljr
Rc
1
$=$
$J$
Ru J
A2 J
with: $R u=U$ or $F u$ and $R=$
or Fm
2
$J$
J
$J$
1
N1
1
1
- contribution of the support $J$ to the movement of drive

Re $J=r j D j$ with $r j=J$ or $F s J$
2.3

Uncertainty on the solution
No (exact analytical solution).
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Titrate:
SDLD30-Spectral seismic response of a system 2 masses 3 springs Dates: 05/03/04
Author (S):
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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The system is modelled by:

## 3 discrete elements $K_{-} T_{-} D \_L$,

2 discrete elements $M_{-} T \_D \_N$.

## 3.2

Characteristics of the grid
The grid consists of 3 meshs SEG2.

3.3 Functionalities<br>tested<br>Orders<br>AFFE_MODELE GROUP_MA<br>"MECHANICAL"<br>"DIS_T"<br>DISCRETE AFFE_CARA_ELEM<br>NODE<br>$M_{-} T_{-} D \_N$<br>NET<br>$K_{-} T \_D \_L$<br>AFFE_CHAR_MECA DDL_IMPO

MACRO_MATR_ASSE

MODE_ITER_SIMULT CALC_FREQ

## METHOD <br> "SORENSEN"

NORM_MODE NORMALIZES "MASS_GENE"<br>MODE_STATIQUE MODE_STAT

PSEUDO_MODE

## COMB_SISM_MODAL MODE_CORR

## EXCIT

## COMB_MODE <br> "SRSS"

## COMB_MULT_APPUI

"QUAD"
"LINE"
DEPL_MULT_APPUI

COMB_DEPL_APPUI "QUAD"<br>"LINE"<br>"ABS"<br>Handbook of Validation<br>V2.01 booklet: Linear dynamics of the discrete systems HT-66/04/005/A

Code_Aster ${ }^{\circledR}$
Version

## Titrate:

SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
05/03/04
Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

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## 4 <br> Results of modeling $A$

4.1 Frequencies
clean
MODE
Reference
Code_Aster
Relative error (\%)
1
2,18815E+00 2,18815E+00 0,000
2
$5,30484 E+005,30484 E+000,000$

## 4.2

Total response on complete modal basis
Modes 1 and 2 are taken into account. Components inertial (primary education) and statics (secondary) response are directly cumulated to the level of the supports.

## calculation $n^{\circ} 1$

COMB_MODE=' SRSS ${ }^{\prime}$
COMB_MULT_APPUI=' QUAD'

- response of the support $j=1$ (node NO1):

2
2
2

```
1
R=
I
Rm+Re1 with
2
I
Rm=
11
Rm+Rm21
- response of the support j=2 (node NO4)
:
2
2
R2 = Rm2 + Re2 with
2
2
Rm2=
12
Rm+Rm22
- total answer:
2
2
R=
l
R + R2
absolute displacements: DEPL
NODE
Reference
Code_Aster
Relative error (%)
NO1
4,00000E-02 4,00000E-02 0,000
NO2
5,43820E-02 5,43820E-02 0,000
NO3
5,75544E-02 5,75544E-02 0,000
NO4
6,00000E-02 6,00000E-02 0,000
nodal reactions: REAC_NODA
NODE
```

```
Reference
Code_Aster
Relative error (%)
NO1
5,36769E+01 5,36769E+01
0,000
NO4
7,44120E+01 7,44120E+01
0,000
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```

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calculation $n^{\circ} 2$
COMB_MODE $=$ ' SRSS $^{\prime}$
COMB_MULT_APPUI=' LINE ${ }^{\prime}$

- response of the support $j=1$ (node NO1):
2
2
2
1
$R=$
1
Rm + Re1 with
2
1

```
Rm=
11
Rm + Rm21
- response of the support j=2 (node NO4)
:
2
2
R2 = Rm2 + Re2 with
2
2
Rm2=
12
Rm + Rm22
-total answer: R = 1
R + R2
absolute displacements: DEPL
NODE
Reference
Code_Aster
Relative error (%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
7,48259E-02 7,48259E-02
0,000
NO3
6,03377E-02 6,03377E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
```

nodal reactions: REAC_NODA
NODE
Reference
Code_Aster
Relative error (\%)
NO1
7,34576E $+017,34576 E+01$
0,000

NO4
9,72617E $+019,72617 E+01$
0,000

### 4.3 Total response on incomplete modal basis without correction statics

Only mode 1 is taken into account. Components inertial (primary education) and statics (secondary) of
response are directly cumulated to the level of the supports.

```
calculation n }\mp@subsup{}{}{\circ}
```


## COMB_MODE=' SRSS'

COMB_MULT_APPUI=' QUAD'

- response of the support $j=1$ (node NO1):

2
2
1
$R=$
1
Rm + Re1 with
1
$R m=$
11
Rm

- response of the support $j=2$ (node NO4):

2
2
$R 2=R m 2+R e 2$ with $R m 2=$
12
Rm

- total answer:

2
2
$R=$
1
$R+R 2$
absolute displacements: DEPL
NODE
Reference

Code_Aster

Relative error (\%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
5,43794E-02 5,43794E-02
0,000
NO3
5,73536E-02 5,73536E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
nodal reactions: REAC_NODA

```
NODE
Reference
Code_Aster
Relative error (\%)
NO1
5,36743E+01 5,36743E+01
0,000
NO4
\(5,68312 E+015,68312 E+01\)
0,000
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
```

Code_Aster ${ }^{\circledR}$
Version
6.4

## Titrate:

SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
05/03/04
Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key
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```
calculation n }\mp@subsup{}{}{\circ
```

$C O M B \_M O D E={ }^{\prime} S R S S^{\prime}$
COMB_MULT_APPUI=' LINE'

## - response of the support $j=1$ (node NO1):

```
2
```

2
1
$R=$
1
Rm + Re1 with
1
$R m=$
11
Rm

- response of the support $j=2$ (node NO4):
2
2
$R 2=R m 2+R e 2$ with $R m 2=$
12
Rm
- total answer: $R=1$
$R+R 2$
absolute displacements: DEPL


## NODE

Reference
Code_Aster
Relative error (\%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
7,48229E-02 7,48229E-02
0,000
NO3
6,01363E-02 6,01363E-02
0,000
NO4
6,00000E-02 6,00000E-02

# nodal reactions: REAC_NODA 

NODE
Reference
Code_Aster
Relative error (\%)
NO1
7,34546E +01 7,34546E +01
0,000
NO4
7,76841E+01 7,76841E+01
0,000
4.4 Total response on incomplete modal basis with correction statics

Only mode 1 intervenes in the calculation of the answer. The static contribution of neglected mode 2 is taking into account.

```
calculation n}\mp@subsup{}{}{\circ}
COMB_MODE=' SRSS'
COMB_MULT_APPUI=' QUAD'
```

- response of the support $j=1$ (node NO1):
2
2
2
1
$R=$
1
$R m+$
1
Rc + Re1 with
1
$R m=$
11
Rm
- response of the support $j=2$ (node NO4):
2

2
2
$R 2=R m 2+R c 2+R e 2$ with $R m 2=$
12
Rm

- total answer:
2
2
$R=$
1
$R+R 2$
absolute displacements: DEPL
NODE
Reference
Code_Aster
Relative error (\%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
5,43820E-02 5,43820E-02
0,000
NO3
5,75544E-02 5,75544E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
nodal reactions: REAC_NODA
NODE
Reference
Code_Aster
Relative error (\%)
NO1
$5,36769 E+015,36769 E+01$
0,000
NO4
7,44120E+01 7,44120E+01
0,000
Handbook of Validation

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Code_Aster ${ }^{\circledR}$
Version
6.4

## Titrate:

SDLD30-Spectral seismic response of a system 2 masses 3 springs Dates:
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Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

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1
$R m+$
1
Rc + Re1 with
1
$R m=$
11
Rm

- response of the support $j=2$ (node NO4):
2
2
2
$R 2=R m 2+R c 2+R e 2$ with $R m 2=$
12
Rm
- total answer: $R=1$
$\boldsymbol{R}+\boldsymbol{R} \mathbf{2}$
absolute displacements: DEPL
NODE
Reference
Code_Aster
Relative error (\%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
7,48259E-02 7,48259E-02
0,000
NO3
6,03377E-02 6,03377E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
nodal reactions: REAC_NODA
NODE
Reference
Code_Aster
Relative error (\%)
NO1
7,34576E $+017,34576 E+01$
0,000
NO4
9,72617E +01 9,72617E +01
0,000
4.5

Partition of the components primary and secondary of the answer
The components inertial (primary education) and statics (secondary) are treated separately.
$\cdot$ calculation $n^{\circ} 1$

- primary response on modal basis supplements (modes 1 and 2)

COMB_MODE=' SRSS'

## COMB_MULT_APPUI=' QUAD'

```
- response of the support j=1 (node NO1):
```

2
2
RI1 =
11
Rm + Rm21

- response of the support $j=2$ (node NO4):
2
2
IH $2=$
12
$R m+R m 22$
- primary answer:
2
2
$I H=R I 1+R I 2$
relative displacements: DEPL
NODE
Reference
Code_Aster
Relative error (\%)
NO1
0,00000E +00 0,00000E +00
NO2
4,12562E-02 4,12562E-02
0,000
NO3
6,60152E-03 6,60152E-03
0,000
NO4
0,00000E $+000,00000 E+00$
nodal reactions: REAC_NODA

NODE
Reference
Code_Aster
Relative error (\%)

```
NO1
4,12562E+01 4,12562E+01
0,000
NO4
6,60152E+01 6,60152E+01
0,000
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```

Code_Aster ${ }^{\circledR}$
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-
secondary answer
COMB_DEPL_APPUI=' QUAD'

- secondary answer:
2
2
$R I I=R e 1+R e 2$
displacements of drive: DEPL
NODE
Reference
Code_Aster
Relative error (\%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
3,54306E-02 3,54306E-02

NO3
5,71746E-02 5,71746E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
nodal reactions: REAC_NODA
NODE
Reference
Code_Aster
Relative error (\%)
NO1
3,43386E $+013,43386 E+01$
0,000
NO4
3,43386E+01 3,43386E +01
0,000
calculation $n^{\circ} 2$

- primary response on incomplete modal basis without static correction

Only mode 1 intervenes in the calculation of the answer
COMB_MODE=' SRSS ${ }^{\prime}$
COMB_MULT_APPUI=' QUAD'

- response of the support $j=1$ (node NO1): RII =

11
Rm

- response of the support $j=2$ (node NO4): $1 H 2=$

12
Rm

- primary answer:

2
2
$I H=R I I+R I 2$
relative displacements: DEPL
NODE

```
Reference
Code_Aster
Relative error (%)
NO1
0,00000E+00 0,00000E+00
NO2
4,12528E-02 4,12528E-02
0,000
NO3
4,52841E-03 4,52841E-03
0,000
NO4
0,00000E+00 0,00000E+00
nodal reactions: REAC_NODA
NODE
Reference
Code_Aster
Relative error (%)
NO1
4,12528E+01 4,12528E+01
0,000
NO4
4,52841E+01 4,52841E+01
0,000
```


## - secondary answer

COMB_DEPL_APPUI=' LINE'

- secondary answer: RII = Re1+ Re2

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Code_Aster ${ }^{\circledR}$
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displacements of drive: DEPL

## NODE

Reference
Code_Aster
Relative error (\%)
NOI
$-4,00000 E-02-4,00000 E-02$
0,000
NO2
7,61905E-03 7,61905E-03
0,000
NO3
5,52381E-02 5,52381E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
nodal reactions: REAC_NODA

NODE<br>Reference<br>Code_Aster<br>Relative error (\%)<br>NOI<br>$-4,76190 E+01-4,76190 E+01$<br>0,000<br>NO4<br>$4,76190 E+014,76190 E+01$

## calculation $n^{\circ} 3$

- primary response on incomplete modal basis with static correction

Only mode 1 intervenes in the calculation of the answer
COMB_MODE $=^{\prime} S R S S^{\prime}$
COMB_MULT_APPUI=' QUAD'

## - response of the support $j=1$ (node NO1):

2
2
RII =
11
$R m+$

Rc

- response of the support $j=2$ (node NO4):

2
2
IH $2=$
12
$R m+R c 2$

- primary answer:

2
2
$I H=R I 1+R I 2$
relative displacements: DEPL

```
NODE
Reference
Code_Aster
Relative error (\%)
NOI
0,00000E+00 0,00000E+00
NO2
4,12562E-02 4,12562E-02
0,000
NO3
6,60152E-03 6,60152E-03
```


## NODE

Reference

## Code_Aster

Relative error (\%)
NO1
$4,12562 E+014,12562 E+01$
0,000
NO4
$6,60152 E+016,60152 E+01$
0,000

- secondary answer
$C O M B \_D E P L \_A P P U I={ }^{\prime} A B S^{\prime}$
- secondary answer: $R I I=R e 1+R e 2$
displacements of drive: $D E P L$

```
NODE
Reference
Code_Aster
Relative error (%)
NO1
4,00000E-02 4,00000E-02
0,000
NO2
4,95238E-02 4,95238E-02
0,000
NO3
5,90476E-02 5,90476E-02
0,000
NO4
6,00000E-02 6,00000E-02
0,000
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```

Code_Aster ${ }^{\circledR}$
Version
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Titrate:
SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
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:
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nodal reactions: REAC_NODA

NODE<br>Reference<br>Code_Aster<br>Relative error (\%)<br>NO1<br>4,76190E+01 4,76190E+01<br>0,000<br>NO4<br>4,76190E+01 4,76190E+01<br>0,000

## calculation $n^{\circ} 4$

- primary response on incomplete modal basis with static correction

Only mode 1 intervenes in the calculation of the answer.
COMB_MODE $=$ ' SRSS' $^{\prime}$
COMB_MULT_APPUI=' QUAD'

```
- response of the support j=1 (node NO1):
2
2
RII =
11
Rm+
l
Rc
```

4 combinations are calculated:

## combination $\boldsymbol{n}^{\circ} 1$

linear office plurality of the cases has and b: TYPE_COMBI=' LINE' NUME_ORDRE=200
secondary answer: $R I I=R a+R b$
1
absolute displacements: DEPL

## NODE <br> Reference

Code_Aster
Relative error (\%)
NOI
-4,00000E-02 -4,00000E-02
0,000
NO2
7,61905E-03 7,61905E-03
0,000
NO3
5,52381E-02 5,52381E-02

## NODE

Reference

## Code_Aster

Relative error (\%)
NO1
$-4,76190 E+01-4,76190 E+01$
0,000
NO4
$4,76190 E+014,76190 E+01$
0,000

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Code_Aster ${ }^{\circledR}$
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SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
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## combination $n^{\circ} 2$

absolute office plurality of the cases has and C: TYPE_COMBI=' ABS' NUME_ORDRE=201
secondary answer: $R I I=R a+R c$
2
absolute displacements: DEPL

## NODE

## Reference

## Code_Aster

Relative error (\%)
NOI
4,00000E-02 4,00000E-02
0,000
NO2
3,52381E-02 3,52381E-02
0,000
NO3
3,04762E-02 3,04762E-02
0,000
NO4
3,00000E-02 3,00000E-02
0,000
nodal reactions: REAC_NODA

NODE<br>Reference<br>Code_Aster<br>Relative error (\%)<br>NO1<br>3,33333E+01 3,33333E+01<br>0,000<br>NO4<br>3,33333E+01 3,33333E+01<br>0,000

## combination $n^{\circ} 3$

quadratic office plurality of the cases D and E: TYPE_COMBI=' QUAD' NUME_ORDRE=202


#### Abstract

secondary answer:


2
2
$R I I 3=R d+R e$
absolute displacements: DEPL

```
NODE
Reference
Code_Aster
Relative error (%)
NO1
7,00000E-02 7,00000E-02
0,000
NO2
4,37189E-02 4,37189E-02
0,000
NO3
4,77356E-02 4,77356E-02
0,000
NO4
5,00000E-02 5,00000E-02
0,000
nodal reactions: REAC_NODA
NODE
Reference
Code_Aster
Relative error (%)
NO1
4,09635E+01 4,09635E+01
0,000
NO4
4,09635E+01 4,09635E+01
0,000
```


## Handbook of Validation

```
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
```


## Code_Aster ${ }^{\circledR}$

Version
6.4

Titrate:
SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
05/03/04
Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

## combination $n^{\circ} 4$

linear office plurality of the cases has and E: TYPE_COMBI=' LINE' NUME_ORDRE=203 secondary answer: $R I I=R a \operatorname{Re}$ 4
absolute displacements: DEPL

NODE<br>Reference<br>Code_Aster<br>Relative error (\%)

NOI
-4,00000E-02 -4,00000E-02
0,000
NO2
2,85714E-03 2,85714E-03
0,000
NO3
4,57143E-02 4,57143E-02
0,000
NO4
5,00000E-02 5,00000E-02
0,000
nodal reactions: REAC_NODA

```
NODE
Reference
Code_Aster
Relative error (\%)
NOI
\(-4,28571 E+01-4,28571 E+01\)
0,000
```

NO4
4,28571E+01 4,28571E+01

The total secondary answer is established by the quadratic office plurality of the 4 combinations precedents:

```
2
2
2
2
RII =
I
RII + RII2 + RII3 + RII4 NUME_ORDRE=204
```

absolute displacements: DEPL

## NODE

Reference

## Code_Aster

Relative error (\%)
NO1
9,84886E-02 9,84886E-02
0,000
NO2
5,67386E-02 5,67386E-02
0,000
NO3
9,13703E-02 9,13703E-02
0,000
NO4
9,74679E-02 9,74679E-02
0,000
nodal reactions: REAC_NODA

## NODE

Reference
Code_Aster
Relative error (\%)
NOI
8,30266E+01 8,30266E+01
0,000
NO4
8,30266E+01 8,30266E+01

## Code_Aster ${ }^{\circledR}$

Version
6.4

Titrate:
SDLD30 - Spectral seismic response of a system 2 masses 3 springs Dates:
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Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key

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## 5 <br> Summary of the results

The results obtained with Code_Aster are in conformity with the analytical results of reference.
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$V 2.01$ booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Simple Oscillating SDLD101 under random excitation

Date:
30/08/01
Author (S):
J. PIGAT Key

V2.01.101-B Page:
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SDLD101 - Simple oscillator under excitation random

## Summary:

An oscillator simple, made up of a mass connected to a support by a spring and a shock absorber, is subjected to a random excitation transmitted by the support, of imposed acceleration type.

This test uses the functionalities of the stochastic analysis and calculates the spectral concentration of power (DSP)
movement of the mass starting from the excitation of the white vibration type data by its DSP also.
The movement is calculated according to various options: relative, absolute, differential movement.
One calculates then the statistical properties of the response while passing in all the options of random dynamic postprocessing.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems

# Code_Aster ${ }^{\circledR}$ 

Version
5.0

Titrate:
Simple Oscillating SDLD101 under random excitation

## Date:

30/08/01
Author (S):

## J. PIGAT Key

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## 1

Problem of reference

### 1.1 Geometry

AX
K
DX
m

The excitation is a seismic movement of type imposed acceleration $A X$ applied to the support in feel DX.

One is interested in the movement of the mass Mr.

## 1.2 <br> Material properties

Specific mass:
$m=100 \mathrm{~kg}$
Arises elastic:
$K=105 \mathrm{~N} / \mathrm{m}$
Modal damping:
$0=0.05$

## 1.3

Boundary conditions and loadings
The problem is unidimensional in direction $X$, and to 1 degree of freedom: the displacement of mass Mr.

The excitation is a spectral concentration of power (DSP), of constant acceleration between 0. and 100 Hz .

## It is applied to the support.

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## Date:

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2
Reference solution

## 2.1

Method of calculation used for the reference solution

## K

The reference solution is analytical [bib1]. The own pulsation of the oscillator is
K
that is to say $0=$
$=100 \mathrm{rad} / \mathrm{s}$, and F
m
$O=15,9155 \mathrm{~Hz}$.
Moving absolute, the DSP of the response in noted acceleration $G$
()
RR
\& \& is connected to the DSP of
excitation GEE
\& \& in acceleration also by:
$4+4222$
G() $=$
0
0
0
.
RR
\& \&
(2-2 2
2
2
2
0
)
G
()
EE
\& \&
$+4$
0
0
Moving relative, one a:
2
2
$\boldsymbol{G}()=$
$\boldsymbol{G}$ ().
RR
\& \&

Moving differential, one a:
$\boldsymbol{G}()=\boldsymbol{G}$
().

RR
\& \&
EE
\& \&
2.2

Results of reference
One tests the DSP of the response for 0, 5, 10, 15, 20 Hz in the three cases of movement: absolute, relative and differential.
2.3

Uncertainty on the solution
Analytical solution.

### 2.4 References <br> bibliographical

## [1]

C. DUVAL "Dynamic response under random excitation in Code_Aster: principles theoretical and examples of use " - Note HP-61/92.148
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-62/01/012/A
Code_Aster ${ }^{\circledR}$
Version
5.0

# Titrate: <br> Simple Oscillating SDLD101 under random excitation 

Date:<br>30/08/01<br>Author (S):<br>J. PIGAT Key<br>V2.01.101-B Page:<br>4/6

## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Discrete element in translation of the type DIS_T
$P 1$
$\boldsymbol{K}$
$P 2$
$\boldsymbol{D X}$
$\boldsymbol{m}$

Characteristics of the elements:
With the nodes P1 and P2: matrices of masses of the type $M_{-} T_{-} D_{-} N$ with $m=100 \mathrm{~kg}$. Between P1 and P2: a matrix of rigidity of the type $K_{-} T_{-} D_{-} L$ with $K x=106 \mathrm{~N} / \mathrm{m}$

Boundary conditions:
All the ddl are blocked except ddl DX of the P2 node.

## 3.2 <br> Characteristics of the grid

## A number of nodes: 2

## A number of meshs and types: 1 SEG2, 2 POII

3.3 Functionalities<br>tested<br>Orders<br>MODE_STATIQUE DDL_IMPO<br>AVEC_CMP<br>DEFI_INTE_SPEC KANAI_TAJIMI

CONSTANT

DYNA_ALEA_MODAL EXCIT

MODE_STAT

ANSWER

REST_SPEC_PHYS

POST_DYNA_ALEA GOING BEYOND
RAYLEIGH

GAUSS

VANMARCKE

MOMENT

## TOO BAD

## Handbook of Validation

V2.01 booklet: Linear dynamics of the discrete systems
HT-62/01/012/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Simple Oscillating SDLD101 under random excitation
Date:30/08/01
Author (S):
J. PIGAT Key
4Results of modeling $A$
4.1 Values
tested
Random dynamic response
Identification Reference Aster \%
Difference
ABSOLUTE: $\mathrm{F}=5 . \mathrm{Hz}$
1.23071.23070.\%ABSOLUTE: $\mathrm{F}=10 . \mathrm{Hz}$
2.7116
2.7116
0.\%
ABSOLUTE: $\mathrm{F}=15 . \mathrm{Hz}$47.2154
47.2157
$0 . \%$
ABSOLUTE: $\mathrm{F}=20 . \mathrm{Hz}$
2.8924
2.8924
0.\%

ABSOLUTE: $\mathrm{F}=25 . \mathrm{Hz}$
0.47047
0.47047
$0 . \%$
RELATIVE: $\mathrm{F}=5 . \mathrm{Hz}$
0.01197
0.01197
0.\%

RELATIVE: $\mathrm{F}=10 . \mathrm{Hz}$
0.04209
0.04209
0.\%

RELATIVE: $\mathrm{F}=15 . \mathrm{Hz}$
36.9225
36.9258
0.\%

RELATIVE: $\mathrm{F}=20 . \mathrm{Hz}$
7.1006
7.1006
0.\%

RELATIVE: $\mathrm{F}=25 . \mathrm{Hz}$
2.7953
2.7953
0.\%

DIFFERENTIAL: $\mathrm{F}=5 . \mathrm{Hz}$
1.0
1.0
0.\%

DIFFERENTIAL: $\mathrm{F}=10 . \mathrm{Hz}$
1.0
1.0
0.\%

DIFFERENTIAL: $\mathrm{F}=15 . \mathrm{Hz}$
1.0
1.0
0.\%

DIFFERENTIAL: $F=20 . \mathrm{Hz}$

DIFFERENTIAL: $\mathrm{F}=\mathbf{2 5} . \mathrm{Hz}$
1.0
1.0
0.\%

Postprocessing on the response in absolute displacement: spectral moments and parameters statistics

## Identification

Aster
version 5.02
Spectral moment $n^{\circ} 0$
2.5285102

Spectral moment $n^{\circ} 1$
2.4524104

Spectral moment $n^{\circ} 2$
2.5125106

Spectral moment $n^{\circ} 3$
2.7647108

Spectral moment $n^{\circ} 4$
3.6031010

Standard deviation 22.49
Factor of irregularity
0.8324

Frequency connects (Hz)
15.86

Numbers average passages by zero a second
31.73

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V2.01 booklet: Linear dynamics of the discrete systems
HT-62/01/012/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Simple Oscillating SDLD101 under random excitation

Date:
30/08/01
Author (S):

## J. PIGAT Key

V2.01.101-B Page:
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Postprocessing on the response in absolute displacement: statistical functions
The recorded values are those printed in the file result.

## Identification Parameter

Aster
Version 5.02
Nb going beyond a second
10.97
25.00
40.55
1.23
60.10
0.025

Distribution of Rayleigh
10.97
0.0342
40.55
0.0062
60.10
0.187

10-3
Distribution of Gauss
10.97
0.0395
40.55

### 4.2 Parameters <br> of execution

Version: 5.02

## Machine: SGI ORIGIN 2000

System:
Obstruction memory:
8 megawords
Time CPU To use:
2.65 seconds

## 5 <br> Summary of the results

It is not astonishing that the results awaited for the random dynamic response are obtained with an accuracy of 0\%. Indeed the DSP of the answers do not result from an iterative process of resolution, but of an analytical expression bringing into play the modal transfer transfer functions. This analytical expression coincides with the reference solution for this problem.

For postprocessing, there is no reference solution. The results of version 4.03.09 are used to check that the results do not evolvelmove from one version to another. Calculation has very well supported the change of platform.

## Code_Aster ${ }^{\circledR}$

Version

4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.01.102-B Page:
1/12
Organization (S): EDF/EP/AMV

## Handbook of Validation

## V2.01 booklet: Linear dynamics of the discrete systems

Document: V2.01.102
SDLD102-Under transitory structuring:

## System 3 masses-4 springs

## Summary:

The applicability of this test relates to the dynamics of the structures. It makes it possible to validate the diagram
of integration to step of adaptive time of operator DYNA_TRAN_MODAL [U4.54.03] as well as the calculation of
linear transitory response on a modal basis calculated by under-structuring (for the 4 diagrams of integration of DYNA_TRAN_MODAL: "EULER", "DEVOGE", "NEWMARK" and "ADAPT"). In particular, the case of
the application of a damping reduced to the dynamic modes of the bases of projection of the substructures is
treaty.
It is a question of determining the transitory response of a system made up of 3 masses and 4 springs, embedded with its
ends and subjected to a constant force as from the initial moment. The springs are modelled by elements of the type "DIS_TR" and masses by elements of the type "DIS_T".
Three modelings are proposed. In the 2 first, the structure is not deadened. Methods of calculation transient by under-structuring with interfaces of the type Craig-Bampton ("CRAIGB") and Mac Neal ("MNEAL") are
tested. The results of reference which are associated for them result from an analytical calculation. In the third,
one imposes a reduced damping of $1 \%$ on the dynamic modes of the bases of projection of
substructures. The transitory equation checked by the complete structure was obtained analytically. Its resolution, which acts as reference, was carried out by the Maple software.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
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1
Problem of reference

### 1.1 Geometry

The studied system is composed of 3 masses $(m)$ and 4 springs ( $K$ ). The unit is embedded with its ends.
With
B
x3
$x 2$
x1
1.2

Material properties
Stiffness of the springs: $K=1 \mathrm{~N} / \mathrm{m}$.
Specific masses: $m=1 \mathrm{~kg}$.
1.3

Boundary conditions and loadings
F
$T$
Embedded points A and B.
Application to the point xl of a constant force $F=1 \mathrm{NR}$, as from the moment $T=0 \mathrm{~S}$.
1.4 Conditions
initial
Structure initially at rest.
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V2.01 booklet: Linear dynamics of the discrete systems

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
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Author (S):
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2
Reference solution
2.1

Method of calculation used for the reference solution
2.1.1 Not deadened structure

In this case, the reference solution can be obtained analytically:
100 X
2-10X
1
1
1
$m 010 X+K-1$
2
$-1 X=0$
2

```
0-12X
O
3
3
The own pulsations of the system mass-arises are worth:
```

```
K
K
K
2
2
2
=
=
=
+
(2 2)
2
2
3
(2 2)
m
m
m
respective modal deformations:
```

2
1

- 2

```
X
```

1
2
1

- 2
1
$X(T)$
$=X$
2
$=$
$=2$
0
22
$X$
3
2-1
23
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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
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2.1.2 Structure

## deadened

Damping is applied to the clean modes of the bases of projection of the substructures embedded (reduced damping). In this case, one leads to the transitory equation in co-ordinates generalized following (bib [1]):
800
3-2 20

- 1

1
1
$m 020+42 \mathrm{~km}$
0
1

This system not being uncoupled, it was solved using the Maple software. One obtained ( = 0.01) :
$T$

```
l
2
l
-2
l
X(T)
= X
```

2
$=$
$=2$
0
22
$X$
3
2-1
23

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Code_Aster ${ }^{\circledR}$
Version
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Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
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2.2

Results of reference
Not deadened structure:

## Displacement, speed and acceleration of the node $x 2$ at the moment $T=80 \mathrm{~S}$ :

X

Deadened case: semi-analytical solution.

### 2.4 Reference

bibliographical
[1]
C. VARE - Report/ratio HP 61/95/025/A - "Implementation of nonlinear transitory calculation by under-structuring in Code_Aster ".
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.01.102-B Page:
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## 3 Modeling

## With

## 3.1

## Characteristics of modeling

The system is divided into 2 substructures:
$m$
$m$
$m$
Substructure 1:
blocking
Substructure 2:
blocking
K
K
NOI
NO3
K
K
NO1
NO3
In situation, the two substructures are connected to the level of the $2 n d$ mass. The dynamic interface

1 st substructure consists of a mass $m$ on the level of node NO3 of the grid and coincide with the dynamic interface of the 2nd substructure which does not comprise any mass and is simply blocked on the level of node NO1.
NOI
NO3 NO1
NO3
m
$m$
Substructure 1
Substructure 2
The clean modes of the complete system are calculated by using the method of calculation modal by under-structuring with interfaces of the type "Craig-Bampton" (blocked interfaces). Bases of each substructure are made up of a dynamic mode and a constrained mode.
The transitory response of the system is calculated on the modal basis calculated by under-structuring.
The steps of times used are equal to: 102 S in "EULER", 102 S in "NEWMARK", 102 S in
"DEVOGE", 10-1 S in "ADAPT" (for this last, it acts of the step of initial time of the algorithm and of no the maximum time of integration).

## 3.2 <br> Characteristics of the grid of the substructure

A number of nodes: 3
A number of meshs and types: 2 SEG2

### 3.3 Functionalities

tested

Orders
Keys
NUME_DDL_GENE
BASE
[U4.55.07]
STORAGE
"DIAG"
PROJ_MATR_BASE
BASE
[U4.55.01]
NUME_DDL_GENE
MATR_ASSE_GENE
PROJ_VECT_BASE
BASE
[U4.55.02]
NUME_DDL_GENE
VECT_ASSE_GENE
DYNA_TRAN_MODAL
METHOD
"ADAPT"

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
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4
Results of modeling A
4.1 Values
tested
Calculation by modal recombination without under-structuring: Method "ADAPT"
Identification
Reference
Aster
\% difference
Node x2, displacement (m)
4.1700101
4.1695101

Node x2, speed (m.s1)
4.3011101
4.2972101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
3.3741101

Calculation by under-structuring

## Method: "EULER"

Node x2, displacement (m)
4.1700101
4.1480101

Node x2, speed (m.s1)
4.3011101
4.2972101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
3.3823101

Method: "DEVOGE"
Node x2, displacement (m)
4.1700101
4.1700101

Node x2, speed (m.s1)
4.3011101
4.3011101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
4.3749101

Method: "NEWMARK"
Node x2, displacement (m)
4.1700101
4.1711101

Node x2, speed (m.s1)
4.3011101
4.3090101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
3.3763101

Method: "ADAPT"
Node x2, displacement (m)
4.1700101
4.1695101

Node x2, speed (m.s1)
4.3011101
4.2972101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):

## G. ROUSSEAU, C. VARE

Key:
V2.01.102-B Page:
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## 5 Modeling

## B

5.1

## Characteristics of modeling

This modeling is identical to the precedent if they are only the clean modes of the complete system are calculated by using the method of calculation modal per under-structuring with interfaces of the type "Mac Neal" (free interfaces). The bases of each substructure are made up of a mode dynamics and of a mode of fastener.
The transitory response of the system is calculated on the modal basis calculated by under-structuring. More precisely, the studied substructures have their free interfaces:
Substructure 1:
Blocked NO1
Free NO3
Substructure 2:
Free NO1
Blocked NO3
The steps of times used are worth: 102 S in "EULER", 102 S in "NEWMARK", 102 S in "DEVOGE", $10-2 S$ in "ADAPT".

## 5.2

## Characteristics of the grid of the substructure

A number of nodes: 3
A number of meshs and types: 2 SEG2

### 5.3 Functionalities

tested
Orders
Keys
NUME_DDL_GENE
BASE
[U4.55.07]
STORAGE
"DIAG"
PROJ_MATR_BASE
BASE
[U4.55.01]
NUME_DDL_GENE
MATR_ASSE_GENE
PROJ_VECT_BASE
BASE
[U4.55.02]
NUME_DDL_GENE
VECT_ASSE_
REST_BASE_PHYS
MODE_MECA
[U4.64.01]
DYNA_TRAN_MODAL
METHOD
"ADAPT"
[U4.54.03]
"EULER"
"NEWMARK"
"DEVOGE"
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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:

G. ROUSSEAU, C. VARE

## Key:

V2.01.102-B Page:
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## Results of modeling B

### 6.1 Values

tested
Identification
Reference
Aster
\% difference
Method: "EULER"
Node x2, displacement (m)
4.1700101
4.1480101

Node x2, speed (m.s1)
4.3011101
4.2972101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
3.3823101

Method: "NEWMARK"
Node x2, displacement (m)
4.1700101
4.1711101

Node x2, speed (m.s1)
4.3011101
4.3090101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
3.3763101

Method: "DEVOGE"
Node x2, displacement (m)
4.1700101
4.1700101

Node x2, speed (m.s1)
4.3011101
4.3011101

Node x2, acceleration (m.s2)
3.3749101
4.3749101

Method: "ADAPT"
Node x2, displacement (m)
4.1700101
4.1695101

Node x2, speed (m.sl)
4.3011101
4.2973101
< $1 \%$
Node x2, acceleration (m.s2)
3.3749101
3.3742101
6.2 Parameters
of execution
Version: 3.4.6
Machine: CRAY C90
Obstruction memory:
8 megawords
Time CPU To use:
14.8 seconds

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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.01.102-B Page:
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7 Modeling
C
7.1

## Characteristics of modeling

The clean modes of the complete system are calculated by using the method of calculation modal by under-structuring with interfaces of the type "Craig-Bampton" (blocked interfaces). Bases of each substructure are made up of a dynamic mode and a constrained mode.
With the dynamic mode of each substructure a damping reduced with $1 \%$ is associated.
The transitory response of the deadened system is calculated on the modal basis calculated by under-structuring.
The steps of time taken are equal to: 102 S in "ADAPT", 102 S in "EULER", 102 S in "NEWMARK".
7.2

Characteristics of the grid of the substructure
A number of nodes: 3
A number of meshs and types: 2 SEG2
7.3 Functionalities
tested
Orders

## Keys

MACR_ELEM_DYNA
AMOR_REDUIT
[U4.55.05]
NUME_DDL_GENE
BASE
[U4.55.07]
STORAGE
"FULL"
PROJ_MATR_BASE
BASE
[U4.55.01]
NUME_DDL_GENE

```
MATR_ASSE_GENE
PROJ_VECT_BASE
BASE
[U4.55.02]
NUME_DDL_GENE
VECT_ASSE_GENE
REST_BASE_PHYS
MODE_MECA
[U4.64.01]
DYNA_TRAN_MODAL
METHOD
"ADAPT"
[U4.54.03]
"EULER"
"NEWMARK"
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
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```

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.01.102-B Page:
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8
Results of modeling C
8.1 Values
tested
Identification

## Reference

Aster
\% difference
Method: "EULER"
Node $x 2$, displacement (m)
4.9867101
$<1 \%$
Method: "NEWMARK"
Node $x 2$, displacement (m)
4.9867101
4.9883101
< $1 \%$
Method: "ADAPT"
Node $x 2$, displacement (m)
4.9867101
4.9863101
< $1 \%$

### 8.2 Parameters

## of execution

Version: 3.4.6
Machine: CRAY C90
Obstruction memory:
8 megawords
Time CPU To use:
11.0 seconds

Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLD102 Under transitory structuring
Date:
01/09/99
Author (S):
G. ROUSSEAU, C. VARE

Key:
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Summary of the results
The precision on displacement, the speed and the acceleration of the node $x 2$ at the moment $\mathrm{T}=80 \mathrm{~S}$ is excellent (relative error < $1 \%$ ).
This test thus validates the operators of calculation of transitory answer linear on calculated modal basis by dynamic under-structuring (with and without damping), as well as the diagram of integration with no the adaptive time of operator DYNA_TRAN_MODAL.
Handbook of Validation

V2.01 booklet: Linear dynamics of the discrete systems
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Seismic SDLD103 Response of a system 3 masses and 4 springs
Date:
30/08/01
Author (S):
Fe Key WAECKEL
:
V2.01.103-B Page:
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Organization (S): EDF/RNE/AMV

## Handbook of Validation

V2.01 booklet: Linear dynamics of the discrete systems
V2.01.103 document

## SDLD103 - Seismic response of a system <br> 3 masses and 4 springs multimedia

## Summary

The problem consists in analyzing the response of a mechanical structure of embed-embedded beam type and
not deadened, modelled by a system $\mathbf{3}$ masses and 4 springs and subjected to a seismic loading unspecified.

One tests the discrete element in traction and rotation, the calculation of the clean modes and the static modes and calculation
transitory response by modal superposition of a structure subjected to a accélérogramme of translation
(modeling A) or of rotation (modeling B).
The results obtained are in very good agreement with the results of reference (analytical results).
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Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

Seismic SDLD103 Response of a system 3 masses and 4 springs
Date:
30/08/01
Author (S):
Fe Key WAECKEL
:
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## 1

Problem of reference

### 1.1 Geometry

The beam is modelled by a whole of 4 springs and 3 specific masses.

## 1.2 <br> Material properties

Stiffness of connection: $K=k 1=k 2=k 3=k 4=104 \mathrm{~N} / \mathrm{m}$;
specific mass: $m=m 1=m 2=m 3=10 \mathrm{~kg}$.

## 1.3 <br> Boundary conditions and loadings

## Boundary conditions:

Only authorized displacements are the translations according to axis $X$.
Points NO1 and NO5 are embedded: $d x=D y=d z=d r x=d r y=d r z=0$.
The other points are free in translation according to direction $X: D y=d z=d r x=d r y=d r z=0$.

## Loading:

The points of anchoring NO1 and NO5 each one are subjected to a transverse acceleration (T)

### 1.4 Conditions

## initial

The system is at rest: with $T=0, d x()$
$0=0, d x / d t()$
$0=0$ in any point.
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Seismic SDLD103 Response of a system 3 masses and 4 springs
Date:
30/08/01
Author (S):
Fe Key WAECKEL
:
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## 2

Reference solution

## 2.1

Method of calculation used for the reference solution
The problem consists in calculating the response of a system to five degrees of freedom subjected to two
accelerations (
1 T) and (
2 T) distinct of an unspecified form. It is explained in detail in the reference [bib2].

One calculates the Eigen frequencies initially fi, the standardized associated clean vectors compared to modal mass Ni and the static modes of the system (analytical values). One calculate then the generalized response of the system multimedia while solving analytically the integral of Duhamel [bib1]. Lastly, one restores on the physical basis the vector of displacements relative (on the active degrees of freedom) Xr, which allows us, after having calculated the vector of displacements of drive Xe, to calculate the vector of absolute displacements

$$
X=X
$$

$$
+X
$$

has
R
E.
2.2

Results of reference

Calculation of the three Eigen frequencies fi, the associated clean vectors standardized compared to modal mass Ni and of the static modes of the system

```
I
F
1 =
= 3 85 Hz
```

$2(2+2)$
m $2 k$
1-2

- 1
31
1
1
1
$F=$
= 7,12
2
Hz
2
0
-2 and $=$

22. 

2 m $2 k$
NR

## Calculation of the generalized response of the multimedia system

The fundamental equation of dynamics, in the relative reference mark on the active degrees of freedom is written:
at2
MR. X
$\&+K X$
R
$R=(M$
$+M x s) X \& s$ with $\& X s=$
, the vector of the accelerations imposed on

## 0

level of the various points of anchoring.
The equation of the movement projected on the basis of dynamic mode standardized compared to modal mass is written, by considering only the active degrees of freedom:
NR
$2+2$

```
m t2 has
&
Q(T)+KQ(T)
T
= -
MR. X = -
2
G
NR
& S
4
2-
2
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Code_Aster [ }\mp@subsup{}{}{\circledR
Version
5 . 0
```


## Titrate:

```
Seismic SDLD103 Response of a system 3 masses and 4 springs
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30/08/01
Author (S):
Fe Key WAECKEL
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```

The response of this linear system, at one moment T, then consists in calculating the integral of Duhamel:
$(3+22)(t 2+(2+2)(\cos T-$
1
) $m$

## K)

m has

```
Q (T) = -
(t2+(cos T -
2
m
l
2
K)
4K
(
3-22)22
2
I
(T+(-) (cos T -
3
) mk)
```

Calculation of displacement relating to the active degrees of freedom: $X r=$ Nor IQ, that is to say: I
2
m
$7 T+10+72$
$)(\cos T-1) 1+(\cos T$
2
$) 1+(10-72)(\cos T$
3
) $1 m$
$K$
$K$

```
m2 has
m
= -
8
X
R
T+102+14
(
)(cosT-1) 1+(-10 2+14)(cosT-3) 1 m
8 K
K
K
2
m
5T+10+72
(
)(cosT-1)1 (cos T
2
) 1+(10-7 2)(cosT
3
) 1m
```

K
$K$

Calculation of displacements of drive to the active degrees of freedom: $X=X$
$S=h a s$
2.

481

Calculation of absolute displacements to the active degrees of freedom: $X=X$
$+X$
has
R
E.
2.3

Uncertainty on the solution
No if one calculates the integral of Duhamel analytically [bib1].

### 2.4 References <br> bibliographical

[1]
J.S. PRZEMIENIECKI: Theory of matrix structural analysis. New York, Mac Graw-Hill, 1968, pages 351-357.
[2]
Fe WAECKEL: Documentations use and validation of the developments carried out for to calculate the seismic response of multimedia structures. HP-52/96/002
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-62/01/012/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Seismic SDLD103 Response of a system 3 masses and 4 springs

## Date:

30/08/01

## Author (S):

## Fe Key WAECKEL

:

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The elements are modelled by discrete elements with 3 degrees of freedom DIS_T.
$X$
2
K 1
k2
k3
k4
Z
1
NO1
NO2
NO3
NO4
NO5

Node NO1 is subjected to an imposed acceleration (
1 T), node NO5 with (
2 T). It is calculated
relative displacement of nodes NO2, NO3 and NO4 compared to their static deformation, them displacement of drive and their absolute displacement.

Temporal integration is carried out with the algorithms of Euler (not of time: 10-3 second), of Devogelaere (not of time: 10-3 second) and with an algorithm with step of adaptive time.

## 3.2 <br> Characteristics of the grid

The grid consists of 5 nodes and 4 discrete elements (DIST_T).

### 3.3 Functionalities <br> tested <br> Orders

Keys Doc. V5
AFFE_MODELE GROUP_MA "MECHANICAL"
"DIS_T"
[U4.41.01]
DISCRETE AFFE_CARA_ELEM
NODE
M_T_D_N
[U4.42.01]
NET
$K_{-} T_{-} D_{-} L$
AFFE_CHAR_MECA DDL_IMPO
[U4.44.01]
MACRO_MATR_ASSE
[U4.61.21]
MODE_ITER_INV CALC_FREQ
ADJUST
[U4.52.04]
CALC_FONC_INTERP
[U4.32.01]
MODE_STATIQUE DDL_IMPO

[U4.52.14]<br>CALC_CHAR_SEISME NODE

[U4.63.01]

[U4.63.11]<br>DYNA_TRAN_MODAL EXCIT<br>MULT_APPUI<br>"YES"[U4.53.21]<br>METHOD<br>EULER

## DEVOGE

ADAPT
REST_BASE_PHYS MULT_APPUI "YES"
[U4.63.21]
MULT_APPUI
"NOT"
RECU_FONCTION RESU_GENE
[U4.32.03]
Handbook of Validation
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Code_Aster ${ }^{\circledR}$
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Author (S):
Fe Key WAECKEL

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```
4
Results of modeling \(A\)
```

```
4 . 1
Values tested of modeling A
```


### 4.1.1 Displacements

relative
nodes NO2, NO3 and NO4

Relative displacements of node NO2 with the numerical algorithm of integration of Euler:

```
Time (S)
Reference
Code_Aster Error
(%)
0,1 8,47734E01
8,47725E01
-0,001
0,3 1,55202E+01
1,55201E+01
0
0,5 4,36449E+01
4,36450E+01
0
0,7 8,50830E+01
8,50832E+01
0
1,0 1,74790E+02
1,74790E+02
0
```

Relative displacements of node NO2 with the numerical algorithm of integration of Devogelaere:
Time (S)
Reference
Code_Aster Error
(\%)

```
0,1 8,47734E01
8,47734E01
0
0,3 1,55202E+01
1,55202E+01
0
0,5 4,36449E+01
4,36449E+01
0
0,7 8,50830E+01
8,50830E+01
0
1,0 1,74790E+02
1,74790E+02
0
```

Displacements relating of node NO2 with the numerical algorithm of integration to step of time adaptive:

Time (S)
Reference
Code_Aster Error
(\%)
0,1 8,47734E01
8,47761E01
0,003
0,3 1,55202E+01
$1,55201 E+01$
0
0,5 4,36449E+01
4,36450E+01
0
0,7 8,50830E+01
$8,50832 E+01$
0
1,0 1,74790E+02
$1,74790 E+02$
0

Relative displacements of node NO3 with the numerical algorithm of integration of Euler:
Time (S)

Reference

Code_Aster Error
(\%)
0,01 9,87666E10
7,32629E05
0
*
0,02 2,49501E07
1,46526E04
0
*
0,03 6,25468E06
2,19789E04
0
*
0,04 6,05829E05
2,93052E04
0
*
0,05 3,47191E04
3,66314E04
0
*
0,06 1,42349E03
1,32757E02
0,012
*
0,07 4,62144E03
2,61852E02
0,022
*
0,08 1,26245E02
3,90946E02
0,026
*
0,09 3,01825E02
5,20040E02
0,022
*
0,1 7,68449E01
7,68420E01
-0,004
0,3 1,76923E+01

```
1,76922E+01
0
0,5 4,99310E+01
4,99311E+01
0
0,7 9,70711E+01
9,70714E+01
0
1,0 1,99722E+02
1,99722E+02
0
* absolute error
Handbook of Validation
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```

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
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Fe Key WAECKEL
:
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Relative displacements of node NO3 with the algorithm of numerical integration of Devogelaere:
Time (S)
Reference
Code_Aster Error
(\%)
0,1 7,68449E01
7,68449E01
0
0,3 1,76923E+01
1,76923E+01

## 0

0,5 4,99310E+01
4,99310E+01
0
0,7 9,70711E+01
9,70711E+01
0
1,0 1,99722E+02
$1,99722 E+02$
0

Displacements relating of node NO3 with the numerical algorithm of integration to step of time adaptive:

Time (S)
Reference
Code_Aster Error
(\%)
0,1 7,68449E01
7,68462E01
0,002
0,3 1,76923E+01
$1,76922 E+01$
0
0,5 4,99310E+01
4,99311E+01
0
0,7 9,70711E+01
9,70715E+01
0
1,0 1,99722E+02
$1,99722 E+02$
0

Relative displacements of node NO4 with the numerical algorithm of integration of Euler:
Time (S)
Reference
Code_Aster Error
(\%)
0,1 4,09632E01
4,09604E01

0,3 1,10372E+01

## 1,10371E+01

0
0,5 3,12415E+01
3,12416E+01
0
$0,76,05833 E+01$
6,05835E+01
0
1,0 1,24803E+02
1,24804E+02
0

Relative displacements of node NO4 with the numerical algorithm of integration of Devogelaere:
Time (S)
Reference
Code_Aster Error
(\%)
0,1 4,09632E01
4,09632E01
0
0,3 1,10372E+01
1,10372E+01
0
0,5 3,12415E+01
3,12415E+01
0
0,7 6,05833E+01
6,05833E+01
0
1,0 1,24803E+02
$1,24803 E+02$
0

Displacements relating of node NO4 with the numerical algorithm of integration to step of time adaptive:

Time (S)
Reference
Code_Aster Error

```
(%)
0,1 4,09632E01
4,09630E01
0
0,3 1,10372E+01
1,10371E+01
0
0,5 3,12415E+01
3,12416E+01
0
0,7 6,05833E+01
6,05835E+01
O
1,0 1,24803E+02
1,24804E+02
0
4.1.2 Absolute displacements of nodes NO2, NO3 and NO4
```

Absolute displacements of node NO2 with the numerical algorithm of integration of Euler:
Time (S)
Reference
Code_Aster Error
(\%)
0,1 4,02266E01
4,02275E01
0,002
0,3 8,57298E+01
$8,57299 E+01$
0
0,5 7,37605E+02
7,37605E+02
0
0,7 2,91617E+03
2,91617E+03
0
1,0 1,23252E+04
$1,23252 E+04$
0
Handbook of Validation
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
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Absolute displacements of node NO2 with the algorithm of numerical integration of Devogelaere:
Time (S)
Reference
Code_Aster Error
(\%)
0,1 4,02266E01
4,02266E01
0
0,3 8,57298E+01
8,57298E +01
0
0,5 7,37605E+02
7,37605E+02
0
0,7 2,91617E+03
2,91617E+03
0
1,0 1,23252E+04
$1,23252 E+04$
0

Absolute displacements of node NO2 with the numerical algorithm of integration to step of time adaptive:

## Time (S) <br> Reference

## Code_Aster Error

(\%)
0,1 4,02266E01
4,02239E01
-0,007
0,3 8,57298E+01
8,57299E+01
0
0,5 7,37605E+02
7,37605E+02
0
0,7 2,91617E+03
2,91617E+03
0
1,0 1,23252E+04
$1,23252 E+04$
0

Absolute displacements of node NO3 with the numerical algorithm of integration of Euler:

## Time (S)

Reference
Code_Aster Error
(\%)
0,1 6,48847E02
6,49134E02
0,044
0,3 4,98077E+01
4,98078E +01
0
0,5 4,70902E+02
4,70902E+02
0
0,7 1,90376E+03
1,90376E+03
0
1,0 8, 13361E+03
8,13361E+03
0

Absolute displacements of node NO3 with the numerical algorithm of integration of Devogelaere:

## Time (S)

## Reference

## Code_Aster Error

(\%)
0,1 6,48847E02
6,48847E02
0
0,3 4,98077E+01
4,98077E+01
0
0,5 4,70902E+02
4,70902E+02
0
0,7 1,90376E+03
$1,90376 E+03$
0
$1,08,13361 E+03$
8,13361E+03
0

Absolute displacements of node NO3 with the numerical algorithm of integration to step of time adaptive:

Time (S)

## Reference

Code_Aster Error
(\%)
0,1 6,48847E02
6,48714E02
-0,021
0,3 4,98077E+01
4,98078E+01
0
0,5 4,70902E+02
$4,70902 E+02$
0
0,7 1,90376E+03
1,90376E+03
0
$1,08,13361 E+03$
8,13361E+03
0

Absolute displacements of node NO4 with the numerical algorithm of integration of Euler:

## Time (S)

## Reference

Code_Aster Error
(\%)
0,1 7,03506E-03
7,06261E-03
0
0,3 2,27128E+01
2,27129E+01
0
0,5 2,29175E+02
2,29175E+02
0
0,7 9,39833E+02
$9,39833 E+02$
0
1,0 4,04186E+03
4,04186E+03
0
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-62/01/012/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Seismic SDLD103 Response of a system 3 masses and 4 springs
Date:
30/08/01
Author (S):

## Fe Key WAECKEL

:
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Absolute displacements of node NO4 with the algorithm of numerical integration of Devogelaere:

```
Time (S)
Reference
Code_Aster Error
(%)
0,1 7,03506E03
7,03504E03
O
0,3 2,27128E+01
2,27128E+01
O
0,5 2,29175E+02
2,29175E+02
O
0,7 9,39833E+02
9,39833E+02
O
1,0 4,04186E+03
4,04186E+03
O
```

Absolute displacements of node NO4 with the numerical algorithm of integration to step of time adaptive:

## Time (S)

Reference

Code_Aster Error

(\%)
0,1 7,03506E03
7,03655E03
0
0,3 2,27128E+01
2,27129E+01
0
0,5 2,29175E+02
2,29175E+02
0
0,7 9,39833E+02
$9,39833 E+02$
0
1,0 4,04186E+03
4,04186E+03
0

### 4.2 Parameters <br> of execution

Version: STA 5.02
Machine: SGI Origin 2000
Time CPU to use: 16,4 seconds
Handbook of Validation
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## Code_Aster ${ }^{\circledR}$

Version
5.0

## Titrate:

Seismic SDLD103 Response of a system 3 masses and 4 springs
Date:
30/08/01
Author (S):
Fe Key WAECKEL
:
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## 5 Modeling <br> B

It is same modeling as the preceding one except for loading which is one accélérogramme of rotation.

## 5.1 <br> Characteristics of modeling

The elements are modelled by discrete elements with 3 degrees of freedom DIS_T.

## X

2
K 1
k2
k3

Node NO1 is subjected to an imposed acceleration $1(T)$, node NO5 to 2 (T). It is calculated relative displacement of nodes $\mathrm{NO} 2, \mathrm{NO} 3$ and NO 4 compared to their static deformation, them displacement of drive and their absolute displacement.

Temporal integration is carried out with the algorithm of Euler (not of time: 103 second).

## 5.2 <br> Characteristics of the grid

The grid consists of 5 nodes and 4 discrete elements (DIST_TR).

### 5.3 Functionalities

tested

## Orders

```
Keys Doc. V5
AFFE_MODELE GROUP_MA
"MECHANICAL"
"DIS_T"
[U4.41.01]
DISCRETE AFFE_CARA_ELEM NODE M_TR_D_N
[U4.42.01]
NET
K_TR_D_L
AFFE_CHAR_MECA DDL_IMPO
[U4.61.21]

MODE_ITER_INV CALC_FREQ
ADJUST
[U4.52.04]
CALC_FONC_INTERP

\author{
[U4.32.01] \\ MODE_STATIQUE DDL_IMPO
}

\author{
[U4.52.14] \\ CALC_CHAR_SEISME NODE
}

\author{
[U4.63.01] \\ MACRO_PROJ_BASE
}

\author{
[U4.63.11] \\ DYNA_TRAN_MODAL EXCIT \\ MULT_APPUI \\ "YES" \\ [U4.53.21] \\ METHOD \\ EULER \\ REST_BASE_PHYS MULT_APPUI \\ "YES" [U4.63.21] \\ MULT_APPUI \\ "NOT"
}

RECU_FONCTION RESU_GENE
[U4.32.03]
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
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Date:
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Author (S):

\section*{Fe Key WAECKEL}

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\section*{6}

Results of modeling B

\section*{6.1 \\ Values tested of modeling B}

\subsection*{6.1.1 Displacements}
relative
nodes NO2, NO3 and NO4

Relative displacements of node NO2:

\section*{Time (S)}

\section*{Reference}

Code_Aster Error
(\%)
0,1 8,47734E01
8,47725E01
-0,001
0,3 1,55202E+01
1,55201E+01
0
0,5 4,36449E+01
\(4,36450 E+01\)
0
0,7 8,50830E+01
\(8,50832 E+01\)
0
1,0 1,74790E+02
1,74790E+02
0

Relative displacements of node NO3:

\section*{Time (S) \\ Reference \\ Code_Aster Error \\ (\%) \\ 0,1 7,68449E01 \\ 7,68420E01 \\ -0,004 \\ 0,3 1,76923E+01 \\ \(1,76922 E+01\) \\ 0 \\ 0,5 4,99310E+01 \\ 4,99311E+01 \\ 0 \\ 0,79,70711E+01 \\ 9,70714E+01 \\ 0 \\ 1,0 1,99722E+02 \\ \(1,99722 E+02\) \\ 0}

Relative displacements of node NO4:

\section*{Time (S)}

Reference

\author{
Code_Aster Error
}
(\%)
0,1 4,09632E01
4,09604E01
-0,007
0,3 1,10372E+01
\(1,10371 E+01\)
0
\(0,53,12415 E+01\)
3,12416E+01
0
0,76,05833E+01
6,05835E+01
0
1,0 1,24803E+02
\(1,24804 E+02\)
0

\subsection*{6.1.2 Absolute displacements of nodes NO2, NO3 and NO4}

Absolute displacements of node NO2:

\section*{Time (S)}

Reference
Code_Aster Error
(\%)
0,1 4,02266E01
4,02275E01
0,002
0,3 8,57298E+01
8,57299E+01
0
0,5 7,37605E+02
7,37605E+02
0
0,7 2,91617E+03
2,91617E+03
0
1,0 1,23252E+04
\(1,23252 E+04\)
0
Handbook of Validation
\(V 2.01\) booklet: Linear dynamics of the discrete systems
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Seismic SDLD103 Response of a system 3 masses and 4 springs
Date:
30/08/01
Author (S):

\section*{Fe Key WAECKEL}

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\section*{Time (S)}

\section*{Reference}

Code_Aster Error
(\%)
0,01 9,87666E10
7,32627E05
0
*
0,02 2,49501E07
1,46525E04
0
*
0,03 6,25468E06
2,19788E04
0
*
0,04 6,05829E05
2,93051E04
0
*
0,05 3,47191E04
3,66313E04
0
*
0,06 1,42349E03
1,32757E02 0,012
*
0,07 4,62144E03
2,61852E02 0,022
*
0,08 1,26245E02
3,90946E02 0,026
*
0,09 3,01825E02
5,20040E02 0,022
*
0,10 6,48847E02
6,49134E02 0,044
0,30 4,98077E+01
4,98078E+01 0

0,50 4,70902E+02
4,70902E+02 0
0,70 1,90376E+03
1,90376E+03 0
1,0 8, 13361E+03
8,13361E+03
0
* absolute error

Absolute displacements of node NO4:
Time (S)
Reference
Code_Aster Error
(\%)
0,1 7,03506E-03
7,06264E-03 0
0,3 2,27128E+01
2,27129E+01 0
0,5 2,29175E+02
2,29175E+02 0
0,7 9,39833E+02
9,39833E+02 0
1,0 4,04186E+03
4,04186E+03 0

\subsection*{6.2 Parameters of execution}

Version: STA 5.02
Machine: Sgi Origin 2000
Time CPU to use: 14,3 seconds

\section*{7 \\ Summary of the results}

The results obtained with Code_Aster are in conformity with the results of reference (the error is in general lower than 0,03\%).
Handbook of Validation
\(V 2.01\) booklet: Linear dynamics of the discrete systems

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLD104 - Extrapolation of local measurements on a complete model
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN

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Organization (S): EDF/RNE/AMV, CS IF

Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
V2.01.104 document

SDLD104-Extrapolation of local measurements on a complete model (discrete)

\section*{Summary:}

It is about a test of linear dynamics discrete.

The goal is to test order PROJ_MESU_MODAL in the case of a discrete system. This order allows to project experimental dynamic transitory answers in a certain number of points on a modal base of a numerical modeling.

This test contains 2 modelings:
projection is done on a basic concept modal of type [mode_meca],
projection is done on a basic concept modal of type [base_modale].
For 2 modelings, provided experimental measurements are identical and make it possible to test seek nodes in opposite, the taking into account of a local orientation and the treatment of one sampling in constant time or not, for measurements in displacement.

In both cases, the reference solution is analytically given (by Maple); projection is realized in the favorable configuration where the number of modes is equal to the number of measurements.
The answers in displacement obtained after projection are identical to displacements of reference provided in data.

The values speeds and the accelerations deduced from the displacements obtained after projection are close relations of those obtained analytically. The weak noted variations are due to the errors of approximation
generated by the determination via a linear diagram in time of speeds and accelerations.
Handbook of Validation
\(V 2.01\) booklet: Linear dynamics of the discrete systems
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLD104 - Extrapolation of local measurements on a complete model
Date:
04/03/02
Author (S):

\section*{S. AUDEBERT, Key P. HERMAN}

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\section*{1 \\ Problem of reference}

\section*{1.1 \\ Description of the system}

We consider the system represented by the diagram below:

\section*{K \\ K \\ \(K\) \\ \(m\) \\ \(m\) \\ X1 \\ X2 \\ 1.2 \\ Masses and rigidity}

The three springs are of identical rigidity: \(K=1000 \mathrm{~N} / \mathrm{m}\).
The two masses are equal to \(m=10 \mathrm{~kg}\).

\section*{1.3 \\ Boundary conditions and loading}

The two ends are embedded.
The loading is a thrust load in traction applied to the mass m1, sinusoidal according to time, of pulsation.

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Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN

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\section*{2 \\ Reference solutions}

\section*{2.1 \\ Method of calculation used for the reference solution}

The analytical solution of this problem is presented below.

Modes and frequencies of vibration:
The following system characterizes the dynamics of the masses:
\(m x \&+2 k x-k x\)
1
1
\(2=0\)
éq 2.1-1
MX
\(\&+2 k x-k x\)
2
2
\(1=0\)
What is equivalent to the following system:

\section*{éq 2.1-2}
(
\(m x \&-X\)
1
\&2) \(+K\)
\(3(X-X\)
1
2) \(=0\)

The 2 Eigen frequencies of the system are thus given by:
```

K
K
=
and
3
l
2=
éq
2.1-3
m
m

```
and the associated modal deformations are:
\(=\) and
1
\(2=\)

The generalized matrices are:
```

I
Im 01
1 2m
O
M=TM=

```
\(=\)
1
10
m 1 -
1
0
2
m
éq
2.1-5
1
\(12 k-k 1\)
\(12 k\)
0
\(K=T K=\)
1
1-K
2kl-

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLD104 - Extrapolation of local measurements on a complete model
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN
:
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The checked dynamic system is as follows:
MX
\(\&+\boldsymbol{K} X=\boldsymbol{F}\)
éq 2.1-6
While projecting on the basis of clean mode, we obtain:
\(T \&+T=T\)
\(\boldsymbol{M}\)
\(\boldsymbol{K}\)
\(\boldsymbol{F}\)
éq
\(2.1-7\)

That is to say:
\(2 m\)
\(0 \& 2 k\)
0111
1
1
\(+\)
=
(
\(\sin T\)
)
éq
2.1-8

0
\(2 m \& 06 k 1-10\)
2
2

We thus end to the following uncoupled system:
1
m
\(\&+K=i f(\)
\(N T\)
)
1
1
2
\(\&+K\)
\(3=\)
\((\)
\(\sin T\)
\()\)
2
2
2

The solution of this system is given by:

\section*{\(\sin T\)}
(T)

1
\(=1\)
(
\(\cos 1 t)+1\)
\(B\) if
\(N 1 t\) )
()
\(+2\)
(m2-2
1
)
éq
2.1-10
\(\sin\)
(
\(T\)
T)
2
\(=2\)
With

Displacements in physical space are obtained by the formula of Ritz:
\(x 1\)
1
11
+
1
2
\(X==\)
=
éq
2.1-11
x2
1 -

\section*{2.1-12}
\(\sin T\)
1
\(X(T)=A \cos\)
\(\sin\)
cos
\(\sin\)
1
1
\((T 1)+B 1(T 1)+A 2(t 2)+B 2(t 2)\)
()
\(+\)
\(+\)
\(2 m 2\)
2
2
2
1
2 -
\(\sin T\)
1
1
\(X(T)=A \cos\)
\(\sin\)
cos
\(\sin\)
2
\((T 1)+B 1(T 1)-A 2(t 2)+B 2(t 2)\)
()
\(+\)
\(2 m 2\)
2
2
2

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Code_Aster \({ }^{\circledR}\)
Version
5.0

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At the initial moment, the system is at rest, from where final expressions of \(X(T)\)
1
and \(X(T)\)
2
```

(
sin}
) -
S
in T
1)
(
sin}
) -
S(
in T
2)
l
X(T)
l
2
l
=
+
2m
2
2
2
2
1
2

```
```

éq
2.1-13

```
\(\sin T\)
) -
S (
in \(T\)
1)
(
\(\sin T\)
) -
S (
in \(T\)
2 )
1
\(X(T)\)
1
2
2
=
\(2 m\)
2
2
2
2

\section*{2.1-14}
```

(

```
\(\cos T\)
) - \(C\) (
bone \(T\)
1 )
Co
\(S T)-C(\)
bone
\(T\)
2 )
\(X(T)\)
\&
2
=
\(2 m\)
2
2
2
1
-
2

Accelerations of the two masses are calculated by deriving speeds compared to time:
```

l
in
T
1)
if(
NT) -
S
2
in
T
2)
\&x (T)

```
1
=
\(+\)
\(2 m\)
2
2

\section*{2.2}

Results of reference
The comparison of the results relates to displacements, speeds and accelerations along the axis of two masses, at five different moments.

\section*{2.3 \\ Uncertainty on the solution}

The reference solution is exact.
The discrete model represents perfectly the problem arising (the modal base is complete; there is not thus not of approximation related to a possible modal truncation). The number of modes of the base of modal projection is equal to the number of measurements, therefore the solution of the inversion is exact (by
opposition to an approximate solution of a generalized opposite problem). If the research of the nodes in opposite is good, the displacements obtained after projection must be in perfect adequacy with the experimental values. Speeds and accelerations are determined by derivation of modal contributions identified via a diagram of linear approximation in time, thus being able to generate some errors.
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling and the grids}

Numerical grid:
The numerical grid is carried out directly with the format ASTER. It comprises 4 nodes and 3 discrete meshs.
```

K
m K
m
K
X
N1
N2
N3
N4
(x=0.) (x=0.1) (x=0.2)(x=0.3)

```

Experimental grid:
The grid of measurement includes/understands only 2 specific elements and 2 nodes:

\section*{\(X\)}

N2 N3
\((x=0.12)(x=0.18)\)

\section*{3.2}

\section*{Characteristics of measurements}

Provided experimental measurements are:

\section*{With the N3 node:}

The data are the displacements axial, multiplied by (2/2), and applied in the direction
\(X\). the local orientation indicated in the command file is (45.0. 0.)
The sampling of time is constant: initial time is \(0 S\), the step of times is 103 S and it a many moments are 1001 (i.e until a final time of 1 S).

With the node N2:
The data are the axial displacements, applied in direction \(X\).
The sampling of time is variable: every moment is indicated of 0 S to \(1 S\), by step of 103 S (1001 moments on the whole).

The values result from the analytical calculation carried out with Maple.

\section*{3.3}

\section*{Characteristics of the modal base}

The two only modes are stored in a concept of the type [mode_meca] created by the order MODE_ITER_SIMULT. Their Eigen frequencies are identical to the analytical Eigen frequencies. Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
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\subsection*{3.4 Functionalities}
tested

\author{
Orders
}

\author{
AFFE_CARA_ELEM \\ DISCRETE \\ CARA \\ 'M_T_D_N \\ " \(K_{-} T_{-} D \_L\) " \\ ORIENTATION \\ "ANGL_NAUT" \\ LOCATE \\ "LOCAL" \\ AFFE_CHAR_MECA \\ DDL_IMPO \\ ALL \\ NODE \\ AFFE_MODELE \\ ALL \\ "MECHANICAL" "DIST_T" \\ ASSE_MATRICE
}

\author{
"RIGI_MECA" \\ MODE_ITER_SIMULT CALC_FREQ OPTION \\ "BAND" \\ \(N U M E \_D D L\)
}

NUME_DDL_GENE

PROJ_MATR_BASE

PROJ_MESU_MODAL
MEASURE

\section*{REGULARIZATION}

REST_BASE_PHYS TOUT_CHAM
"YES"

TEST_RESU NOM_CHAM
"DEPL"

\section*{CRITERION}
"QUICKLY"
"ACCE"
"RELATIVE"
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Code_Aster difference
with \(T=0.1 \mathrm{~S}\)
1.745 10-4 1.745

10-4 0.01
\%
with \(T=0.3 S\)
6.797 10-4 6.797

10-4 0.01
\%
DEPL_X
with the node N2
with \(T=0.5 \mathrm{~S}\)
-1.217 10-3 -1.217
10-3 0.01
\%
(m) (mass
1)
with \(T=0.7 S\)
5.214 10-4 5.214

10-4-0.01
\%

\title{
with \(T=0.9 \mathrm{~S}\)
}
9.031 10-4 9.031

10-4 0.00
\%
with \(T=0.1 S\)
9.154 10-6 9.154

10-6 0.00
\%
with \(T=0.3 S\)
6.414 10-4 6.414

10-4 0.00
\%
DEPL_X
with the N3 node
with \(T=0.5 \mathrm{~S}\)
-8.636 10-4 -8.636
10-4 0.00
\%
(m) (mass
2)
with \(T=0.7 S\)
-1.107 10-4 -1.107
10-4 0.03
\%
with \(T=0.9 \mathrm{~S}\)
1.633 10-3 1.633

10-3 0.02
\%
with \(T=0.1 S\)
4.586 10-3 4.616

10-3 0.65
\%
with \(T=0.3 S\)
\%
VITE_X
with the node N2
with \(T=0.5 \mathrm{~S}\)
-1.581 10-4 -8.000
10-5 7.81
\(105 \mathrm{~m} / \mathrm{s}\)
( \(\mathrm{m} / \mathrm{s}\) ) (mass
1)
with \(T=0.7 S\)
9.382 10-3 9.354
10-3-0.30
\%
with \(T=0.9 S\)
-7.481 10-3 -7.537
10-3 0.75
\%
with \(T=0.1 \mathrm{~S}\)
4.328 10-4 4.405
10-4 1.79
\%
with \(T=0.3 \mathrm{~S}\)
3.671 10-3 3.640
10-3-0.84
\%
VITE_X
with the N3 node
with \(T=0.5 \mathrm{~S}\)
-1.539 10-2 -1.536
10-2-0.20
\%
\((\mathrm{m} / \mathrm{s})(\mathrm{mass}\)
2)
with \(T=0.7 S\)
2.453 10-2 2.457
10-2 0.15
with \(T=0.9 S\)
-1.899 10-2 -1.912
10-2 0.68
\%
with \(T=0.1 S\)
6.112 10-2 6.100

10-2 -0.20
\%
with \(T=0.3 S\)
-1.306 10-1 -1.300
10-1-0.46
\%
ACCE_X
with the node \(N 2\)
with \(T=0.5 S\)
1.571 10-1 1.600

10-1 1.85
\%
(m/s2) (mass
1) with \(T=0.7 S\)
\(-5.65710-2-5.800\)
10-2 2.53
\%
with \(T=0.9 S\)
-1.124 10-1 -1.130
10-1 0.53
\%
with \(T=0.1 S\)
1.562 10-2 1.618

10-2 3.58
\%
with \(T=0.3 S\)
\%

\section*{Note:}

Speed with the node \(N 2\) at the moment \(T=0.5 S\) being relatively close to zero, the comparison is realized for this case in absolute value.

\subsection*{4.2 Parameters \\ of execution}

Version: STA5 (5.05)
Machine: CLASTER
Obstruction memory: 100 Mo
Time CPU To use: 9.05 seconds
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

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Date:
04/03/02
Author (S):

\section*{S. AUDEBERT, Key P. HERMAN}

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\section*{5 Modeling}

\section*{B}
5.1

Characteristics of modeling and the grids

Numerical grid:
The numerical grid is carried out directly with the format ASTER. It comprises 4 nodes and 3 discrete meshs.

\section*{K \\ \(m K\) \\ m \\ K \\ \(X\) \\ N1 \\ N2 \\ N3 \\ N4}
\((x=0).(x=0.1)(x=0.2)(x=0.3)\)

Experimental grid:
The grid of measurement includes/understands only 2 specific elements and 2 nodes:

\section*{\(X\)}

N2
N3
\((x=0.12)(x=0.18)\)

\section*{5.2}

\section*{Characteristic of measurements}

Provided experimental measurements are:

With the N3 node:
The data are the displacements axial, multiplied by (2/2), and applied in the direction
\(X\). The local orientation indicated in the command file is (45. 0. 0.)
The sampling of time is constant: initial time is 0 S, the step of times is 103 S and it a many moments are 1001 (i.e until a final time of \(1 S\) ).

With the node N2:
The data are the axial displacements, applied in direction \(X\).
The sampling of time is variable: every moment is indicated of \(0 S\) to \(1 S\), by step of \(103 S\) (1001 moments on the whole).

The values result from the analytical calculation carried out with Maple.

\section*{5.3}

Characteristics of the modal base
The two only modes are stored in a concept of the type [base_modale], created by the order
DEFI_BASE_MODALE. The interface, of Craig-Bampton type, is placed on the degree of freedom in displacement following \(X\) of the node N2 (corresponding to the mass \(m 1\) ). The modal base thus contains a dynamic mode (with blocked N2) and a static mode.
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Code_Aster \({ }^{\circledR}\)
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5.0

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Date:
04/03/02
Author (S):

\section*{S. AUDEBERT, Key P. HERMAN}

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\subsection*{5.4 Functionalities}
tested
Orders

AFFE_CARA_ELEM
DISCRETE
CARA
'M_T_D_N
\[
\text { " } K_{-} T_{-} D_{-} L \text { " }
\]

ORIENTATION
"ANGL_NAUT"

LOCATE
"LOCAL"

AFFE_CHAR_MECA
DDL_IMPO
ALL

\author{
NODE \\ AFFE_MODELE \\ ALL \\ "YES" \\ PHENOMENON \\ "MECHANICAL" \\ MODELING \\ "DIST_T"
}

\section*{REGULARIZATION}

REST_BASE_PHYS TOUT_CHAM

\section*{CRITERION}
"QUICKLY"
"АССЕ"
"RELATIVE"
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

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04/03/02
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\section*{6}

Results of modeling B

\subsection*{6.1 Values}
tested

\section*{Identification Reference}

Code_Aster difference
with \(T=0.1 \mathrm{~S}\)
1.745 10-4 1.745

10-4 0.01
\%
with \(T=0.3 S\)
6.797 10-4 6.797

10-4 0.01
```

%
DEPL_X

```
with the node N2
with \(T=0.5 \mathrm{~S}\)
-1.217 10-3-1.217
10-3 0.01
\%
(m) (mass
1)
with \(T=0.7 S\)
5.214 10-4 5.214
    10-4-0.01
    \%
with \(T=0.9 S\)
9.031 10-4 9.031
    10-4 0.00
\%
with \(T=0.1 \mathrm{~S}\)
9.154 10-6 9.154
10-6 0.00
\%
with \(T=0.3 S\)
6.414 10-4 6.414
    10-4 0.00
\%
DEPL_X
with the N3 node
with \(T=0.5 \mathrm{~S}\)
-8.636 10-4 -8.636
10-4 0.00
\%
(m) (mass
2)
with \(T=0.7 \mathrm{~S}\)
-1.107 10-4 -1.107
10-4 0.03
\%
with \(T=0.9 S\)
1.633 10-3 1.633

10-3 0.02
\%
with \(T=0.1 S\)
4.586 10-3 4.616

10-3 0.65
\%
with \(T=0.3 S\)
-7.598 10-3 -7.663
10-3 0.85
\%
VITE_X
with the node N2
with \(T=0.5 S\)
-1.581 10-4 -8.000
10-5 7.81
\(105 \mathrm{~m} / \mathrm{s}\)
(m/s) (mass
1)
with \(T=0.7 S\)
9.382 10-3 9.354

10-3-0.30
\%
with \(T=0.9 S\)
-7.481 10-3 -7.537
10-3 0.75
\%
with \(T=0.1 S\)
4.328 10-4 4.405

10-4 1.79
\%
with \(T=0.3 S\)
3.671 10-3 3.640
with \(T=0.9 S\)
-1.124 10-1 -1.130
10-1 0.53
\%
with \(T=0.1 S\)
\(1.56210-21.618\)
10-2 3.58
\%
with \(T=0.3 S\)
-6.031 10-2 -6.223
10-2 3.18
\%
ACCE_X
with the N3 node
with \(T=0.5 S\)
5.102 10-2 5.374

10-2 5.33
\%
(m/s2) (mass
2) with \(T=0.7 S\)
7.428 10-2 7.043

10-2 -5.19
\%
with \(T=0.9 S\)
-2.364 10-1 -2.263
10-1-4.28
\%

\section*{Note:}

Speed with the node \(N 2\) at the moment \(T=0.5 S\) being relatively close to zero, the comparison is realized for this case in absolute value.

\subsection*{6.2 Parameters of execution}

\section*{Version: STA5 (5.05)}

\title{
Machine: CLASTER
}

Obstruction memory: 100 Mo
Time CPU To use: 9.24 seconds
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Code_Aster \({ }^{\circledR}\)
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\section*{7}

Summary of the results
For two modelings, the answers in displacement obtained after projection are identical with the displacements of reference calculated analytically with Maple and provided in data.

Values speeds and the accelerations deduced from the displacements obtained after projection are close to those obtained analytically. The weak noted variations are due to the errors of approximation generated by the determination by a linear diagram in time speeds and accelerations.

\section*{Handbook of Validation}

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Code_Aster \({ }^{\circledR}\)
Version
7.0

Titrate:
SDLD313 - System mass-arises to 2 ddl (damping hysteretic)
Date:

\section*{23/06/03}

Author (S):
O. Key NICOLAS
:

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Organization (S): EDF-R \& D /AMA

\author{
Handbook of Validation \\ V2.01 booklet: Linear dynamics of the discrete systems \\ Document: V2.01.313
}

SDLD313 - System masses spring with 2 DDL with
damping hysteretic

\section*{Summary:}

This one-way problem consists in carrying out a harmonic analysis of a mechanical structure composed of a whole of mass-springs with damping hysteretic and subjected to an excitation sinusoidal. This test of mechanics of the structures corresponds to a dynamic analysis of a discrete model
having a linear behavior. It includes/understands three modelings.
Via modeling A, one tests the discrete elements in translation (mass, arises), them options AMOR_HYST of AFFE_CARA_ELEM.

Via modeling B, one tests the elements of beam (POU_D_T), options AMOR_HYST of DEFI_MATERIAU,
Via modeling C, one tests calculation modal (MODE_ITER_SIMULT) complex.
On the first two modelings, one tests the definition of a force of specific excitation harmonic and the operator of harmonic calculation of answer (DYNA_LINE_HARM [U4.54.02]). In addition, several are tested
operators of postprocessing: RECU_FONCTION [U4.62.03], TEST_FONCTION [U4.72.02], RECU_CHAMP
[U4.62.01].
Results obtained for the first two modelings (field of displacement for different frequencies of excitation) are in concord with the results of guide VPCS. Results obtained for the third modeling are in concord with the semi-analytical results.
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Date:
23/06/03
Author (S):
O. Key NICOLAS

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

We consider the system represented by the diagram below:

\section*{Specific masses:}
\(\boldsymbol{m} \boldsymbol{\&} \boldsymbol{m}\)
1
2
Stiffnesses of connection:
\(k 1 \& k 2\)
Damping hysteretic:
\(1 \& 2\)
1.2

Properties of material
Comes out from linear elastic translation
\(K 1=28000\)
\(N / m\)
\(K 2=\)
\(28000 \mathrm{~N} / \mathrm{m}\)
Specific mass
M1 =
10 kg
M2 =
5 kg
Damping hysteretic
\(1=\)
0.1
\(2=\)
0.0

\section*{1.3}

Boundary conditions and loadings
Boundary conditions:
Points A, B, C embedded out of DY and DZ
Embedded points \(a:(D X=0)\).
Loading: Force concentrated sinusoidal of variable frequency at the point \(C\)

\section*{Not C}
\(F x=0\)
\(F \sin t=2 F 0 \mathrm{~Hz}\)

F
Hz
21.0543

4

0
\(F=\) constant \(=\)
NR
100

\subsection*{1.4 Conditions}
initial
Without object for the study of the permanent harmonic mode.
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O. Key NICOLAS

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

The system of differential equations of the second order coupled is form:
\(M u \&+K u=F\)
000
\(1+\).
01 J
-1-0 1
. J
0
with \(M=0100\)
and
\(K=28000-1-\)
01 J
\(2+01\)
. J
-
1

005
0
-1
1

The solution with a harmonic excitation \(F=F 0 E J T j 2=-1\)
(

\section*{2}
\(\boldsymbol{K}-\boldsymbol{M}) \boldsymbol{U}=\boldsymbol{F}\)
0
0
This system is solved for all.

\section*{2.2 \\ Sizes and results of reference}

Displacement according to \(X\) of the point \(C\) for certain frequencies.
Eigen frequencies and damping reduced.

\section*{2.3 \\ Uncertainties on the solution \\ Semi-analytical solution.}

\subsection*{2.4 References \\ bibliographical}
[1]
J. PIRANDA: Note of use of the software of modal analysis MODAN - Version 0.2 (1990). Laboratory of Mechanics Applied - University of Frank County - Besancon (France).
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O. Key NICOLAS

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

\section*{Discrete element of rigidity in translation}
\(y\)
\(X\)
WITH B
C

Characteristics of the elements
DISCRETE:
with nodal masses
\(M_{-} T \_D \_N\)
and matrices of rigidity
K_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the node end \(A\)

Names of the nodes:
Not \(A=N 1\)
With \(=\) N1
Not \(B=N 2\)
\(B=N 2\)
Not \(C=\) N3
\(C=N 3\)

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 3 \\ A number of meshs and types: 2 SEG2
}

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

DISCRETE AFFE_CARA_ELEM GROUP_MA " \(K_{\_} T \_D \_L\) "
GROUP_MA
AMOR_HYST
"A_T_D_L"
"MECHANICAL" AFFE_MODELE VERY "DIS_T"
GROUP_NO
"DIS_T"
AFFE_CHAR_MECA DDL_IMPO GROUP_NO

\author{
FORCE_NODALE \\ NODE \\ DEFI_LIST_REEL BEGINNING
}

\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.0

Titrate:
SDLD313 - System mass-arises to 2 ddl (damping hysteretic)
Date:
23/06/03
Author (S):
O. Key NICOLAS

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\section*{3.4 \\ Sizes tested and results}

Parts real and imaginary of component \(D X\) of the displacement of the point \(C\).

\section*{Frequency Reference}

Aster \%
Difference
0.00
7.1075E-03
7.1074964639321E-03
1.08E-04
3.5360E-04
3.5360678925035E-04
3.36870E+00 9.388216E-03
9.3882649899583E-03
5.31E-04
7.31196E-04
7.3120610001073E-04
6.48480E+00 5.0269E-03
5.0349198344062E-03
0.012
7.07103E-02
7.0708581052416E-02

\author{
8.00060E+00 9.54931E-03 \\ 9.5490053525137E-03 \\ 0.003 \\ 2.2154E-03 \\ 2.2153458282190E-03 \\ 1.18746E+01 4.23259E-05 \\ \(4.2266734408325 E-05\) \\ 0.016 \\ 3.57193E-04 \\ 3.5719325443817E-04 \\ \(1.34747 E+012.35524 E-03\) \\ 2.3552527130123E-03 \\ 5.34E-04 \\ 5.01765E-04 \\ 5.0176685846530E-04 \\ 1.55802E+01 1.6395374E-02 \\ 1.6420641488151E-02 \\ 0.039 \\ \(6.871471 E-02\) \\ \(6.8704047854161 E-02\) \\ 2.10543E+01 1.88977E-03 \\ 1.8897660707219E-03 \\ 2.08E-04 \\ 5.53314E-06 \\ 5.5328629109043E-06
}

\subsection*{3.5 Remarks}

\section*{Contents of the file results:}

Values of the displacement of component DX of the point C for all the frequencies of 0 with 2.10543E+01Hz by step of 3.3687.

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V2.01 booklet: Linear dynamics of the discrete systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.0

Titrate:

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Date:
23/06/03
Author (S):
O. Key NICOLAS

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\section*{4 Modeling}

B

\section*{4.1}

Characteristics of modeling

\section*{Continuous element of beam type in traction}
y
\(X\)
WITH B
C

Characteristics of the elements
DISCRETE: masses
nodal
\(M_{-} T_{-} D_{-} N\)
BEAM:
matrices of rigidity
POU_D_T
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the node end \(A\)
(GROUP_NO: WITH DX: 0. )
Names of the nodes:
Not \(A=N 1\)
With \(=N 1\)

Not \(B=N 2\)
\(B=N 2\)
Not \(C=N 3\)
\(C=N 3\)

\section*{4.2}

Characteristics of the grid
A number of nodes: 3
A number of meshs and types: 2 SEG2

\subsection*{4.3 Functionalities}
tested

\author{
Orders
}

DISCRETE AFFE_CARA_ELEM
GROUP_MA " \(M_{-} T \_D \_L "\)
DEFI_MATERIEU ELAS
AMOR_HYST
AFFE_MODELE ALL
"MECHANICAL" "POU_D_T"
GROUP_NO
"DIS_T"
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\author{
FORCE_NODALE \\ NODE \\ CALC_MATR_ELEM RIGI_MECA_HYST
}

DEFI_LIST_REEL BEGINNING

\author{
Handbook of Validation
}
\(V 2.01\) booklet: Linear dynamics of the discrete systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.0

Titrate:
SDLD313 - System mass-arises to 2 ddl (damping hysteretic)
Date:
23/06/03
Author (S):
O. Key NICOLAS
:
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\section*{4.4 \\ Sizes tested and results}

Parts real and imaginary of component DX of the displacement of the point \(C\).

\section*{Frequency Reference}

Aster \%
Difference
0.00
7.1075E-03
7.1074964639321E-03
1.08E-04
3.5360E-04
3.5360678925035E-04
3.36870E+00 9.388216E-03
9.3882649899583E-03
5.31E-04
7.31196E-04
7.3120610001073E-04
6.48480E+00 5.0269E-03
5.0349198344064E-03
0.012
7.07103E-02

\title{
9.5490053525137E-03
}
0.003

\subsection*{2.2154E-03}
2.2153458282190E-03
1.18746E+01 4.23259E-05
4.2266734408325E-05
0.016
3.57193E-04
3.5719325443817E-04
1.34747E+01 2.35524E-03
2.3552527130123E-03
5.34E-04
5.01765E-04
5.0176685846530E-04
\(1.55802 E+011.6395374 E-02\)
1.6420641488152E-02
0.039
6.871471E-02
\(6.8704047854161 E-02\)
2.10543E+01 1.88977E-03
1.8897660707219E-03
2.08E-04
5.53314E-06
5.5328629109043E-06

\subsection*{4.5 Remarks}

\section*{Contents of the file results:}

Values of the displacement of component DX of the point C for all the frequencies of 0 with \(2.10543 E+01 H z\) by step of 3.3687 .

\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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7.0

Titrate:

SDLD313 - System mass-arises to 2 ddl (damping hysteretic)
Date:
23/06/03
Author (S):
O. Key NICOLAS

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\section*{5 Modeling}

C

\section*{5.1}

\section*{Characteristics of modeling}

\section*{Discrete element of rigidity in translation}
y
\(X\)
WITH B
C

Characteristics of the elements
DISCRETE:
with nodal masses
\(M_{-} T \_D \_N\)
and matrices of rigidity
K_T_D_L
Limiting conditions:
in all the nodes
DDL_IMPO:
(ALL: "YES" DY: 0. , DZ: 0. )
with the node end \(A\)
(GROUP_NO: WITH DX: 0. )
Names of the nodes:
Not \(A=N 1\)
With \(=N 1\)

Not \(B=N 2\)
\(B=N 2\)
Not \(C=N 3\)
\(C=N 3\)

\section*{5.2 \\ Characteristics of the grid \\ A number of nodes: 3 \\ A number of meshs and types: 2 SEG2}

\subsection*{5.3 Functionalities}
tested

\author{
Orders \\ DISCRETE AFFE_CARA_ELEM GROUP_MA " \(K_{-} T \_D \_L\) " \\ GROUP_MA \\ AMOR_HYST \\ "A_T_D_L" \\ "MECHANICAL" AFFE_MODELE VERY "DIS_T" \\ GROUP_NO \\ "DIS_T" \\ AFFE_CHAR_MECA DDL_IMPO GROUP_NO \\ FORCE_NODALE \\ NODE \\ MODE_ITER_SIMULT SORENSEN \\ "COMPLEX" \\ DEFI_LIST_REEL BEGINNING
}

INTERVAL
RECU_FONCTION LIST_FREQ

Handbook of Validation

Code_Aster \({ }^{\circledR}\)
Version
7.0

Titrate:
SDLD313 - System mass-arises to 2 ddl (damping hysteretic)
Date:
23/06/03
Author (S):
O. Key NICOLAS

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\section*{5.4 \\ Sizes tested and results}

Eigen frequencies and reduced depreciation.

Eigen frequencies:

\section*{Sequence number}

Reference
Aster \%
Difference
1
6.45376 .44568
-0.124
215.58061 .55612
-0.124

Reduced depreciation:

\section*{Sequence number \\ Reference \\ Aster \% \\ Difference}

\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
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Titrate:
SDLD313 - System mass-arises to 2 ddl (damping hysteretic)
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O. Key NICOLAS
:

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\section*{6}

Summary of the results
The results obtained are excellent.

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V2.01 booklet: Linear dynamics of the discrete systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLD320 - Transitory response of a free system of 3 masses and 2 springs
Date:
01/03/04
Author (S):
E. BOYERE, T. QUESNEL Key

V2.01.320-A Page:
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Organization (S): EDF-R \& D /AMA, IRCN

\section*{Handbook of Validation}

V2.01 booklet: Linear dynamics of the discrete systems
V2.01.320 document

SDLD320-Transitory response of a free system of 3 masses and 2 springs under excitation harmonic

\section*{Summary:}

One considers the transitory analysis of a discrete system masses/arises linear with three degrees of freedom completely free. This system has a non-proportional damping. A sinewave excitation is applied at an end of the system.

In this problem, one tests, through a discrete model, the calculation of the transitory response of a system
whose rigid modes are not fixed. One is interested only in the transient state. For that, one will seek the solution by an integration on the complete modal basis (DYNA_TRAN_MODAL [U4.53.21]).

\title{
The results obtained (displacement, speed and acceleration) are compared with an average of results
} coming from industrial codes and a method of integration numerical of type - Newmark improved.

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V2.01 booklet: Linear dynamics of the discrete systems
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Titrate:
SDLD320-Transitory response of a free system of 3 masses and 2 springs
Date:
01/03/04
Author (S):
E. BOYERE, T. QUESNEL Key

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1
Problem of reference
1.1 Geometry
k1
k2
m1
m
m
2
3
\(F 0 \sin (T)\)
P1
P
\(P\)
2

\section*{1.2}

Properties of materials
Stiffnesses of connection: \(k 1=4.109\) N.m1, \(k 2=5.33108\) N.m1

Specific masses: \(m 1=106 \mathrm{~kg}, \mathrm{~m} 2=\mathrm{m} 3=12.106 \mathrm{~kg}\)

One-way viscous damping: \(C 1=1.2566106 \mathrm{~kg} . \mathrm{sl}, C 2=9.0478106 \mathrm{~kg} . \mathrm{sl}\)

\section*{1.3 \\ Boundary conditions and loadings}

Completely free system.
Loading at the P3 point following axis X: \(F(T) 0=F 0 \sin (T)\) for \(T 0\) with \(F 0=5.104 \mathrm{NR}\) and \(=19\) rad.s 1.

\subsection*{1.4 Conditions}

\section*{initial}

The system is at rest with \(t=0:(\)
\(U 0)=0\) and
()
\(0=0\).
\(d t\)
Handbook of Validation
\(V 2.01\) booklet: Linear dynamics of the discrete systems
HT-66/04/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

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Titrate:
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Date:
01/03/04
Author (S):
E. BOYERE, T. QUESNEL Key

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

The research of the transitory response of this problem to damping nonproportional, and where them rigid modes are not fixed, can be carried out by numerical integration in real space:
\([M]\{u \&\}+[C]\{u \&\}+[K]\{U\}=\{F\)
\(N\)
\(N\)
\(N\)
\}.
For that, the answer was calculated with two industrial codes:

PERMAS: Diagram of integration of Newmark \((=0,25,=0,5), t=104 \mathrm{~s}\),
Diagram of integration with cubic interpolation of Hermit [bib1], \(t=104 s\),

ABAQUS: Diagram of integration of Hilber-Hughes-Taylor [bib2] ( \(=-0,05\) ), \(t=104 s\), and method of integration of - Newmark improved [bib3]:

\footnotetext{
\(+\)
\(+\)
\(+\)
+
+
+
\(F\)
\(F\)
\(F\)
}

To start, one takes:
\(u 0\) and \(U 1=U\)

\section*{2.2 \\ Results of reference}

Displacement, speed and acceleration of the P3 point.
Differential of displacement enters the points P3 and P1.
Relative displacement of the P3 point compared to the P1 point
1,00E-05
5,00E-06
(m) \(10,00 E+00\)
- \(\boldsymbol{U}\)

0,0
0,5
1,0
1,5
2,0
2,5
3,0
3,5
4,0
4,5
5,0
U 3
-5,00E-06
-1,00E-05
time (S)

\section*{Handbook of Validation}

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Author (S):
E. BOYERE, T. QUESNEL Key

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\section*{2.3 \\ Uncertainty on the solution}

Average of numerical solutions.

\subsection*{2.4 References \\ bibliographical}
[1]
J.H. ARGYRIS, PC DUNNE and T. ANGELOPOULOS "Non-linear oscillations using the finite technical element" comp. Meth. Appl. Mech. Engng., Vol.2, 1972, pp. 203-254 [2]
H.M. HILBER, T.J.R. HUGHES and R.L. TAYLOR "Improved numerical dissipation for time integration algorithms in structural dynamics" Earthquake Structural Engineering and Dynamics, Vol.5, 1977, pp. 283-292
[3]
Structural N.M. NEWMARK "A method of computation for dynamics" Proceeding ASCE
J.Eng.Mech. Div E-3, July 1959, pp. 67-94

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Titrate:
SDLD320 - Transitory response of a free system of 3 masses and 2 springs

\section*{Date:}

01/03/04
Author (S):
E. BOYERE, T. QUESNEL Key

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\section*{3 Modeling}

\section*{With}

\section*{3.1 \\ Characteristics of modeling}

Discrete elements of rigidity, damping and mass.
```

y
P
P2
P
l
3
*

DISCRETE:
nodal masses
$M \_T R \_D \_N$
rigidities
linear $K \_T R \_D \_L$
depreciation
linear
A_TR_D_L

No boundary conditions, in all the nodes: DX, DY, DZ, DRX, DRY, DRZ free.

Names of the nodes: $P 1=N 1, P 2=N 2, P 3=N 3$.
Method of calculation:
Integration on the modal basis supplements with Newmark $(=0,25,=0,5)$,
No time: $t=10-4$ s then modal recombination.
Duration of observation: 5 s .

## 3.2 <br> Characteristics of the grid

A number of nodes: 3
A number of meshs and type: 2 meshs SEG2

## 3.3 <br> Functionalities tested

## Orders

DISCRETE AFFE_CARA_ELEM
NET
" $K_{-} T R_{-} D \_L$ "

NET<br>"A_TR_D_L"<br>NODE<br>"M_TR_D_N"<br>MODE_ITER_SIMULT<br>CALC_FREQ<br>(Option: "CENTER")<br>DYNA_TRAN_MODAL NEWMARK

## CALC_FONCTION COMB

Handbook of Validation<br>V2.01 booklet: Linear dynamics of the discrete systems<br>HT-66/04/005/A

Code_Aster ${ }^{\circledR}$
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6.4

## Titrate:

SDLD320 - Transitory response of a free system of 3 masses and 2 springs
Date:
01/03/04
Author (S):
E. BOYERE, T. QUESNEL Key
:
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## 4 <br> Results of modeling $A$

### 4.1 Values

tested

Displacement of the P3 point
Time Displacement
Displacement
Difference
(S)

Reference (m)
Aster (m)
(\%)
0,09

## Time Speed Speed Difference <br> (S) <br> Reference

(m.s l)

Aster (m.s l)
(\%)
0,05
1,3425 E-4
1,34131 E-4
-0,088
0,32
-6,4111 E-5
-6,41097 E-4
-0,002
1,18
1,6104 E-5
1,60598 E-5
-0,274
3,55
4,4262 E-5
4,41720 E-5

# Acceleration of the P3 point 

## Time Acceleration

## Acceleration Difference

(S)

Reference
(m.s 2)

Aster (m.s 2)
(\%)
0,09
-3,5694 E-3
-3,56634 E-3
-0,086
0,18
-4,3924 E-3
-4,38933 E-3
-0,070
0,55
4,3766 E-3
4,37283 E-3
-0,086
1,18
4,2459 E-3
4,24264 E-3
-0,077
4,92
-4,2233 E-3
-4,21962 E-3
-0,087

Relative displacement of the P3 point compared to the P1 point

## Time u3-u1

## u3-u1 Difference

(S)

Reference (m)
Aster (m)
(\%)

0,18
8,0987 E-6
8,04800 E-6
-0,626
0,55
-6,2246 E-6
-6,21194 E-6
-0,203
0,82
5,3064 E-6
5,34121 E-6
0,656
1,18
-4,5552 E-6
-4,52071 E-6
-0,757
1,92
-3,0416 E-6
-3,04417 E-6
0,085
3,55
1,8448 E-6
1,82742 E-6
-0,942
4,92
1,4832 E-6
1,47526 E-6
-0,535

### 4.2 Remarks

In addition to the comparison for the values tested, one checks that the variables different kinematics that those related to the translation according to $X$ remain null.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems

## HT-66/04/005/A

## Code_Aster ${ }^{\circledR}$

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6.4

## Titrate:

SDLD320-Transitory response of a free system of 3 masses and 2 springs
Date:

To obtain a good precision of the results, it is initially necessary to obtain a base modal precise and perfectly orthogonal (MODE_ITER_SIMULT):
by avoiding the multiple modes (different rigidity on the nonexcited ddl),
by calculating the rigid modes of body correctly (to prefer the option "Centers" in MODE_ITER_SIMULT with the other options),
by specifying method "JACOBI" for a modal complete extraction.

The precision of the results is good as well for displacements for speeds and accelerations.

For the elastic response of the system (relative displacements u3-u1), the numerical precision is one little worse because of the numerical office plurality of the errors on the absolute values.
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V2.01 booklet: Linear dynamics of the discrete systems
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Titrate:
SDLD320-Transitory response of a free system of 3 masses and 2 springs
Date:
01/03/04
Author (S):
E. BOYERE, T. QUESNEL Key

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Titrate:
SDLD321-Transitory dynamic response of a harmonic oscillator
Date:
17/02/04
Author (S):
E. BOYERE, T. QUESNEL Key

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Organization (S): EDF-R \& D /AMA, IRCN

# Handbook of Validation 

V2.01 booklet: Linear dynamics of the discrete systems
V2.01.321 document

SDLD321-Transitory dynamic response of one
harmonic oscillator with damping
variable

## Summary:

The system considered is a harmonic oscillator with 1 d.d.l under harmonic excitation with resonance.
Various depreciation will be considered:

## critical damping,

average damping,
very weak damping.
Via this problem, one tests the various algorithms of order DYNA_TRAN_MODAL [U4.54.03] and their capacities to deal with problems with extreme damping. The results are compared with the exact analytical solutions.

# Handbook of Validation <br> V2.01 booklet: Linear dynamics of the discrete systems <br> HT-66/04/005/A 

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6.4

## Titrate:

SDLD321-Transitory dynamic response of a harmonic oscillator
Date:
17/02/04
Author (S):
E. BOYERE, T. QUESNEL Key

$$
:
$$

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## 1

Problem of reference

### 1.1 Geometry

The system is composed of a mass, a spring and a shock absorber. It admits a single degree of freedom in translation.
$y, v$
With
K
B
$X, U$
m
$F 0 \sin (T)$
C
: pulsation of excitation corresponding
with the resonance of the system not deadened
$=\mathrm{km}$

## 1.2

Material properties
Stiffness of connection: $K=25.103$ N.m-1
Specific mass: $m=10 \mathrm{~kg}$

Viscous damping:
$C=$ ccritic $; C=0,01$ ccritique $; C=10-5$ ccritique
with ccritic $=1.000 \mathrm{~kg} . \mathrm{s}-1$

## 1.3 <br> Boundary conditions and loadings

Embedded end A.
Force harmonic according to $X$ at the frequency of resonance at point $b$ :
$F(T)=F 0 \sin (T)$ for $T 0$ with $F 0=5 N R$ and $=$ $K=50$ rad.s-1.
m

### 1.4 Conditions initial

The system is at rest with $T=0:($
$U 0)=0$ and
()
$0=0$.
$d t$
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD321 - Transitory dynamic response of a harmonic oscillator
Date:
17/02/04
Author (S):
E. BOYERE, T. QUESNEL Key
:
V2.01.321-A Page:
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## 2 <br> Reference solution

## 2.1

Method of calculation used for the reference solution
The simple oscillator checks the following equation:
$m u \&+C u \&+K U=F \sin$
0
(T
)
with $U(0)=0$ and $u \&(0)=0$
: own pulsation of the oscillator $=$
K
m
Critical damping is ccritic $=2 m$.
The solution for $C=$ ccritic is:
(
F
UT)
0 [and
(1+T
) $-\cos (T$
)]
$2 k$

## C

The solution for a subcritical damping such as
= is:
ccritic
(

- $T$

F
F
F
$U T)=E$

```
2 K
D
with
12
D=
```


## 2.2

## Results of reference

## Displacement and speed of the point B.

## Displacement for critical damping

1,50E-04
1,00E-04
5,00E-05
)
(m 0,00E+00
$\boldsymbol{U} \boldsymbol{B}$
0
0,1
0,2
0,3
0,4

0,5
-5,00E-05
-1,00E-04
-1,50E-04
time (S)

## Handbook of Validation

V2.01 booklet: Linear dynamics of the discrete systems
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Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD321 - Transitory dynamic response of a harmonic oscillator
Date:
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E. BOYERE, T. QUESNEL Key
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Displacement for the damping of $1 \%$
1,00E-02
5,00E-03
(m) 0,00E+00
$\boldsymbol{U} \boldsymbol{B}$
0
0,5
1
1,5
2
2,5
3
3,5
4
4,5
5
-5,00E-03
-1,00E-02
time (S)

## Displacement for the damping of 0,001\%

## 2,50E-02

2,00E-02
1,50E-02
1,00E-02
5,00E-03
(m) $0,00 E+00$
$\boldsymbol{U} \boldsymbol{B}$
0
0,5
1
1,5
2
2,5
3
3,5
4
4,5
5
-5,00E-03
-1,00E-02
-1,50E-02
-2,00E-02
-2,50E-02
time (S)

## 2.3

Uncertainty on the solution
Exact analytical solution.
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Version
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Titrate:
SDLD321-Transitory dynamic response of a harmonic oscillator

## Date:

17/02/04
Author (S):
E. BOYERE, T. QUESNEL Key

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling <br> Discrete elements of rigidity, damping and mass.

$y$
. B
$A$.
$X$
N1
N2

Characteristics of the elements:
DISCRETE: nodal mass
$M_{-} T \_D \_N$
rigidity
linear
$K_{-} T_{-} D \_L$
damping
linear
$A_{-} T_{-} D_{-} L(c=c c r i t i c)$
Boundary conditions: with the node N1 DDL_IMPO DX $=D Y=D Z=0$.
Names of the nodes: $P 1=N 1, P 2=N 2$.
Methods of calculation:

Integration on the modal basis with Newmark $(=0,25,=0,5)$
No time $T=10-3 S$

Integration on the modal basis with Euler
No time $T=10-3 \mathrm{~S}$
Duration of observation: 0,5 S.

## 3.2

Characteristics of the grid
A number of nodes: 2
A number of meshs and type: 1 mesh SEG2

## 3.3

Functionalities tested
Orders

DISCRETE AFFE_CARA_ELEM
NET "K_T_D_L"

## NET

"A_T_D_L"
NODE
"M_T_D_N"
MODE_ITER_SIMULT
OPTION: "CENTER"

## ! FORMULATE REAL

CALC_FONC_INTERP FUNCTION

## DYNA_TRAN_MODAL NEWMARK AMOR_GENE

## EULER

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## 4 <br> Results of modeling $A$

### 4.1 Values

tested

Displacement of the point $B$

## Displacement displacement

## Displacement

## Time

Reference
NEWMARK
Difference
EULER
Difference
(S)
(m)

Aster (m)
(\%)
Aster (m)
(\%)
0,06
1,18914 E4
1,18886 E4 0,023 1,18886
E4

```
-0,024
0,12
9,42819 E5
9,42574 E5 0,026 9,47822
E5 0,531
0,19
9,97958 E5
9,97765 E5 0,019 9,96206
E5
-0,176
0,25
9,97748 E5
9,97526 E5 0,022 9,99152
E5
-0,141
0,31
9,78457 E5
9,78210 E5 0,025 9,83436
E5
0,509
0,38
9,88705 E5
9,88530 E5 0,018 9,84730
E5
-0,402
0,44
9,99961 E5
9,99754 E5 0,021 9,99525
E5
-0,044
```

Speed of the point B

Speed Speed<br>Speed<br>\section*{Time}<br>Reference NEWMARK<br>Difference<br>EULER<br>Difference<br>(S)

```
(m.s-1)
Aster (m.s-1)
(%)
Aster (m.s-1)
(%)
0,03
3,31400 E3
3,31363 E3 0,011 3,32568
E3
0,353
0,09
5,13760 E3
5,13729 E3 0,006 5,13627
E3
-0,026
0,16
4,93337 E3
4,93354 E3 0,003
4,93088
E3
-0,050
0,22
5,00087 E3
5,00087 E3 0,000
-5,00133
E3
0,009
0,28
4,95298 E3
4,95284 E3 0,003 4,95297
E3
0,000
0,35
4,87813 E3
4,87836 E3 0,005
-4,87801
E3
-0,002
0,41
4,98415 E3
4,98423 E3 0,002
4,98409
E3
```


### 4.2 Remarks

The results are tested on the level of the peaks for the grain of observation selected (10-2s) where values
are most significant.
The mode becomes quasi-permanent after the first period, it is what one must observe in carrying out a transitory analysis.

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## 5 Modeling <br> B

## 5.1 <br> Characteristics of modeling

Discrete elements of rigidity, damping and mass.
$y$
A.

```
X
N1
N2
```

Characteristics of the elements:
DISCRETE: nodal mass
$M_{-} T \_D \_N$
rigidity
linear $K_{-} T$ _D_L
damping
linear
$A_{-} T \_D \_L(C=0,01$ ccritic $)$
Boundary conditions: with the node N1 DDL_IMPO DX $=D Y=D Z=0$.
Names of the nodes: $P 1=N 1, P 2=N 2$.
Methods of calculation:

## Integration on the modal basis with Fu-Devogelaere

No time $\boldsymbol{T}=10-3 \mathrm{~s}$

Integration on the modal basis with T adaptive
No initial time $\boldsymbol{T}=\mathbf{1 0}-\mathbf{3 s}$
Duration of observation: 5 S.
5.2 Characteristics of the grid

A number of nodes: 2
A number of meshs and type: 1 mesh SEG2
5.3

Functionalities tested
Orders

```
DISCRETE AFFE_CARA_ELEM
NET
"K_T_D_L"
```


## NET

```
"A_T_D_L"
NODE
"M_T_D_N"
MODE_ITER_SIMULT
OPTION: "CENTER"
```


## ! FORMULATE REAL

## CALC_FONC_INTERP FUNCTION

## DYNA_TRAN_MODAL DEVOGE <br> AMOR_REDUIT

ADAPT<br>AMOR_GENE

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## 6 <br> Results of modeling B

### 6.1 Values

tested

Displacement of the point $B$

## Displacement displacement

## Displacement

## Time

Reference DEVOG Difference
ADAPT
Difference
(S)
(m)

Aster (m)
(\%)
Aster (m)
(\%)
0,06
3,06503 E4
3,06503 E4 0,000 3,06481
E4
-0,007
0,13
5,93807 E4
5,93807 E4 0,000 5,93696
E4
-0,019
0,25
1,17872 E3
1,17872 E3 0,000 1,17862
E3
-0,009
0,69
2,91788 E3

## Speed Speed

## Speed

Time<br>Reference DEVOG Difference ADAPT<br>Difference<br>(S)<br>(m.s-1)<br>Aster (m.s-1)<br>(\%)<br>Aster (m.s-1)<br>(\%)<br>0,04<br>8,95997 E3<br>8,95997 E3 0,000<br>8,97145<br>E3 0,128

## 0,10

2,33271 E2
2,33271E2
0,000-2,33492
E2
0,095
0,22
5,20590 E2
5,20590 E2
0,000-5,21002
E2
0,079
0,66
1,40500 E1
1,40500 E1 0,000
1,40593
E1 0,066
1,04
1,99889 E1
1,99889 E1 0,000
1,99923
E1 0,017
2,36
3,39933 E1
3,39933 E1
0,000-3,39703
E1
-0,068
3,68
4,10585 E1
4,10585 E1 0,000
4,09991
E1
-0,145
5,00
4,45309 E1
4,45309 E1
0,000-4,42239
E1
-0,689

### 6.2 Remarks

The results are tested on the level of the peaks where the values are most significant.
The duration of selected observation makes it possible to see the effect of damping. However, in this interval, the response of the point B remains always transitory but one is close to steady operation whose scale of displacement is 10-2m.

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## 7 Modeling

C

## 7.1

Characteristics of modeling
Discrete elements of rigidity, damping and mass.
$y$
. B
A.

X
N1
N2

Characteristics of the elements:
DISCRETE:
nodal mass

Boundary conditions: with the node N1 DDL_IMPO $D X=D Y=D Z=0$.
Names of the nodes: $P 1=N 1, P 2=N 2$.
Methods of calculation:

Integration on the modal basis with Newmark ( $=0,25,=0,5$ )
No time $\boldsymbol{T}=10-3 \mathrm{~s}$

## Integration on the modal basis with Euler

No time $\boldsymbol{T}=\mathbf{1 0 - 3 s}$
Duration of observation: 5 S.

## 7.2

Characteristics of the grid
A number of nodes: 2
A number of meshs and type: 1 mesh SEG2

## 7.3

Functionalities tested
Orders

## DISCRETE AFFE_CARA_ELEM

NET "K_T_D_L"
NET
"A_T_D_L"
NODE

## ! FORMULATE REAL

CALC_FONC_INTERP FUNCTION

DYNA_TRAN_MODAL NEWMARK AMOR_GENE

## EULER

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$V$
10
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## Results of modeling $C$

### 8.1 Values

tested

## Displacement of the point $B$

## Displacement displacement

Displacement
Time
Reference NEWMARK Difference
EULER
Difference
(S)
(m)

Aster (m)
(\%)
Aster (m)
(\%)
0,06
3,11105 E4
3,10936 E4 0,054 3,11181
E4
0,024
0,13
6,13250 E4
6,13016 E4 0,038 6,13380
E4
0,021
0,25
1,25380 E3
1,25304 E3 0,060 1,25418
E3
0,030
0,69
3,44945 E3
3,44691 E3 0,074 3,45069
E3
0,036
1,01
4,88729 E3
4,89081 E3 0,072 4,88547
E3
-0,037
2,32

```
1,12876 E2
1,12475 E2 0,355 1,13069
E2
0,171
3,64
1,77960 E2
1,77100 E2 0,484 1,78360
E2
0,225
4,96
2,43613 E2
2,42198 E2 0,581 2,44242
E2
0,258
Speed of the point B
Speed Speed
Speed
Time
Reference NEWMARK Difference
EULER
Difference
(S)
(m.s-1)
Aster (m.s-1)
(%)
Aster (m.s-1)
(%)
0,04
9,09284 E3
9,08897 E3 0,043 9,08230
E3
-0,116
0,10
2,39724 E2
2,39637 E2 0,036 2,40269
E2 0,227
0,22
5,49964 E2
```

5,49680 E2 0,052 5,48752
E2
-0,220
0,66
1,64958 E1
1,64879 E1 0,048 1,64882
E1
-0,046
1,04
2,56456 E1
2,56547 E1 0,035
2,57280
E1
0,321
2,36
5,79010 E1
5,80019 E1 0,174
-5,81033
E1
0,349
3,68
8,97631 E1
9,00729 E1 0,345
9,00668
E1
0,338
5,00-1,21164-1,21829
0,549-1,21531
0,303

### 8.2 Remarks

The results are tested on the level of the peaks where the values are most significant.
In the interval of observation, one remains very in lower part of steady operation in resonance of which
the scale of displacement is 10 Mr .
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## 9

Summary of the results
As modeling A, as result obtained as well in displacement of speed have an error absolute largely lower than $1 \%$ compared to the analytical solution.

The diagram of integration of Newmark is shown more precise than the diagram of Euler.
To $1 \%$ of damping criticizes (modeling B), the diagram of Fu-Devogelaere integration is of one frightening precision (not of error compared to the reference solution).

The diagram with step of adaptive time also gives results to very small percentage of error.
For very weak depreciation (modeling C), one will note a better precision for diagram of integration of the Euler type that for a diagram of the Newmark type. For this last, the error
increase according to time but remains lower than $1 \%$ all the same.

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SDLD325 - Transitory dynamic response of a system mass-arises
Date:
16/02/04
Author (S):
E. BOYERE, T. QUESNEL Key

\author{

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V2.01.325 document

SDLD325-Transitory dynamic response of one system mass-arises deadened to 2 ddl

## Summary:

This problem consists in analyzing the dynamic response of a system made up of a unit of mass-spring-shock absorbers with 2 ddl from which the stiffnesses of the springs are very different under excitation from crenel type in 1 ddl.

Via this problem, one tests the sensitivity of diagrams of integration on physical space (DYNA_LINE_TRAN [U4.53.02]) or modal space (DYNA_TRAN_MODAL [U4.53.21]) with respect to the report/ratio of rigidities.

The results in displacement and speed are compared with an average of results coming from codes industrialists and of a method of integration numerical of type - Newmark improved.

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1
Problem of reference

### 1.1 Geometry

$\boldsymbol{K}$
$C$
$K$
B
1
2

With
m
m
F
$\boldsymbol{X}, \boldsymbol{U}$
$0(T)$
C
C
$T(S)$
0
1

## 1.2

Material properties

# Stiffnesses of connection: $K=28.103$ N.m1 

2 cases:
$k 1=k / 10, k 2=10 k$
$k 1=10 k, k 2=k / 10$
Specific mass: $m=10 \mathrm{~kg}$
One-way viscous damping: $C=50 \mathrm{~kg} . \mathrm{s} 1$

## 1.3 <br> Boundary conditions and loadings

Embedded end A.

$$
(T)=1 \text { if } 0 T S
$$

1
Force applied at end b: $F(T)=F 0(T)$ with and $F 0=5 N$.
$(T)=0$ if not

### 1.4 Conditions initial

The system is at rest with $T=0$ : (
U)
$0=0$ and
()
$0=0$.
$d t$
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2
Reference solution

## 2.1

Method of calculation used for the reference solution
The research of the transitory response of this problem to damping nonproportional can be carried out by numerical integration in real space:
$[M]\{u \&\}+[C]\{u \&\}+[K]\{U\}=\{F$
$N$
$N$
$N$
\}.

For that, the answer was calculated with two industrial codes:

PERMAS: Diagram of integration of Newmark $(=0,25$ and $=0,5) T=104 \mathrm{~s}$;
ABAQUS: Diagram of integration of Hilbert-Hugues-Taylor [bib1] $(=0,05) T=104 s$; and method of integration of - Newmark improved [bib2]:
[M] [C] [K]

```
+
+
+
{
2M
K
U
N2
N1
N
n+2}
{
} {
} {}
[ ] [ ]
=
+
{U
n+}
t2
2t
3
3
t2
1
3
[M][C][K]
+
```

where $N, n+1, n+2$ respectively indicate the calculations carried out at times $t n, t n+1=t n+t$ and $t n+2$ $=t n+2 t$
where $T$ is the increment of appointed time.
To start, one takes:
$u 0$ and $U 1=U$

0-T
u\&0
$\cdot$
F
$2 F$
F

- =

1
0
1

The step of adopted time is $T=105 \mathrm{~s}$.

## 2.2

Results of reference
Displacement and speed of the point end B.
Displacement of the point B for $k 2 / k 1=100$
3,20E-03
2,40E-03
1,60E-03
(m) 8,00E-04
$\boldsymbol{U} \boldsymbol{B}$
0,00E+00
0,00
0,50
1,00
1,50

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Displacement of the point $B$ for $\mathbf{k 2} / \mathbf{k} 1=0,01$
3,00E-03
2,50E-03
2,00E-03
1,50E-03
1,00E-03
(m)

U B 5,00E-04
0,00E+00
0,00
0,50
1,00
1,50
2,00
2,50
-5,00E-04
-1,00E-03
-1,50E-03
time (S)

## 2.3 <br> Uncertainty on the solution

Average of numerical solutions.

### 2.4 References <br> bibliographical

H.M. HILBERT, T.J.R HUGUES and R.L. TAYLOR "Improved numerical dissipation for time integration algorithms in structural dynamics" Earthquake Structural Engineering and Dynamics, Vol.5, 1977, pp. 283-292
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Structural N.M. NEWMARK "A method of computation for dynamics" Proceeding ASCE
J.Eng.Mech. DIV E-3, July 1959, pp. 67-94

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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

Discrete elements of rigidity, damping and mass.

Characteristics of the elements:
DISCRETE: nodal mass
M_T_D_N
rigidity
linear
$K_{-} T \_D \_L(k N 1 N 2=k / 10, k N 2 N 3=10 k)$
damping
linear
$A_{-} T_{-} D \_L$
Boundary conditions: with the node N1 DDL_IMPO $D X=D Y=D Z=0$.
Names of the nodes: With $=$ N1, $C=N 2, B=N 3$.
Methods of calculation:

Integration on physical space with Newmark $(=0,25,=0,5)$
No time $T=103 \mathrm{~s}$

Integration on the modal basis supplements with Euler
No time $T=103$ s then modal recombination

Integration on the modal basis supplements with T adaptive
No initial time $T=103 \mathrm{~s}$ then modal recombination
Duration of observation: 3 S.

## 3.2

Characteristics of the grid
A number of nodes: 3
A number of meshs and type: 2 meshs SEG2

## 3.3 <br> Functionalities tested

## Orders

DISCRETE AFFE_CARA_ELEM<br>NET " $K_{-} T \_D \_L$ "<br>NET<br>" $A_{-} T_{-} D_{-} L$ "<br>NODE<br>" $M_{-} T \_D \_N$ "<br>MODE_ITER_SIMULT<br>OPTION: "CENTER"<br>DYNA_LINE_TRAN NEWMARK<br>MATR_AMOR<br>DYNA_TRAN_MODAL EULER<br>AMOR_GENE<br>ADAPT

REST_BASE_PHYS

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## 4 <br> Results of modeling $A$

### 4.1 Values

tested

Displacement (m) of the point B

## Displacement displacement

Displacement

Time Reference
Aster Difference
Aster Difference
Aster Difference
(S)

NEWMARK
(\%)

## EULER

(\%)
ADAPT
(\%)
0,27 3,0927 E-3 3,09263 E-3
-0,002
3,09254 E-3
-0,005
3,09278 E-3
0,003
0,53 8,7953 E-4 8,79902 E-4
0,042
8,79515 E-4
-0,002
8,79583 E-4
0,006
0,80 2,4669 E-3 2,46677 E-3
-0,005
2,46666 E-4

```
-0,010
2,46688 E-4
-0,001
1,25-1,0980 E-3-1,09829 E-3
0,026
-1,09248 E-4
-0,502
-1,09844 E-4
0,040
1,51 7,8754 E-4 7,87625 E-4
0,011
7,82702 E-4
-0,614
7,87760 E-4
0,028
1,78-5,6508 E-4 -5,65131 E-4
0,009
-5,61709 E-4
-0,597
-5,65265 E-4
0,033
2,05 4,0502 E-4 4,05155 E-4
0,033
4,02581 E-4
-0,602
4,05168 E-4
0,037
2,31 -2,9012 E-4 -2,90070 E-4
-0,017
-2,88252 E-4
-0,644
-2,90192 E-4
0,025
2,58 2,0831 E-4 2,08323 E-4
0,006
2,06960 E-4
-0,648
2,08376 E-4
0,032
2,85-1,4943 E-4 -1,49462 E-4
0,022
-1,48425 E-4
-0,672
```


## Speed (m.sl) of the point B

## Speed Speed Speed

Time Reference
Aster Difference
Aster Difference
Aster Difference
(S)

NEWMARK
(\%)

## EULER

(\%)

## ADAPT

(\%)
0,11 1,8347 E-2 1,82400 E-2
-0,583
1,84067 E-2
0,326
1,83510 E-2
0,022
0,39-1,3140 E-2 -1,31120 E-2
-0,213
1,31472 E-2
0,055
-1,31407 E-2
0,006
0,66 9,3509 E-3 9,34550 E-3
-0,058
9,36556 E-3
0,157
9,35335 E-2
0,026
0,93-6,7080 E-3-6,71303 E-3
0,075
-6,70399 E-3
-0,060
-6,70788 E-3
-0,002
1,11-1,5863 E-2 -1,57872 E-2

### 4.2 Remarks

The results are tested on the level of the respective peaks of displacement and speed where values are most significant.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A

Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD325-Transitory dynamic response of a system mass-arises
Date:
16/02/04
Author (S):
E. BOYERE, T. QUESNEL Key

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## 5 Modeling

B

## 5.1

Characteristics of modeling
Discrete elements of rigidity, damping and mass.
$y$

Characteristics of the elements:
DISCRETE: nodal mass
M_T_D_N
rigidity
linear
$K_{-} T_{-} D_{-} L(k N 1 N 2=10 k, k N 2 N 3=k / 10)$
damping
linear
$A_{-} T_{-} D_{-} L$
Boundary conditions: with the node N1 DDL_IMPO $D X=D Y=D Z=0$.
Names of the nodes: With $=$ N1, $C=N 2, B=N 3$.
Methods of calculation:

Integration on physical space with Newmark $(=0,25,=0,5)$
No time $T=103 \mathrm{~s}$

Integration on the modal basis supplements with Euler
No time $T=103$ s then modal recombination

Integration on the modal basis supplements with T adaptive
No initial time $\boldsymbol{T}=103$ s then modal recombination
Duration of observation: 2,5 S.

## 5.2

Characteristics of the grid
A number of nodes: 3
A number of meshs and type: 2 meshs SEG2

### 5.3 Functionalities

tested

# DISCRETE AFFE_CARA_ELEM 

NET "K_T_D_L"

## NET

" $A_{-} T_{-} D_{-} L "$

## NODE

" $M_{-} T_{-} D_{-} N "$
MODE_ITER_SIMULT
OPTION: "CENTER"

DYNA_LINE_TRAN NEWMARK<br>MATR_AMOR<br>DYNA_TRAN_MODAL EULER<br>AMOR_GENE<br>\section*{ADAPT}

$R E S T \_B A S E \_P H Y S$

## Handbook of Validation

V2.01 booklet: Linear dynamics of the discrete systems

## HT-66/04/005/A

## Code_Aster ${ }^{\circledR}$

Version
6.4

## Titrate:

SDLD325 - Transitory dynamic response of a system mass-arises
Date:
16/02/04
Author (S):

## E. BOYERE, T. QUESNEL Key

:

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6<br>Results of modeling B<br>\subsection*{6.1 Values}<br>tested<br>\section*{Displacement (m) of the point B}

## Displacement displacement

Displacement
Time Reference
Aster Difference
Aster Difference
Aster Difference
(S)

NEWMARK
(\%)
EULER
(\%)
ADAPT
(\%)
0,19 2,9334 E-3 2,93325 E-3
-0,005
2,93308 E-3
-0,011
2,93355 E-3
0,005
0,38 1,0959 E-3 1,09605 E-3
0,014
1,09625 E-3
0,032
1,09573 E-3
-0,015
0,57 2,2468 E-3 2,24664 E-3
-0,007
2,24647 E-3

```
-0,015
2,24690 E-3
0,005
0,76 1,5260 E-3 1,52615 E-3
0,010
1,52627 E-3
0,017
1,52595 E-3
-0,003
0,95 1,9773 E-3 1,97725 E-3
-0,002
1,97718 E-3
-0,006
1,97739 E-3
0,005
1,19-1,2107 E-3 -1,21113 E-3
0,036
-1,20839 E-3
-0,191
-1,21142 E-3
0,060
1,38 7,5880 E-4 7,59030 E-4
0,030
7,56994 E-4
-0,238
7,59422 E-4
0,082
1,57 -4,7553 E-4 -4,75637 E-4
0,023
-4,74180 E-4
-0,284
-4,75974 E-4
0,093
1,76 2,9796 E-4 2,98011 E-4
0,017
2,97002 E-4
-0,322
2,98273 E-4
0,105
1,95-1,8668 E-4 -1,86695 E-4
0,008
-1,86012 E-4
-0,358
```

-1,86890 E-4<br>0,113<br>2,14 1,1694 E-4 1,16943 E-4<br>0,002<br>1,16489 E-4<br>-0,385<br>1,17076 E-4<br>0,116<br>2,33-7,3246 E-5 -7,32415 E-5<br>-0,006<br>-7,29453 E-5<br>-0,411<br>-7,33309 E-5<br>0,116

Speed (m.s1) of the point B

## Speed Speed Speed

Time Reference
Aster Difference
Aster Difference
Aster Difference
(S)

NEWMARK
(\%)
EULER
(\%)
ADAPT
(\%)
0,09 2,4261 E-2 2,42719 E-2
0,045
2,42772 E-2
0,067
2,42563 E-2
-0,019
0,28-1,5210 E-2 -1,52159 E-2
0,039
-1,52111 E-2
0,007
-1,52087 E-2
-0,009
0,47 9,5332 E-3 9,53598 E-3

```
0,029
9,52994 E-3
-0,034
9,53446 E-3
0,013
0,66 -5,9745 E-3 -5,97590 E-3
0,023
-5,97018 E-3
-0,072
-5,97614 E-3
0,028
0,85 3,7438 E-3 3,74438 E-3
0,015
3,73979 E-3
-0,107
3,74519 E-3
0,037
1,08 -2,6037 E-2 -2,60274 E-2
-0,037
-2,59908 E-2
-0,177
-2,60402 E-2
0,012
1,27 1,6302 E-2 1,62945 E-2
-0,046
1,62664 E-2
-0,218
1,63040 E-2
0,013
1,46-1,0204 E-2 -1,01990 E-2
-0,049
-1,01797 E-2
-0,238
-1,02065 E-2
0,024
1,66 6,3887 E-3 6,39331 E-3
0,072
6,37778 E-3
-0,171
6,39477 E-3
0,095
    1,85-4,0059 E-3 -4,00851 E-3
0,065
```

```
3,99659 E-3
-0,232
-4,01048 E-3
0,114
2,04 2,5114 E-3 2,51292 E-3
0,061
2,50425 E-3
-0,285
2,51465 E-3
0,130
2,23-1,5743 E-3-1,57516 E-3
0,055
-1,56902 E-3
-0,355
-1,57652 E-3
0,141
2,42 9,8676 E-4 9,87206 E-4
0,045
9,82986 E-4
-0,382
9,88220 E-4
0,148
```


### 6.2 Remarks

```
The results are tested on the level of the respective peaks of displacement and speed where values are most significant.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
6.4
Titrate:
SDLD325-Transitory dynamic response of a system mass-arises
Date:
16/02/04
Author (S):
E. BOYERE, T. QUESNEL Key
```

V2.01.325-A Page:
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7
Summary of the results
For two modelings, the results are precise with an error lower than $1 \%$.
Integration on modal basis with a diagram with adaptive step gives the best results for one restricted computing time.

For information, here various times CPU To use used for the resolution of modelings $A$ and B.

CPU To use
DYNA_LINE_TRAN DYNA_TRAN_MODAL DYNA_TRAN_MODAL
(dryness) (NEWMARK) (EULER) (Adaptive)
Modeling A
69,82
0,50 *
0,81 *
Modeling B
57,29
0,42 *
0,50 *
(*): MODE_ITER_SIMULT = 0,23 S: computing time of the modal base to add.
Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLD325-Transitory dynamic response of a system mass-arises
Date:
16/02/04
Author (S):
E. BOYERE, T. QUESNEL Key

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Handbook of Validation
V2.01 booklet: Linear dynamics of the discrete systems
HT-66/04/005/A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.001-C Page:
1/8
Organization (S): EDF/IMA/MMN
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
V2.02.001 document
SDLL01 - Short beam on simple supports

## Summary:

This two-dimensional problem consists in seeking the frequencies of vibration of a mechanical structure
composed of a beam in simple supports at its two ends. This case test of Mechanics of the Structures corresponds to a dynamic analysis of a linear model having a linear behavior. One studies the influence of the position of the points considered as points of supports (points on neutral fibre or points
offset at the base of the beam) compared to neutral fibre of a thick beam.
This test which comprises only one modeling, makes it possible to test part of the functionalities which concern the beams of Timoshenko, the connections rigid and the search for Eigen frequencies by iterations
opposite.
Results obtained, either with the points of supports on neutral fibre, or with the points of offset supports
are compared with results VPCS. In the second configuration, the reference solution is an average results of several software packages.
When the points of supports are offset, one observes a coupling between the various modes of traction and compression and of inflection.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035-Ind A

## Code_Aster ${ }^{\circledR}$

## Version

4.0

## Titrate:

SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.001-C Page:
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1

## Problem of reference

### 1.1 Geometry

$y$
$y$
$y$
With
With
B

B
H
X
Z
$X$
C
D
$X, U$
B
L
$L$
Rectangular cross-section:
height:
$H=0.2 \mathrm{~m}$
width:
$B=0.1 \mathrm{~m}$
surface:
$T o=2.102 \mathrm{~m} 2$
inertia:
$l z=6.667105$
shearing:
$A y=A z=1.17692$
torsion:
$J x=0.45776042104$
Length of the beam
L: 1. m
Co-ordinates of the points (m):
With
B
C
D
$X$
0.
1.
0.
1.
$y$
0.
0.
0.1
0.1
1.2

# Material properties <br> $E=2.1011 \mathrm{~Pa}$ <br> $=0.3$ <br> $=7.800 . \mathrm{kg} / \mathrm{m} 3$ <br> 1.3 <br> Boundary conditions and loadings <br> Problem 1: <br> Not $A$ <br> $U=v=0$. <br> Not B <br> $v=0$. <br> Problem 2: <br> Not C <br> $U=v=0$. <br> Not D <br> $v=0$. <br> 1.4 Conditions <br> initial <br> Without object for the modal analysis. <br> Handbook of Validation <br> V2.02 booklet: Linear dynamics of the beams <br> HI-75/96/035 - Ind A 

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.001-C Page:
3/8
2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLL01/89 of the guide VPCS which presents method of calculation in the following way:
Problem 1: Analytical calculation
The equation of inflection of the nonslim beams gives the formulation of Timoshenko, in
superimposing the effects of the pure bending, deformations of shearing action and the inertia of rotation.
The Eigen frequencies of reference are determined by a digital simulation of this equation, independent of any software package.
The Eigen frequencies in traction and compression are given by:

## E

2 -
I
(I) 1
$F=$
with
=
$I=12$
I
$2 L$
I

## 2

Problem 2:
The problem not having an analytical solution, the solution is established by average of several software package: model of Timoshenko with effect of the deformations of shearing action and inertia of
rotation.
The modes of inflection and traction and compression are coupled.
2.2

Results of reference
Problem 1: the first 6 clean modes.
Problem 2: the first 5 clean modes.
2.3

Uncertainty on the solution
Problem 1: analytical solution. `
Problem 2: $\pm 0.1 \%$
2.4 References
bibliographical
[1]
S.P. TIMOSHENKO, D.H. YOUNG, W. WEAVER. Problems vibrations in Engineering.

New York: Wiley \& Sounds, $4^{\circ}$ edition, p. 415 (1974).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.001-C Page:
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3 Modeling
With
3.1

Characteristics of modeling
One uses the element of right beam of Timoshenko: POU_D_T
y
y
B
B
With
With
X
X
C
D

## Problem 1:

Cutting:
beam AB: 40 meshs SEG2
Limiting conditions:
in all the nodes
DDL_IMPO: (GROUP_NO: AB DZ: 0. , DRX: 0, DRY: 0.)
in a :
(NODE: WITH DX: 0. , DY: 0. )
in b :
(NODE: B DY: 0. )

## Problem 2:

Cutting:
beam AB: 40 meshs SEG2
2 rigid elements AC, BC: 2 meshs SEG2
Limiting conditions:
in all the nodes

DDL_IMPO: (ALL: "YES" DZ: 0. , DRX: 0, DRY: 0.)
out of C:
(NODE: C DX: 0. , DY: 0. )
in D :
(NODE: D DY: 0. )
Names of the nodes:
Not A = N100
Not C $=$ N300
Not B = N200
Not D $=$ N400
3.2

Characteristics of the grid
A number of nodes:
43
A number of meshs and types:
42 SEG2
3.3 Functionalities
tested
Orders

## Keys

AFFE_CARA_ELEM
BEAM
"GENERAL"
ALL
[U4.24.01]
GROUP_MA
"RIGHT-ANGLED"
GROUP_MA
AFFE_CHAR_MECA
DDL_IMPO
ALL
[U4.25.01]
GROUP_NO
NODE
AFFE_MATERIAU
GROUP_MA
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_T"
ALL
[U4.22.01]
GROUP_MA

## DEFI_MATERIAU

ELAS
[U4.23.01]
MODE_ITER_INV
CALC_FREQ
OPTION
"ADJUSTS"
[U5.23.01]
FREQ

### 3.4 Remarks

Definition of the rigid beams AC and data base:

- Section: $\mathrm{Hy}=0.2, \mathrm{~Hz}=0.2$.
- Matériau: E=2.1016, = 0 .

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.001-C Page:
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4
Results of modeling A
4.1 Values
tested
Frequency (Hz)
Clean mode
Reference
Aster
\% difference
Problem 1
inflection 1
431.555
431.8916
0.078

## traction 1

1265.924
1266.0056
0.006
inflection 2
1498.295
1500.7635
0.165
inflection 3
2870.661
2873.5344
0.100
traction 2
3797.773
3799.9692
0.058
inflection 4
4377.837
4370.8206
0.160

## Problem 2

1
$392.8 \pm 2.7 \%$
394.4774
0.427
coupling 2
$922.2 \pm 5.7 \%$
922.6072
0.044
inflection 3
$1592.0 \pm 2.9 \%$
1638.2311
2.903
traction 4
$2629.2 \pm 5.7 \%$
2778.7000
5.686
compression 5
$3126.2 \pm 4.3 \%$
3261.6699
4.333
4.2 Remarks

Calculations carried out by:

```
Problem 1:
MODE_ITER_INV OPTION: LIST_FREQ "ADJUSTS": (430. , 4500. )
Problem 2:
MODE_ITER_INV OPTION: LIST_FREQ "ADJUSTS": (380. , 3300. )
```


## Contents of the file results:

```
Problem 1:
the first 6 Eigen frequencies, clean vectors and modal parameters.
Problem 1:
the first 5 Eigen frequencies, clean vectors and modal parameters.
```


### 4.3 Parameters

## of execution

```
Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 Megawords
Time CPU To use:
9.7 seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
```


## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.001-C Page:
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## 5 Modeling

## B

## 5.1

## Characteristics of modeling

POU_D_TG

## Problem 1:

## Cutting:

beam AB: 40 meshs SEG2
Limiting conditions:
in all the nodes
DDL_IMPO: (GROUP_NO: AB DZ: 0. , DRX: 0, DRY: 0.)
in a:
(NODE: WITH DX: 0. , DY: 0. )
in b :
(NODE: B DY: 0. )

## Problem 2:

Cutting:
beam AB: 40 meshs SEG2
2 rigid elements AC, data base: 2 meshs SEG2
Limiting conditions:
in all the nodes
DDL_IMPO: (ALL: "YES" DZ: 0. , DRX: 0, DRY: 0.)
out of C:
(NODE: C DX: 0. , DY: 0. )
in D :
(NODE: D DY: 0. )
Names of the nodes:
Not A = N100
Not $\mathrm{C}=\mathrm{N} 300$
Not B $=$ N200
Not D $=$ N400

## 5.2

Characteristics of the grid
A number of nodes:
43
A number of meshs and types:
42 SEG2
5.3 Functionalities
tested
Orders
Keys

AFFE_CARA_ELEM<br>BEAM<br>"GENERAL"<br>ALL<br>[U4.24.01]<br>GROUP_MA<br>"RIGHT-ANGLED"<br>GROUP_MA<br>AFFE_CHAR_MECA<br>DDL_IMPO<br>ALL<br>[U4.25.01]<br>GROUP_MA<br>NODE<br>AFFE_MATERIAU<br>GROUP_MA<br>[U4.23.02]<br>AFFE_MODELE<br>"MECHANICAL"<br>"POU_D_TG"<br>ALL<br>[U4.22.01]<br>GROUP_MA<br>DEFI_MATERIAU<br>ELAS<br>[U4.23.01]<br>MODE_ITER_INV<br>"ADJUSTS"<br>[U4.52.01]<br>\subsection*{5.4 Remarks}

Definition of the rigid beams AC and data base:

- Section: $\mathrm{Hy}=0.2, \mathrm{~Hz}=0.2$.
- Matériau: $\mathrm{E}=2.1016,=0$.

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:

## 07/01/98

Author (S):

## B. QUINNEZ

Key:
V2.02.001-C Page:
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6
Results of modeling B
6.1 Values
tested
Frequency (Hz)
Clean mode
Reference
Aster
\% difference
Problem 1
inflection 1
431.555
431.8916
0.078
traction 1
1265.924
1266.0056
0.006
inflection 2
1498.295
1500.7635
0.165
inflection 3
2870.661
2873.5344
0.100
traction 2
3797.773
3799.9692
0.058
inflection 4
4377.837
4370.8206
0.160

## Problem 2

1
$392.8 \pm 2.7 \%$

### 6.2 Remarks

Calculations carried out by:
Problem 1:
ITERATIONS_INVERSES OPTION: LIST_FREQ "ADJUSTS": (430. , 4500. )
Problem 2:
ITERATIONS_INVERSES OPTION: LIST_FREQ "ADJUSTS": (380. , 3300. )

## Contents of the file results:

Problem 1:
the first 6 Eigen frequencies, clean vectors and modal parameters.
Problem 1:
the first 5 Eigen frequencies, clean vectors and modal parameters.

### 6.3 Parameters

of execution
Version: 3.06
Machine: CRAY C90
System:

## UNICOS 8.0

Obstruction memory:
8 Megawords
Time CPU To use:
$1.1875 \mathrm{E}+1$ seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL01 short Beam on simple supports
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.001-C Page:
8/8
7
Summary of the results
The problem without eccentricity is correctly dealt with.
With eccentricity, the problem is dealt with with a dispersion from 3 to $6 \%$ by various software
packages. Aster
remain in this fork.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SDLL02 - Beam hurled, embed-free, folded up on it even
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.02.002-B Page:
1/10
Organization (S): EDF/AMA, DeltaCAD

# Handbook of Validation <br> V2.02 booklet: Linear dynamics of the beams <br> V2.02.002 document 

SDLL02-Beam hurled, embed-free, folded up on it even

## Summary:

This two-dimensional problem consists in seeking the frequencies and the modes of vibration of a structure mechanics, made up of a hurled, embedded beam free and folded up on itself.

The problem arising does not have physical significance. It on the other hand makes it possible to validate the research of
Eigen frequencies of inflection multiples and the research of the modes double in a subspace of order 2.

In this test, one carries out three different modelings:

- in the first modeling, the boundary conditions are imposed using parameters of

Lagrange (order AFFE_CHAR_MECA) and the clean values and vectors are calculated by method of Lanczos (order MODE_ITER_SIMULT, method: "TRI_DIAG"), - in the second modeling, the boundary conditions are imposed by removing degrees of freedom in the matrices of mass and stiffness (order AFFE_CHAR_CINE) and the values and clean vectors are calculated by the method of Bathe and Wilson (order MODE_ITER_SIMULT, method: "JACOBI"),

- in the third modeling, one checks the behavior of modeling COQUE_C_PLAN in
dynamics. The eigenvalues and the clean modes are calculated with the order
MODE_ITER_SIMULT and with the method of SORENSEN.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams


# Code_Aster ${ }^{\circledR}$ 

Version
5.0

## Titrate:

SDLL02 - Beam hurled, embed-free, folded up on it even
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.02.002-B Page:
2/10

1
Problem of reference

### 1.1 Geometry

Rectangular cross-section:

## Height: <br> $H=0.005 \mathrm{~m}$ <br> Width: <br> $B=0.050 \mathrm{~m}$ <br> Surface: <br> $T o=2.5104 \mathrm{~m} 2$ <br> Moment of inertia: <br> $I z=5.2081010 \mathrm{~m} 4$

The co-ordinates (in meters) of the points characteristic of the whole of the beams are:

## WITH B

C
$X 0$.
0.5
0.
$y 0$.
0.0.
1.2

Material properties
The properties of material constituting the beam are:
$E=2.11011 \mathrm{~Pa}$
$=0.3$
$=7.800 . \mathrm{kg} / \mathrm{m} 3$

## 1.3

Boundary conditions and loadings
The boundary condition which characterizes this problem is the embedding of point $A$ and is written:
$U=v=0 .,=0$.

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams

## HT-66/02/001/A

Code_Aster ${ }^{\circledR}$

## Version

5.0

Titrate:
SDLL02-Beam hurled, embed-free, folded up on it even
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
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## 2

Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLL02/89 of the guide VPCS which presents method of calculation in the following way:

By the method of stiffness dynamic, one shows that the folded up beam admits frequencies double, solution of:
cos
() $=0$
$I=2$
(I -1) 2
2

EI
F
I
Z
$I=1$
$I=1,2, \ldots$
2 L2
With

For a rectangular section, one obtains:
$\boldsymbol{E}$
F

This formulation neglects the deformations of shearing action and inertia of rotation (beam of EulerBernoulli).

For the clean modes, the forms are given in guide VPCS. They are normalized to 1 or 1 with not greater amplitude. There are results only for modes 1, 2, 3, 4, 7 and 8. By example, the forms of the first two clean modes are as follows:

```
1
I
0.707
0 . 7 0 7
mode 1
F
mode 2
1=11.76 Hz
```


## 2.2 <br> Results of reference

The results of reference are the first eight Eigen frequencies and displacements of the points B, C for the clean modes 1, 2, 3, 4, 7 and 8. In Code_Aster, the modes are normalized to 1 with not greater amplitude (order NORM_MODE). To be able to make comparisons with the results of reference, the latter were corrected (multiplication by 1 if necessary).
2.3

Uncertainty on the solution
There is no uncertainty on the solution because it is analytical.

### 2.4 References

bibliographical
[1]
PIRANDA J.: Run and Directed Work of Vibrations of the Structures - Mechanical Option École Nationale Supérieure of Mechanics and Micromechanics - Laboratory of Mechanics Applied-Besancon (France (1983).)

## Handbook of Validation

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

$y$
B
With
$X$
C

The beam in 20 meshs SEG2 was cut out (10 for part AB and 10 for part BC).
The modeling used for the beams is that of Euler Bernoulli (POU_D_E).
Two-dimensional solutions are sought. One can thus block for all the nodes it displacement DZ and rotations DRX and DRY.

The end of the beam (not $A$ ) is embedded from where in this point:
$D X=D Y=0 . D R Z=0$.

## 3.2

Characteristics of the grid
The grid contains 21 nodes and 20 meshs of the type SEG2.
The points characteristic of the grid are as follows:
Not $\boldsymbol{A}=\boldsymbol{A}$
Not $\boldsymbol{B}=\boldsymbol{B}$
Not $C=C$

### 3.3 Functionalities <br> tested

The functionalities tested are summarized in this table:
Orders

AFFE_CARA_ELEM<br>BEAM<br>"RIGHT-ANGLED"<br>ALL<br>AFFE_CHAR_MECA<br>DDL_IMPO<br>ALL<br>\section*{NODE}<br>AFFE_MATERIAU<br>ALL<br>AFFE_MODELE "MECHANICAL"<br>"POU_D_E"<br>ALL<br>RECU_CHAMP<br>"NUMERO_ORDRE"<br>"DEPL"<br>DEFI_MATERIAU<br>ELAS

MODE_ITER_SIMULT<br>METHOD<br>"TRI_DIAG"<br>CALC_FREQ<br>OPTION<br>"PLUS_PETITE"<br>NORM_MODE<br>"TRAN"

One insists on orders MODE_ITER_SIMULT (method of Lanczos) for the calculation of the modes
clean and AFFE_CHAR_MECA for the imposition of the boundary conditions.
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## 4

Results of modeling $A$

### 4.1 Values

tested

For the frequencies of vibration of the structure, there are the following results:

Identification Reference Aster \%
difference

Frequency 1

11.76
11.7642
0.04

Frequency 2
11.76
11.7642
0.04

Frequency 3
105.88
105.8811
0.00

Frequency 4
105.88
105.8812
0.00

Frequency 5
294.10
294.1780
0.03

Frequency 6
294.10
294.1806
0.03

Frequency 7
576.44
576.9802
0.09

Frequency 8
576.44
577.0079
0.10

For the modes of vibration of the structure, there are the following results:

Identification Points<br>Size Reference<br>Aster \%<br>difference<br>Frequency 1<br>B

## DY

-0.707
-0.69845
-1.2
C
DY
1.
1.

Frequency 2
B
DY
0.707
0.72615
2.7

C
DY
1.
1.

Frequency 3
B
DY
0.707
0.70711
0.01

C
DY
1.
1.

Frequency 4 B
DY
-0.370
-0.37015
0.04

C
DY
0.523
0.52347
0.09

Frequency 7
B
DY
0.707

## B

DY
-0.388
-0.38847
0.12

C
DY
0.549
0.54937
0.07

### 4.2 Remarks

For the Eigen frequencies, the results obtained are correct. It is the same for the results obtained concerning the clean modes. The tolerance is lower than $2 \%$ for the whole of the modes except for the mode 2 where the tolerance lies between 2 and 3\%.

4.3 Parameters<br>of execution<br>Version: NEW 3.03.09<br>Machine: CRAY C90

## Obstruction memory:

8 MW
Time CPU To use:
5 seconds
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL02 - Beam hurled, embed-free, folded up on it even
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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## 5 Modeling <br> B

## 5.1 <br> Characteristics of modeling

y
B
With
$X$
C

The beam in 20 meshs SEG2 was cut out (10 for part $A B$ and 10 for part $B C$ ).
The modeling used for the beams is that of Euler Bernouilli (POU_D_E).
Two-dimensional solutions are sought. One can thus block for all the nodes it displacement DZ and rotations DRX and DRY.

The end of the beam ( $\operatorname{not} A$ ) is embedded from where in this point:
$D X=D Y=0 . D R Z=0$.

## 5.2 <br> Characteristics of the grid

The grid contains 21 nodes and 20 meshs of the type SEG2.

The points characteristic of the grid are as follows:
Not $A=A$
Not $B=B$
Not $C=C$

### 5.3 Functionalities

tested
The functionalities tested are summarized in this table:

## Orders

## AFFE_CARA_ELEM

BEAM
"RIGHT-ANGLED"
ALL
AFFE_CHAR_CINE
MECA_IMPO
ALL
NODE
AFFE_MATERIAU
$A L L$
AFFE_MODELE
"MECHANICAL"
"POU_D_E"
ALL
RECU_CHAMP
"NUMERO_ORDRE"
"DEPL"
DEFI_MATERIAU
ELAS
MODE_ITER_SIMULT
METHOD
"JACOBI"

## NORM_MODE

"TRAN"

One insists on orders MODE_ITER_SIMULT (method of Bathe and Wilson) for the calculation of clean modes and AFFE_CHAR_CINE for the imposition of the boundary conditions.
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
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## Code_Aster ${ }^{\circledR}$

Version
5.0

## Titrate:

SDLL02 - Beam hurled, embed-free, folded up on it even
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.02.002-B Page:
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## 6 <br> Results of modeling B

### 6.1 Values <br> tested

For the frequencies of vibration of the structure, there are the following results:

## Identification Reference Aster \% difference

Frequency 1
11.76
11.7642
0.04

Frequency 2
11.76
11.7642
0.04

Frequency 3
105.88
105.8811
0.00

Frequency 4
105.88
105.8812
0.00

Frequency 5
294.10
294.1780
0.03

Frequency 6
294.10
294.1806
0.03

Frequency 7
576.44
576.9802
0.09

Frequency 8
576.44
577.0079
0.1

For the modes of vibration of the structure, there are the following results:

## Identification Points <br> Size Reference <br> Aster \% <br> difference <br> Frequency 1 <br> B <br> DY <br> -0.707 <br> -0.69658

C
DY
1.
1.

Frequency 2
B
DY
0.707
0.73038
3.31

C
DY
1.
1.

Frequency 3
B
DY
0.707
0.70711
0.02

C
DY
1.
1.

Frequency 4 B
DY
-0.370
$-0.37014$
0.04

C
DY
0.523
0.52347
0.09

Frequency 7
B
DY
0.707
0.70711
0.02

C

### 6.2 Remarks

For the Eigen frequencies, the results obtained are correct. It is the same for the results obtained concerning the clean modes. The tolerance is lower than $2 \%$ for the whole of the modes except for the mode 2 where the tolerance lies between 3 and $4 \%$.

### 6.3 Parameters of execution

Version: NEW 3.03.09
Machine: CRAY C90

Obstruction memory:
$8 M W$
Time CPU To use:
5 seconds
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SDLL02 - Beam hurled, embed-free, folded up on it even

## 7 Modeling

C

## 7.1

Characteristics of modeling

The beam in 20 meshs SEG3 was cut out (10 for part $A B$ and 10 for part $B C$ ).
Modeling used is COQUE_C_PLAN.
The end of the beam $(\operatorname{not} A)$ is embedded from where in this point:
$D X=D Y=D R Z=0$.

## 7.2

Characteristics of the grid
The grid contains 41 nodes and 20 meshs of the type SEG3.
The points characteristic of the grid are as follows:
Not $A=A$
Not $B=B$
Not $C=C$

### 7.3 Functionalities

## tested

The functionalities tested are summarized in this table:

Orders

## AFFE_CARA_ELEM

HULL
THICK
AFFE_MODELE
"MODELING"
"COQUE_C_PLAN"
MODE_ITER_SIMULT
METHOD
"SORENSEN"
CALC_FREQ
OPTION
"PLUS_PETITE"
Handbook of Validation
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## Code_Aster ${ }^{\circledR}$

Version
5.0

Titrate:
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Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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## 8 <br> Results of modeling $C$

### 8.1 Values

tested

For the frequencies of vibration of the structure, there are the following results:

## Identification Reference Aster \% difference <br> Frequency 1 <br> 11.76 <br> 11.767 <br> 0.063

Frequency 2
11.76
11.804
0.377

Frequency 3
105.88
106.533
0.616

Frequency 4
105.88
107.556
1.583

Frequency 5
294.10
299.685
1.899

Frequency 6
294.10
304.643
3.585

Frequency 7
576.44
599.025
3.918

Frequency 8
576.44
613.506
6.430

For the modes of vibration of the structure, there are the following results:

Identification Points

## Size Reference

Aster \%
difference
Frequency 1
B
DY
-0.707
-0.707
0.015

C
DY
1.
1.
0.

Frequency 2
B
DY
0.707
0.707
0.015

C
DY
1.
1.
0.

Frequency 3
B
DY
0.707
0.707
0.015

C
DY
1.
1.
0.

Frequency 4
B
DY
-0.370
-0.373
0.713

C

DY
0.523
0.527
0.763

Frequency 7
B
DY
0.707
0.707
0.015

C
DY
1.
1.
0.

Frequency 8
B
DY
-0.388
-0.403
3.990

C
DY
0.549
0.571
3.936

### 8.2 Remarks

In this case-test, where the results are independent of the Young modulus, it is not necessary of to modify the Young modulus retained for modeling, as in the case of the static analysis linear, to take account of the real width of the beam.

### 8.3 Parameters <br> of execution

Version: NEW 5.04.17
Machine: SGI-Origin2000 R12000

Obstruction memory: 16 megabytes
Time CPU To use:
2.49 seconds

Handbook of Validation

# V2.02 booklet: Linear dynamics of the beams 

HT-66/02/001/A

## Code_Aster ${ }^{\circledR}$

Version
5.0

Titrate:
SDLL02 - Beam hurled, embed-free, folded up on it even
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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## 9 <br> Summary of the results

- Modélisations A and B of the Beam type:

The problem is dealt with with very good precision on the first eight frequencies (tolerance < $0.1 \%$ ) for two modelings tested. The components of modes 3, 4, 7 and 8 are also obtained with a good precision of about $0.1 \%$. The precision on mode 1 is of the order of $1 \%$ for the method of Lanczos and $0.5 \%$ for the method of Bathe and Wilson. In it who relates to mode 2, the precision degrades himself: it is about $2.7 \%$ for the method of Lanczos and about 3.3\% for the method of Bathe and Wilson. Complementary tests (use of the method of Lanczos by imposing boundary conditions by the order AFFE_CHAR_CINE) make it possible to think that these differences come from the method of seek eigenvalues used.

## - Modélisation COQUE_C_PLAN

The precision on the results is good for the first three frequencies, the error is of the order from 0.6\%. It is degraded as the frequency increases, the error passes from 4th frequency with 8 th of $1.5 \%$ to $6.4 \%$. More the frequency is high plus the difference between the frequencies
double is important. The error on the modes is satisfactory for the first 7 modes ( $<0.7 \%$ ), it is higher for 8 th ( $<4 \%$ ). A finer grid should allow best to represent the modal deformations associated with the high frequencies.

## Handbook of Validation

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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL04 Beam hurled on two supports
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.004-C Page:
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Organization (S): EFD/IMA/MMN

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams

## V2.02.004 document

## SDLL04-Beam hurled on two supports,

 coupled to a system mass-arises
## Summary:

This plane problem consists in seeking the frequencies of vibration of a mechanical structure made up of one
beam embed-slide and of a mass connected to the beam by a spring. Stiffness of the spring and mass depend on a variable parameter, which will make it possible to highlight the displacement of the frequencies
clean for a small disturbance of the model. This test of Mechanics of the Structures corresponds to an analysis
dynamics of a linear model having a linear behavior. It includes/understands only one modeling.
This problem makes it possible to test the element of beam of Timoshenko in inflection, the calculation of the Eigen frequencies
by the method of the iterations opposite and the method of Lanczos, the discrete elastic connection
between one
specific mass and a node of a beam.
The results obtained are in concord with the results given in guide VPCS. It well is observed unfolding of the Eigen frequencies induced by the disturbance of the initial model (beam hurled on two supports).
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$V 2.02$ booklet: Linear dynamics of the beams
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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:

## SDLL04 Beam hurled on two supports

Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
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1
Problem of reference
1.1 Geometry

C
$y, v$
With

## D

B
$X, U$
L/4
$L$
Length: $L=10$
has
( $=A D B=d B$ )
$m=$
E
WITH L $=780 \mathrm{~kg}$
$K=4$
=
E
me
7804 Nm
Cross-section:
surface
$A=1.102 \mathrm{~m} 2$
moment of inertia
$I z=3.9106 \mathrm{~m} 4$
3 cases studied:
$=0$.
$=0.001$
$=0.01$
Co-ordinates of the points (meters):
With
B
C
D
X
0.
10.
2.5
2.5
$y$
0 .
0.
qcq_0
0.
1.2
Material properties
$E=2.1011 P a$
$=7.800 . \mathrm{kg} / \mathrm{m} 3$
1.3
Boundary conditions and loadings

Not a:
$U=v=0$.
Not b:
$v=0$.
Not C:
$U=0 .=0$. vertical slide
1.4 Conditions
initial
Without object for the modal analysis.
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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL04 Beam hurled on two supports
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
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## 2

## Reference solution

2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLL04/89 of the guide VPCS which presents method of calculation in the following way:
The equation with the own pulsations of the complete system is written:

```
= m
With
E
R4=2
= ke
+B has =L
WITH L
I
I E I
C
me
In absence of system secondary, K
=
E,me
0, one find well the Eigen frequencies of the beam
hurled on two supports.
l
I.E.(internal excitation)
F=i2
= i2
I
2 L2
With
2
When the secondary system is granted exactly on the first mode of this beam, them
new Eigen frequencies of the system can be obtained by the approximate formulas:
```

$m$
*
$E$
F
$=1 \pm 05$
F
*
$= \pm$
12
1
(1 05

## 2.2

## Results of reference

The first two Eigen frequencies for $=0$.
The first three Eigen frequencies for $=0.001$ and $=0.01$.

## 2.3

## Uncertainty on the solution

< $4 \%$ for the first modes if the system is granted to the first mode.

### 2.4 References <br> bibliographical

[1]
NOUR-OMID, SACKMAN, KIUREGHIAN. Modal characterisation of equipment continous structure system. Newspaper of Sound and Vibration, V. $88 n^{\circ} 4$, p. 459, 472 (1983).
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B. QUINNEZ

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3 Modeling
With
3.1

Characteristics of modeling

One uses right beams of Timoshenko POU_D_T and discrete elements DIS_T.

AD: 5 meshs SEG2
DB: 15 meshs SEG2
CD: 1 mesh SEG2
Modeling:
$P O U \_D \_T$ for all the meshs of beam $A B$
DIS_T for the mesh CD and the point $C$
For all the structure $D Z=D R X=D R Y=0$
Limiting conditions:
in all the nodes of
beam $A B$ :
DDL_IMPO: (GROUP_NO: NPOUTRE DZ: 0. , DRX: 0, DRY: 0.)
with the nodes
ends:
(GROUP_NO: WITH DX: 0. , DY: 0. ) (GROUP_NO: B DY: 0. )
out of $C$ :
(GROUP_NO: C DX: 0. , DZ: 0. )
Names of the nodes:
Not $A=N 1$
Not $C=N 22$
Not $B=N 21$
Not $D=N 6$
3.2

Characteristics of the grid
A number of nodes:
22
A number of meshs and types:
21 meshs SEG2
1 mesh POII

### 3.3 Functionalities

tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"GENERAL"

## ALL

[U4.24.01]
GROUP_MA
DISCRETE
"K_T_D_L"
" $M_{-} T_{-} D_{-} N$ "
AFFE_CHAR_MECA
DDL_IMPO
ALL
[U4.25.01]
GROUP_NO
AFFE_MATERIAU
GROUP_MA
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_T"
GROUP_MA
[U4.22.01]
"DIS_T"
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
MODE_ITER_INV
CALC_FREQ
OPTION
"NEAR"
[U5.52.01]
FREQ
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Titrate:
SDLL04 Beam hurled on two supports
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
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4
Results of modeling A
4.1 Values
tested
Frequency (Hz)

## Order of the clean mode

Reference
Aster
\% difference
0 .
inflection 1
1.5707
1.5707

0 .
inflection 2
6.2831
6.2812
0.03
0.001

1 inflection
1.5460
1.5460
0.

2 inflection
1.5958
1.5957
0.

3 inflection 2
6.2336
6.2817
0.77
0.01

1 inflection
1.4937
1.4936
0.

2 inflection
1.6506
1.6506
0.

3 inflection 2
6.2874
6.2854
0.03

### 4.2 Remarks

For
$=0$, one carried out:
MODE_ITER_SIMULT METHOD: "TRI_DIAG"
OPTION: "PLUS_PETITE"
NMAX_FREQ: 2
For $=0.001$, one carried out:
MODE_ITER_INV
OPTION: "NEAR"
LIST_FREQ: (1.5, 1.6, 6.5)
For $=0.01$, one carried out:
MODE_ITER_INV
OPTION: "ADJUSTS"
LIST_FREQ: (1. , 7.)
Contents of the file results:
Case 1: the first 2 Eigen frequencies, clean vectors and modal parameters.
Case 2: the first 3 Eigen frequencies and modal parameters.
Case 3: the first 3 Eigen frequencies, clean vectors and modal parameters.

### 4.3 Parameters

of execution
Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
8 seconds
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HI-75/96/035 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL04 Beam hurled on two supports
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
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## 5

Summary of the results
The unfolding of the Eigen frequencies induced by the disturbance of the initial model is perfectly represented.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

Transitory SDLL06 Response of a embed-free post
Date: 14/09/01
Author (S): Fe WAECKEL
Key: V2.02.006-C Page: 1/8
Organization (S): EDF/RNE/AMV

Handbook of Validation<br>V2.02 booklet: Linear dynamics of the beams<br>V2.02.006 document

SDLL06-Transitory response of a post embed-free

## Summary

In this case test, one analyzes the transitory response of a not deadened embed-free beam, modelled by one system masses - arises and subjected to an unspecified dynamic loading.

One tests the discrete element in inflection, the calculation of the clean modes by the method of Lanczos and calculation of transitory response by modal recombination of the subjected structure is with a accélérogramme (modeling
With) is with an equivalent imposed force (modeling B).
The diagram of Euler is used.
The results obtained are in concord with the results of reference (analytical results).
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1

## Problem of reference

### 1.1 Geometry

It is a problem suggested initially in the reference [bib1] and contained in [bib2].

## xr (T)

## Co-ordinates

Fx (T)
B
m
points (m)
0
0
To 0
B 10
Iz
0
0
I
$y$
$y$
$X$
With
$X$
(T)
(T)

- beam AB: beam hurled without mass length $A B, L=10 m$ and of moment of inertia $I Z=0,3285 \mathrm{~m} 4$.


## 3

specific mass in b: $m=43,810 \mathrm{~kg}$
1.2

Properties of materials
Young modulus:
10
$E=4.10 P a$
Density:
$=0 \mathrm{~kg} / \mathrm{m} 3$

## 1.3

Boundary conditions and loadings

Boundary conditions:
Only authorized displacements are the translations according to axis $X$.
Point $A$ is embedded: $d x=D y=d z=d r x=d r y=d r z=0$.
Loadings:

- modeling a: transverse acceleration at point a: (T)

Time (S)
00.0250 .05
(T)
(m/s2)
Acceleration according to $X$ (ms2)
09.810
$P 0=9,81$
$T(S)$
0
$t 0=0.0250 .05$

- modeling b: forces transverse at point b: $F x(T)$ with $F x(T)=M r .(T)$


### 1.4 Conditions

initial

The system is at rest: with $T=0, d x(0)=0, d x / d t(0)=0$ in any point.
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2
Reference solution
2.1

Method of calculation used for the reference solution
The problem is dealt with by model with degree of freedom. The post is regarded as one slim not deadened and nonheavy beam of rigidity $k=3 E I Z / l 3=3,942.107 \mathrm{~N} / \mathrm{m}$. the superstructure 3
located at the top of the post by a specific mass $m=43,810 \mathrm{~kg}$ are modelled.
The two loading cases lead to the calculation of the response of a system to a degree of subjected freedom with an acceleration (T) of an unspecified form:

## K

3E.I
\&
$X+2 X=-(T$
$Z$
R
R
) with $=$
=
the Eigen frequency of the system and $X$
m
m. 13

R it
relative displacement of the point $B$ compared to point $A$. the solution is obtained by integration of the integral of Duhamel [bib3]:
m
$X(T)=-$
$(T) \sin (T-) D$
R

0
2.2

Results of reference
Displacement relating to the point B.
For a triangular imposed acceleration, one can calculate the integral of Duhamel analytically [bib3]:

P0
$\sin T$
$T$
$<T$
: X
0
=
$T$
R

0
$\boldsymbol{P}$
$2 \sin$
0
$(T-t 0) \sin T$
$T$
$0<T<2 t: X$
0
= -
2t0-T

2
$T$
$\boldsymbol{P}$
$T$
$>2 t$
: X
0
0
$\boldsymbol{R}=-$
$2 \sin$
$\sin$
2
$\sin$
3
[
( $T-t 0$ ) -
( $\boldsymbol{T}-\boldsymbol{t} \mathbf{0}$ ) -
$T]$
$T$
0

## 2.3

Uncertainty on the solution

No if one calculates the integral of Duhamel analytically [bib3]. About the precision of method of integration numerical employed to calculate the integral of Duhamel ([bib1], [bib2]): method of Simpson with 40 points per period.

### 2.4 References <br> bibliographical

[1]
R.W. Clough and J. Penzien: Dynamics of New York structures, Mac Graw-Hill, 1975, p. 102-105
[2]
Guide Technical VPCS AFNOR - 1990
[3]
J.S. Przemieniecki: Theory of matrix structural analysis New York, Mac Graw-Hill, 1968,

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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

The elements are modelled by discrete elements with 6 degrees of freedom "DIS_TR".
$y$
$y$
NO2
m
K
NO1
X
(T)

Node NO1 is subjected to an imposed acceleration (T). One calculates the relative displacement of the node
NO2 compared to the displacement of node NO1 and one compares it with calculated displacement analytically.
Temporal integration is carried out with the algorithm of Euler (not of time: 5. 104 S).

## 3.2

Characteristics of the grid
The grid consists of 2 nodes and a discrete element (DIS_TR).

3.3 Functionalities<br>tested<br>Orders<br>AFFE_MODELE<br>GROUP_MA<br>"MECHANICAL"<br>"DIS_TR"<br>AFFE_CARA_ELEM<br>DISCRETE<br>NODE<br>M_TR_D_N<br>NET<br>K_TR_D_L<br>AFFE_CHAR_MECA<br>DDL_IMPO<br>MODE_ITER_INV<br>CALC_FREQ<br>NEAR<br>CALC_CHAR_SEISME<br>MONO_APPUI<br>MACRO_PROJ_BASE<br>DYNA_TRAN_MODAL<br>METHOD<br>EULER<br>REST_BASE_PHYS<br>RECU_FONCTION<br>RESU_GENE<br>FORMULATE<br>CALC_FONC_INTERP

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```
4
Results of modeling A
```


### 4.1 Values

tested
Relative displacement of node NO1 (in meters).
Time (S)
Analytical calculation
Code_Aster Error
(\%)
0,010 6,511E05
6,495E05
0
0,015 2,185E04
2,183E04
0
0,020 5,139E04
5,136E04
-0,058
0,024 8,809E04
8,806E04
-0,039
0,026 1,115E03
1,115E03
-0,041
0,030 1,679E03
1,679E03
-0,014
0,035 2,523E03
2,523E03
-0,004
0,040 3,457E03

3,457E03<br>0<br>0,045 4,412E03<br>4,412E03<br>0,004<br>0,049 5,143E03<br>5,143E03<br>0,005<br>0,051 5,485E03<br>5,485E03<br>0,005<br>0,055 6,109E03<br>6,109E03<br>0,005<br>0,060 6,765E03<br>6,765E03<br>0,005<br>0,065 7,269E03<br>7,269E03<br>0,005<br>0,070 7,610E03<br>7,610E03<br>0,005<br>0,075 7,779E03<br>7,780E03<br>0,005<br>0,080 7,774E03<br>7,775E03<br>0,004<br>0,085 7,595E03<br>7,595E03<br>0,004<br>4.2 Parameters of execution<br>Version:<br>STA 5.02

Machine:<br>SGI ORIGIN 2000

# Time CPU To use: 

3,16 seconds

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## 5 Modeling

B

## 5.1

Characteristics of modeling
The elements are modelled by discrete elements with 6 degrees of freedom "DIS_TR".

## $y$ <br> $y$ <br> Fx (T) <br> Fx (T) <br> m <br> NO2 <br> K <br> NO1 <br> $X$ <br> $X$

Node NO2 is subjected to an imposed force $\mathrm{Fx}(\mathrm{T})$. One calculates the relative displacement of node NO2 compared to the displacement of node NO1 and one compares it with the displacement calculated in

### 5.2 Functionalities

tested

Orders

AFFE_MODELE<br>GROUP_MA<br>"MECHANICAL"<br>"DIS_TR"<br>AFFE_CARA_ELEM<br>DISCRETE<br>NODE<br>$M_{-} T R_{-} D \_N$

NET<br>K_TR_D_L<br>AFFE_CHAR_MECA<br>DDL_IMPO<br>FORCE_NODALE<br>MODE_ITER_INV<br>CALC_FREQ<br>NEAR<br>CALC_CHAR_SEISME<br>MONO_APPUI<br>MACRO_PROJ_BASE<br>DYNA_TRAN_MODAL<br>METHOD<br>EULER<br>REST_BASE_PHYS<br>RECU_FONCTION<br>RESULT

5.3

Characteristics of the grid
It is the same grid as for modeling $A$.
Handbook of Validation

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6
Results of modeling $B$

### 6.1 Values

tested

Relative displacement of node NO1 (in meters).
Time (S)
References
Code_Aster Error
(\%)
[bib1], [bib2]
0,01
6,500E05 6,447E05 0,82
0,02
5,130E04 5,127E04 0,064
0,03
1,679E03 1,678E03 0,037
0,04 3,457E03
3,457E03
0,013
0,05 5,316E03
5,317E03
0,022
0,06 6,764E03
6,766E03
0,035
0,07 7,609E03
7,611E03
0,027
0,08 7,774E03
7,776E03
0,024
0,09 7,244E03
7,246E03
0,028
0,1 6,068E03
6,069E03
0,014
0,12
2,242E03 2,242E03 0,0170,14
2,367E03 2,369E03 0,071
0,16
6,149E03 6,152E03 0,041
0,18
7,783E03 7,785E03 0,029
0,2
6,698E03 6,699E03 0,018
6.2 Parametersof execution
Version:
STA 5.02
Machine:
SGI ORIGIN 2000
Time CPU To use:
1,9 seconds
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## 7

Summary of the results and remarks general
The simplified model presented in this case test makes it possible to validate the method of numerical resolution.
To deal with the real physical problem, it would be necessary to take into account the effects of inertia (mass of
post, effect of inertia of rotation around $B$ of the superstructure) and of compression of the post (actual weight).

For modeling A, the error made with a step of time of 5. 104 S is about 0,01\%;
for modeling $B$ (not of times of 103 S) it is about 0,6\%.
One will be able to supplement this case test by checking the convergence of the results for other step values
time and by comparing the results obtained with other diagrams of integration.
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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL08 Fits latticework on plane beams (metal sections)
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.008-C Page:
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Organization (S): EDF/IMA/MMN
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
V2.02.008 document

SDLL08 - Fit latticework on plane beams
(metal sections)
Summary:
This three-dimensional problem first of all consists in carrying out a modal analysis and then to study harmonic response of a mechanical structure of a plane netting of beams. This test of Mechanics of Structures corresponds to a dynamic analysis of a linear model having a linear behavior. It only one modeling includes/understands.
This problem thus makes it possible to test the element of beam of Euler Bernouilli in transverse inflection, the calculation of
frequencies and of the modes of vibration by the method of Lanczos and the use of linear relations enters
displacements of two points in modal analysis and harmonic answer.
The results are in agreement with the analytical results of guide VPCS.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

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Author (S):

## B. QUINNEZ

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1
Problem of reference
1.1 Geometry

Z, W
F
C
FG
y, v
L1
B = H
G
$\mathrm{E}=\mathrm{I}$
X, U
With
D
L2
Length: L1 = L2 = 5 m
Cross-section (section out of I): IPE 200
surface
To $=2.872103 \mathrm{~m} 2$
moment of inertia
$\mathrm{Iz}=1.943105 \mathrm{~m} 4$
(other parameters of beam not used)
Co-ordinates of the points (in meters):
With
B $=\mathrm{H}$
C
D
E = I
F
G
X2.5
2.5
2.52.5
0.
0.
0.
0.
0 .
0.

## 1.2

Material properties
$E=2.1011 \mathrm{~Pa}$
$=7.800 . \mathrm{kg} / \mathrm{m} 3$
1.3
Boundary conditions and loadings
Points A, C, D, F: $(U=v=W=0$. $)$
Points B, E: rotulée connection (continuity of U, v, W)
Sinusoidal force at the point G

## F

() $=F$
G
$0 \sin T$
$F=$
0
1105 NR
$=80 \mathrm{rad} \mathrm{S}$
1.4 Conditions

## initial

With $T=0$, structure at rest.
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Date:
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Author (S):

## B. QUINNEZ

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2
Reference solution

## 2.1

## Method of calculation used for the reference solution

The reference solution is that given in card SDLL08/89 of the guide VPCS which presents method of calculation in the following way:
A method of Rayleigh-Ritz makes it possible to make calculation with two degrees of freedom from assumptions of following symmetrical deformations:

- for the point of X-coordinate there of the members AC and DF L1 length
$y+L 1$
2
W
$=W$
$A B$
$B \sin$
L1
- for the point of X-coordinate X of the cross-piece BE L2 length
$X+L 2$


## Results of reference

The first two Eigen frequencies and symmetrical clean modes (other frequencies clean of this system are not studied). For the clean modes, one with the value following: $W B / W G$
In harmonic answer one a:
$W B$ max and $W G$ max,
$G$ max at the point G .
2.3

## Uncertainty on the solution

Analytical solution.

### 2.4 References

bibliographical
[1]
J.M. BIGGS. Introduction to Structural Dynamics. New York: Mc Graw Hill, p. 184 (1964).

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Author (S):

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3 Modeling
With

## 3.1

Characteristics of modeling
One uses the element of beam of Euler Bernouilli POU_D_E
y
C
F
B H
G
I
E
X
With
D
3 beams:
ABC, DEF, cut out HGI each one in 10 meshs SEG2
The nodes (B, H) and (E, I) have the same co-ordinates.
Limiting conditions:
beams ABC and DEF
DDL_IMPO:
(GROUP_NO: (PABC, PDEF)
DX: 0. , DY: 0. , DRY: 0. )
beam HGI
(GROUP_NO: (PHGI)
DX: 0. , DY: 0. , DRX: 0. )
nodes ends
(GROUP_NO: (NACDF)
DZ: 0. )
Liaison_ddl:
DZB DZH $=0$. and DZE DZI $=0$.
Force_nodale:
Node: G Fz: 1.E5
Names of the nodes:
With = N1
B = N6
$\mathrm{C}=\mathrm{N} 11$
$\mathrm{D}=\mathrm{N} 21$
$\mathrm{E}=\mathrm{N} 26$
$\mathrm{F}=\mathrm{N} 31$
$\mathrm{H}=\mathrm{N} 41$
$\mathrm{G}=\mathrm{N} 46$
I = N51
3.2

Characteristics of the grid
A number of nodes:
33
A number of meshs and types:
$3^{*} 10=30$ SEG2
3.3 Functionalities
tested
Orders

## Keys

AFFE_CARA_ELEM
BEAM
"GENERAL"
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
LIAISON_DDL
FORCE_NODALE
NODE
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"

'POU_D_E<br>ALL<br>[U4.22.01]<br>DEFI_MATERIAU<br>ELAS<br>[U4.23.01]<br>MODE_ITER_SIMULT<br>METHOD<br>"TRI_DIAG"<br>[U4.52.02]<br>CALC_FREQ<br>OPTION<br>"PLUS_PETITE"<br>COMB_MATR_ASSE<br>[U4.53.01]

### 3.4 Remarks

The blocking of ddl DX and DY in all the nodes makes it possible to select only the modes of inflection transverse (in the "vertical" plane).
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4
Results of modeling A
4.1 Values
tested
Frequency (Hz)
Order of the clean mode

## Reference

Aster
\% difference

# Clean mode: value of WB/WG 

Order of the clean mode

## Reference

Aster*
\% difference
symmetrical
1
1.213
1.213
0.

2
0.412
0.412
0.
*
$\mathrm{WB}=\mathrm{DZ}$ out of B (N6)
WG + WB = DZ in G (N46)
mode 1:
WB $=0.5480$
$\mathrm{WG}+\mathrm{WB}=1$.
mode 2:
WB $=0.6698$
$\mathrm{WG}+\mathrm{WB}=0.9559$

## Harmonic answer:

Not
Type of value
Reference
Aster
\% difference
(m)

B, E
WB max
0.098
0.1003
2.45

### 4.2 Remarks

Calculations carried out by:
MODE_ITER_SIMULT METHOD: "TRI_DIAG"
OPTION: "PLUS_PETITE"
NMAX_FREQ: 3
One obtains an antisymmetric mode for a frequency $\mathrm{F}=22.5676 \mathrm{~Hz}$. This Eigen frequency depends on the constant of provided torsion; this one is not defined in the bench-mark data.
Values WB/WG are not checked in the test but are obtained manually starting from WB and WG + WB.
Value (WG) max is not checked in the test. One has only access to WB max and (WB + WG) max. WG max is obtained manually by difference.

## Contents of the file results:

the first 3 Eigen frequencies, displacement of the nodes B, E, G in harmonic answer.

### 4.3 Parameters <br> of execution

Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5 seconds
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## Summary of the results

The values of the Eigen frequencies and the clean vectors are obtained with a precision $<0.3 \%$.
The variation of $2.5 \%$ on the maximum arrows at the points B and E would deserve to check the solution of
reference, to supplement the validation of the harmonic answer.
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HI-75/96/035 - Ind A

## Code_Aster ®

Version
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Titrate:
SDLL09 Vibration of a slim beam of rectangular section
Date:
07/01/98
Author (S):

## B. QUINNEZ, A. PENET

Key:
V2.02.009-A Page:
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Organization (S): EDF/IMA/MMN

## Handbook of Validation

## V2.02 booklet: Linear dynamics of the beams

## V2.02.009 document

## SDLL09 - Vibration of a slim beam <br> of variable rectangular section (embed-free)

Summary:
This plane problem consists in seeking the frequencies of vibration of a free fixed beam with section rectangular variable. This test comprises only one modeling.
The variation of section of the beam is either homothetic, or nonhomothetic. Characteristics of section of the beam are given according to meshs' in two different ways:

- section and inertias,
- height and width.

This problem thus makes it possible to thus test the element of beam with variable section for a prismatic structure
that the calculation of the frequencies of vibration by iterations opposite. In addition, in the operator AFFE_CARA_ELEM [U4.24.01], one tests the remanence of certain key words.
The results obtained are in concord with those given in guide VPCS.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
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Code_Aster ${ }^{\circledR}$
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Titrate:
SDLL09 Vibration of a slim beam of rectangular section
Date:
07/01/98
Author (S):
B. QUINNEZ, A. PENET

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1
Problem of reference

### 1.1 Geometry

y, v
L
y
y
bo
b1
B
H
O
h1
H
With
1
X, U
Z
Z
O
B

## Z, W

1 bo
Rectangular sections
Length of the beam:
$\mathrm{L}=1 \mathrm{~m}$
Rectangular section:

## Initial cross-section

Final cross-section
Case 1
Case 2
height:
$\mathrm{Ho}=0.04 \mathrm{~m}$
$=0.04 \mathrm{~m}$
$\mathrm{h} 1=0.01 \mathrm{~m}$
width:
bo $=0.04 \mathrm{~m}$
$=0.05 \mathrm{~m}$
$\mathrm{b} 1=0.01 \mathrm{~m}$
surface:
Ao $=1.6103 \mathrm{~m} 2$
$=2.103 \mathrm{~m} 2$
$\mathrm{A} 1=1.104 \mathrm{~m} 2$
inertia:
lzo $=2.1333107 \mathrm{~m} 4$
$=2.6667107 \mathrm{~m} 4$
Iz1 $=8.33331010 \mathrm{~m} 4$
Co-ordinates of the points (m):
With
B
X
0.
1.
0.
0.

## 1.3

## Boundary conditions and loadings

Not a: embedded $\mathrm{U}=\mathrm{v}=0=0$.
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## Code_Aster ®

Version
4.0

Titrate:
SDLL09 Vibration of a slim beam of rectangular section
Date:
07/01/98
Author (S):

## B. QUINNEZ, A. PENET

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2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLL09/89 of the guide VPCS which presents method of calculation in the following way:
Exact calculation by numerical integration of the differential equation of the inflection of the beams (Theory
of Euler-Bernouilli).

## With

x2
$t 2$
where $I z$ and $A$ vary with the X -coordinate.
One obtains:
E
F
=
() hl

I
,
$2 I$
L2
12
with:
$=h 0=4$
hl
$=b 0=4$ or 5
bl

1
2
3
4
5
$=4$
23.289
73.9
165.23
299.7
478.1
$=5$
24.308
75.56
167.21
301.9
480.4
2.2

Results of reference
the first 5 clean modes of inflection.
2.3

## Uncertainty on the solution

Semi-analytical solution.

### 2.4 References

## bibliographical

H.H. MABIE, C.B. ROGERS, Transverse vibrations of double-tapered cantilever beams - Newspaper of the
Acoustical Society of America, ${ }^{\circ}$ 51, p. 1771-1774 (1972).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL09 Vibration of a slim beam of rectangular section
Date:
07/01/98
Author (S):

## B. QUINNEZ, A. PENET

Key:
V2.02.009-A Page:
4/6
3 Modeling
With

## 3.1

Characteristics of modeling
Modeling: Elements of beam POU_D_E
y
B
With
X
Cutting:
beam AB: 30 meshs SEG2 of section variable
15 meshs in "General section"
15 meshs in "Rectangular section"
Limiting conditions:
in all the nodes
DDL_IMPO: (ALL: "YES" DZ: 0. , DRX: 0. , DRY: 0. ) at end A
(Node: WITH DX: 0. , DY: 0. , DRZ: 0. )

```
Names of the nodes:
Not A = N100
Not B = N200
```


## 3.2

```
Characteristics of the grid
Grid:
A number of nodes: 31
A number of meshs and types: 30 SEG2
```


### 3.3 Functionalities

## tested

```
Orders
```


## Keys

```
AFFE_CARA_ELEM
BEAM
"GENERAL"
NET
[U4.24.01]
AFFE_CARA_ELEM
BEAM
"RIGHT-ANGLED"
NET
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
AFFE_MODELE
"MECHANICAL"
"POU_D_E"
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_INV
OPTION
"ADJUSTS"
[U4.52.01]
Test of the remanence of a key word (SECTION and CARA).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
```

Code_Aster ${ }^{\circledR}$
Version

## 4.0

Titrate:
SDLL09 Vibration of a slim beam of rectangular section
Date:
07/01/98
Author (S):
B. QUINNEZ, A. PENET

Key:
V2.02.009-A Page:
5/6
4
Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference

## frequency in HZ

Cases $1 \mathrm{ho} / \mathrm{h} 1=4 \mathrm{bo} / \mathrm{b} 1=4$
homothetic
inflection 1
54.18
54.1354
0.08
inflection 2
171.94
171.7122
0.13
inflection 3
384.40
383.8764
0.14
inflection 4
697.24
696.1877
0.15
inflection 5
1112.28
1110.4727
0.16

Cases $2 \mathrm{ho} / \mathrm{h} 1=4 \mathrm{bo} / \mathrm{b} 1=5$
nonhomothetic

## inflection 1

56.55
56.4984
0.09
inflection 2
175.19
175.5306
+0.19
inflection 3
389.01
388.3426
0.17
inflection 4
702.36
700.9879
0.20
inflection 5
1117.63
1115.4275
0.20

### 4.2 Remarks

Calculations by
MODE_ITER_INV (...
CALC_FREQ: (FREQ: (53. , 1150.))) ;
4.3 Parameters
of execution
Version: NEW3.03.15
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
7.72 seconds

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL09 Vibration of a slim beam of rectangular section
Date:
07/01/98
Author (S):

## B. QUINNEZ, A. PENET

Key:
V2.02.009-A Page:
6/6
5
Summary of the results
Good establishment of the element of non-prismatic beam with a fine grid.
A coarser modeling would be sufficient.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SDLL10 Beam of rectangular section variable
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.010-C Page:
1/6
Organization (S): EDF/IMA/MMN

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
V2.02.010 document
SDLL10 - Rectangular beam of section
variable (embed-embedded)

## Summary:

This plane problem consists in seeking the frequencies and the modes of vibration of a mechanical structure composed of an embed-embedded beam whose surface of the cross-section varies exponentially. This test
of Mechanics of the Structures corresponds to a dynamic analysis of a linear model having one linear behavior. It includes/understands only one modeling.
Via this problem, one tests the element of beam in inflection of variable Timoshenko of section as well as the calculation of the frequencies and modes of vibration by the method of Lanczos. One also tests
functionality "normalizes to 1. " at the point of maximum amplitude in translation " of the modes of vibration.
By using a fine space discretization, the results obtained are in concord with the results analytical given in guide VPCS.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL10 Beam of rectangular section variable
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.010-C Page:
2/6
1
Problem of reference

### 1.1 Geometry

bo
b1
initial
finale
Z, W
Length of the beam: $\mathrm{L}=0.6 \mathrm{~m}$
Constant thickness: $\mathrm{H}=0.01 \mathrm{~m}$

## Rectangular section:

Initial cross-section:
Variation of the section:
width:
$\mathrm{b} 0=0.03 \mathrm{~m}$
$\mathrm{B}=\mathrm{b} 0 \mathrm{e} 2 \mathrm{x}$ with $=1$.
surface:
$\mathrm{A} 0=3.104 \mathrm{~m} 2$
With $=\mathrm{A} 0 \mathrm{e} 2 \mathrm{x}$
moment of inertia:
$\mathrm{Iz} 0=0.25108 \mathrm{~m} 4$
Iz = Iz0 e2x
Co-ordinates of the points (m):
With
B
X
0.
0.6
y
0.
0.
1.2

Material properties
$E=2.1011 \mathrm{~Pa}$
$=0.3$
$=7.800 . \mathrm{kg} / \mathrm{m} 3$
1.3

## Boundary conditions and loadings

Embedded points A and b:, $\mathrm{U}=\mathrm{v}=0 .,=0$.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

## Titrate:

SDLL10 Beam of rectangular section variable

## Date:

07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.010-C Page:
3/6
2

## Reference solution

## 2.1

## Method of calculation used for the reference solution

The reference solution is that given in card SDLL10/89 of the guide VPCS which presents method of calculation in the following way:
The pulsation is given by the roots of the equation:
I
$1-\cos r L$
() $\mathrm{CH}(s L)+s 2-r 2 \mathrm{HS}(s L) \sin (r L)=0$

2rs
with:
To 2
I
$S$
I
(I) 0
E IzO

Components of translation of the mode $F()$
$I X$ are then:
cos

- CH
()

X
$r L$
$s L$
$X=E$
(
$\cos X$-ray) - C (
H $s x$ )
()
()
$+$
$\sin$

I
(S X-ray $R$ HS sx)
$R$
(
HS $s L$ )

- S
(
$\sin r L)$
()
(
2.2


## Results of reference

the first 4 Eigen frequencies and normalized clean modes with 1 for the largest component in translation.
2.3

Uncertainty on the solution
Analytical solution.

### 2.4 References <br> bibliographical

[1]
Working group Analyzes Dynamic. Committee of Validation of the Software packages of Calculation of Structure. French company of the Mechanics (1988).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL10 Beam of rectangular section variable
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.010-C Page:
4/6
3 Modeling

## With

3.1

Characteristics of modeling
Element of beam POU_D_T (right Beam of Timoshenko)
y
B
With
X
Cutting: beam AB: 120 meshs SEG2 of section variable.
Limiting conditions:
in all the nodes
DDL_IMPO
(ALL: "YES", DZ: 0. , DRX: 0. , DRY: 0. )
with the nodes
ends
(NODE: (AB) DX: 0. , DY: 0. , DRZ: 0. )
Names of the nodes:
Not A
$\mathrm{X}=0$.
= N 1
$\mathrm{X}=0.1$

$$
\begin{aligned}
& =\mathrm{N} 21 \\
& \mathrm{X}=0.2
\end{aligned}
$$

$$
=\mathrm{N} 41
$$

$$
\mathrm{X}=0.3
$$

= N61

$$
\mathrm{X}=0.4
$$

$$
=\text { N81 }
$$

$$
\mathrm{X}=0.5
$$

= N101

Not B
$\mathrm{X}=0.6$
= N121
3.2

Characteristics of the grid
A number of nodes:
121
A number of meshs and types:
120 SEG2
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"GENERAL"
NET
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
ALL
[U4.25.01]
NODE
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_T"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]

```
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
NORM_MODE
NORMALIZES
"TRAN"
[U4.64.02]
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
```


## Code_Aster ®

Version
4.0

Titrate:
SDLL10 Beam of rectangular section variable
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.010-C Page:
5/6
4
Results of modeling A
4.1 Values
tested
Order of the mode
Frequency
Aster
\% difference
\% tolerance
clean

## Reference

1
143.303
145.924
1.598

The clean modes of Aster were normalized to 1. at the point of maximum amplitude in translation like in the reference.

## Clean mode $\mathrm{Fi}(\mathbf{X})$ normalized with 1 at the point of maximum amplitude

## I

$\mathrm{X}=0.1$
$\mathrm{X}=0.2$
$\mathrm{X}=0.3$
$\mathrm{X}=0.4$
$\mathrm{X}=0.5$

## Reference

0.2349
0.6962
0.98960
0.8505
0.3507

Aster
1
0.2363
0.6970
0.9895
0.8516
0.3529
\% difference
0.583
0.119
0.
0.132
\% tolerance
0.6
0.15
0.1
0.15
0.7

Reference
0.4653
0.7558
0.
0.9232
0.6941

Aster
2
0.4670
0.7555
2.9104
0.9226
0.6971
\% difference
0.37
0.041
2.9104
0.063
0.435
\% tolerance
0.4
0.1
1.103
0.1
0.45

Reference
0.6278
0.1969
0.7783
0.2406
0.9366

Aster
3
0.6290
0.1952
0.7782
0.2377
0.9387
\% difference
0.192
0.89
0.014
1.226
0.228
\% tolerance
0.2
0.9
0.1
1.23
0.25

Reference
0.666
0.4832
0.
0.5901
0.9937

Aster
4
0.6656
0.4840
4.6104
0.5919
0.9928
\% difference
0.081
0.18
4.6104
0.31
0.089
\% tolerance
0.1
0.2
1.103
0.35
0.1

### 4.2 Remarks

Calculations carried out by:
MODE_ITER_SIMULT METHOD: "TRI_DIAG" OPTION: "PLUS_PETITE"

## NMAX_FREQ: 4

Contents of the file results:
the first 4 Eigen frequencies, clean vectors.

### 4.3 Parameters <br> of execution

Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
9.8 seconds

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL10 Beam of rectangular section variable
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.010-C Page:
6/6
5

## Summary of the results

Suitable modeling (frequencies and modes suitable for less than $2 \%$ ) with a fine grid.
A calculation carried out on a coarse grid ( 12 meshs) shows more important variations with reference solution. This is especially due to the way in which the modes are normalized.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SDLL14 - Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key :
V2.02.014-A Page:
1/8
Organization (S): EDF/AMA, DeltaCAD

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
Document: V2.02.014

SDLL14-Modes of vibration of an elbow of thin piping

## Summary:

This test consists in seeking the Eigen frequencies and the modes of vibration associated with a piping bent. It makes it possible to validate modelings finite elements PIPE (SEG3 and SEG4) and TUYAU_6M (SEG4).

The results obtained are compared with an analytical reference solution.
Handbook of Validation

Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SDLL14-Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.02.014-A Page:
2/8

1
Problem of reference

### 1.1 Geometry

$y, v$
Hollow circular cross-section
L
$y$
$L=$
2.0
m
With
R
$=1 . m$
C
D
$=0.020 \mathrm{~m}$
E
$D=$
0.016
m
I
With
$=1.131 \times 10-4 \mathrm{~m}^{2}$
$I=I=4.637 X 10-9 \mathrm{m4}$

```
y
Z
R
I
= 9.274 X 10-9 m4
p
=90
O
B
X
X,U
Z
L
Z,W
D
1.2
Properties of material
```

The properties of material constituting the plate are:

$$
E=2.11011 \mathrm{~Pa}
$$

Young modulus
$=0.3$
Poisson's ratio
= 7800. Kg/m3 Masses
voluminal

## 1.3

Boundary conditions and loadings
-C.L. :

- sections out of C and D embedded
- Not a: displacements following y and Z null
- Not b: displacements according to $X$ and $Z$ null

1.4 Conditions<br>initial

Without object

# Handbook of Validation 

V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL14-Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.02.014-A Page:
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## 2

Reference solution

## 2.1

Method of calculation used for the reference solution
Method of Rayleigh applied to elements of hurled right beam and to an element of thin curved beam makes it possible to determine parameters such as:

2
I.E.(internal excitation)

- inflection in the plan: $F$

I
Z
$I=$
$i=1,2$;
R
2
2
With
$\mu 2$
GI
$I$
$p$

Values 2
2
$I$ and $\mu i$ are drawn from an abacus.
This formulation is usable only for piping very slim:
L

- Elancement of the right parts higher than

20
of
I

- Thin Coude such as R

Z
100
with, angle in the center in radian. It is not
With
necessary to use here a coefficient of flexibility of the elbow.

## 2.2

Results of reference

- The first Four Eigen frequencies,
- The first Four clean modes (2 transverse modes, 2 modes in the plan).
- Frequency (transverse mode 1)
17.9 Hz
- Frequency (mode in plan 1)
24.8 Hz
- Frequency (transverse mode 2)
25.3 Hz
- Frequency (mode in plan 1)
27.0 Hz


## 2.3

Uncertainties on the solution
$\cdot \pm 0.1 \%$ for the first transverse Eigen frequency,

### 2.4 References <br> bibliographical

## [1]

VPCS: Guide validation of the software packages of structural analysis: "test SDLL14", SFM, Technical AFNOR.
[2]
R.D. Blevins, formulated for natural frequency and shape mode, New York, Van Nostrand, 1979, P. 215.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL14 - Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key :
V2.02.014-A Page:
4/8

## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

$y, v$
Modeling PIPE (SEG3)
Boundary conditions:
C
10
With
Points C and D:
8

- DDL of Beam: $D X=D Y=D Z=D R X=D R Y=D R Z=0$
- DDL of Hull: UIm $=$ VIm $=$ Wim $=0(m=2,3)$
$X, U U O m=V O m=W O m=0(m=2,3)$
$W I I=W O 1=W O=0$
B
Not a:
- $D D L$ of Beam: $D Y=D Z=0$

Not b:
10
Z, W

- DDL of Beam: $D X=D Z=0$

D

## 3.2

## Characteristics of the grid

A number of nodes: 57<br>A number of meshs and types: 28 SEG3

3.3 Functionalities<br>tested<br>Orders Key word<br>factor<br>Key word<br>AFFE_MODELE<br>AFFE<br>MODELISATION =' TUYAU'<br>AFFE_CARA_ELEM<br>BEAM: (SECTION: "CIRCLE")<br>ORIENTATION: (CARA: "GENE_TUYAU"<br>VALE: (X Y Z)<br>MACRO_MATR_ASSE<br>MATR_ASSE<br>OPTION=' RIGI_MECA'<br>OPTION=' MASS_MECA'<br>MODE_ITER_SIMULT<br>CALC_FREQ<br>OPTION=' PLUS_PETITE'<br>\section*{4}<br>Results of modeling $A$<br>\subsection*{4.1 Values}<br>tested

Identification Moments
Reference Aster \%
difference
Frequency (Hz) Transverse 1
17.9
17.84
-0.308
Frequency $(\mathrm{Hz})$ in plan 1
24.8
24.82
25.3
25.82
2.039

Frequency ( Hz ) in plan 2
27.0
27.65
2.406

### 4.2 Parameters <br> of execution

Version: NEW 5.04.20
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 13.36 seconds
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL14 - Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.02.014-A Page:
5/8

## 5 Modeling <br> B

5.1

Characteristics of modeling
$y, v$
Modeling TUYAU_6M (SEG3)
Boundary conditions:
C
With
Points $C$ and D:
8

- $D D L$ of Beam: $D X=D Y=D Z=D R X=D R Y=D R Z=0$
- DDL of Hull: UIm $=$ VIm $=$ Wim $=0(m=2,6)$
$X, U U O m=V O m=W O m=0(m=2,6)$
$W I 1=W O 1=W O=0$
B
Not a:
- DDL of Beam: $D Y=D Z=0$
Not b:
10
Z, W
- DDL of Beam: $D X=D Z=0$
D


## 5.2 <br> Characteristics of the grid

A number of nodes: 57
A number of meshs and types: 28 SEG3

### 5.3 Functionalities

tested

## Orders Key word

factor
Key word
AFFE_MODELE
AFFE
MODELISATION =' TUYAU_6M'
AFFE_CARA_ELEM
BEAM: (SECTION:"CIRCLE")
ORIENTATION: (CARA: "GENE_TUYAU"
VALE: (X Y Z)
MACRO_MATR_ASSE

MATR_ASSE<br>OPTION=' RIGI_MECA'<br>OPTION=' MASS_MECA'<br>MODE_ITER_SIMULT<br>CALC_FREQ<br>OPTION=' PLUS_PETITE'

## 6 <br> Results of modeling B

### 6.1 Values

tested

## Identification Moments

Reference Aster \%
difference
Frequency (Hz) Transverse 1
17.9
17.74
-0.914
Frequency (Hz) in plan 1
24.8
24.67
-0.525
Frequency (Hz) Transverse 2
25.3
25.66
1.439

Frequency $(\mathrm{Hz})$ in plan 2
27.0
27.49
1.800

### 6.2 Parameters

of execution
Version: NEW 5.04.20
Machine: SGI-Origin2000 R12000

Obstruction memory: 16 megabytes
Time CPU To use: 42.82 seconds
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL14 - Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.02.014-A Page:
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## 7 Modeling <br> C

## 7.1

## Characteristics of modeling

## Modeling PIPE (SEG4)

$y, v$
Boundary conditions:

## C

Points $C$ and D:
8

- DDL of Beam: $D X=D Y=D Z=D R X=D R Y=D R Z=0$
- DDL of Hull: UIm $=\operatorname{VIm}=\operatorname{Wim}=0(m=2,3)$
$X, U U O m=V O m=W O m=0(m=2,3)$
$W I I=W O 1=W O=0$
B
Not a:
- DDL of Beam: $D Y=D Z=0$

Not b:

## 7.2

Characteristics of the grid
A number of nodes: 85
A number of meshs and types: 28 SEG4

### 7.3 Functionalities

tested

## Orders Key word

factor
Key word
CREA_MAILLAGE
MODI_MAILLE
OPTION: "SEG3_4"
AFFE_MODELE
AFFE
MODELISATION=' TUYAU'
AFFE_CARA_ELEM
BEAM: (SECTION: "CIRCLE")
ORIENTATION: (CARA: "GENE_TUYAU"
VALE: (X Y Z)
MACRO_MATR_ASSE
MATR_ASSE
OPTION = ' RIGI_MECA'
OPTION=' MASS_MECA'
MODE_ITER_SIMULT
CALC_FREQ
OPTION = ' PLUS_PETITE'

## 8

Results of modeling $C$

### 8.1 Values

tested

## Identification Moments

Reference Aster \%
difference

Frequency (Hz) Transverse 1
17.9
17.66
-1.332
Frequency $(\mathrm{Hz})$ in plan 1
24.8
24.45
-1.400
Frequency (Hz) Transverse 2
25.3
24.97
-1.322
Frequency $(\mathrm{Hz})$ in plan 2
27.0
26.75
-0.914

### 8.2 Remarks

The grid in SEG4 is obtained starting from a grid SEG3 with order CREA_MAILLAGE, MODI_MAILLE with option "SEG3_4". It is important that the node medium of the SEG3 is well with
medium, Code_Aster checks this condition with a tolerance.

### 8.3 Parameters

of execution
Version: NEW 5.04.20
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 28.44 seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SDLL14-Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.02.014-A Page:
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## 9

Summary of the results
The results obtained with modeling PIPE (SEG3 and SEG4) and TUYAU_6M (SEG4) are satisfactory. The maximum change observed is lower than $2.1 \%$.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL14 - Modes of vibration of a thin elbow of piping
Date:
19/08/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.02.014-A Page:
8/8

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL15 Beam hurled, embed-free
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.015-B Page:
1/6
Organization (S): EDF/IMA/MMN

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
V2.02.015 document
SDLL15-Beam hurled, embed-free, with mass or offset inertia
Summary:
This three-dimensional problem consists in calculating the frequencies and the modes of vibration of a structure mechanics made up of a right beam slim, embed-free, with tubular section and of a mass offset attached at the loose lead of the beam. This test of Mechanics of the Structures corresponds to one
analyze dynamic of a linear model having a linear behavior. It comprises only one modeling.

This problem makes it possible to test the element of beam of Euler Bernouilli, the model of specific mass and calculation
modal by the method of Lanczos.
The results obtained are in concord with those of guide VPCS. Two calculations carried out (eccentricity of
the specific mass null or different from zero) make it possible to highlight the coupling of different modes when the specific mass is offset.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL15 Beam hurled, embed-free
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.015-B Page:
2/6
1
Problem of reference
1.1 Geometry
$Z, W$
$y, v$
C
With
$y c$
B
L
$X, U$
Co-ordinates of the points (in m):
With
B
C
$X$
0.
10.
10.
$y$
length of the beam: $A B=L=10 \mathrm{~m}$ specific mass out of $\mathbf{C}: \mathbf{m c}=1000 \mathrm{~kg}$
Tubular section:
external diameter
$o f=0.350 \mathrm{~m}$
internal diameter
$d i=0.320 \mathrm{~m}$
surface
$T o=1.57865102 \mathrm{~m} 2$
inertia
$I y=I z=2.21899104 \mathrm{m4}$
polar inertia
$I P=4.43798104 \mathrm{m4}$
2 studied cases:

1) $y c=0$.
2) $y c=1 . m$
1.2

Material properties
$E=2.11011 \mathrm{~Pa}$
$=7800 \mathrm{~kg} / \mathrm{m} 3$
1.3

Boundary conditions and loadings
Not A embedded: $(U=v=W=0,==$
$X$
$y$

0 ).
1.4 Conditions
initial
Without object for the modal analysis.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035-Ind A
Code_Aster ${ }^{\circledR}$
Version

## 4.0

Titrate:
SDLL15 Beam hurled, embed-free
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.015-B Page:
3/6
2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLL15/89 of the guide VPCS which presents method of calculation in the following way:
The problem with not offset mass leads to uncoupled modes:

- traction and compression (effect of the mass alone),
- torsion (effect of inertia around neutral fibre),
$\cdot$ inflection in plans $X, y$ and $X, Z$ (effect of the mass).
The various Eigen frequencies are given with a model by finite elements of beam of Euler (slim beam).
For the first mode with an unbalance, a method of Rayleigh gives the formula approached:
3rd I
$F=1$
Z
1
2
L3 m
)
C
0.24 M
with $M=$ total mass of the beam.
When the mass is offset, the modes of inflection ( $X, Z$ ) and of torsion are coupled, as well as modes of inflection $(X, y)$ and of traction and compression.
For the clean mode, the components at the point B make it possible to calculate the components in the center
of gravity of the mass (point C) by:
$U$
0
$Z$
$-y$
$C$
$B$
$C$
$C$
$x B$

$$
\begin{aligned}
& v \\
& =v \\
& +-Z \\
& 0 \\
& +X \\
& C \\
& C \\
& B C \\
& C \\
& y B \\
& \\
& \\
& W \\
& W+y-X \\
& 0 \\
& C \\
& B \\
& C \\
& C \\
& Z \\
& B \\
& U \\
& C \\
& C \\
& B \\
& \text { B }- \\
& z B \\
& \text { for this test }
\end{aligned}
$$

```
v}=
C
B
W=W+
C
B
xB
2.2
Results of reference
Case 1: the first }10\mathrm{ clean modes.
Case 2: the first 8 clean modes.
2.3
Uncertainty on the solution
Problem 1:
f1 analytical solution
other frequencies }\pm\mathbf{1%
Problem 2:
\pm1%
2.4 References
bibliographical
[1]
Working group Analyzes Dynamic. Commission of Validation of the Software packages of
Calculation of
Structures. French company of Mécaniens. (1988)
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
```

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL15 Beam hurled, embed-free
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.015-B Page:
4/6
3 Modeling
With
3.1

## Characteristics of modeling

Element of beam POU_D_E and discrete element DIS_TR
$y$
C
With
B
$X$
Cutting: beam AB: 20 meshs SEG2.
Limiting conditions:
with the node end $A$
DDL_IMPO: (NODE: WITH DX: 0. , DY: 0. , DZ: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Nodal mass out of B with an eccentricity
ey $=0$.
Case 1
ey $=1$.
Case 2
Names of the nodes:
Points
With $=$ N100
$B=N 200$

## 3.2

Characteristics of the grid
A number of nodes:
21
A number of meshs and types:
20 SEG2
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"CIRCLE"
ALL
[U4.24.01]
DISCRETE
"M_TR_D_N"
AFFE_CHAR_MECA
DDL_IMPO
NODE
[U4.25.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_E"
ALL
[U4.22.01]
"DIS_TR"
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
METHOD
"TRIA_DIAG"
[U4.52.01]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
: 10 cases 1
: 8 cases 2
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

## Titrate:

SDLL15 Beam hurled, embed-free
Date:
07/01/98
Author (S):
B. QUINNEZ

Key:
V2.02.015-B Page:
5/6
4
Results of modeling $A$
4.1 Values
tested
Case
Nature of the mode
Frequency Hz

Reference
Aster
inflection 1,2
1.65
1.6554
0.33
inflection 3,4
16.07
16.0712
0.

CASE 1
inflection 5,6
50.02
50.0240
0.
traction 1
76.47
76.4727
0.
$y c=0$.
torsion 1
80.47
80.4688
0.
inflection 7,8
103.20
103.20444
0.
$f z+t o 1$
1.636
1.6363
0.
$f y+\operatorname{tr} 2$
1.642
1.6416
0.

CASE 2
$f y+\operatorname{tr} 3$
13.46
13.4551
0.

$$
f z+\text { to } 4
$$

$$
13.59
$$

$$
13.5919
$$

0. 

$y c=1$.
$f z+$ to 5
28.90
28.8972
0.
$f y+\operatorname{tr} 6$
31.96
31.9594
0.
$f z+$ to 7
61.61
61.6091
0.
$f y+\operatorname{tr} 8$
63.93
63.9289
0.

Mode
0.03
3.039102
1.321
$\boldsymbol{X} \boldsymbol{B}$
1
$w C / w B$
1.030
1.030
0.

2
$u C / v B$
0.148
0.148
0.

3
$u C / v B$
2.882
2.880
0.07

4

## $w C / w B$

0.922
0.923
0.108

5
1.922
1.92268
0.036
$\boldsymbol{X} \boldsymbol{B}$
with:
$f z+t o=$ inflection $X, Z+$ torsion
$f y+t r=$ inflection $X, y+$ traction
4.2 Remarks

Calculations carried out by:
MODE_ITER_SIMULT
METHOD: "TRI_DIAG"
OPTION: "PLUS_PETITE" NMAX_FREQ:
10 Cases 1
8 Cases 2
U
In the test, one cannot check the values of the reports/ratios $C$ for modes 2 and 3 (except $\boldsymbol{v B}$
W
manually). With regard to the values of
$C$, the technique is as follows: if one imposes
$\boldsymbol{w} \boldsymbol{B}$
W
$W=$
$C=$
B
1 (order NORM_MODE), one has then
$1+X$ and one can make checks on
W
B
B
values of $X$.
B
Contents of the file results:
Case 1: the first 11 Eigen frequencies, clean vectors and modal parameters.
Case 2: the first 9 Eigen frequencies, clean vectors and modal parameters.
4.3 Parameters
of execution

Version: 3.02.21
Machine: CRAYC90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
7.2 seconds

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL15 Beam hurled, embed-free
Date:
07/01/98
Author (S):

## B. QUINNEZ

Key:
V2.02.015-B Page:
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5
Summary of the results
The modeling of unbalance gives exact results for the 8 frequencies of reference.
The precision of the clean modes is about $0.1 \%$ until mode 4.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/035 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
Embed-free SDLL23 Beam subjected to a seism
Date:
07/01/98
Author (S):

## P. GUIHOT

Key:
V2.02.023-B Page:
1/6
Organization (S): EDF/EP/AMV

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
V2.02.023 document
SDLL23 - Subjected embed-free beam
with a seism (spectral answer)

## Summary

This one-way problem consists in carrying out a spectral seismic analysis of a free fixed beam provided with two localised masses subjected to a three-dimensional excitation provided in the shape of a spectrum
oscillators in pseudo-acceleration.

Via this problem, one tests modal combinations DPC, SRSS, CQC and DSC of the operator COMB_SISM_MODAL [U4.54.04]. Combination SRSS is tested with taking into account of the neglected modes.
In addition, one tests the operators of preprocessing MODE_ITER_SIMULT [U4.52.02],
NORM_MODE [U4.64.02],
MODE_STATIQUE [U4.52.04], DEFI_FONCTION [U4.21.02] and DEFI_NAPPE [U4.21.03].
The results obtained are in agreement with the card of validation suggested in guide VPCS.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-785/96/051 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
Embed-free SDLL23 Beam subjected to a seism
Date:
07/01/98
Author (S):
P. GUIHOT

Key:
V2.02.023-B Page:
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1

## Problem of reference

### 1.1 Geometry

B
C
With
Length of the beam $\mathrm{L}=10 \mathrm{~m}$, the mass B is at a distance of 0.5 m of point A .
Cross section of the beam
Surface
$\mathrm{To}=78.1104 \mathrm{~m} 2$
Moments of inertia
$\mathrm{L}=5696108 \mathrm{~m} 4$
y
$\mathrm{L}=2003108 \mathrm{~m} 4$
Z
1.2

## Material properties

Beam
$\mathrm{E}=2.1011 \mathrm{~Pa}$
$=0 \mathrm{~kg} / \mathrm{m} 3$
(mass of the null beam)
Mass out of B
$\mathrm{mB}=50000 \mathrm{~kg}$
Mass out of C
$\mathrm{mC}=5000 \mathrm{~kg}$
1.3

Boundary conditions and loadings
Not A embedded
Spectrum of oscillator in acceleration applied in A in the three directions.

## ms2

25
2.4
0.050 .13
0.5
5.0

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-785/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:

## Embed-free SDLL23 Beam subjected to a seism

Date:
07/01/98
Author (S):

## P. GUIHOT

Key:
V2.02.023-B Page:
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2

## Reference solution

2.1

Method of calculation used for the reference solution
Guide VPCS (to be appeared): comparison with other codes.
For the comparison between the method CQC, comparison and Castem2000.

## 2.2

Results of reference

Absolute acceleration according to X at the points $\mathrm{A}, \mathrm{P} 1, \mathrm{P} 2, \mathrm{P} 3, \mathrm{P} 4$.

### 2.3 References

## bibliographical

[1]
J. PIRANDA: Note of use of the software of modal analysis MODAN - Version 0.2 (1990).

Laboratory of Mechanics Applied - University of Frank County Besancon (France).
Guide VPCS to be appeared.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-785/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
Embed-free SDLL23 Beam subjected to a seism
Date:
07/01/98
Author (S):
P. GUIHOT

Key:
V2.02.023-B Page:
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3 Modeling
With
3.1

Characteristics of modeling
y
B
C
With
X
Right beam modelled by 3 nodes and 2 SEG2 of the type POU_D_E
Mass modelled by discrete elements M_T_N
3.2

## Characteristics of the grid

A number of nodes: 3
A number of meshs and types: 2 SEG2 (POU_D_E), 2 POI1 (DIS_T_N)
3.3 Functionalities
tested
Orders
Keys
MODE_ITER_SIMULT

CALC_FREQ
[U4.52.02]
NORM_MODE
"MASS_GENE"
[U4.64.02]
MODE_STATIQUE
[U4.52.04]
DEFI_FONCTION
FREQ
LOG
[U4.21.02]
DEFI_NAPPE
[U4.21.03]
COMB_SISM_MODAL
COMB_MODE
"DPC"
[U4.54.04]
"SRSS"
"CQC"
"DSC"
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-785/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
Embed-free SDLL23 Beam subjected to a seism
Date:
07/01/98
Author (S):
P. GUIHOT

Key:
V2.02.023-B Page:
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4
Results of modeling A
4.1 Values
tested

## Identification

## Reference

Aster

## \% difference

## Eigen frequencies

## 1

0.24691
0.24672
-0.07
2
0.41666
0.41606
-0.14
3
7.4074
7.3932
-0.19
4
12.5
12.467
-0.26
5
27.777
27.507
-0.97
6
41.666
41.740
0.17

Direction numero_mode
Factor of participation
DY 1
73.3
73.31
0.02

DZ 2
73.3
73.31
0.02

DY 3
223.
222.8
-0.10
DZ 4
-223.
-222.8
5.969105
1.005

Response on 6 modes (DPC)
DEPL B DX
1.32 10-4
1.333 10-4
1.031

B DY
1.255 10-2
1.247 10-2
-0.607
B DZ
3.829 10-3
3.814 10-3
-0.393
C DX
5.999 10-4
6.022 10-4
0.368

C DY
1.269
1.282
1.027

C DZ
7.579 10-1
7.673 10-1
1.230

REAC A DX
4.12105
4.166105
1.121

WITH DY
1.227106
1.240106
1.118

WITH DZ
7.96105
7.816105
-1.807
WITH DRY
4.49105
4.481105

```
3 . 4 3 1 0 5
3.495 105
1.919
WITH DRZ
5.91105
5 . 9 6 9 1 0 5
1.005
Response on 6 modes (CQC)
DEPL B DX
1.337 10-4
1.334 10-4
-0.243
B DY
1.247 10-2
1.247 10-2
0 . 0
B DZ
3.814 10-3
3.814 10-3
-0.001
C DX
6.012E-4
6.019E-4
0 . 1 2
C DY
1.282
1.282
0 . 0
C DZ
0.767
0.767
0 . 0
REAC A DX
4.18E+5
4.169E+5
-0.24
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-785/96/051 - Ind A
```

Code_Aster ${ }^{\circledR}$
Version
4.0

## Titrate:

Embed-free SDLL23 Beam subjected to a seism
Date:
07/01/98
Author (S):
P. GUIHOT

Key:
V2.02.023-B Page:
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WITH DY
$12.40 \mathrm{E}+5$
$12.40 \mathrm{E}+5$
0.0

WITH DZ
7.816E+5
$7.816 \mathrm{E}+5$
0.0

WITH DRY
$4.481 \mathrm{E}+5$
$4.481 \mathrm{E}+5$
-0.01
WITH DRZ
$5.969 \mathrm{E}+5$
$5.969 \mathrm{E}+5$
0.0

Response on 6 modes (DSC, lasted 5
S)

DEPL B DX
1.339 10-4
1.335 10-4
-0.290
B DY
1.248 10-2
1.247 10-2
-0.025
B DZ
3.816 10-3
3.815
-0.028
C DX
6.009 10-4
6.018 10-4
0.144
4.183105
4.171105
-0.290
WITH DY
1.240106
1.240106
-0.001
WITH DZ
7.816105
7.816105
-0.002
WITH DRY
4.483105
4.482105
-0.027
WITH DRZ
5.971105
5.970105
-0.018

### 4.2 Remarks

Value of the spectrum (interpolation):
Mode
1
2
3
4
5
6
Spectrum
2.972
5.058
25.
15.74
10.

```
10.
1
1.38E-3
5.05E-6
2.27E-6
6.85E-7
3.65E-7
```

1
1.13E-5
5.05E-6
$1.51 \mathrm{E}-6$
8.04E-7
1
$1.38 \mathrm{E}-3$
$1.64 \mathrm{E}-4$
7.48E-5

## Stamp correlation CQC:

1
5.61E-4
$2.04 \mathrm{E}-4$
1
$2.21 \mathrm{E}-3$
1

### 4.3 Parameters

## of execution

Version: 3.05.02
Machine: CRAY C90
System: UNICOS 8
Obstruction memory: 8 megawords
Time CPU To use: 14.43 seconds
5

## Summary of the results

Perfect agreement of the Aster results with the card of validation suggested by the commission VPCS which
indicate a tolerance of $2 \%$ on the values of reference.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-785/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
Transitory SDLL100 Dynamic response of a beam
Date:
01/12/98
Author (S):
A.C. LIGHT

Key:
V2.02.100-D Page:
1/6
Organization (S): EDF/EP/AMV

## Handbook of Validation

## V2.02 booklet: Linear dynamics of the beams

Document: V2.02.100
SDLL100 - Transitory dynamic response

## of a beam in simple traction

## Summary:

This problem-test corresponds to a direct transitory analysis of a deadened linear system or not, made up of a beam in simple traction, subjected to a loading of the Heaviside type applied as from the initial moment.
The problem discretized with 1 single element of beam has an analytical reference solution.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
Transitory SDLL100 Dynamic response of a beam
Date:
01/12/98
Author (S):

## A.C. LIGHT

Key:
V2.02.100-D Page:
2/6
1

## Problem of reference

### 1.1 Geometry

y, v
With
B
$F(T)=(T) . F x$
X, U
1
R
X
T
$\mathrm{R}=0.05 \mathrm{~m} \mathrm{I}=1 . \mathrm{m}$
y
N01
N02
X
1.2

Material properties
$\mathrm{E}=98696.044 \mathrm{MPa}$
$=0$.
$=3.106 \mathrm{~kg} / \mathrm{m} 3$
Without damping
: $\mathrm{C}=0$. or with damping proportional of Rayleigh
:
$\mathbf{C}=\mathbf{K}+\mu \mathbf{M}$
-4
$,=5.10, \mu=5$.

## 1.3

## Boundary conditions and loadings

Force applied to the N02 node in b: $\mathrm{Fx}=1.106 \mathrm{NR}$
Function (T) evolution of the loading: $(\mathrm{T})=1 ., \mathrm{T} 0$.

### 1.4 Conditions

 initialInitial displacement no one.
Null initial speed.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams

## Code_Aster ®

Version
4.0

Titrate:
Transitory SDLL100 Dynamic response of a beam
Date:
01/12/98
Author (S):

## A.C. LIGHT

Key:
V2.02.100-D Page:
3/6
2
Reference solution
2.1

Method of calculation used for the reference solution

- Without damping: the analytical solution of the problem with 1 element is:

F
X
$X$
=
$1-\cos$
$B$ (T)
2 (
(T
0
)
m 0
1
3rd
2
2
m
$I, 0=$
, $T$

3
where $S$ is the surface of the section $(R 2)$.

- With damping: the analytical solution of the problem with 1 element is:

```
F
```

$\mu+2$
2
0
$\mu+$
$X(T)$
$X$
$=$
1 - exp-
$T$
0 sin
cos
2
$(T 1)+(T$
B
1 )
m
2
2
0
1
,$\mu$ coefficient damping proportional $\mathbf{C}=\mu \mathbf{M}+\mathbf{K}$
$(-\mu) 2224$
4
2
0

## Uncertainty on the solution

Analytical solution.

## Note:

The reference solution corresponds to the solution obtained with the discretization to an element and by keeping a matrix masses full. That makes it possible to validate the algorithm but it is not the solution of the physical problem.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
Transitory SDLL100 Dynamic response of a beam
Date:
01/12/98
Author (S):
A.C. LIGHT

Key:
V2.02.100-D Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling
POU_D_T
y
N01
N02
X
Cutting:
N01
N02
1 mesh SEG2
Limiting conditions: DDL_IMPO with the N01 node:
DX: 0. , DY: 0. , DZ: 0. , DRX: 0, DRY: 0, DRZ: 0
No time:
105 S.
Integration NEWMARK
$=0.25$, $=0.5$
WILSON integration
$=1.4$
3.2

Characteristics of the grid
A number of nodes: 2
A number of meshs and types: 1 mesh SEG2

### 3.3 Functionalities

tested
Orders
Keys
COMB_MATR_ASSE
[U4.53.01]

DYNA_LINE_TRAN<br>NEWMARK<br>[U4.54.01]<br>WILSON<br>MATR_AMOR<br>Handbook of Validation<br>V2.02 booklet: Linear dynamics of the beams<br>HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
Transitory SDLL100 Dynamic response of a beam
Date:
01/12/98
Author (S):

## A.C. LIGHT

Key:
V2.02.100-D Page:
5/6
4
Results of modeling A

### 4.1 Values

tested
Without damping:
Moment in dryness.

## Reference

Aster
\% diff.
WILSON aster
\% diff.
NEWMARK
2.E3
2.4638E04
2.4519 E 04
0.5
2.4424 E 04
0.86
4.E3
8.9141E04
8.8948E04
1.93

| 8.8794 E 04 |
| :--- |
| 0.38 |
| $6 . \mathrm{E} 3$ |
| 1.6887 E 03 |
| 1.6868 E 03 |
| 0.11 |
| 1.6852 E 03 |
| 0.20 |
| $8 . \mathrm{E} 3$ |
| 2.3337 E 03 |
| 2.3325 E 03 |
| 0.05 |
| 2.3316 E 03 |
| 0.09 |
| $1 . \mathrm{E} 2$ |
| 2.5801 E 03 |
| 2.5801 E 03 |
| 0.03 |
| 2.5801 E 03 |
| 0 |
| 1.2 E 2 |
| 2.3337 E 03 |
| 2.3349 E 03 |
| 0.05 |
| 2.3359 E 03 |
| 0.09 |
| 1.4 E 2 |
| 1.6887 E 3 |
| 1.6906 E 03 |
| 0.43 |
| 1.6922 E 03 |
| 0.21 |
| 1.6 E 2 |
| 8.9141 E 04 |
| 8.9334 E 04 |
| 0.21 |
| 8.9489 E 04 |
| 0.4 |
| 1.8 E 2 |
| 2.4638 E 04 |
| 2.4758 E 04 |
| 0.48 |
| 2.4854 E 04 |

### 9.3188E09

With damping:
Moment in dryness.
Reference
Aster
\% diff.
WILSON aster
\% diff.
NEWMARK
2.E3
2.3775E04
2.3662E04
0.47
2.3572 E 04
0.85
4.E3
8.3189E04
8.3015E04
0.21
8.2877E04
0.37
6.E3
1.5307 E 03
1.5290E03
0.11
1.5277 E 03
0.2
8.E3
2.0704E03
2.0694 E 03
0.04
2.0686E03
0.09
1.E2
2.2721E03
2.2721 E 03

0 .

### 2.2720 E 03

0.004
1.2E2
2.0976 E 03
2.0984 E 03
0.04
2.0991 E 03
0.07
1.4 E 2
1.6488 E 03
1.6501 E 03
0.08
1.6511 E 03
0.14
1.6E2
1.1164 E 03
1.1176 E 03
0.11
1.1186 E 03
0.2
1.8 E 2
7.0165E04
7.0241 E 04
0.11
7.0302 E 04
0.19
2.E2
5.4263 E 04
5.4266 E 04
0.005
5.4269 E 04
0.01
4.2 Remarks

After the first two steps of time, the solution with damping is obtained with an error lower than $0.2 \%$.

### 4.3 Parameters

of execution
Version: 3.02.19
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords

# Time CPU To use: 

150 seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
Transitory SDLL100 Dynamic response of a beam
Date:
01/12/98
Author (S):
A.C. LIGHT

Key:
V2.02.100-D Page:
6/6
5
Summary of the results
The two algorithms give a solution with an error lower than $0.2 \%$ of the solution of reference after the first two steps of time.
This problem requires a step of time of integration of 105 S .
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
1/8
Organization (S): EDF/IMA/MMN

## Handbook of Validation

## V2.02 booklet: Linear dynamics of the beams

## V2.02.101 document

## SDLL101 - Vibration of a beam with <br> prestressed <br> Summary:

This plane problem consists in seeking the frequencies of vibration of a mechanical structure made up of one
hurled beam, of circular section, under tension embed-slide. This test of Mechanics of the Structures corresponds to a dynamic analysis of a linear model having a linear behavior. This test comprises two modelings.
In the first modeling, one tests the element of beam of Timoshenko subjected to a prestressing, it calculation of geometrical rigidity and the calculation of the Eigen frequencies by the method of Lanczos. In
the second modeling, one tests the element of beam of Euler-Bernouilli subjected to a prestressing, the calculation of geometrical rigidity and the calculation of the Eigen frequencies by the method of Bathe and Wilson. The results obtained are in concord with the results of guide VPCS. One notices a shift towards high frequencies of vibration when prestressing in the beam increases.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
2/8
1
Problem of reference

### 1.1 Geometry

With
B
P
P
Full circular section
diameter D: 0.01 m
Length of the beam
L: 2 m

## 1.2

## Material properties

$\mathrm{E}=21011 \mathrm{~N} / \mathrm{m} 2$
$=0.3$
$=7.800 . \mathrm{kg} / \mathrm{m} 3$

## 1.3

Boundary conditions and loadings

- Poutre pose-posed,
. 4 loadings are studied $\mathrm{P}=0$. , $\mathrm{P}=10$. , $\mathrm{P}=100$. , $\mathrm{P}=1.000$. NR
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A


## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
3/8
2
Reference solution

## 2.1

## Method of calculation used for the reference solution

The equation of vibration of a prestressed beam is:
4y
2y
2 y
I.E.(internal excitation)
$+P$
$=-S$
Z x4
$x 2$
$x 2$
prestressed traction if $\mathrm{P}>0$, of compression if $\mathrm{P}<0$, and led to the Eigen frequencies of inflection (assumption of Euler-Bernoulli)
I.E.(internal excitation)
$F=i 2$

Z
$I=$
I.E.(internal excitation) i2 $2 S$

Z
2.2

Results of reference
the first 5 Eigen frequencies.
2.3

Uncertainty on the solution
Analytical solution (assumption of the beams of Euler-Bernouilli).

### 2.4 References

bibliographical
[1]
Robert D. BLEVINS Formulated for natural frequency and shape mode - 1979 p. 144 (formula 8.20 rectified).

Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
4/8
3 Modeling
With

## 3.1 <br> Characteristics of modeling <br> Elements of beam POU_D_T (right Beam of Timoshenko)

## y

With
B
X
Cutting: 10 elements of beam
node a: translations in X and y blocked
node b: translation in y blocked.
Note:
The force P applied out of B generates a reaction - P in A .

## 3.2

Characteristics of the grid
A number of nodes:
21
A number of meshs and types:
20 SEG2
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"CIRCLE"
[U4.24.01]
AFFE_CHAR_MECA
FORCE_NODALE
NODE
[U4.25.01]
DDL_IMPO
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_T"
"ALL"
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_MATR_ELEM
[U4.41.01]
SIEF_ELGA
CALC_VECT_ELEM
OPTION
CHAR_MECA
[U4.41.02]
CALC_CHAM_ELEM

## DEPL

[U4.61.01]
OPTION
SIEF_ELGA_DEPL
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
NMAX_FREQ
"PLUS_PETITE"
COMB_MATR_ASSE
[U4.53.01]
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):
B. QUINNEZ

Key:
V2.02.101-B Page:
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4
Results of modeling A
4.1 Values
tested

## Pre constraint/order of the mode

## Reference

Aster
\% difference
clean
$\mathrm{P}=01$
4.97137
4.9711
0.01

2
19.8851
19.8829
0.01

3
44.7414
44.7320
0.02

4
79.5403
79.5203
0.02

5
124.2818
124.2706
0.01
$\mathrm{P}=101$
5.0728
5.0727
0.0

2
19.9874
19.9852
0.01

3
44.8439
44.8345
0.02

4
79.6429
79.6229
0.03

5
124.3844
3
45.7561
45.7467
0.02
4
80.5600
80.5400
0.03
5
125.3037
125.2923
0.01
$\mathrm{P}=1.0001$
11.2577
11.2576
0.00
2
28.3462
28.3442
0.01
3
54.0370
54.0281
0.02
4
89.2134
89.1935
0.02
5
134.1511
134.1379
0.01
4.2 Parameters

## of execution

Version: 3.02.21
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
10 seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
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5 Modeling
B
5.1

Characteristics of modeling
Elements of beam POU_D_E (Beam of Euler-Bernouilli)

## y

With
B
X
Cutting: 19 elements of beam
node a: translations in X and y blocked
node b: translation in y blocked.
Note:
The force P applied out of B generates a reaction - P in A .

## 5.2

Characteristics of the grid
A number of nodes:
21

A number of meshs and types:
20 SEG2

### 5.3 Functionalities

tested

Orders

## Keys

AFFE_CARA_ELEM
BEAM
"CIRCLE"
[U4.24.01]
AFFE_CHAR_MECA
FORCE_NODALE
NODE
[U4.25.01]
DDL_IMPO
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_T"
"ALL"
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_MATR_ELEM
OPTION
"RIGI_GEOM"
[U4.41.01]
SIEF_ELGA
CALC_VECT_ELEM
OPTION
CHAR_MECA
[U4.41.02]
CALC_CHAM_ELEM
DEPL
[U4.61.01]
OPTION
SIEF_ELGA_DEPL
MODE_ITER_SIMULT
METHOD
"JACOBI"
[U4.52.02]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
COMB_MATR_ASSE
[U4.53.01]
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
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6
Results of modeling B
6.1 Values
tested

## Pre constraint/order of the mode

## Reference

Aster
\% difference
clean
$\mathrm{P}=01$
4.97137
4.9713
0.00

2
19.8851
19.8853
0.00

3
44.7414
44.7439
4
79.5403
79.5574
0.02
5
124.2818
124.3594
0.06
$\mathrm{P}=101$
5.0728
5.0728
0.00
2
19.9874
19.9876
0.00
3
44.8439
44.8464
0.01
4
79.6429
79.6599
0.02
5
124.3844
124.4619
0.06
$\mathrm{P}=1001$
5.9090
5.9090
0.00
2
20.8860
20.8862
0.00
3
45.7561
45.7585
0.01
4
80.5600

### 6.2 Parameters

## UNICOS 8.0

Obstruction memory:
8 megawords
Time CPU To use:
11 seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

Code_Aster ®
Version
4.0

Titrate:
SDLL101 Vibration of a beam with prestressing
Date:
01/09/99
Author (S):

## B. QUINNEZ

Key:
V2.02.101-B Page:
8/8
7
Summary of the results
The results obtained are in concord with the results of reference. It is noticed well that them frequencies of vibration increase when prestressing increases.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SDLL102 Gantry subjected to forces electrodynamics
Date:
06/03/98
Author (S):
G. DEVESA

Key:
V2.02.102-B Page:
1/6
Organization (S): EDF/EP/AMV

## Handbook of Validation

V2.02 booklet: Linear mechanics

## V2.02.102 document

SDLL102 - Gantry subjected to
electrodynamic forces

## Summary:

This test is a three-dimensional problem of direct transitory dynamic calculation with forces distributed of electrodynamic origin applied to a gantry (bars on 3 insulating columns of a station of transformation).
This test was provided by the Center of Studies of the Grid system (EDF-DEPT). It was supplemented
since by
a benchmark international bench starting from experimental measurements (results of several foreign codes):
test CIGRE-structure D.
It makes it possible to compare results of displacements compared to those obtained by other industrial codes
using a finished method finite elements or differences.
This test contains a modeling with elements of the type SEG2.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SDLL102 Gantry subjected to forces electrodynamics
Date:
06/03/98
Author (S):
G. DEVESA

Key:
V2.02.102-B Page:
2/6
1
Problem of reference
1.1 Geometry

D
M3
S4
H
M3
S8
L
Z
S3
0.16

C
M3
S7
G
M3
S11 K

```
M3
S2
2.1 Y
S6
M2
M2
S10
M2
X
B
F
J
2.135
S1
M1
S5
M1
S9
M1
With
E
I
11.5
11.5
Cross sections of beams:
- frame support
S1:
To = 1.2061 102 m2,
Iz =
2.3681 105 m4;
S5:
To = 1.4621 102 m2,
Iz =
2.8709 105 m4;
S9:
To = 1.5530 102 m2,
Iz =
3.0493 105 m4
- insulating columns
S2:
To = 3.1428 102 m2,
Iz =
4.5070 105 m4;
```


## S6:

$\mathrm{To}=3.2592102 \mathrm{~m} 2$,
$\mathrm{Iz}=$
4.6738105 m 4 ;

S10:
$\mathrm{To}=3.3416102 \mathrm{~m} 2$,
Iz =
4.7927105 m 4

- connections

S3, S11:
$\mathrm{To}=3.1944102 \mathrm{~m} 2$,
Iz =
1.15105 m 4 ;

S7:
$\mathrm{To}=4.2130102 \mathrm{~m} 2$,
Iz =
1.15105 m 4 ;

- conducting

S4, S8:
circular $\mathrm{R}=6.055102 \mathrm{~m}$
$\mathrm{E}=6.2103 \mathrm{~m}$
1.2

Material properties
M1:
$\mathrm{E}=2.1011 \mathrm{~Pa}$
$=8000 \mathrm{~kg} / \mathrm{m} 3$
(frame support)
M2:
$\mathrm{E}=5.1010 \mathrm{~Pa}$
$=2500 \mathrm{~kg} / \mathrm{m} 3$
(insulating column)
M3:
$\mathrm{E}=7.1010 \mathrm{~Pa}$
$=2700 \mathrm{~kg} / \mathrm{m} 3$
(connection and conducting aluminium)
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ®

Version
4.0

Titrate:

SDLL102 Gantry subjected to forces electrodynamics
Date:
06/03/98
Author (S):
G. DEVESA

Key:
V2.02.102-B Page:
3/6
1.3

Boundary conditions and loadings
Points A, E, I: embedding
Points D, L: not-continuity of ux, y, Z
Forces of Laplace on drivers DH, HL;
two-phase current
$==100 \mathrm{~m}$

- conducting infinite separated from 1 m

I
2 (C
bone
$T$
) $E T$

EFF
cos)
Ieff
effective intensity of the current
time-constant

- two short-circuit with reset

T
$0<\mathrm{T}^{2} 0.135$
$0.135<\mathrm{T}<0.580$
$0.580^{2} \mathrm{~T}^{2} 0.885$
I
15.6 kA

0

### 1.4 Conditions

## initial

$\mathrm{T}=0$, speed and zero acceleration.

## 2

## Reference solution

## 2.1

Method of calculation used for the reference solution

- experimental measurements,
- numerical methods D.F or E.F.


## 2.2 <br> Uncertainty on the solution

$\pm 5$ to $10 \%$ of dispersion of the computed values.

### 2.3 References

bibliographical
[1]
G. DEVESA: "Calculation of the electrodynamic strains on structures of drivers rigid electric stations: establishment in the mechanical computer code Aster and Validation ".
Note HM-72/5904
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL102 Gantry subjected to forces electrodynamics
Date:
06/03/98
Author (S):
G. DEVESA

Key:
V2.02.102-B Page:
4/6
3 Modeling
With
3.1

## Characteristics of modeling

Modeling POU_D_E
D2
H1 H2
L1
D1

## Z

L2
y
C
G
K
X
B
F
J
With
E
I
Discretization:

- elements AB, EF, IJ: 10 meshs: SEG2
- elements BC, FG, JK: 10 meshs: SEG2
- elements CD1, GH1, KL1: 1 mesh: SEG2
- elements D2H1, H2L1: 30 meshs: SEG2

Dynamic evolution on 1s discretized in step of time of 5. 104 S with the algorithm of NEWMARK (= 0.25 have, $\mathrm{D}=0.5$ ).
Storage of the results all the 20 steps of times is 102 S .

## 3.2

## Characteristics of the grid

A number of nodes: 126
A number of meshs and types: 123 meshs SEG2

### 3.3 Functionalities

## tested

Orders
Key word
Option
Keys
DEFI_FONC_ELEC
[U4.21.06]
AFFE_CHAR_MECA
FORCE_ELEC
[U4.25.01]
DYNA_LINE_TRAN

## NEWMARK

[U4.54.01]
CALC_ELEM
EFGE_ELNO_DEPL
[U4.61.02]
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL102 Gantry subjected to forces electrodynamics
Date:
06/03/98
Author (S):
G. DEVESA

Key:
V2.02.102-B Page:
5/6
4
Results of modeling A
4.1 Values
tested
Identification
Reference test
References
\% difference
Aster 3.04.02
$\mathrm{T}=0.12 \mathrm{~S}$
uy in C2
60.49 mm

MX in S1
3140. Nm
3071. Nm
2.2

MX in S2
10150. Nm
9239. Nm
-9.
MX in S3
3130. Nm

### 4.2 Remarks

The results obtained by ASTER are satisfactory compared to the other codes. They are almost always lower than measurements (effects of frames AB, EF, IJ overestimated). The maximum ones are chopped because of periodic storage.

## Contents of the file results:

Displacements all 102 S and efforts in the elements at times $\mathrm{T}=0.12 \mathrm{~S}, \mathrm{~T}=0.27 \mathrm{~S}, \mathrm{~T}=0.70 \mathrm{~S}$.

### 4.3 Parameters

of execution
Version: 3.04.12
Machine: CRAY C90
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
74.19 seconds

Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL102 Gantry subjected to forces electrodynamics
Date:
06/03/98
Author (S):

## G. DEVESA

Key:
V2.02.102-B Page:
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5
Summary of the results
The results are acceptable compared to the test results and locate values produced by
Code_Aster in good place among ten results of other software.
The values of reference of the file of test are those of version 3.04.02 of the code (not regression).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SDLL104 Structures primary education and secondary

Date:
30/08/01
Author (S):
J. PIGAT Key

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Organization (S): EDF/RNE/AMV

Handbook of Validation<br>V2.02 booklet: Linear dynamics of the beams<br>V2.02.104 document

SDLL104-Structures primary education and secondary subjected to a random excitation

## Summary:

A principal beam of strong rigidity and important mass (primary structure), embedded at its base, support in three points of transmission of efforts a light and flexible beam (secondary structure).

The primary structure is excited at its base by an acceleration given by its DSP.
The test compares a direct calculation of the whole of the two structures and a chainé calculation where the answer of principal beam at the points of connection is used like the excitation of the secondary beam.

The following functionalities are tested:
stamp interspectrale analytical preset by the function of KANAI-TAJIMI, random dynamic response moving absolute, imposed acceleration, modal answer interspectrale, modal excitation interspectrale.

This approach is representative of what is required for the industrial studies: to determine them answers of various secondary structures knowing the answer of the primary structure.
Handbook of Validation
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Code_Aster ${ }^{\circledR}$
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## Titrate:

SDLL104 Structures primary education and secondary

Date:
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Author (S):
J. PIGAT Key
:
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## 1

Problem of reference

### 1.1 Geometry

Observation of the result
Dy
PB
$d x$
Pa

## Beam Pa:

Section: $0.1 \mathrm{~mm} X 0.1 \mathrm{~mm}$
Length: 3.0 m

## Beam PB:

Section: 0.001 mm X 0.001 mm

## 1.2 <br> Material properties

Young modulus beams A and b:
$E=2.1 E+11 \mathrm{~N} / \mathrm{M} 2$
Poisson's ratio beams A and b:
$=0.3$
Density beam Pa:
With $=2000 \mathrm{~kg} / \mathrm{m} 3$
Density beam PB:
$B=1000 \mathrm{~kg} / \mathrm{m} 3$

## 1.3

Boundary conditions and loadings
The movement is authorized in the plan ( $D X, D Y$ ).
Beam Pa is embedded in the support.
The beam PB is connected to beam Pa by three points. In each one, displacements in direction DX and DY of the node of Pa and the PB node are identical. Rotations are not dependent.

The matrix interspectrale which transmits displacements of structure Pa to the structure PB in chained calculation is of dimension 6 ( 6 ddl of transmission).
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Date:
30/08/01

## Author (S):

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2
Reference solution

## 2.1

Method of calculation used for the reference solution

The reference solution is the direct calculation of the whole of the two structures.

It is supposed that the mass and the rigidity of secondary structure Pa do not disturb it behavior of the primary structure. Thus chained calculation is supposed to be equivalent to direct calculation.
One can check on the table below that the structure PB modifies the Eigen frequencies little of structure Pa.

Calculated Eigen frequencies ( Hz )

Pa
$P B$
Pa and PB

1
6.5711
6.5711
0.

2
10.2655
10.2654
0.001
318.3759
18.37590.

4
26.2871
26.2871
0.

5
33.2716
33.2716
0.

6
59.1708
59.1708

0. 

## 7

69.4570
69.4571
0.0001
8
105.3094105 .3091
0.0001
9114.5567
114.55590 .0007
10
118.9369
118.93760 .0006

## 2.2

Results of reference
One observes the spectral concentration of acceleration on the node of the beam PB of X-coordinates 2.4 m (it node PB25).

2.3 References<br>bibliographical<br>\section*{[1]}<br>C. DUVAL "harmonic Response under random excitation in Code_Aster: principles theoretical and examples of use " - Note HP-61/92.148<br>\section*{Handbook of Validation}<br>V2.02 booklet: Linear dynamics of the beams<br>HT-62/01/012/A

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Date:
30/08/01
Author (S):

## J. PIGAT Key

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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

## Discrete element in translation of the type DIS_T

PB11
PB15
PB21 PB25
PB31
PA1
PAll
PA21
PA31

Elements used: POU_D_T. The characteristics of beam are defined by:

## CARELEM

=
AFFE_CARA_ELEM
( BEAM

The method of calculation asks for the calculation of static modes corresponding to the DDL excitation.
The Eigen frequencies taking into account in calculations are all the frequencies in the band [ $0,35 \mathrm{~Hz}$ ].

## 3.2 <br> Characteristics of the grid

A number of nodes:
Pa: 31
PB: 21
A number of meshs and types:
Pa: 30 SEG2
PB: 20 SEG2

### 3.3 Functionalities <br> tested

## Orders

AFFE_CHAR_MECA DDL_IMPO

LIAISON_DDL
MODE_ITER_INV

MODE_STATIQUE DDL_IMPO<br>AVEC_CMP<br>DEFI_INTE_SPEC KANAI_TAJIMI

## DYNA_ALEA_MODAL EXCIT

MODE_STAT<br>ANSWER<br>REST_SPEC_PHYS<br>\section*{Handbook of Validation}<br>V2.02 booklet: Linear dynamics of the beams<br>HT-62/01/012/A

## Code_Aster ${ }^{\circledR}$

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Results of modeling $A$

### 4.1 Values

tested

Values of the spectral concentration of acceleration at point PB25:

Frequency Direct Node<br>\section*{Chains}<br>\%<br>difference<br>5. Hz<br>PB25<br>3.6913<br>3.7052<br>0.38\%<br>10. Hz<br>PB25<br>75.439<br>75.797<br>0.47\%<br>15. Hz<br>PB25<br>1.6777<br>1.6929<br>0.91\%<br>20. Hz<br>PB25<br>1.1367<br>1.0987<br>3.34\%<br>25. Hz<br>PB25<br>0.2927<br>0.2630<br>10.12\%

### 4.2 Parameters

of execution
Version: STA 5.02

# Code_Aster ${ }^{\circledR}$ 

Version
5.0

## Titrate:

SDLL104 Structures primary education and secondary

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Author (S):
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## 5 <br> Summary of the results

The results obtained confirm on the one hand the assumption of equivalence between direct calculation and calculation
chained, in addition the good coherence of the calculation algorithm of dynamic response random.
A variation is inevitable between calculations of the two methods: that obtained to 25 Hz is too high (11\%).

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SDLL105 Pipe subjected to sources of random fluid excitations
Date:
30/08/01
Author (S):
J. PIGAT Key
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Organization (S): EDF/RNE/AMV

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
V2.02.105 document

## SDLL105 - Pipe subjected to sources random fluid excitations

## Summary:

A right piping embedded at an end in the wall of a tank and supporting a mass with the other end is subjected to a fluid excitation.

The excitation is defined by its spectral concentration of power in the form of a "white vibration".
It covers all the types of source established in the code:
source of flow-volume,
source of flow-mass,
source of pressure,
source of force,
imposed effort.
One is interested in the spectral concentration of power of the response in a degree of freedom of pressure located on
node supporting the mass.

The random dynamic response is given here moving absolute.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-62/01/012/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
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Author (S):
J. PIGAT Key
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## 1 <br> Problem of reference

### 1.1 Geometry

# section accoustics-mechanics 

## SOURCE

Tank
$d x$
$m=10 \mathrm{~kg}$

Circular pipe of section:
External diameter: 0.1 m
Thickness: 3 mm
One does not take account of the field of gravity.

## 1.2 <br> Material properties

Young modulus of the pipe:
$E=2.1 E+11 \mathrm{NR}$
Coefficient of compressibility of the pipe:
$=0.3$
Density of the pipe:
$=7800 \mathrm{~kg} / \mathrm{m} 3$
Density of the fluid:
$F=8.3 \mathrm{~kg} / \mathrm{m} 3$
Celerity of the fluid:
$C=495 \mathrm{~m} / \mathrm{s}$

## 1.3 <br> Boundary conditions and loadings

The $D D L D y, d z, d r x, d r y, d r z$ are blocked for all the pipe.
On the acoustic section dx is also blocked and the only free DDL are NEAR and PHI.
At the end on the side tank: $C L O S E=0 . P H I=0$.
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## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

No reference solution. The values tested for the not-regression are those obtained with version 3.02.17.

## 2.2 <br> Results of reference

## Spectral concentration of power of the pressure to the node at the right end of the tube, the frequencies

 10, 12, 14, 36, 38, 40 Hz . These frequencies are close to the two Eigen frequencies catches in count ( 12.38 and 37.36 Hz ).
### 2.3 References <br> bibliographical

## [1]

C. DUVAL "Dynamic response under random excitations in Code_Aster: principles theoretical and examples of use" - Note HP-61/92.148
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams

## Code_Aster ${ }^{\circledR}$

Version
5.0

Titrate:
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Author (S):
J. PIGAT Key

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

4.0 m
6.0 m
$d x$
A7
A6
A5
A4
AM5 AM6 AM7 AM8 AM9 AM10
AM4
A3A2 Al AMIAM2 AM3

Elements used for the pipes: FLUI_STRU
Element used for the mass in AM10: DIS_T
In all the calculation cases, the exiting spectral concentration is a white vibration of level 1.
The sources of flow-volume and flow-rate pressures are applied to node AM1.
The sources of mass and force are applied between nodes AM1 and AM2.
The last calculation case corresponds to a force imposed on node AM10 in the direction dx.

The clean modes of frequency in the interval $[0,100 \mathrm{~Hz}]$ were taken into account in calculation, that is to say the first two modes.

Damping is introduced in modal form into the operator of dynamic response random. For all the calculation cases, it is taken equal to 1\%

## 3.2 <br> Characteristics of the grid

A number of nodes: 17
A number of meshs and types: 16 SEG2, 1POII

### 3.3 Functionalities

tested
Orders
MODE_ITER_SIMULT

## CONSTANT DEFI_INTE_SPEC

## DYNA_ALEA_MODAL EXCIT SIZE

### 3.4 Remarks

The spectral concentrations of fluid source are expressed in their physical units. For a source of volume flow rate in (m3/s) $2 / \mathrm{Hz}$.
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30/08/01
Author (S):

## J. PIGAT Key

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## 4

Results of modeling $A$

## 4.1

Values of not-regression tested
Values of the spectral concentration of acceleration at point PB25:

## Frequency

Type of source
Aster
10 Hz
SOUR_DEBI_VOLU
$9.1954 E+11$
12 Hz
SOUR_DEBI_VOLU
$4.3709 E+13$
$3.6428 E+12$
36 Hz
SOUR_DEBI_VOLU
$1.1142 E+13$
38 Hz
SOUR_DEBI_VOLU
$3.6976 E+13$
40 Hz
SOUR_DEBI_VOLU
$2.6238 E+12$

10 Hz
SOUR_DEBI_MASS
$1.3347 E+10$
12 Hz
SOUR_DEBI_MASS
$6.3448 E+11$
14 Hz
SOUR_DEBI_MASS
$5.2879 E+10$
36 Hz
SOUR_DEBI_MASS
1.6173E+11

38 Hz
SOUR_DEBI_MASS
$5.3675 E+11$
40 Hz
SOUR_DEBI_MASS
$3.8088 E+10$

10 Hz
SOUR_PRESS
$9.5991 E+00$
12 Hz
SOUR_PRESS
$2.5952 E+02$
14 Hz
SOUR_PRESS

36 Hz
SOUR_PRESS
$3.2428 E+00$
38 Hz
SOUR_PRESS
1.3681E+01

40 Hz
SOUR_PRESS
1.1649E+00

10 Hz
SOUR_FORCE
1.9931E+05

12 Hz
SOUR_FORCE
$5.3887 E+06$
14 Hz
SOUR_FORCE
$2.5675 E+05$
36 Hz
SOUR_FORCE
$6.7334 E+04$
38 Hz
SOUR_FORCE
$2.8408 E+05$
40 Hz
SOUR_FORCE
$2.4189 E+04$

10 Hz
EFFO
2.6542E03

12 Hz
EFFO
4.5780E02

14 Hz
EFFO
9.0980E04

36 Hz

## EFFO

3.3472E02

38 Hz
EFFO
0.1186

40 Hz
EFFO
8.8587E03

### 4.2 Parameters <br> of execution

Version: STA 5.02
Machine: SGI-Origin 2000
System:
IRIX 64
Obstruction memory:
8 megawords
Time CPU To use:
3.21 seconds

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## 5 <br> Summary of the results

This test makes it possible to pass in the options corresponding to the various types of source. It acts
primarily of a test developer.
Not having a reference solution, it is simply a question of not regressing between the versions.
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL106 Beam subjected to a random excitation distributed
Date:
29/08/00
Author (S):
J. PIGAT Key
:
V2.02.106-B Page:
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Organization (S): EDF/RNE/AMV

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
V2.02.106 document

SDLL106-Beam subjected to an excitation

## Summary:

An Bi-embedded beam is subjected over all its length to an effort distributed. Profile of distribution of the force is identical to all the frequencies.

The random movement of this beam is evaluated by a stochastic approach: the density is determined spectral of power of displacement in various points of the beam.

The two possibilities are tested:
space function of the efforts applied with interspectre unit (method 1),
interspectre builds directly for the excited ddl (method 2).
This test is an illustration of the response of a structure subjected to a Eolienne excitation.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-62/01/012/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SDLL106 Beam subjected to a random excitation distributed
Date:
29/08/00
Author (S):
J. PIGAT Key
:
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1
Problem of reference

### 1.1 Geometry

obstacle<br>flexible<br>excitation<br>DY<br>DX

## Beam:

Square section: $0.001 m X 0.001 \mathrm{~m}$
Length: 0.8 m
One does not take account of the field of gravity.

## 1.2

Material properties
Young modulus:
$E=2.1 E+11 \mathrm{NR}$
Coefficient of compressibility:
$=0.3$
Density:
$=7000 \mathrm{~kg} / \mathrm{m} 3$

## 1.3 <br> Boundary conditions and loadings

The beam is embedded at the two ends.
Ddl DZ is blocked in any point.
The effort applied is distributed with the following space distribution:

Titrate:
SDLL106 Beam subjected to a random excitation distributed
Date:
29/08/00
Author (S):
J. PIGAT Key

## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

Direct calculation defines an assembled vector of space distribution of the effort and applies the density spectral of effort $G$
()

FF on this distribution (method 1).
Broken up calculation defines the excitation as a matrix interspectrale of dimension 3 (equalizes with a many excited nodes) and apply, in effort imposed on the nodes, the following matrix interspectrale (method 2):

The two results must be identical without any approximation.

## 2.2 <br> Results of reference

Spectral concentration of power of displacement of the P3 node at the frequencies: 4. , 6. , 8. , 10. and 12 Hz .

### 2.3 References <br> bibliographical

[1]
C. DUVAL "Dynamic response under random excitation in Code_Aster: principles theoretical and examples of use" - Note HP-61/92.148
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

## Discrete element in translation of the type DIS_T

Observation of the DSP of displacement
P1
$P 2$
P3
P4
P5
P6
P7
P8
P9
Load application

Elements of beam: $P O U \_D \_T$
The exiting spectral concentration is a white vibration of level 1.
The first 2 clean modes were taken into account in calculation.
Damping is introduced in the form of modal damping into the operator of answer random dynamics. For all the calculation cases, it is taken equal to 5\%

## 3.2 <br> Characteristics of the grid

A number of nodes: 9
A number of meshs and types: 8 SEG2

### 3.3 Functionalities

tested

## Orders

AFFE_CHAR_MECA FORCE_NODALE
$M O D E \_I T E R \_I N V$

CONSTANT DEFI_INTE_SPEC

DYNA_ALEA_MODAL EXCIT
SIZE:

### 3.4 Remarks

The spectral concentrations are expressed in their physical unit. For a force it will be in $N 2 / \mathrm{Hz}$.
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Date:
29/08/00
Author (S):
J. PIGAT Key
:
V2.02.106-B Page:
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## 4

Results of modeling $A$

### 4.1 Values

tested

## Spectral concentration of displacement at point AM10:

## Frequency

Method 1
Method 2
\% difference

### 4.2 Parameters of execution

Version: STA 5.02
Machine: SGI-Origin 2000

## System:

IRIX 64
Obstruction memory:
8 megawords
Time CPU To use:
2.39 seconds

Handbook of Validation
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Version
5.0

## Titrate:

SDLL106 Beam subjected to a random excitation distributed
Date:
29/08/00
Author (S):

## J. PIGAT Key

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## 5

Summary of the results
Method 1 (space distribution of the efforts) and indirect method (by decomposition on the three excited nodes) provide the same result.

This checking ensures a good coherence of the two methods and the quality of their programming.

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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL108 "Counts with coffee" of NEUBERT
Date:
08/01/98
Author (S):

## P. GUIHOT

Key:
V2.02.108-A Page:
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Organization (S): EDF/EP/AMV

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams

## V2.02.108 document

## SDLL108 - "Table with coffee" of NEUBERT

## Summary

This multidirectional problem consists in carrying out a spectral seismic analysis of a made up structure elements of beams without masses and discrete masses to the nodes. It includes/understands two
modelings
correspondent with two smoothnesses of discretization.
The seismic excitation is provided in the shape of three spectra of response of oscillators in acceleration to
supports according to axes' $X, Y$ and $Z$.
Via this problem, one tests order MODE_STATIQUE [U4.52.04] and the options of
quadratic combination of the modes and quadratic combination of the directions of the excitations of order COMB_SISM_MODAL [U4.54.04].
The results obtained are in concord with the results of reference.
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## Code_Aster ${ }^{\circledR}$

Version
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Titrate:
SDLL108 "Counts with coffee" of NEUBERT
Date:
08/01/98
Author (S):

## P. GUIHOT

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1
Problem of reference

### 1.1 Geometry

438,15
692,15
$L=0.69215 \mathrm{~m}$
$L=0.43815 \mathrm{~m}$
$H=0.473075 \mathrm{~m}$
473,075
y
$X$
Z
hollow circular section: $D=0.060 \mathrm{~m}$
$D=0.052 \mathrm{~m}$
I
$S=0.7037 \times 103 \mathrm{~m} 2$
$I y=I z=0.2772 \times 106 \mathrm{~m} 4$

$$
\text { WITH }=A=2 .
$$

y
Z
$C=0.5545 \times 106 \mathrm{~m} 4$
$y$
1.2

Material properties
$E=1.92276 E 11 \mathrm{~N} / \mathrm{m} 2$
$=0.3$
$=0 . \mathrm{kg} / \mathrm{m} 3$
1.3

Boundary conditions and loadings

- Structure embedded at its base,
- Modal Amortissements of $2 \%$.

Definition of the spectrum of acceleration to the supports
Freq
X Z
Y
100
17.3
11.5

110
16.3
10.9

120
15.3
10.2

130
14.3
9.6

300
10.2
6.66

- for a damping of $2 \%$,
- acceleration in $G$.

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Titrate:

## P. GUIHOT

Key:
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2
Reference solution
2.1

Method of calculation used for the reference solution
Comparison with other codes.
2.2

Results of reference

- Déplacements at the points constituting the corners of the table,
- Réactions of supports to anchorings,
- efforts intern with the "corners".


### 2.3 References

bibliographical
[1]
NEUBERT and EZELL: Dynamics behavior of has foundation like structure.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A
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Author (S):

## P. GUIHOT

Key:
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3 Modeling
With
3.1

Characteristics of modeling
219,075

Net E2: N1-N7
7
13
219,075
3
E3: N7-N6
6
16
1
E12: N7-N8
14
254,
5
17
X
15
4
Z
18
Masses of corners
: 4,444 kg
Intermediate masses: 1,566 kg
3.2
Characteristics of the grid
A number of nodes: 18
A number of meshs and types: 18 MECA_POU_D_T and 14 MECA_DIS_T_N

### 3.3 Functionalities

tested
Orders
Keys
MODE_STATIQUE
[U4.52.04]
MODE_ITER_INV

### 3.4 Remarks

The modes are standardized with the generalized mass with 1.
The total answer is obtained by a quadratic combination of the modes and a combination quadratic of the directions of the excitations.
Handbook of Validation
$V 2.02$ booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SDLL108 "Counts with coffee" of NEUBERT
Date:
08/01/98
Author (S):

## P. GUIHOT

Key:
V2.02.108-A Page:
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4
Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference
Displacement:
N5 DX
3.408E-04
3.409E-04
0.013

DY
4.364E-06
4.417E-06
1.218

DZ
3.019E-04
3.019E-04
0.010

DRX
3.684E-04
3.686E-04
0.044

DRY
4.988E-05
5.024E-05
0.718

DRZ
5.086E-04
5.090E-06
0.073

Reactions:
N15 FX
$1.255 E+03$
$1.257 E+03$
0.051

FY
$1.257 E+03$
$1.273 E+03$
1.282

FZ
$1.220 E+03$
$1.224 E+03$
0.381

MX
$3.247 E+02$
$3.248 E+02$
0.016

MY
$4.345 E+00$
$4.355 E+00$
0.239

MZ

```
3.483E+02
3.482E+02
-0.010
Efforts:
E2 N7 FX
1.135E+03
1.135E+03
-0.024
FY
1.241E+03
1.253E+03
0.922
FZ
1.101E+03
1.102E+03
0.145
MX
2.275E+02
2.275E+02
0.004
MY
4.346E+00
4.355E+00
0.218
MZ
2.196E+02
2.197E+02
0.048
E3 N7 FX
1.899E+02
1.913E+02
0.729
FY
1.039E+03
1.039E+03
0 . 0 1 6
FZ
1.306E+02
1.319E+02
0.987
MX
2.275E+02
2.275E+02
```


### 4.2 Remarks

- Displacements of the corners (N5, N7, N10, N12) are identical,
- The reactions to the supports (N15, N16, N17, N18) are identical,
- Efforts generalized in the total reference mark.


### 4.3 Parameters

of execution
Version: 3.05.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory: 8 megawords

# Time CPU To use: 17.13 seconds 

Handbook of Validation
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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
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Date:
08/01/98
Author (S):
P. GUIHOT

Key:
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5 Modeling
B
5.1

Characteristics of modeling
155,75
155,75
127
127
12
127
13
155,75
127
14
11
155,75
15
5
10
16
17
9
4
Net E3: N6-N17
18
24
8
155,75
22
E4: N17-N18

Masses of corners

: 4,444 kg

Intermediate masses: $0,783 \mathrm{~kg}$
5.2

Characteristics of the grid
A number of nodes: 24
A number of meshs and types: 28 (MECA_POU_D_T) and 24 MECA_DIS_T_N

### 5.3 Functionalities

tested

## Orders

## Keys

MODE_STATIQUE
[U4.52.04]
MODE_ITER_INV
CALC_FREQ
[U4.52.02]
NORM_MODE
[U4.64.02]
DEFI_FONCTION
[U4.21.02]
DEFI_NAPPE
[U4.21.03]
COMB_SISM_MODAL
[U4.54.04]

### 5.4 Remarks

The modes are standardized with the generalized mass with 1.

The total answer is obtained by quadratic combination of the modes and a quadratic combination directions of the excitations.
Handbook of Validation
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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL108 "Counts with coffee" of NEUBERT
Date:
08/01/98
Author (S):
P. GUIHOT

Key:
V2.02.108-A Page:
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6
Results of modeling B
6.1 Values
tested
Identification
Reference
Aster
\% difference
Displacement:
N17 DX
$3.429 \mathrm{E}-04$
$3.419 \mathrm{E}-04$
-0.278
DY
4.364E-06
4.392E-06
0.658

DZ
3.036E-04
$3.027 \mathrm{E}-04$
-0.300
DRX
$3.701 \mathrm{E}-04$
$3.698 \mathrm{E}-04$
-0.078

## DRY

4.750E-05
4.762E-05
0.253

DRZ
$5.110 \mathrm{E}-04$
5.105E-04
-0.102
Reactions:
N25 FX
$1.254 \mathrm{E}+03$
$1.253 \mathrm{E}+03$
-0.072
FY
$1.254 \mathrm{E}+03$
$1.263 \mathrm{E}+03$
0.706

FZ
$1.219 \mathrm{E}+03$
$1.219 \mathrm{E}+03$
0.037

MX
$3.249 \mathrm{E}+02$
$3.245 \mathrm{E}+02$
-0.132
MY
$4.124 \mathrm{E}+00$
$4.128 \mathrm{E}+00$
0.104

MZ
$3.486 \mathrm{E}+02$
$3.482 \mathrm{E}+02$
-0.133
Efforts:
E3 N17 FX
$1.130 \mathrm{E}+03$
$1.131 \mathrm{E}+03$
0.062

FY
$1.245 \mathrm{E}+03$
$1.249 \mathrm{E}+03$
0.351

FZ

| $1.098 \mathrm{E}+03$ |
| :--- |
| $1.098 \mathrm{E}+03$ |
| -0.007 |
| MX |
| $2.286 \mathrm{E}+02$ |
| $2.282 \mathrm{E}+02$ |
| -0.174 |
| MY |
| $4.125 \mathrm{E}+00$ |
| $4.128 \mathrm{E}+00$ |
| 0.072 |
| MZ |
| $2.208 \mathrm{E}+02$ |
| $2.205 \mathrm{E}+02$ |
| -0.134 |
| E 4 N 17 FX |
| $1.881 \mathrm{E}+02$ |
| $1.893 \mathrm{E}+02$ |
| 0.670 |
| FY |
| $1.042 \mathrm{E}+03$ |
| $1.042 \mathrm{E}+03$ |
| -0.023 |
| FZ |
| $1.311 \mathrm{E}+02$ |
| $1.320 \mathrm{E}+02$ |
| 0.703 |
| MX |
| $2.286 \mathrm{E}+02$ |
| $2.282 \mathrm{E}+02$ |
| -0.175 |
| MY |
| $2.910 \mathrm{E}+01$ |
| $2.918 \mathrm{E}+01$ |
| 0.269 |
| MZ |
| $1.860 \mathrm{E}-01$ |
| $1.857 \mathrm{E}-01$ |
| -0.159 |
| E 19 N 17 FX |
| $2.935 \mathrm{E}+02$ |
| $2.966 \mathrm{E}+02$ |
| 1.053 |

## FY

$6.367 \mathrm{E}+02$
$6.390 \mathrm{E}+02$
0.359

FZ
$2.653 \mathrm{E}+02$
$2.653 \mathrm{E}+02$
0.006

MX
$1.860 \mathrm{E}-01$
$1.860 \mathrm{E}-01$
0.040

MY
$3.231 \mathrm{E}+01$
$3.236 \mathrm{E}+01$
0.148

MZ
$2.208 \mathrm{E}+02$
$2.205 \mathrm{E}+02$
-0.134

### 6.2 Remarks

- Displacements of the corners (N9, N12, N17, N20) are identical, - The reactions to the supports (N25, N26, N27, N28) are identical, - Efforts generalized in the total reference mark.


### 6.3 Parameters

## of execution

Version: 3.05.02
Machine: CRAY
System: UNICOS
Obstruction memory: 8 megawords
Time CPU To use: 18.03 seconds
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SDLL108 "Counts with coffee" of NEUBERT
Date:
08/01/98
Author (S):

## P. GUIHOT

Key:
V2.02.108-A Page:
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## Summaries of the results

One obtains a relatively good agreement between the solution calculated with Aster and the reference solution
(\% of difference lower than 1\%), except in modeling A, Depl N5 DY and Réaction N15 FY where variations reach $1,2 \%$.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/96/051 - Ind A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Transitory SDLL113 dynamic Under-structuring

Date:
18/02/00
Author (S):
G. Key ROUSSEAU

V2.02.113-B Page:
1/8
Organization (S): EDF/RNE/AMV

SDLL113-Under-structuring dynamic
transient: beam in simple traction

## Summary:

The structure considered is an annular beam of section in simple traction, embedded on a side, and subjected
at its end with a force of the Heaviside type. Its transitory dynamic response is calculated by under-structuring.
The beam is modelled by elements of the beams type of Timoshenko (linear model). Two modelings are proposed according to whether the beam is deadened or not. Damping tested is of type RAYLEIGH (damping proportional).
The results of reference result from a direct transitory calculation by modal recombination without under-structuring with operator DYNA_TRAN_MODAL [U4.54.03]. This test thus makes it possible to validate the tools of transitory calculation of response per under-structuring, in the linear case.

## Handbook of Validation <br> V2.02 booklet: Linear dynamics of the beams <br> HT-62/01/012/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Transitory SDLL113 dynamic Under-structuring

Date:
18/02/00
Author (S):
G. Key ROUSSEAU

## 1

Problem of reference

### 1.1 Geometry

$y$
With
N5
N10
F
$X$
Z
section
L

Length of the beam: $L=1 \mathrm{~m}$
Section: Interior ray $=0.09 \mathrm{~m}$
External ray $=0.10$ m

## 1.2 <br> Properties of materials <br> $E=1 x 1010 \mathrm{~Pa}$ <br> $=0.3$ <br> $=1 x 104 \mathrm{~kg} / \mathrm{m} 3$ <br> Modeling a: not of damping <br> Modeling b: Damping proportional (RAYLEIGH):

$C=K+M$ with $=6.5 \times 106 S$ and $=16.0 \mathrm{~s} 1$.
These values correspond to a reduced damping of $1 \%$ on the first mode of the structure.

## 1.3

Boundary conditions and loadings

On all the structure one imposes $D Y=D Z=D R X=D R Y=D R Z=0$.
On point A one imposes the condition of embedding $D X=0$.
In N10 one applies a constant force as from the moment $t=0: F x=100$ NR.
Fx (NR)
$T(S)$
-100

### 1.4 Conditions

initial
The structure is initially at rest.
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V2.02 booklet: Linear dynamics of the beams
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2
Reference solution

## 2.1

Method of calculation used for the reference solution
There is an analytical solution detailed in the reference [bib1].
The following notations are adopted:

## : Young modulus

: mass
voluminal
L
: length of the bar
With
: section of the bar
NR
: normal effort directed according to axis $X$ : damping coefficients of Rayleigh

One also poses:
$=(2-)$
$N$
N 1
where
$N=12$
, 3
, ,...
2

1
$=(+/$
$N$
$N$
$N)$.
2

Displacement in a point $M(X)$ unspecified is given by:

- $T$
(
Nx
8NL
NE
$N N$

2
$U X, T)=$

2
(-) 1
C
bone 1
$T$
$N$
$+$
$\sin 1-$
$T$
EA
2
$(N N)$
2
( $N N$ )
EA $\boldsymbol{n}=1$
(2n-)
1
1 -
$N$

## 2.2

Results of reference
Values of the fields of displacement, speed and acceleration of the loose lead (N10 node) are worth at the moment $T=0.0195 S$ :

Displacement (m)
Speed (Mr. s1) Acceleration (Mr. s2)
Calculation without damping
8.3766x107
1.6753 X 1030

Calculation with structural damping

## 2.3 <br> Uncertainty on the solution

Analytical solution.

### 2.4 References <br> bibliographical

[1]
G. ROBERT: Analytical solutions in dynamics of the structures. Report/ratio Samtech $n^{\circ} 121$, March 1996.

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
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5.0

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Transitory SDLL113 dynamic Under-structuring

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18/02/00
Author (S):
G. Key ROUSSEAU
:
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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The beam is cut out in two parts of equal size. Each substructure considered is with a grid in segments to which are affected of the elements of the type "POU_D_T".
$F$
$\boldsymbol{X}$
L/2
$L / 2$

The structure is studied using the method of under-structuring transitory with interfaces of the type "Craig-Bampton" (blocked interfaces).

The modal base used is made up of 4 clean modes for the substructure of left, of 5
clean modes for the substructure of right-hand side to which the constrained modes associated are added with
degrees of freedom of interface (2).
Base projection of the substructure of left:
With
N5
Modes with interface
With
N5
Constrained mode
blocked
$d x=1$
Base projection of the substructure of right-hand side:
N5
N10
N5
N10
N5
N10
Fx
$d x=1$
Modes with interface
blocked
Constrained mode

## 3.2

Characteristics of the grids
The grid of the complete beam to carry out the calculation of reference shows the characteristics following:
file of the type grid Aster (.mail)

```
A number of nodes \(=\mathbf{1 1}\)
A number of meshs \(=10\) SEG2
```

The grid of the half-beam to carry out calculation by under-structuring, presents them following characteristics:
file of the type Ideas (.msup)
A number of nodes $=6$
A number of meshs $=5 \mathbf{S E G} 2$

## 3.3

Functionalities tested
Orders
Key word factor
Key word
Argument
Keys
MODE_ITER_SIMULT CALC_FREQ
NMAX_FREQ
[U4.52.02]
DEFI_INTERF_DYNA INTERFACES
TYPE
"CRAIGB" [U4.55.03]
TRADITIONAL DEFI_BASE_MODALE
[U4.55.04]
MACR_ELEM_DYNA
"TRADITIONAL" OPTION
[U4.55.05]
DYNA_TRAN_MODAL EXCIT
VECT_GENE
[U4.54.03]
REST_BASE_PHYS INTERPOL
"FLAX"
[U4.64.01]
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-62/01/012/A
Code_Aster ${ }^{\circledR}$

Version

5.0

Titrate:
Transitory SDLL113 dynamic Under-structuring

## Date:

18/02/00
Author (S):

## G. Key ROUSSEAU

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## 4

Results of modeling $A$

### 4.1 Values

tested
The values are restored on a grid skeleton made up of the two substructures. Grid initial which contains 6 nodes is duplicated to create the substructure of right-hand side. The node of end
thus corresponds to node 12.
Identification Reference
Under-structuring Difference
(\%)
(beam supplements)
Node 12: displacement (m)
6.2818E7
$6.2818 E 7$
< 0.1
Node 12: speed (m.s1)
2.0957E3 2.0957E3 <
0.1

Node 12: acceleration (m.s2)
1.1139E+1 1.1139E+1 <
0.1

### 4.2 Remarks

One can be astonished that the adopted reference corresponds to the complete beam modelled by 10 elements and not with the analytical solution. It is that the development of the solution in series of clean modes converges very slowly: the modal solution is very far away from the solution here theoretical. The relevant comparison is thus well that selected.

Calculation by modal recombination is carried out on the basis of complete modal structure (11 modes) taking into account the adopted discretization. In the same way, the dimension of the base of projection
used for calculation by dynamic under-structuring 11 (substructure of left is: 4 modes clean +1 constrained mode; substructure of right-hand side: 5 clean modes +1 constrained mode). It is
thus normal to obtain an excellent agreement between the modeling of the complete beam and that of beam divided into two substructures.

4.3 Parameters<br>of execution<br>Version: STA 5.03<br>Machine: SGI ORIGIN 2000<br>Obstruction memory: 64 Mo, Time CPU To use: 10,29 seconds.<br>Handbook of Validation<br>V2.02 booklet: Linear dynamics of the beams<br>HT-62/01/012/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Transitory SDLL113 dynamic Under-structuring

Date:
18/02/00
Author (S):
G. Key ROUSSEAU
:
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5 Modeling
B

## 5.1

Characteristics of modeling
The characteristics of this modeling are identical to the preceding one (modeling A). Only difference lies in the fact that the structure is deadened. Damping used is of type proportional:
$C=K+M$ with $=6.5 x 106 S$ and $=16.0$ s 1.
These values correspond to a reduced damping of $1 \%$ on the first mode of the structure.

## 5.2

Characteristics of the grid
The characteristics of the grid are also identical to those of modeling A (cf [§ 3.2]).

## 5.3

Functionalities tested

Orders
Key word factor
Key word
Argument
Keys
MODE_ITER_SIMULT CALC_FREQ
NMAX_FREQ
[U4.52.02]
DEFI_INTERF_DYNA INTERFACES
TYPE
"CRAIGB" [U4.55.03]
TRADITIONAL DEFI_BASE_MODALE
[U4.55.04]
MACR_ELEM_DYNA MATR_AMOR
OPTION
"TRADITIONAL"
[U4.55.05]
ASSE_MATR_GENE
OPTION
"AMOR_GENE"
[U4.55.08]

DYNA_TRAN_MODAL EXCIT<br>VECT_GENE<br>\section*{[U4.54.03]}<br>\section*{AMOR_GENE}<br>REST_BASE_PHYS INTERPOL<br>"FLAX"[U4.64.01]

Handbook of Validation<br>V2.02 booklet: Linear dynamics of the beams<br>HT-62/01/012/A

> Code_Aster ${ }^{\circledR}$
> Version
> 5.0

> Titrate:
> Transitory SDLL113 dynamic Under-structuring

## Date:

18/02/00
Author (S):
G. Key ROUSSEAU

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6
Results of modeling B

### 6.1 Values

tested
The results are restored on a grid skeleton made up of the two substructures. Grid initial which contains 6 nodes is thus duplicated to create the substructure of right-hand side. The node
of end thus corresponds to node 12.

## Identification Reference <br> Under-structuring Difference <br> (\%) <br> (complete model) <br> Node 12: displacement (m) <br> 9.54882E7 <br> 9.54882E7 <br> < 0.1 <br> Node 12: speed (m.s1) <br> 1.22190E3 1.22190E3 < <br> 0.1 <br> Node 12: acceleration (m.s2) <br> $1.91712 E+0$ 1.91712E+0 <br> < <br> 0.1

### 6.2 Remarks

One can be astonished that the adopted reference corresponds to the complete beam modelled by 10 elements and not with the analytical solution. Important differences between the numerical solutions and
theoretical are ascribable with the reduced number of elements. The use of 50 elements instead of 10 would have allowed to approach theoretical acceleration with a margin of $1 \%$. That put aside, one can note that
the use of a method of under-structuring provides the same results as those of the beam complete.

Calculation by modal superposition is carried out on the basis of complete modal structure (11 modes). In the same way, the dimension of the base of projection used for calculation by understructuring
dynamics is 11 (substructure of left: 4 clean modes +1 constrained mode; substructure of right-hand side: 5 clean modes +1 constrained mode). It is thus normal to obtain an excellent agreement enters
the modeling of the complete beam and its modeling in two substructures.

### 6.3 Parameters <br> of execution

Version: STA 5.03
Machine: SGI ORIGIN 2000
Obstruction memory: 64 Mo, Time CPU To use: 13,41 seconds.

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
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## 7 <br> Summary of the results

As well in the case not deadened as in the deadened case, the results obtained using the model complete and by under-structuring do not present significant variations. Operators of calculation linear transient by under-structuring are thus validated.

In the deadened case, the agreement between the solutions numerical and analytical would have been better while taking more elements ( 50 instead of 10 for example).

Lastly, let us announce that the results obtained by Code_Aster were compared with results obtained by the SAMCEF software. They are included in the table below. One notes that in the case not deadened, the two software provides nearby results, quite as far away from the solution analytical.

## Identification <br> Case not deadened <br> Deadened case

Code_Aster
The SAMCEF software
Code_Aster
The SAMCEF software
N12, displacement ( $m$ )
6.282E7 6.290E7

Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
SDLL118 - Beam subjected to an axial excitation fluid-rubber band
Date:
01/03/04
Author (S):
A. ADOBES, Key Mr. LAINET
:
V2.02.118-A Page:
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Organization (S): EDF-R \& D /MFTT, CS

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
Document: V2.02.118

SDLL118-Beam subjected to an excitation fluid-rubber band axial

## Summary:

One considers a PVC tube placed at the center of a cylindrical enclosure of section circular and subjected to
the action of an axial water flow. This hardware configuration corresponds to the experimental device of
Tanaka and Al [bib1] which is used to measure the evolutions of frequency and reduced damping of the first mode tube according to the mean velocity of the flow.

The goal of this case-test is to validate the resorption of model MEFISTEAU [R4.07.04] making it possible to calculate them modal characteristics of a telegraphic structure under confined axial flow, by taking account of one excitation of the fluid-rubber band type.

The functionalities particular to test are as follows:

- operator DEFI_FLUI_STRU [U4.25.01]: definition of the parameters for the taking into account of coupling fluid-rubber band, in the case of a configuration of the type "beam of tubes under axial flow" (key word factor FAISCEAU_AXIAL), - operator CALC_FLUI_STRU [U47.66.02]: calculation of the evolutions of the frequencies and depreciation
modal tiny rooms according to the mean velocity of the flow, by the implementation of the model MEFISTEAU.

The numerical results of the simulation of the device of Tanaka and Al are validated by comparison with
experimental results. Taking into account relatively important uncertainties on the values experimental, the results of reference for nonthe regression of the code are those obtained numerically
during the restitution of the case-test.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/04/005/A

# Code_Aster ${ }^{\circledR}$ 

Version
6.4

Titrate:
SDLL118 - Beam subjected to an axial excitation fluid-rubber band
Date:
01/03/04
Author (S):
A. ADOBES, Key Mr. LAINET
:
V2.02.118-A Page:
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## 1 <br> Problem of reference

### 1.1 Geometry

The tube considered is a hollow roll whose characteristic dimensions are as follows:
length
L
$=1 \mathrm{~m}$,
external diameter
ext. $=13 \mathrm{~mm}$,
internal diameter
int $=8,8$ Misters.
The tube is placed in the center of a cylindrical enclosure of circular section. The internal diameter of the enclosure is worth $D=5 \mathrm{~cm}$.

The surface roughness of the tube is worth $=105 \mathrm{Mr}$.

## 1.2 <br> Properties of materials

The physical characteristics of material PVC constituting the tube are as follows:
Young modulus
$E=2,80.109 \mathrm{~Pa}$,
Poisson's ratio $=0,3$,
density
$=1500 \mathrm{~kg} / \mathrm{m} 3$.
Water surrounding the tube has the following properties:
density
water $=1000 \mathrm{~kg} / \mathrm{m} 3$,
kinematic viscosity water $=1,1.106 \mathrm{~m} 2 / \mathrm{s}$.

## 1.3 <br> Boundary conditions and loadings

The two ends of the tube are connected to fixed supports by two metal stems. The relative one flexibility of inflection of these stems releases the degrees of freedom of rotation of the ends of the tube. One
can thus estimate that the conditions of self-supporting quality of the tube are of the rotulé-rotulé type, the stems metal introducing of each end an additional stiffness of rotation.

Moreover, these stems make it possible to apply an axial load to the tube, which can thus be prestressed in
traction or in compression. In practice, two configurations are studied:

- nonprestressed tube: no effort is applied. This configuration corresponds to modeling A of the case-test,
- tube prestressed in compression by application of an axial load of 40 NR at an end.

This configuration corresponds to modeling B of the case-test.
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Author (S):
A. ADOBES, Key Mr. LAINET

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### 1.4 Reference <br> bibliographical

[1]
Mr. TANAKA, K. FUJITA, A. HOTTA and NR. KONO: "Parallel flow-induced damping of PWR fuel assembly ", ASME Conference, Pittsburgh, Pa, PVP vol. 133 (1988)
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## 2 <br> Reference solution

The experimental measurements taken on the device of Tanaka and Al provide the values of reference for the validation of the model.

Two graphs below, representing the evolutions of the frequencies and damping tiny room of the first mode doubles inflection according to the mean velocity of the flow, allow to compare the results of the model with the experimental results.

Taking into account uncertainties on measurements, the tolerance of relative variation for the validation of the model
is rather broad. This is why experimental measurements cannot be used as values of reference for the case-test, a narrower tolerance being necessary to guarantee nonthe regression of the code. values of reference used are thus those obtained numerically during the restitution of the case test.

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The tube is represented by 100 elements of right beams of Timoshenko (MECA_POU_D_T), supported per as many meshs segments to 2 nodes (SEG2). Two elements MECA_DIS_TR are additions with the nodes ends of the tube, allowing to model the metal stems by stiffnesses of rotation discrete.

One carries out with the elements of beam the characteristics of circular section:
external ray
Rext $=6,5.103 \mathrm{~m}$,
thickness E
=
2,1.103 Mr.
(cf paragraph [§1.1])
One also assigns to these elements a material of behavior ELAS:
Young modulus
$E=2,80.109 \mathrm{~Pa}$,

Poisson's ratio $=0,3$,
density
$=1500 \mathrm{~kg} / \mathrm{m} 3$.
(cf paragraph [§1.2])
One assigns to the discrete elements the same stiffness of rotation around the two orthogonal axes to the neutral fibre of the tube:
$\mathrm{Kr}=6,29 \mathrm{Nm} / \mathrm{rad}$

This stiffness of rotation was adjusted in order to find the Eigen frequency of the first double mode in air.

The degrees of freedom in translation DX and DZ of the nodes ends N001 and N101 are blocked so to prohibit a rigid movement of body of the tube (axial translatory movement). One blocks also the DY of the N001 node. Moreover, in each node, one blocks the degree of freedom of rotation $D R Y$, in order to prohibit any movement of torsion.

The tube is immersed in a cylindrical enclosure of $2,5 \mathrm{~cm}$ interior ray (cf paragraph [§1.1]). The profiles of density and kinematic viscosity of surrounding water are presumedly constant along the tube:
density
water $=1000 \mathrm{~kg} / \mathrm{m} 3$,
kinematic viscosity water $=1,1.106 \mathrm{~m} 2 / \mathrm{s}$.
(cf paragraph [§1.2])
No axial load is applied to the tube which is thus not prestressed.
The evolutions of the frequency and the reduced damping of the first double mode of inflection are calculated for a beach mean velocities of flow from 0 to $8 \mathrm{~m} / \mathrm{s}$, by step of $1 \mathrm{~m} / \mathrm{s}$.
One takes account of an initial reduced damping of the tube of the $4,8 \%$.

## 3.2 <br> Characteristics of the grid

The total number of nodes used for the grid is 101.
The meshs (of type SEG2) are 100.
The file of grid is with the format ASTER.
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## 3.3 <br> Stages of calculation

Validation of the operators of coupling fluid-structure, for configurations of the type "beam tubes under axial flow" is made in two principal stages.

The first consists in defining the parameters of taking into account of the coupling fluid-structure with operator DEFI_FLUI_STRU followed by key word FAISCEAU_AXIAL.
The second is the calculation of the evolutions of modal frequency and reduced damping according to mean velocity of the flow, with operator CALC_FLUI_STRU and by the implementation of model MEFISTEAU.

3.4 Functionalities<br>tested<br>\section*{Orders Key word}<br>factor Key word<br>DEFI_FLUI_STRU<br>FAISCEAU_AXIAL<br>TYPE_PAS: "CARRE_LIGN"

NOT: 1.5

TYPE_RESEAU: 1/3

## Code_Aster ${ }^{\circledR}$

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## 4 <br> Results of modeling $A$

### 4.1 Values <br> tested

The tests relate to the frequency and the reduced damping of the first double mode of inflection of tube, at the mean velocity of flow of $0 \mathrm{~m} / \mathrm{s}$ and $4 \mathrm{~m} / \mathrm{s}$. 2 types of test are carried out:

- a test of comparison with experimental measurements,
- a test to guarantee nonthe regression of the code.


### 4.1.1 Frequency of the first mode doubles inflection

- Test of comparison with the experiment, at the rate of flow of $0 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the experimental value is worth 0,1\%.
Number of the mode
Experimental value

## Computed value

Relative variation
1
7 Hz
7,0011331924304 Hz
+0,016\%
2
7 Hz
7,0011331924505 Hz
+0,016\%

- Test of nonregression of the code, at the rate of flow of $4 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth $10 \mathbf{8 \%}$.
Number of the mode
Value of reference
Computed value
Relative variation
1
6,812275 Hz
6,8122749601350 Hz
-5,85.10-9\%
2
6,812275 Hz
6,8122749601557 Hz
-5,85.10-9\%
4.1.2 Reduced damping of the first mode doubles inflection

- Test of comparison with the experiment, at the rate of flow of $4 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth $1 \%$.
Number of the mode
Experimental value
Computed value
Relative variation
1
17 \%
16,972486655473 \%
-0,162 \%
2
$17 \%$

- Test of nonregression of the code, at the rate of flow of $4 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth $106 \%$.
Number of the mode
Value of reference
Computed value
Relative variation
1
16,97249 \%
16,972486655473 \%
-1,97.10-7\%
2
16,97249 \%
16,972486655445 \%
-1,97.10-7\%

### 4.2 Remarks

The values of reference those are obtained by Code_Aster during the restitution of the case-test, which
thus allows to check nonthe regression of the code during its evolution.
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## 5 Modeling

Characteristics of modeling
Modeling B is identical to modeling A (cf paragraph [§3.1]), but this time the tube is prestressed in compression.

An axial load of compression of 23,7 NR is applied to the node N101 end. The intensity of the effort has
thus readjusted compared to the provided experimental value of 40 NR , in order to find correctly the value of frequency of the first mode doubles in air (cf paragraphs [§1.2], [§1.3]). It readjustment can apply by the summary modeling of the metal stems ensuring it self-supporting quality and the setting in compression.

One deduces from the nodal effort the vector of elementary efforts, then an assembled vector which is built
according to the classification of the degrees of freedom of the tube. Static deformation due to the setting in
compression is then obtained by multiplying the vector assembled by the reverse of the matrix of rigidity
structural. Using this static deformation, one calculates then a stress field with elements, whose is deduced a geometrical matrix of rigidity. This one is then added to stamp structural rigidity in order to obtain the matrix of rigidity of the tube in compression, which is finally used for the calculation of the modes in air.

The evolutions of the frequency and the reduced damping of the first double mode of inflection are calculated for a beach mean velocities of flow from 0 to $8 \mathrm{~m} / \mathrm{s}$, by step of $1 \mathrm{~m} / \mathrm{s}$. One holds count of an initial reduced damping of the tube of 4,3\%.

## 5.2 <br> Characteristics of the grid

The characteristics of the grid of this second modeling are the same ones as those of modeling $A$, is:
101 nodes used and 100 meshs of the type SEG2.
The file of grid is with the format ASTER.
5.3

Stages of calculation
Just as for modeling A, the functionalities to be validated are those of the operators of coupling fluid-structure for configurations of the type "beam of tubes under axial flow" (cf paragraph [§3.3]).

Moreover, modeling B makes it possible to test other functionalities.
The first makes it possible to carry out the calculation of a field of displacements to the nodes by inversion of
stamp rigidity structural and product of the reverse by a vector of assembled effort, with operators FACT_LDLT and RESO_LDLT.
The second allows the calculation of a geometrical matrix of rigidity using a stress field with the elements, with the operator CALC_MATR_ELEM, option RIGI_GEOM.
5.4 Functionalitiestested
Orders Key word
factor Key word
FACT_LDLT
RESO_LDLT
CALC_MATR_ELEM RIGI_GEOM
DEFI_FONC_FLUI
DEFI_FLUI_STRU FAISCEAU_AXIAL
DEFI_MATERIAU ELAS_FLUI
CALC_FLUI_STRU BASE_MODALE
MODE_MECA
MODI_BASE_MODALE
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## 6

Results of modeling B

### 6.1 Values

tested
The tests relate to the frequency and the reduced damping of the first double mode of inflection of tube, at the mean velocity of flow of $0 \mathrm{~m} / \mathrm{s}$ and $4 \mathrm{~m} / \mathrm{s}$. 2 types of test are carried out:

- a test of comparison with experimental measurements,
- a test to guarantee nonthe regression of the code.
6.1.1 Frequency of the first mode doubles inflection
- Test of comparison with the experiment, at the rate of flow of $0 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth 0,1\%.
Number of the mode
Experimental value
Computed value
Relative variation
1
5,1 Hz
$5,1046169521712 \mathrm{~Hz}$
$+0,091 \%$
2
5,1 Hz
$5,1046169521914 \mathrm{~Hz}$
+0,091\%

- Test of nonregression of the code, at the rate of flow of $4 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth $107 \%$.
Number of the mode
Value of reference
Computed value
Relative variation
1
$4,842109 \mathrm{~Hz}$

### 6.1.2 Reduced damping of the first mode doubles inflection

- Test of comparison with the experiment, at the rate of flow of $4 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth $10 \%$.
Number of the mode
Experimental value
Computed value
Relative variation
1
21,1 \%
21,935720674426 \%
+3,96 \%
2
21,1 \%
21,935720674292 \%
+3,96 \%

- Test of nonregression of the code, at the rate of flow of $4 \mathrm{~m} / \mathrm{s}$ :

The tolerance of relative variation compared to the reference is worth $107 \%$.
Number of the mode
Value of reference
Computed value
Relative variation
1
21,93572 \%
21,935720674426 \%
+3,07.10-8\%
2
21,93572 \%
21,935720674292 \%
$+3,07.10-8 \%$
6.2 Remarks

The values of reference those are obtained by Code_Aster during the restitution of the case-test, which
allows to check nonthe regression of the code during its evolution.
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Code_Aster ${ }^{\circledR}$
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## Titrate:

SDLL130 - Seismic response of a beam BA to linear behavior
Date:
15/10/03
Author (S):
S. MILL, L. DAVENNE, F.GATUINGT

Key: V2.02.130-A Page:
1/8
Organization (S): EDF-R \& D /AMA, LMT Cachan

## Handbook of Validation

V2.02 booklet: Linear dynamics of the beams
Document: V2.02.130

SDLL130-Seismic response of a beam in reinforced concrete (rectangular section) with behavior linear

## Summary:

The problem consists in analyzing the seismic response of a concrete beam reinforced via one modeling beam multifibre (POU_D_EM, modeling B).
The calculation of reference (modeling A) is made using Code_Aster with "traditional" elements of beam Euler Bernoulli (POU_D_E).

## Handbook of Validation

## V2.02 booklet: Linear dynamics of the beams

HT-66/03/008/A

Code_Aster ${ }^{\circledR}$
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Titrate:
SDLL130 - Seismic response of a beam BA to linear behavior
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Author (S):
S. MILL, L. DAVENNE, F.GATUINGT

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1 Characteristics
general

### 1.1 Geometry

It is about a beam simply supported on its two supports [Figure 1.1-a].

```
y
X
5.400 mm
500 mm
Frameworks HA8 separated by 100 mm
5.000 mm
20 mm
44 mm
20 mm
32 mm
28
y
500 mm
Z
Tally HA8 spaced 100 mm
232
200 mm
44 mm
```

Appear 1.1-a: geometry of the structure

## 1.2

Material properties

steel: $E=200.000 \mathrm{MPa}=0.33,=7800 \mathrm{~kg} / \mathrm{m} 3$
damping: of type Rayleigh ( $K+M$ ), with 5\% on modes 1 and 2

## 1.3

Boundary conditions and loadings
Simple support in b: $\boldsymbol{D y}=\mathbf{0}$
Support "doubles" in a: dx=Dy=0
To avoid the clean modes except plan, one blocks the following degrees of freedom on all the beam:
$X-r a y=r y=d z=0$
Loading: seism ac_s2_c_1 [Figure 1.3-a], in axis OY applied to the two supports (factor of amplification of the signal $=137$ ).

NB: the transverse reinforcements are not taken into account in calculations
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## 2 <br> Reference solution Modeling A

The reference is obtained by a Code_Aster calculation with traditional elements of beam of Euler (POU_D_E). The characteristics for this calculation of reference are obtained while homogenizing steel-concrete section:
Ea
200000
Section:
2
S
$=S+$
$S=1$
+
$\times$,
$00017=109$

The density selected is that of the concrete (the weight of steel is neglected).
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3
Modeling B (POU_D_EM)

## 3.1

Characteristics of modeling
Longitudinal grid of the beam:
It is composed of 17 nodes and 16 pairs of elements POU_D_EM (16 elements for the concrete and 16 for steel).

Cross section of the beam:
The concrete is modelled by a grid (AFFE_SECT) composed of $2 \times 20$ quadrilaterals (40 fibres)
Appear 3.1-a: Discretization of the section
Steel is modelled by 4 specific fibres (AFFE_FIBRE)
The coefficients and for damping are calculated using the following formula
and 2 are the first two own pulsations $(F=$
2
) and 1 and 2 is them
depreciation wished on the first two modes.
With $\boldsymbol{F}$
378
, Hz
$1=$
and $F$
,
1492 Hz
2 =
(see paragraph [§4]), for modal depreciation of
5\%, we find:
5
5
8.10-
$=$
and $=$
985

# For the calculation of the temporal answer, the step of selected time is 1/100ème of second. 

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### 3.2 Functionalities

tested
Orders

CREA_MAILLAGE
CREA_GROUP_MA

## AFFE_MODELE GRID

AFFE
ALL
"YES"
PHENOMENON
"MECHANICAL"
MODELING
"POU_D_EM"
DEFI_MATERIAU
"ELAS"

```
AFFE_MATERIAU
GROUP_MA
MATER
AFFE_CARA_ELEM BEAM
GROUP_MA
SECTION
ORIENTATION
GROUP_MA
```

CARA

```"ANGL_VRIL"
AFFE_SECT
GROUP_MA
MAILLAGE_SECT
"YES"
TOUT_SECT
AFFE_PONCT
GROUP_MA
"DIAMETER"
CARA
VALE
MODEL AFFE_CHAR_MECA
```

DDL_IMPO

GROUP_NO
CALC_MATR_ELEM OPTION
RIGI_MECA
MASS_MECA
AMOR_MECA
NUME_DDL MATR_RIGI

MATR_B

CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
CALC_CHAR_SEISME MONO_APPUI
"YES"
DIRECTION
DYNA_LINE_TRAN MATR_MASS

## MATR_RIGI

NEWMARK

## EXCIT

VECT_ASSE
FONC_MULT
INCREMENT

## CALC_ELEM OPTION

`SIEF_ELGA_DEPL

CALC_NO OPTION "REAC_NODA"

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4 Results
The curves of reaction according to time and arrow in the center according to time are presented on the figures [Figure 4-a] with [Figure 4-d].

300
200
100
0
-100
-200
Aster ref.
ASTER
-300
0
2
4
6
8
10
12
14
16
Time (S)
Appear 4-a: Reaction to the first supports according to time

# Appear 4-b: Detail of the reaction between 2 and 3 seconds 

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10
8
6
4
2
0
-2
-4
-6
-8

## ASTER ref.

ASTER
-10
0
5
10
15
Time (S)
Appear 4-c: Arrow in the center according to time

9
7
5
3
1
ASTER ref.
ASTER
-1
-3
2,5
2,55
2,6
2,65
2,7
2,75
2,8
Appear 4-d: Detail of the arrow between 2,5 and 2,8 seconds

Tests of results (TEST_RESU) are carried out for the first three Eigen frequencies. One also test the reaction on the first support and the arrow in the center is tested at the moments 1s (not 100) and 2s (not 200), then for the 2 first extremums of the curves, at the moments 2,68s (not 268) and
4,68s (not 468).
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Eigen frequency
ASTER ref.
ASTER
Relative error \%

## REACTION

ASTER ref.

```
ASTER
Relative error %
```


## 1,00 S

1,8878.104
1,8479.104
2,1

2,00 S<br>6,3393.104<br>6,2184.104<br>1,9

## 2,68 S

-2,3222.105
-2,2443.105
3,4

## 4,68 S

2,4692.105
2,3979.105
2,9

## ARROW ASTER

Ref.
ASTER
Relative error \%

## 1,00 S

-6,0694.10-4
-5,9846.10-4
1,4

2,00 S
-2,3507.10-3
-2,3362.10-3
0,6

## 2,68 S

8,5790.10-3
8,3929.10-3
2,2

## 4,68 S

-9,1084.10-3
-8,9530.10-3
1,7

5
Summary of the results
The results obtained using modeling beam multifibre (POU_D_EM) are in concord with the traditional modeling of right beam of Euler (POU_D_E) of Code_Aster. Handbook of Validation
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# Version 

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Seismic SDLL131- Calculation of a piping VVP. Comparison Aster-SYSPIPE Dates:
05/03/04
Author (S):
Y. PONS, D. NUNEZ*, L. VIVAN* Key
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Organization (S): EDF-R \& D/AMA, (*) CS IF

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Document: V2.02.131

SDLL131 - Seismic calculation of a piping VVP.
Comparison Aster-SYSPIPE

## Summary:

This test contributes to the validation of the operator of spectral analysis COMB_SISM_MODAL of Code_Aster.
The studied structure is a line of piping VVP subjected to an excitation multi supports. It is with a

## grid in

elements of right beam $P O U_{-} D_{-} T$ and curve $P O U_{-} C_{-} T$.
The numerical reference solution is given by code SYSPIPE of FRAMATOME.
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1
Problem of reference

### 1.1 Geometry

## - Isométrie:

## C1

N10 N12
N4
SP14
N3
C2
N13
N49
DAB
N1
M55
DAB
N17
PRICKING STEAM GENERATOR
M61
N48
SP13
N19
M60
N47
M54

## CROSSING BR

Studied line VVP presents a developed length of 28,4m.
The supports weight (SP12, SP13 and SP14) and the blocking device car (DAB) are not taken in count in the seismic analysis.

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## Co-ordinates of the points (m):

NODES<br>COOR X (m)<br>COOR Y (m)<br>COOR Z (m)<br>N3<br>-0,91514 10,46 30,861<br>bend C1<br>N4<br>$-1,719411,368$<br>32,081<br>center curve<br>-1,7194<br>11,368<br>30,861<br>N12<br>-4,5175 14,526<br>32,069<br>bend C2<br>N13<br>-5,3224 15,435<br>30,849<br>center curve<br>-4,5175<br>14,526<br>30,849<br>N43<br>-5,3224 15,435<br>15,409<br>bend C3<br>N44<br>$-5,718416,583$<br>14,189<br>center curve<br>-5,7184<br>16,583<br>15,409

-0,91514 10,46 29,944
crossing Br
N45
$-5,881917,058$
14,187
support SP14
N10
-3,6221 13,515
32,076
N17
$-5,322415,435$
29
N46
-4,7079 15,646
29
DAB+attache
N47
-4,6449 15,569
29
N48
$-5,936915,223$
29
N49
-5,9386 15,123
29
N19
-5,3224 15,435
28
support SP13
N50
-5,4169 15,402
28
N51
$-5,227815,467$
28
N39
$-5,322415,435$
17,4
support SP12
N52
-5,4169 15,402

## - Caractéristiques of the sections:

Rext (m)<br>$e p$ (m)<br>GROUP_MA<br>ring Steam Generator<br>0,408 0,038 GMAT04<br>current right portion<br>0,406 0,03 GMAT02<br>attach DAB<br>0,406 0,03 GMAT06<br>anchoring support<br>0,406 0,4 GMAT05<br>elbows C1, C2 and C3<br>0,406 0,032 GMAT03

Coefficient of flexibility RCC-M of the elbows: $k=6,43$.
Radius of curvature: $R c=1,22 \mathrm{~m}$.

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1.2Material properties
materials $\boldsymbol{E}$
(Pa)
(Kg/m3)
( ${ }^{\circ} \mathrm{C}-1$ )
GROUP_MA
MAT02
2,04E+11 8100,2 0,3
1,092E-05 GMAT02
MAT03
2,04E+11 8098 0,3
1,092E-05
GMAT03
MAT04
2,04E+11 8053,6 0,3
1,092E-05 GMAT04
MAT05
2,04E+1100,3
1,092E-05
GMAT05
MAT06
2,04E+11 1774,4 0,3
1,092E-05 GMAT06
1.3
Boundary conditions and loadings

## Boundary conditions

Embedding pricking Steam Generator and crossed Br: $D X=D Y=D Z=D R X=D R Y=D R Z=0$ with the nodes N1 and N45

## Seismic loading Normally Acceptable Seism (1/2 seism of dimensioning)

- Multiple Excitation (several spectra by direction):
node N1 (pricking Steam Generator) in directions $X$ and $Y$
frequency (Hz)
0,2.0,5.1,7 2,3 3.3,3.4,5 691020100
acceleration (G) 0,031 0,1 0,53
0,65
0,73
0,69
0,52
0,39
0,29 0,284 0,23 0,23
node $N 45$ (crossed $B r$ ) in directions $X$ and $Y$
frequency (Hz)
1.1,4 2,65

4,8 102025100
acceleration (G) 0,13 0,43 1,48
0,86
0,38
0,29
0,22
0,22
nodes N1 and N45 (pricking Steam Generator and crossed Br) in direction Z
frequency ( Hz )
0,2 0,3 0,5
1
234571020
25
100
acceleration (G) 0,0155 0,025 0,044 0,1.0,2 0,25 0,28 0,28 0,22 0,15 0,12 0,1 0,1

## - Seismic Differentials Displacement

$u=0,004 m$ with the node $N 45$ (crossed $B r$ ) according to the local direction $U$.
$U$ results from axis $X$ by a rotation of angle $=109,02^{\circ}$ around axis $Z$.
This loading amounts applying ucos displacements and usin along axes $X$ and $Y$.
Several cases of displacements are mentioned in the note of calculation FRAMATOME [bib1] but only one is retained in this test.

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## 2 <br> Reference solution

## 2.1 <br> Method of calculation used for the reference solution

The reference solution is obtained by:

- software SYSPIPE of FRAMATOME,
- Code_Aster: test of nonregression based on an equivalent command set.


## 2.2 <br> Results of reference

The line of piping was the subject of a complete lawful study. Results of the analysis seismic are mentioned in the note of the manufacturer [bib1].

## 2.3 <br> Uncertainty on the solution

Numerical solution, obtained with identical data and comparable elements.

### 2.4 References <br> bibliographical

Note calculation FRAMATOME ITMC/DC/414: Lines interior vapor Br of the sections Fessenheim.
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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

The model comprises 12 elements $P O U_{-} C_{-} T$ (4 by elbow) and 49 elements $P O U_{-} D \_T$.

## 3.2 <br> Characteristics of the grid

A number of nodes: 62
A number of meshs and type: 61 SEG2

## -Connectivité:

```
net
node 1
node 2
net
node 1
node 2
M1 N4 N5 M32
```


## N36

N37
M2 N5 N6 M33
N37

## N38

M3 N6 N7 M34
N38
N39
M4 N7 N8 M35
N39
N40
M5 N8 N9 M36
N40
N41
M6 N9
N10
M37
N41
N42
M7 N10 N11 M38 N42

## N43

M8 N11 N12 M39 N44
N45
M9 N13
N14 M40 N3
N55
M10 N14 N15 M41 N55
N56
M11 N15 N16 M42 N56
N57
M12 N16 N17 M43 N57
N4
M13 N17 N18 M44 N12
N59
M14 N18 N19 M45 N59
N60
M15 N19 N20 M46 N60
N61
M16 N20 N21 M47 N61
N13
M17 N21 N22 M48 N43
N63
M18 N22 N23 M49 N63

```
N64
M19 N23 N24 M50 N64
N65
M20 N24 N25 M51 N65
N44
M21 N25 N26 M52 N1
N2
M22 N26 N27 M53 N2
N3
M23 N27 N28 M54 N46
N47
M24 N28 N29 M55 N48
N49
M25 N29 N30 M56 N19
N50
M26 N30 N31 M57 N19
N51
M27 N31 N32 M58 N39
N52
M28 N32 N33 M59 N39
N53
M29 N33 N34 M60 N17
N46
M30 N34 N35 M61 N17
N48
M31 N35 N36
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```

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## - Groupe of mesh:

GROUP_MA Nb nets
list meshs contained in the group
M1 m2 m3 M4 M5
M6 M7 M8 M9 M10
M11 M12 M13 M14 M15
M16 M17 M18 M19 M20
GMAT02
39
M21 M22 M23 M24 M25
M26 M27 M28 M29 M30
M31 M32 M33 M34 M35
M36 M37 M38 M39
M40 M41 M42 M43 M44
GMAT03
12M45 M46 M47 M48 M49M50 M51
GMAT04

2

M52 M53
M54 M55 M56 M57 M58
GMAT05
6
M59
GMAT06
2
M60 M61

3.3 Functionalities tested<br>\section*{Orders}

"MECHANICAL" AFFE_MODELE "POU_C_T"<br>AFFE_CARA_ELEM BEAM<br>GROUP_MA<br>RING<br>DEFI_ARC<br>GROUP_MA<br>CENTER<br>GROUP_MA<br>COEF_FLEX<br>AFFE_CHAR_MECA DDL_IMPO<br>GROUP_NO<br>DEFI_MATERIAU ELAS

AFFE_MATERIAU

MACRO_MATR_ASSE

POST_ELEM MASS_INER

MACRO_MODE_MECA CALC_FREQ

NORM_MODE

## EXCIT

GROUP_NO

COMB_MODE

## $I M P R \_R E S U$

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## 4

Results of modeling $A$

### 4.1 Frequencies <br> clean

mode<br>Code_Aster<br>Reference \%<br>variation<br>1 5,0791<br>5,0793<br>-0,004<br>2 8,7391

## 4.2 <br> Spectral analysis multi supports

### 4.2.1 Primary component (inertial effect)

## - Method of calculation

- Modal Base comprising the 4 preceding modes,
- Reduced Damping of 5\% for all the modes,
- Taking into account of the static contribution of the neglected clean modes ( $>25 \mathrm{~Hz}$ ),
- Combination of the modal answers according to method CQC:
$C O M B \_M O D E=\_F\left(T Y P E={ }^{\prime} C Q C^{\prime}\right)$,
- Quadratic Office plurality of the answers by supports:
$C O M B \_M U L T \_A P P U I=\_F(T Y P E=' Q U A D ')$,
- Quadratic Office plurality of the directional answers:
$C O M B \_D I R E C T I O N=\_F(T Y P E=' Q U A D ')$,

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## - Référence SYSPIPE

Generalized efforts: $E F G E \_E L N O \_D E P L$ (N.m)

NET NODE CMP<br>Code_Aster<br>Reference \%<br>variation<br>M52 N1 MT 132314,352 131471 0,641<br>MFY<br>34671,9174 34622 0,144<br>MFZ<br>34062,3639 33951 0,328<br>M40 N3 MT 132347,543 1314660,671<br>MFY<br>13271,7861 13716-3,239<br>MFZ<br>12698,7018 126790,155<br>M43 N4 MT 25427,8116 25624 -0,766<br>\section*{MFY}<br>96610,7623 959150,725<br>MFZ<br>8474,33445 8335 1,672<br>M5 N8 MT 24995,7612 24868 0,514<br>\section*{MFY}<br>12762,0643 12564 1,576<br>MFZ<br>42196,6397 42006 0,454<br>M44 N12 MT 25027,5711 24931 0,387<br>MFY<br>15984,253 15811 1,096<br>MFZ<br>17688,0827 17390 1,714<br>M47 N13 MT 40765,3048 40491 0,677

MFY
3066,98979 3201,1
-4,190
MFZ
6599,21353 6521 1,199
M24 N28 MT 41227,5405 40943 0,695
MFY
37631,0924 37086 1,470
MFZ
45605,1023 44926 1,512
M48 N43 MT 41265,0496 41104 0,392
MFY ..... 51189,9272 507260,915
MFZ
13728,1788 136280,735
M51 N44 MT 67834,6989 67395 0,652
MFY
27343,0908 27167 0,648
MFZ
33365,2076 32890 1,445
M39 N45 MT 67893,3287 673960,738
MFY
38018,4979 37285 1,967
MFZ
23350,1075 23551-0,853
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## Titrate:

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[Figure 4.2.1-a] the evolution of a total size of equivalent moment gives
)
SYSPIPE
$T$
éq

```
0
O
5
10
15
2 0
25
30
ABSC_CURV (m)
```

Appear 4.2.1-a: Primary component - Field of moment are equivalent
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:

The reference is obtained by calculating the total response with differential displacements null.

## Generalized efforts: EFGE_ELNO_DEPL (N.m)

NET NODE CMP Code_Aster<br>Reference \%<br>variation<br>M52 N1 MT<br>132314,352 132314,352 0,000<br>MFY<br>34671,9174 34671,9174 0,000<br>MFZ<br>34062,3639 34062,3639 0,000<br>M40 N3 MT<br>132347,543 132347,543 0,000<br>MFY<br>13271,7861 13271,7861 0,000<br>MFZ<br>12698,7018 12698,7018 0,000<br>M43 N4 MT 25427,8116 25427,8116 0,000<br>MFY<br>96610,7623 96610,7623 0,000<br>MFZ<br>8474,33445 8474,33445<br>0,000<br>M5 N8 MT 24995,7612 24995,7612 0,000<br>\section*{MFY}<br>12762,0643 12762,0643 0,000<br>MFZ<br>42196,6397 42196,6397 0,000<br>M44 N12 MT 25027,5711 25027,5711 0,000

MFY
15984,253 15984,253 0,000
MFZ
17688,0827 17688,0827 0,000
M47 N13 MT 40765,3048 40765,3048 0,000
MFY
3066,98979 3066,98979
0,000
MFZ
6599,21353 6599,21353
0,000
M24 N28 MT 41227,5405 41227,5405 0,000
MFY
37631,0924 37631,0924 0,000
MFZ45605,1023 45605,1023 0,000
M48 N43 MT 41265,0496 41265,0496 0,000
MFY
51189,9272 51189,9272 0,000
MFZ
13728,1788 13728,1788 0,000
M51 N44 MT 67834,6989 67834,6989 0,000
MFY
27343,0908 27343,0908 0,000
MFZ
33365,2076 33365,2076 0,000
M39 N45 MT 67893,3287 67893,3287 0,000
MFY
38018,4979 38018,4979 0,000
MFZ
23350,1075 23350,1075 0,000

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### 4.2.2 Secondary component (static effect of differential displacements)

## - Method of calculation

- Linear Office plurality of the loadings in displacement imposed of crossing Br (node N45) according to DX and DY:

DEPL_MULT_APPUI= (
_F (NOM_CAS=' DDS Br U local following DX',
NUME_CAS $=1$,
MODE_STAT=MSTAT,
NOEUD=N45,
DX=ucos,),
_F (NOM_CAS=' DDS Br U local following DY',
NUME_CAS=2,
MODE_STAT=MSTAT,
$N O E U D=N 45$,
DY=usin,),),
COMB_DEPL_APPUI=_F $($
TOUT=' OUI',
TYPE=' LINE', ,
with $u=0,004 \mathrm{~mm}$ and $=109,02^{\circ}$
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## - Référence SYSPIPE

Generalized efforts: EFGE_ELNO_DEPL (N.m)

## NET NODE CMP Code_Aster

Reference \%
variation
M52 N1 MT
-3514,643-3519-0,124
MFY
-8718,626-8731-0,142
MFZ
3839,959 3847 -0,183
M40 N3 MT
-3498,378-3519-0,586
MFY*
4627,678 4620 0,166
MFZ*
-6559,864-6572-0,185
M43 N4 MT*
$-5466,298-5482-0,286$
MFY
-2661,552-2645
0,626
MFZ
707,119 704 0,443
M5 N8 MT

## MFY

-7813,834-7819-0,066

## MFZ

-1373,902-1376-0,153
M44 N12 MT -5452,502-5459 -0,119
MFY*
232,577 221 5,239
MFZ*
16264,520 16280 -0,095
M47 N13 MT*
-1080,825-1091 -0,933

## MFY

-4608,238-4612-0,082

## MFZ

17959,849 17978 -0,101
M24 N28 MT -1090,004-1091-0,091
MFY
616,317 $617-0,111$
MFZ
-581,559-581 0,096
M48 N43 MT -1087,843-1091-0,289

## MFY*

1008,866 1008 0,086

## MFZ*

18334,269 18351 -0,091
M51 N44 MT*
$-1295,762-1299-0,249$

## MFY

-803,728-799 0,592
MFZ
16745,356 16759 -0,081
M39 N45 MT*

# Code_Aster ${ }^{\circledR}$ 

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[Figure 4.2.2-a] the evolution of a total size of equivalent moment gives
2
2
2
$M T+M r . F Y+M r . F Z$ according to the curvilinear $X$-coordinate of the analyzed section. A variation is noted
maximum of 0,93\% between SYSPIPE and Code_Aster with the N4 node of the M1 mesh.

```
) 16000
```

Code_Aster
.$m$

```
14000
```

$T$
in 12000
I
valley 10000
$U$
8000

```
T
éq
```

E
$N$
6000
m
O
4000
m
2000
0
0
5
10
15
20
25
30
ABSC_CURV (m)

## Appear 4.2.2-a: secondary component - field of moment are equivalent

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- Référence Code_Aster

The solution is obtained with operator MECA_STATIQUE by imposing on the N45 node them displacements $D X=u c o s$ and $D Y=u s i n$.

Generalized efforts: EFGE_ELNO_DEPL (N.m)
NET NODE
CMP Code_Aster
Reference \%
variation
M52 N1 MT -3514,643-3514,643
0,000
MFY
-8718,626-8718,626
0,000
MFZ
3839,959 3839,959 0,000
M40 N3 MT -3498,378 -3498,378
0,000
MFY
4627,678 4627,678 0,000

```
MFZ
-6559,864 -6559,864
0,000
M43 N4 MT -5466,298 -5466,298
0,000
MFY
-2661,552 -2661,552
0,000
MFZ
707,119 707,119 0,000
M5 N8 MT -5451,862 -5451,862
0,000
MFY
-7813,834 -7813,834
0,000
MFZ
-1373,902 -1373,902
0,000
M44 N12 MT -5452,502 -5452,502
0,000
MFY
232,577 232,577 0,000
MFZ
16264,520 16264,520
0,000
M47 N13 MT -1080,825 -1080,825
0,000
MFY
-4608,238-4608,238
0,000
MFZ
17959,849 17959,849
0,000
M24 N28 MT -1090,004 -1090,004
0,000
MFY
616,317 616,317 0,000
MFZ
-581,559 -581,559 0,000
M48 N43 MT -1087,843-1087,843
0,000
MFY
1008,866 1008,866 0,000
```

```
MFZ
18334,269 18334,269
0,000
M51 N44 MT -1295,762 -1295,762
0,000
MFY
-803,728 -803,728 0,000
MFZ
16745,356 16745,356
0,000
M39 N45 MT -1298,075 -1298,075
0,000
MFY
14896,226 14896,226
0,000
MFZ
680,911 680,911 0,000
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```

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## 5

Summary of the results
The answers obtained with the two codes are very close. Variations surplus not $1 \%$ on primary and secondary components. More important differences are observed in elbows. The latter are modelled with a curved element in SYSPIPE and 4 elements POU_C_T in Code_Aster.

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Titrate:
SDLL132-Clean modes of a frame

## Date:

15/04/03
Author (S):
J-L. Key FLEJOU
:
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Organization (S): EDF-R \& D /TESE

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Document: V2.02.132

SDLL132-Clean modes of a frame in multifibre beams

## Summary:

This test relates to the validation of option MASS_INER, as well as calculation of the clean modes of the frame
when the model contains POU_D_TGM (multifibre beams). The results of the reference solution are obtained by making the same study but with a model of beams based on POU_D_E.

This test makes it possible to validate, by making a modal analysis of the structure:

- linear finite elements of type $P O U_{-} D_{-} T G M$.
$\cdot$ results of the orders: POST_ELEM, NORM_MODE, EXTR_MODE.
- results of MACRO_MODE_MECA.

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Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SDLL132-Clean modes of a frame

Date:
15/04/03
Author (S):
J-L. Key FLEJOU
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## 1

Problem of reference

### 1.1 Geometry

The case test is a metal frame made up of beams and posts.

Co-ordinates of the principal nodes of the grid:

|  |  |
| :---: | :---: |
| Coord. XCoord. Y |  |
|  | Coord. Z |
|  | Not |
|  | (in m) |
|  | (in m) |
|  | (in m) |
|  | To 2.0 |
|  | 2.5 |
|  | 0.0 |
|  | B 4.0 |
|  | 0.0 |
|  | 0.0 |
|  | C 2.0 |
|  | -2.5 |
|  | 0.0 |
|  | D 0.0 |
|  | 0.0 |
|  | 0.0 |
|  | E 2.0 |
|  | 2.5 |
|  | 3.0 |
|  | F 4.0 |
|  | 0.0 |
|  | 3.0 |
|  | G 2.0 |
|  | -2.5 |
|  | 3.0 |
|  | H 0.0 |
|  | 0.0 |
|  | 3.0 |
|  | I 2.0 |
|  | 0.03 .0 |

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## 1.2

Mechanical characteristics of the beams
The beams of the case test are standard sections of the steel construction. Units of theirs mechanical characteristics are homogeneous with [m].

HEA200 IPE220 IPE160 HEA140 IPE120

## Beams

BF, DH
HF
I.E.(INTERNAL EXCITATION), CG

AE
FG, GH
With 5.39E03
3.34E03
2.01E03 3.14E03 1.32E03

IY 3.69E05
2.77E05
8.70E06 1.03E05 3.18E06

IZ 1.34E05
2.05E06
6.83E07 3.89E06 2.77E07

AY
1.4749941 .7898651 .7928841 .4640321 .774392

AZ
4.4660382 .6337542 .5861994 .4641732 .590182

JX 1.97E07
8.66E08
3.37E08 7.76E08 1.63E08

JG 1.06E07

### 2.23E08

3.89E09 1.47E08 8.73E10

Sizes EY, EZ, IYR2, IZR2 are null for all the beams.

## 1.3 <br> Properties of material

Only one material is used:
Young 2.10e+11
Pa
Rho 7.85e+03
kg/m3

## 1.4 <br> Boundary conditions

The points $A, B, C, D$ are embedded.

$D X=0$<br>$D Y=0$<br>$D Z=0$<br>$D R X=0$<br>$D R Y=0$<br>$D R Z=0$

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## 2 <br> Reference solution

## 2.1

Method of calculation used for the reference solution
The values of the reference solution are obtained with the frame, produced with a model with base POU_D_E.

## 2.2

Results of reference
Values obtained by order POST_ELEM, with key word MASS_INER:
Sizes Value
MASS
$5.85759 E+02$
CDG_X
$2.00000 E+00$
CDG_Z
2.03968E+00

IX_PRIN_G
$1.56562 E+03$
IY_PRIN_G
$1.81822 E+03$
IZ_PRIN_G
$2.23486 E+03$
The table below gives the clean modes calculated with a model of POU_D_E. Modes are filtered by order EXTR_MODE with criterion MASS_EFFE_UN and a threshold of 5.0E-04.

```
NUME FREQUENCY MASS_EFFE_UN CUMUL_DX MASS_EFFE_UN CUMUL_DY
MASS_EFFE_UN CUMUL_DZ
MODE
DX
DY
DZ
1 1.00E+01 2.41E01 2.41E01
```

2 1.24E+01 4.33E01 6.74E01
3.99E24 4.01E24
2.64E27 2.65E27

3 1.31E+01 3.84E24 6.74E01
5.29E01 5.29E01
4.13E04 4.13E04

4 1.75E+01 7.73E04 6.74E01
4.54E26 5.29E01
2.74E28 4.13E04

5 1.91E+01 7.92E02 7.54E01
1.88E27 5.29E01
4.25E28 4.13E04

6 2.24E+01 6.29E27 7.54E01
1.38E01 6.67E01
2.22E04 6.35E04

7 2.69E+01 1.21E29 7.54E01
6.00E02 7.27E01
1.50E07 6.35E04

10 3.36E +01 2.42E30 7.54E01
6.37E04 7.27E01
6.26E06 6.41E04

13 3.53E+01 1.67E03 7.55E01
7.43E30 7.27E01
1.13E29 6.41E04
$143.70 E+01$ 1.21E02 7.68E01
9.09E30 7.27E01
2.84E33 6.41E04

## 2.3 <br> Uncertainty on the solution <br> Without object. <br> Handbook of Validation <br> V2.02 booklet: Linear dynamics of the beams <br> HR-17/02/019/A

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling and the grid

The model is composed of POU_D_TGM (multifibre beams). All the sections, are in form of "I" and are described with 30 fibres: 1 in the sole and web thickness, 10 in the width of soles and 10 in the height of the heart.

### 3.2 Functionalities

tested
The case test comprises three stages:

- the first calculates the structural features with the order post_elem,
$\cdot$ the second stage consists in calculating the clean modes of the structures using orders calc_matr_elem, nume_ddl, asse_matrice, mode_iter_simult and with to normalize with the result of the concept post_elem, - the third stage calculates the clean modes of the structure with the macro-order MACRO_MODE_MECA by normalizing the modes with the result of concept POST_ELEM. Orders

AFFE_CARA_ELEM BEAM

AFFE_FIBRE<br>POST_ELEM MASS_INER

CALC_MATR_ELEM OPTION
"RIGI_MECA"

## 3.3 <br> Sizes tested and results

The table below summarizes the results obtained by orders POST_ELEM, with the key word MASS_INER, for model POU_D_TGM and compares them with the values of reference obtained with one model of POU_D_E.

## Sizes Values Values

Error
References
POU_D_TGM
Relative
MASS
$5.8576 E+025.8576 E+02$ 7.04E07
CDG_X
$2.0000 E+002.0000 E+000.00 E+00$
CDG_Z
$2.0397 E+002.0397 E+002.16 E 06$
IX_PRIN_G
$1.5656 E+031.5656 E+03$ 1.11E06
IY_PRIN_G
1.8182E+03 1.8182E+03 1.80E07

IZ_PRIN_G
2.2349E+03 2.2349E+03 4.72E07

In this test, the authorized maximum relative error is fixed at 2.0E-05.
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The table below gives the clean modes calculated with a model of POU_D_TGM. Modes are filtered by order EXTR_MODE with criterion MASS_EFFE_UN and a threshold of 5.0E-04.
The maximum relative error authorized on the calculation of the frequencies is fixed at $2.0 E-02$. The absolute error maximum authorized on the calculation of the MASS_EFFE_UN is fixed at 1.0E-02.

|  | NUME FREQUENCY MASS_EFFE_UN CUMUL_DX MASS_EFFE_UN CUMUL_DY MASS_EFFE_UN CUMUL_DZ |
| :---: | :---: |
|  | MODE |
|  | DX |
|  | $D Y$ |
|  | DZ |
|  | $11.00 E+012.45 E 012.45 E 01$ |
|  | 1.38E27 1.38E27 |
|  | 7.12E31 7.12E31 |
|  | 2 1.23E+01 4.30E01 6.75E01 |
|  | 1.28E25 1.30E25 |
|  | 7.59E29 7.66E29 |
|  | 3 1.29E+01 1.10E25 6.75E01 |
|  | 5.35E01 5.35E01 |
|  | 4.02E04 4.02E04 |
|  | $41.73 E+011.90 E 036.77 E 01$ |
|  | 4.08E28 5.35E01 |
|  | 5.99E31 4.02E04 |
|  | 5 1.91E+01 7.67E02 7.54E01 |
|  | 2.18E27 5.35E01 |
|  | 3.36E30 4.02E04 |

6 2.21E+01 7.04E28 7.54E01<br>1.40E01 6.75E01<br>2.28E04 6.30E04<br>7 2.68E+01 7.31E28 7.54E01<br>5.57E02 7.31E01<br>1.34E07 6.30E04<br>8 3.49E+01 2.32E03 7.56E01<br>5.33E29 7.31E01<br>2.56E31 6.30E04<br>9 3.76E+01 1.72E02 7.73E01<br>1.77E29 7.31E01<br>8.92E34 6.30E04

The table below compares the results obtained with model POU_D_TGM and the values of reference obtained with a model of $P O U_{-} D_{\_} E$ (NUME_MODE 1, 2, 3, 5, 6, 7), for which there is not no particular remarks.

NUME<br>FREQUENCY MASS_EFFE_UN<br>MASS_EFFE_UN MASS_EFFE_UN_DZ<br>MODE<br>DX<br>DY<br>Values references<br>1<br>10.0386 2.4062E01<br>1.6855E26 7.0440E30<br>POU_D_TGM<br>110.0299<br>2.4507E01<br>1.3770E27<br>7.1158E31<br>Relative error<br>8.7E04<br>1.8E02

$\qquad$

Absolute error
8.7E03
4.4E03

# Values references 

 2$12.36314 .3310 E 01$
3.9884E24 2.6399E27

POU_D_TGM
212.2722
4.3008E01
1.2845E25
7.5937E29

Relative error
7.4E03
7.0E03

Absolute error
9.1E02
3.0E03

## 

Values references
3
$13.06133 .8389 E 24$
5.2880E01 4.1267E04

POU_D_TGM
312.8877
1.1018E25
5.3471E01
4.0172E04

Relative error
1.3E02
1.1E02
2.7E02

Absolute error
1.7E01 --- 5.9E03
1.1E05

Values references
5
$19.14217 .9213 E 02$

# 1.8759E27 4.2482E28 

POU_D_TGM
519.0752
7.6706E02
2.1807E27
3.3632E30

Relative error

```
\(3.5 E 03\)
3.3E02
```

$\qquad$
---
Absolute error
$6.7 E 02$
2.5E03
---
Values references 6
$22.3596 .2877 E 27$
1.3777E01
2.2223E04

POU_D_TGM
622.1023
7.0371E28
1.4041E01
2.2790E04

Relative error
$1.2 E 02$
---
$1.9 E 02$
2.5E02

Absolute error

2.6E01 --- 2.6E03<br>5.7E06<br>Values references<br>7<br>26.9214 1.2099E29<br>6.0029E02 1.5014E07<br>POU_D_TGM<br>726.7993

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The table below compares the results obtained with model POU_D_TGM and the values of reference obtained with a model of $P O U \_D \_E$ (NUME_MODE 4, 8, 9) for which some remarks are necessary.

## NUME <br> FREQUENCY MASS_EFFE_UN MASS_EFFE_UN MASS_EFFE_UN_DZ <br> MODE

```
DX
DY
Values references
4
17.5316 7.7262E04 4.5440E26 2.7423E28
POU_D_TGM
417.3126
1.9020E03
4.0831E28
5.9912E31
Relative error
1.3E02
5.9E01
Absolute error
2.2E01
1.1E03
Values references
13
35.2798 1.6712E03 7.4262E30 1.1337E29
POU_D_TGM
834.9301
2.3155E03
5.3345E29
2.5636E31
Relative error
1.0E02
2.8E01
Absolute error
3.5E01
6.4E04
Values references
14
``` MASS_EFFE_UN_DX (1.9E-03 to be compared with 7.7E-04), and an absolute error of 1.1E-03. The value of
MASS_EFFE_UN_DX indicates a weak participation of this mode to the dynamic response of structure. The two analyses are thus in agreement, there is well a mode in the vicinity of 17.5 Hz , and its contribution is very weak with respect to the dynamic response of the structure.

After analysis of the results, calculated mode with 34.9 Hz (NUME_MODE 8 for model POU_D_TGM) corresponds to the mode of reference to 35.3 Hz ( \(N U M E \_M O D E 13\) ). On the other hand there is a relative error
from \(28 \%\) on the MASS_EFFE_UN_DX (2.3E-03 to be compared with 1.7E-03) and an absolute error of 6.4E-04.
The value of MASS_EFFE_UN_DX indicates a weak participation of this mode to the dynamic response structure, the two analyses are thus in agreement.

After analysis of the results, calculated mode with 37.6 Hz (NUME_MODE 9 for model POU_D_TGM) corresponds to the mode of reference to 37.0 Hz ( \(N U M E \_M O D E 14\) ). On the other hand there is a relative error
from \(30 \%\) on the MASS_EFFE_UN_DX (1.7E-02 to be compared with 1.2E-02) and an absolute error of 5.1E-03.
The value of MASS_EFFE_UN_DX indicates a weak participation of this mode to the dynamic response structure, the two analyses are thus in agreement.

Mode, of the reference solution, which corresponds to the frequency of 33.6 Hz (NUME_MODE 10) is not
not found by modeling in POU_D_TGM. The MASS_EFFE_UN_DY, corresponding to this mode is of \(6.37 E-04\), lower than \(0.1 \%\). This value indicates a weak participation of this mode to dynamic response of the structure. The fact that one does not find this mode, with a modeling \(P O U \_D \_T G M\), thus will not influence the dynamic analysis which one could make thereafter.

\subsection*{3.4 Parameters \\ of execution}

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Machine:
IRIX64
Obstruction memory: 16Mo
Time CPU To use
: 9.0sec

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\section*{4}

\section*{Summary of the results}

The results of reference are obtained with a model of \(P O U \_D \_E\).
The analyses carried out with a modeling in \(P O U_{\_} D_{-} T G M\) are in agreement with the solution of reference.

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Date:
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Author (S):
A. ADOBES, L. SALMONA Key
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Organization (S): EDF-R \& D /MFTT, CS IF

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V2.02.134 document

SDLL134 - Method of Connors for the analysis of vibratory behavior of the tubes of Steam Generator

\section*{Summary:}

This case test aims at the validation of the establishment in Code_Aster of the method of Connors, in so much
that method of analysis of the vibratory behavior of the tubes of Steam Generator.
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1
Problem of reference

\subsection*{1.1 Geometry}
```

K1
K2

```
J1
J2

\section*{With}

E
Co-ordinates of the points:
WITH B C
\(D\)
\(E\)
\(X-0.2944\)
-0.2944
0.0 .2944
0.2944
\(y 0\).
9.693
9.98749 .6930.

F1 G1 H1
II
```

J1
X -0.2944
-0.2944
-0.2944
-0.2944
-0.2944
y 1.068 2.136
3.204 4.272 5.34
K1 L1 M1
X -0.2944
-0.2944
-0.2944
y 6.408 7.476
8.544
F2 G2 H2
I2
J2
X 0.2944
0 . 2 9 4 4
0 . 2 9 4 4
0 . 2 9 4 4
0.2944
y 1.068 2.136
3.204 4.272 5.34
K2 L2 m2
X 0.2944
0 . 2 9 4 4
0 . 2 9 4 4
y 6.408 7.476
8.544
1.2
Properties of materials and characteristic of the tube

```

\section*{Elastic properties}
```

E=202000 MPa

```
\(=0.3\)
\(=8330 \mathrm{~kg} / \mathrm{m} 3\)

The tube is hollow. Its external ray is worth 9.525 mm and its thickness 1.09 Misters.
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\section*{1.3}

Boundary conditions

\section*{Boundary conditions:}
- on the points \(A, B, D\) and \(E\) : embedding ( \(D X=D Y=D Z=R X=R Y=R Z=0)\).
\(\cdot\) on the points F1, G1, H1, I1, J1, K1, L1, M1, F2, G2, H2, I2, J2, K2, L2 and m2: support ( \(D X=D Y=D Z=0\) ).

\section*{Fluid loading:}

The internal fluid and the external fluid are distinguished. Each one has a dependent density curvilinear \(\boldsymbol{X}\)-coordinate along the tube. Moreover, for the external fluid, the profile speed inter-tubes transverse with the tube in the plan of the tube is provided.

\section*{1.4 \\ Type of network and characteristic of the model of Connors}

The network is a square step of step reduces 1.439895. The constant of Connors is supposed to be ranging between 3.0 and 5.0 with 3 values équiréparties in this interval, that is to say 3.0, 4.0 and 5.0.
Damping necessary to the application of the method of Connors is damping in fluid at rest. It is taken equal to \(\mathbf{0 . 6 4 \%}\).

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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
One is interested in the mode 21 which is more penalizing on the vibratory level. Its frequency (estimated with assistance MODE_ITER_SIMULT with 61.53761 Hz ) is regarded as a data of problem. The values of interest for the case-test are, on the one hand effective speed in
\(V\), and
in addition the critical engine failure speed Cn
\(V\). Their values are given by the equations (see nomenclature of the variables in [R4.07.04]):

2
2
\(S(S) V(S) N(S)\)
\(d s\)
Cn
V
\(=\)
tube
\(m N\)
Nex
\[
F
\]
\[
2
\]
\[
N
\]

By using the spreadsheet Excel, it is possible to calculate these values for the different ones constants of Connors requested.

\section*{2.2}

Results of reference
The value of reference is the sum of the various values for each constant of Connors. One thus has for effective speed and the critical engine failure speed of the respective values of
3.0039 and 6.9156 .

\section*{2.3}

Uncertainty on the solution
It is about an semi-analytical solution. Only uncertainty relates to the frequency of mode 21.

\subsection*{2.4 References \\ bibliographical}
[1]
T. KESTENS, Mr. LAINET: Coupling fluid-structure for the tubular structures and them coaxial hulls, document [R7.07.04].
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Elements 1D POU_D_T (SEG2)}

\author{
With \\ E \\ \section*{3.2} \\ Characteristics of the grid \\ A number of nodes: 311 \\ A number of meshs and types: 310 SEG2 \\ 3.3 Functionalities \\ tested \\ Orders \\ DEFI_FLUI_STRU CSTE_CONNORS \\ NB_CONNORS \\ CALC_FLUI_STRU AMOR_REDUIT_CONN \\ \section*{4} \\ Results of modeling \(A\)
}
4.1 Valuestested
Size Reference
Aster \%
difference
Summon Cn
V
6.91560370782016 .9156037078201 1.03E-13
Summon of in
V
3.00394899782013 .0039544634875 1.82E-04
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\section*{5}

Summary of the results
The error made by Code_Aster on speeds (critical and effective) is with more than \(1.82104 \%\) it who is acceptable and valid the establishment of the method of Connors in the software.
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\section*{Titrate:}

SDLL311-Transitory dynamic response of a beam in traction

\section*{Date:}

13/06/03
Author (S):
E. BOYERE, T. QUESNEL Key
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Organization (S): EDF-R \& D /AMA, IRCN

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
V2.02.311 document

SDLL311-Transitory dynamic response of one beam in traction under imposed displacement

\section*{Summary:}

This problem-test corresponds to a linear transitory analysis of a bar requested in traction by application
of a displacement imposed at an end, the other end being embedded. Function displacement of time is of type "Heaviside" imposed as from the initial moment.

The results obtained in the middle of the beam for a modeling with four elements are compared with analytical solution of the problem discretized by four elements by not taking into account the peaks instantaneous speed and of acceleration at the initial moment on the level of the end where displacement is imposed.

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1
Problem of reference

\subsection*{1.1 Geometry}
\(y, v\)
With
B
C
/2
/2
\(X, U\)
\(U(C)=F(T) . u\)
\(\boldsymbol{R}\)
F
\(R=0,05 m\)
\(=1 \mathrm{~m}\)
1
\(T\)

\section*{1.2}

Material properties
E \(=98\) 696,044 MPa
\(=0\)
\(=3.106 \mathrm{~kg} / \mathrm{m} 3\)

\title{
Damping proportional of Rayleigh: \(C=K\)
}
\(+\mu M\), \(=\)
5104
, \(\mu=5\)

\section*{1.3}

Boundary conditions and loadings
Displacement imposed at the end \(C: U(C)=U F(T)\) with \(U=-\)
\(103 m\) and \(F(T)\) evolution in function
time of the Heaviside type: \(F(T)=\),
1 T 0.
Embedded end A.

\subsection*{1.4 Conditions}
initial
Initial displacement no one in any point.
Null initial speed in any point.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLL311-Transitory dynamic response of a beam in traction
Date:
13/06/03
Author (S):
E. BOYERE, T. QUESNEL Key
:
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The discretized problem checks:
M
M
ll
ld \(U\)
C
C
ll
ld
\&
\(U\)
K
K
ll
ld
\&
L
L
\(L\)
+
+
\(=\)
MR. T
\(M \boldsymbol{U}\)
\(T\)
\(T\)
\& \(D\)
\& C
C U
\&d K
\(\boldsymbol{K} \boldsymbol{U}\)
F
ld
\(d d\)
ld
\(d d\)
with index L: ddl free
index D: ddl imposed

\begin{abstract}
F (T
D
) external loadings applied to the nodes ends and leading to displacements
imposed ud is unknown, one thus eliminates these equations and one obtains:
\end{abstract}
\([M]\{u \&\}+[C]\{u \&\}+[K]\{U\}=-[M]\{u \&\}-[C]\{u \&\}-[K]\{U\)
ll
L
\(l l\)
\(L\)
ll
L
ld
D
ld
D
ld
D\}.
The only nonnull terms of the second member of this system are related to the variables kinematics relating to the node end where displacement is imposed. However, with \(t=0, u d C\)
\&
and u\&dC is not
defined but in \(t=0\) - and \(t=0+, u d C\)
\&
and \(u \& d C\) is null. All the complexity of the problem comes from that.
To obtain a reference solution, we considered udC
\&
and u\&dC uniformly null it
who amounts not considering that the forces intern elastic at the end C. This is debatable of one physical point of view but, by adopting the same assumptions at the time of the modeling of the problem, the validation of Code_Aster can be concluded.

One calculates the reference solution by dealing with the following problem:
\([M I l]\{u l \&\}+[C l l]\{u \& l\}+[\) Kll \(]\{u l\}=[-K l d]\{U(\)
D T\}
) with \(\{u l(0)\}=0\) and \(\{u \& l(0)\}=0\).
With this intention, one transports the problem in the modal base of the system which checks:
\([M]\{u \&\}+[K]\{U\)
ll
\(L\)
ll
\(L\}=0\).
Damping being diagonal, the diagonal system is obtained:
\([M g]\{X \&\}+[c g]\{X \&\}+[k g]\{X\}=\{G(T)\}\) where \(\{G(T)\}=\{G\}\) for \(T 0\), with \(\{X(0)\}=0\) and \(\{X \&(0)\}=0\).

In modal space, one thus solves three equations (3 ddls free) differential of the second order then one returns in physical space. One obtains then the displacement of the point medium:

3
\(U(T)=e-i t(h a s\)

I
(
\(\cos T\)
\(I)+B i \sin (T\)
B
I),
\(i=1\)
with I: ième own pseudo-pulsation of the deadened system.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

SDLL311-Transitory dynamic response of a beam in traction
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\section*{2.2 \\ Results of reference}

Displacement, speed and acceleration of the point medium B of the beam.
Displacement of the point medium B
1,00E-03
8,00E-04
6,00E-04
(m) 4,00E-04
\(\boldsymbol{U} \boldsymbol{B}\)
2,00E-04
0,00E+00
0
0,005
0,01
0,015
0,02
0,025
0,03
-2,00E-04
time (S)

\section*{2.3}

Uncertainty on the solution
Analytical solution of the problem discretized in four elements length equalizes while considering speed and acceleration uniformly null at the point \(C\) where displacement is imposed.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
6.4

\section*{Titrate:}

SDLL311 - Transitory dynamic response of a beam in traction
Date:
13/06/03
Author (S):
E. BOYERE, T. QUESNEL Key

\section*{:}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Modeling in element of beam 3D: POU_D_T}
```

Cutting:
$A C=4$ meshs SEG2 equal length

```

\section*{Limiting conditions:}
```

- Noeud N1 (A) embedded
$D D L \_I M P O D X=D Y=D Z=D R X=D R Y=D R Z=0$

```

\title{
- Noeud N5 (C) in imposed displacement following \(X\)
}
\(D D L \_I M P O D Y=D Z=D R X=D R Y=D R Z=0 D X(T)=U\)

\section*{Resolution:}

Algorithm of direct integration of Newmark
No time: \(T=105 \mathrm{~S}\)
Duration of observation: 0,03 S

\section*{3.2}
Characteristics of the grid
Node S numbers: 5
A number of meshs and type: 4 meshs SEG2

\section*{3.3}
Functionalities tested
Orders

\section*{DYNA_LINE_TRAN NEWMARK}
C.L. DIRICHLET BY VECTASS
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
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Titrate:
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E. BOYERE, T. QUESNEL Key

\section*{:}

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{- Déplacement at the point medium B}

\section*{Time}

\section*{Displacement}

Displacement
Difference
(S)

Reference (m)
Aster (m)
(\%)
0,0054
87,376 e3
87,3763 e3
28,6 E 3\%
0,0055
87,360 e3
87,3598 e3
21,9 E 3\%
0,0108
26,818 e3
26,8178 e3
57,0 E 3\%
0,0109
26,800 e3
26,8000 e3
10,3 E 3\%
0,0163
64,386 e3
64,3865 e3
84,9 E 3\%
0,0164
64,366 e3
64,3663 e3
42,7 E 3\%
0,0217
41,083 e3
41,0828 e3
49,6 E 3\%
0,0218
41,084 e3

94,0 E 3\%
0,0271
55,525 e3
55,5247 e3
62,6 E 3\%
0,0272
55,530 e3
55,5305 e3
93,3 E 3\%

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.4

Titrate:
SDLL311-Transitory dynamic response of a beam in traction Date:
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Author (S):
E. BOYERE, T. QUESNEL Key

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\section*{5 Modeling \\ B}

\section*{5.1}

Characteristics of modeling
idem that modeling \(A\)

\section*{5.2}

Characteristics of the grid
idem that modeling \(A\)
5.3

\title{
Functionalities tested
}

\author{
Orders
}

\section*{DYNA_LINE_TRAN NEWMARK C.L. DIRICHLET BY LOAD}

\author{
6 \\ Results of modeling B
}

\subsection*{6.1 Values}
tested

\section*{- Déplacement at the point medium B}

```

85,0 E 3%
0,0164
64,366 e3
64,3663 e3
42,9 E 3%
0,0217
41,083 e3
41,0827 e3
49,6 E 3%
0,0218
41,084 e3
41,0844 e3
94,0 E 3%
0,0271
55,525 e3
55,5247 e3
62,6 E 3%
0,0272
55,530 e3
55,5305 e3
93,2 E 3%

```

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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6.4

\section*{Titrate:}

SDLL311-Transitory dynamic response of a beam in traction
Date:
13/06/03
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E. BOYERE, T. QUESNEL Key
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\section*{7}

Summary of the results

The results given by Code_Aster are in perfect agreement with the results of the analytical model, that displacement boils about it beam is imposed by a VECTOR ASSEMBLES or by a LOAD.

Caution: the questions of Dirichlet in DYNA_LINE_TRAN are compatible only with method of integration of NEWMARK.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLL400-Beam in vibration with center of excentré torsion
Date:
12/04/02
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK Key :

V2.02.400-A Page:
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Organization (S): EDF/AMA, IAT St CYR, CNAM

V2.02 booklet: Linear dynamics of the beams
V2.02.400 document

SDLL400-Beam in vibration with center
of excentré torsion

\section*{Summary:}

This test results from the validation independent of version 4 of the models of beam.
It makes it possible to test the taking into account of an eccentricity of the center of torsion on the calculation of frequencies
clean of a right beam (a modeling with elements POU_D_E, right beam of Euler).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLL400-Beam in vibration with center of excentré torsion
Date:
12/04/02
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK Key
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

With
B
\(L=7,5 \mathrm{~m}\)

\section*{Appear 1.1-A}

Right beam length 7,5 Mr.
Characteristics of the section:

\section*{It is about the \(U\)-shaped beam presented [Figure 1.1-b].}

\section*{B}

G: centre of gravity

\section*{E}

C: center torsion
Z
C
G
H
E

\section*{\(y\)}

Appear 1.1-B: Section of the \(U\)-shaped beam
\(H=200 \mathrm{~mm}\)
\(B=273 \mathrm{~mm}\)
\(E=8,2 \mathrm{~mm}\)
One has by [bib1] the following data:
\(I y=I z=5,022105 \mathrm{m4}\)
\(Z G C=221,5 \mathrm{~mm}\)
One calculates starting from the geometry of the section:
\(S=\)
6,117
103 m 2
\(J x=1,28\)
107 m4
\(A y=3,65\)

\section*{1.2}

Properties of materials
Young modulus:
\(E=2.071011 \mathrm{~Pa}\)
Poisson's ratio: \(=0,3\)
Density:
\(=7850 \mathrm{~kg} / \mathrm{m} 3\)
1.3

Boundary conditions
Boundary condition:

Plane problem: Blocked DZ and DRY.
Supported nodes A and B: Blocked DX and DY
The taking into account of the eccentricity is done using operand LIAISON_DDL of the order AFFE_CHAR_MECA.

The ddl are always in \(G\), and one takes account of the eccentricity by: \(D Y(G)=D Y(C)+G C X\)

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
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SDLL400-Beam in vibration with center of excentré torsion
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J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK Key
\(:\)
V/
2
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Reference solutions

\section*{2.1}

Method of calculation used for the reference solutions
They are the Eigen frequencies solutions of the homogeneous problem without damping.
It is partially given in [bib1]. The method of resolution, of finite elements type, concerns a model POU_D_TG. However, a series of results is provided if the effects of torsion of warping are neglected, which brings back modeling to a POU_D_T.
\(N^{\circ}\) mode
1

\section*{5}

Frequency (Hz)
3,797
7,788
11,74
15,68
19,62
Table 2.1-A: Results of reference according to [bib1]
One can grant a certain confidence to these results published in a newspaper at reading panel. However uncertainties exist if one wants to reproduce these calculations: constants of Jx torsion and of shearing ky are not provided in the article. They should have been recomputed starting from the geometry of the section.

\section*{2.2 \\ Results of reference \\ Eigen frequencies of the beam without damping}

\section*{2.3}

Uncertainty on the solution
Comparison between codes (STONE [bib2] and ASTER), and analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

WU J.S. \& CHEN K.Z. : Dynamic Analysis of has channel beam had to has moving load. J. of Sound and Vibration, vol. 188, \(n^{\circ}\) 3, pp 337-345, 1995.
[2]
Code STONE version 4 of October 30, 1996, IAT
[3]
Report/ratio n 2314/A of the Institute Aerotechnics "Proposal and realization for new cases tests missing with the validation beams ASTER"
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

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SDLL400 - Beam in vibration with center of excentré torsion
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Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK Key :
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

The model is composed of 15 elements right beam of Euler.

\section*{3.2 \\ Characteristics of the grid}

15 elements \(P O U_{-} D_{-} E\)

\subsection*{3.3 Functionalities}
tested
Orders

MODE_ITER_SIMULT METHOD
JACOBI

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Results}

Results mode
```

STONE
Results
Aster Variation
(%)
1 3,79432
3.7966
0 . 0 6 3
2 7,43340
7 . 4 5 1 3
0.242
3 11,4450
11.5108
0 . 5 7 5
4 15,3439
15.5027
1.036
5 19,4766
19.8060
1.692
Table 4.1-A: Comparison ASTER/CAILLOU in POU_D_E with eccentricity
Mode
Reference results
Aster results
Variation (%)
13.79700
3.7966
-0.008
27.78800
7.4513
-4.322
311.7400
11.5108
-1.952
415.6800
15.5027
-1.130
519.6200
19.8060
0 . 9 4 8

```

Table 4.1-B: Comparison ASTER/Référence [bib1] in POU_D_E with eccentricity

\section*{5}

Summary of the results
The results are rather close to the reference solution (numerical). (variation < 5\%), for which certain data missed and thus had to be estimated. They correspond on the other hand very well with the results of the code STONE of the IAT (given identical to those of Code_Aster).

This makes it possible to validate the taking into account of the offsetting of the center of torsion in the matrices of mass and of rigidity.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLL401 tilted right Beam with \(20^{\circ}\), subjected to sinusoidal efforts
Date:
01/12/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
V2.02.401-A Page:
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Organization (S): EDF/IMA/MMN, IAT, CNAM

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
Document: V2.02.401
SDLL401-Tilted right beam with \(20^{\circ}\), subjected
with sinusoidal efforts
Summary:
This test results from the validation independent of version 4 of the models of beams.
It makes it possible to check the internal efforts on an inclined beam, for sinusoidal loadings in function
time (a modeling with elements POU_D_T, right beam of Timoshenko).

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)

\section*{Version}
4.0

Titrate:
SDLL401 tilted right Beam with \(20^{\circ}\), subjected to sinusoidal efforts
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01/12/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
V2.02.401-A Page:
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1
Problem of reference
1.1 Geometry

B
With
\(20^{\circ}\)
X
Appear 1.1-a
B
With
\(20^{\circ}\)
\(X\)
Appear 1.1-b
Right beam length 1 Mr.
slope \(20^{\circ}\) compared to \(X\) (trigonometrical direction).
Characteristics of the section:
\(S=* 0.01^{2} \mathrm{~m}^{2}\)
1.2

Properties of materials
Young modulus
\(E=2.1011 \mathrm{~Pa}\)
Poisson's ratio
= 0,3
Density
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)
1.3

Boundary conditions and loading
Boundary condition:
- For the loading distributed [1.1-1]

Embedded nodes A and B: DX, DY, DZ, DRX, DRY, DRZ blocked
- For the specific loading [1.1-2]

Embedded node A: DX, DY, DZ, DRX, DRY, DRZ blocked

\section*{Loadings:}
\(F(T)=1000 * \cos (T)\) according to direction \(A B\) either distributed or applied at the end \(B\)

M
\(T=1000\)
(T)
\(T\) ()
* cos
applied at the end \(B\)
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLL401 tilted right Beam with \(20^{\circ}\), subjected to sinusoidal efforts
Date:
01/12/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
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2
Reference solutions
2.1

Method of calculation used for the reference solutions

\subsection*{2.1.1 Loading distributed of traction and compression}

A right beam length \(L\) working only in traction and compression is subjected to one loading distributed constant according to \(X\) but varying in a sinusoidal way according to time. It is embedded at its two ends.
\(2 u\)
2
\(U\)
\(S\)
- E S
\(=F(T\)
```

)=
cos
L
1
2
2

```
ES 4
has
\(2 O\)
\(O\)
\(\sin\)
\(L\)

has


2

0
-
\(\cos\)
\(X\)
1
\(\cos\)
(2 T \(\boldsymbol{O}\) ).

\section*{has -}
```

E
2

```

\section*{with: has}
= .
The use of the law of behavior gives us the tractive effort compression:

20
cos
X
F has

2
has

NR (X, T
O
) =
1 -
cos
L
2
has

20
O
\(\sin\)
\(L\)
has
\(O\)
\(+\sin\)
X

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLL401 tilted right Beam with \(\mathbf{2 0}^{\circ}\), subjected to sinusoidal efforts
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01/12/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

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\subsection*{2.1.2 Loadings}
specific
A beam comforts length L working only in traction compression (or torsion) is subjected to a sinusoidal force in time, (or a moment) applied at its loose lead.
2.1.2.1 Traction
\(2 u\)
2
\(\boldsymbol{U}\)
\(S\)
- \(\boldsymbol{E S}\)
\(=0\)
\(t 2\)
\(x 2\)
\(U\)
1
\(U(0)=0\)
```

,()
L=
F (T).
X
ES

```
The technique of resolution is equivalent to that of the paragraph [§ 2.1.1.1].
For \(F(T)=F \cos (2 T\)
\(O\) ), we have:
2
\(\sin\)
OX
F has
has
\(\boldsymbol{U}\) (,
\(X T)=\)
\(\cos (2\)
OT)
ES 2
2
\(O\)
cos
OL
has
\(E\)
2
with A
=
2
cos
OX
has
```

and NR (,
XT) =F
cos (2
O T)
2
cos
OL
has

```

\subsection*{2.1.2.2 Torsion}
\(2 X\)
2
G
I
\(-I\)
\(X=F(T\)
p
)
2
\(X\)
\(X\)
\(t 2\)
\(U(0)=0, U(L)=0\)
E
\(G=(21+)\)
2
0
4
\(\sin\)
X
00
, 1

```

2
X
(
T
0)
2
0
cos
L
B
G
with B =
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
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```

Code_Aster \({ }^{\circledR}\)
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Titrate:
SDLL401 tilted right Beam with \(20^{\circ}\), subjected to sinusoidal efforts
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Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

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2.2

Results of reference
Interior efforts (NR and MT)
2.3

Uncertainty on the solution
Analytical solution.
2.4 References
bibliographical
[1]
Report/ratio n \({ }^{\circ}\) 2314/A of the Institute Aerotechnics "Proposal and realization for new cases tests missing with the validation beams ASTER"
3 Modeling
With

\section*{3.1}

Characteristics of modeling
The model is composed of 2 elements right beam of Timoshenko.
3.2

Characteristics of the grid
2 elements POU_D_T
3.3 Functionalities
tested
Orders
Keys
DYNA_LINE_TRAN
NEWMARK
[U4.54.01]
EXCIT
CHARGE
FONC_MULT
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J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

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4
Results of modeling A
4.1 Results
4.1.1 Charge divided into traction

Analytical results
Aster results
Variation (\%)
Normal effort for \(X=0\)
\(T=1 / 3 S\)
\(4.7247 E+02\)
\(4.7247 E+02\)
\begin{tabular}{|c|}
\hline 9.12E07 \\
\hline \(T=2 / 3 S\) \\
\hline 3.92944E+02 \\
\hline 3.9294E+02 \\
\hline \(6.08 E 07\) \\
\hline Normal effort for \(X=L / 2\) \\
\hline \(T=1 / 3 S\) \\
\hline 0.0000E+00 \\
\hline 2.1985E12 \\
\hline 2.20E12* \\
\hline \(T=2 / 3 S\) \\
\hline 0.0000E+00 \\
\hline \(2.5087 E 12\) \\
\hline 2.51E12* \\
\hline * Absolute deviation \\
\hline 4.1.2 Charge specific \\
\hline 4.1.2.1 Loading in traction \\
\hline Normal effort for \(X=0\) \\
\hline Analytical results \\
\hline Aster results \\
\hline Variation (\%) \\
\hline \(T=1 / 3 \mathrm{~S}\) \\
\hline \(9.44957 E+02\) \\
\hline \(9.44956 E+02\) \\
\hline 7.59E07 \\
\hline \(T=2 / 3 S\) \\
\hline \(7.8588 E+02\) \\
\hline \(7.8588 \mathrm{E}+02\) \\
\hline 3.01E06 \\
\hline 4.1.2.2 Loading in torsion \\
\hline Torque for \(X=0\) \\
\hline Analytical results \\
\hline Aster results \\
\hline Variation (\%) \\
\hline \(T=1 / 3 \mathrm{~S}\) \\
\hline \(9.4495 E+02\) \\
\hline 9.4495E+02 \\
\hline 1.88E06 \\
\hline \(T=2 / 3 S\) \\
\hline \(7.8588 E+02\) \\
\hline \(7.8589 E+02\) \\
\hline .29E06 \\
\hline
\end{tabular}
4.2 Parameters
of execution
Version: 4.02
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU to use:
10 seconds
5Summary of the results
This test makes it possible to check that the efforts intern elements of beam in dynamics are correct.
The results show a very good agreement with the analytical solution, for a made up grid
only of two elements \(P O U \_D \_T\).
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
7.3
Titrate:SDLL403 - Vibrations of a pendulum in rotation
Date:
04/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key:V2.02.403-A Page:1/8
Organization (S): EDF-R \& D/AMA, SAMTECH

\author{
Handbook of Validation \\ V2.02 booklet: Linear dynamics of the beams \\ V2.02.403 document \\ SDLL403 - Vibrations of a pendulum in rotation
}

\section*{Summary}

The applicability of this test is the modal analysis of the structures. The studied structure is a pendulum in
rotation around an axis fixed and plunged in a field of gravity. The pendulum itself is articulated around one
center perpendicular to the axis of rotation and located at a certain distance from this one. One is interested in the first six Eigen frequencies.

The interest of this test lies in the following aspects:
analyze modal with taking into account of initial constraints (geometrical stiffness)
analyze modal with taking into account of the centrifugal stiffening
important relative difference between two successive frequencies of the spectrum
Currently, the taking into account of the centrifugal stiffening is not possible that with voluminal elements.
The element used is element HEXA20 and one employs the method of Sorensen for the calculation of the frequencies
clean.
The first Eigen frequency is compared with an analytical reference. The following frequencies are compared with numerical values obtained by a software independent of Code_Aster and using modelings "beam" and "plane constraint".

\author{
Handbook of Validation
}

Code_Aster \({ }^{\circledR}\)
Version
7.3

Titrate:
SDLL403 - Vibrations of a pendulum in rotation

\author{
Date: \\ 04/10/04 \\ Author (S): \\ E. BOYERE, G. ROBERT, F. SOULIE Key
}

\section*{:}

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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{H}
has
L

\section*{Characteristics:}

Length of the pendulum
\(L=0.6 \mathrm{~m}\)
Eccentricity
\(=0.1 \mathrm{~m}\) has
Height of the profile
H \(=0.01 \mathrm{~m}\)
Width of the profile
\(B=0.004 \mathrm{~m}\)
Section
\(\boldsymbol{S}=\boldsymbol{b} \boldsymbol{h}\)

\section*{Inertia of inflection}
\(I Z=b h 3 / 12\)
1.2
Properties of materials
Young modulus
\(E=7 . E 10 \mathrm{~N} / \mathrm{m} 2\)
Poisson's ratio
\(=0.3\)
Density
\(=2700 \mathrm{~kg} / \mathrm{m} 3\)
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.3
Titrate:
SDLL403 - Vibrations of a pendulum in rotation
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\section*{1.3}

Boundary conditions and loading
The beam is articulated at point A. the clevis pin is axis \(Y\). the initial state of stress which allows to carry out the geometrical calculation of the stiffnesses and centrifuges is obtained by imposing a speed of rotation and gravity.

Acceleration of gravity
\(G=-9.81 \mathrm{~m} / \mathrm{s}^{2}(\) parallel with axis \(Z\) )

\section*{Number of revolutions}
\(=10 \mathrm{rad} / \mathrm{s}\)
The static position of balance 0 correspondent with loading is calculated by the relation:
```

3
2
Gcos=(3a+2
0
L}\operatorname{cos}0)\operatorname{sin}

```

One finds \(0=11.269931365^{\circ}\)
The conditions on displacements at point \(A\) are as follows:
\(U=v=W=0 ; X=Z=0\)
One considers moreover than the section passing by A remains rigid.

\subsection*{1.4 Conditions initial}

Without object in modal analysis.

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}

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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

\section*{first Eigen frequency}

The facts of the case are selected in such a way that the stiffnesses in inflection and in extension are large with respect to the stiffnesses geometrical and centrifugal. Under these conditions, the value of the first Eigen frequency is obtained analytically by considering a pendulum rigid.

While taking as degree of freedom the angle enters the pendulum and axis \(X\) the equation of movement is written:
\(2 \&\)
\(=3\)
2
\(L\)
\(G \cos -(3 a+2 L \cos ) \sin\)
One considers here the small oscillations of the pendulum around a position of balance statics 0 . By linearizing the equation of the movement in the vicinity of this position, one obtains the equation with the small disturbances:
\(2 L\) \&
\(G \sin +(3 a \cos +2 L \cos 2\)
0
0
)] = 0
0

One deduces the pulsation from it from the first mode:
```

3g
3a
2
=
sin +
cos+cos
2
2
O
L
2
O
O
L

```

This own pulsation can be still written in the form
\(K()+K(2\)
)
\(=\)
\(I\)
with
2
2
has
\(L\)
2
=
\(S L\)
\(G \sin +S L \cos +\cos\)
0
0
0
2
2
3
E
géométriqu
stiffness
)
K (2
)
1
3
2
2
= -
\(S L\)
\(\sin 0\)
3
(
centrifugal
stiffness
)
1
3
\(I=S L\)
3
(
rotation

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.3

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\section*{other Eigen frequencies}

The values of reference of frequencies 2 to 6 are obtained numerically by means of version 7 of software the SAMCEF software. Two different modelings were used: 20 elements of deformable beam with the shearing action and 20X4 elements of membrane with 8 nodes. results obtained in both cases are identical if one limits oneself to the first 4 digits significant. Considering the corrections of stiffness are small with respect to the terms of stiffness linear, one can check that frequencies 2 to 6 differ little from the analytical values obtained for a nondeformable beam hurled with the sharp effort. In fact, the maximum variation between the numerical and analytical values \(1 \%\) do not exceed.

\section*{2.2 \\ Results of reference}

The first 5 critical loads are classified by order of increasing module.

\section*{Mode}

\section*{Eigen frequency (Hz)}
11.75556
2100.2
3324.0
4674.4
51150.
61748.

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution for the first frequency. Numerical solution for the others. The tolerance estimated numerical results is \(1 \%\).

\subsection*{2.4 References \\ bibliographical}

Without object.
Handbook of Validation
V2.02 booklet: Linear dynamics of the beams
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\section*{3 Modeling}

With

\section*{3.1}

\section*{Characteristics of modeling}

The beam is with a grid by means of elements HEXA20.
Boundary conditions:
At point \(A\) such as \(X=0.1, Y=0, Z=0\) :
\(D X=D Y=D Y=D R X=D R Z=0\)
In addition, all the nodes of the section passing by \(A\) are rigidly dependent.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes:
1077
A number of meshs:
160 HEXA20
8
QUAD8

\subsection*{3.3 Functionalities \\ tested}

Orders

AFFE_MODELE AFFE
MODELING
" \(3 D\) "
AFFE_CHAR_MECA GRAVITY

ROTATION
DDL_IMPO
LIAISON_SOLIDE
CALC_MATR_ELEM OPTION
"RIGI_GEOM"
```

"RIGI_ROTA"
"MASS_MECA"
MODE_ITER_SIMULT METHOD

```
"SORENSEN"
CALC_FREQ
OPTION
PLUS_PETITE

\author{
NMAX_FREQ
}

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Frequencies in Hz

\section*{Reference mode}

Code_Aster

\section*{Relative variation (\%)}
11.75556
1.7871
2.121

2
100.2100 .2720 .072
3324.0
324.65
0.200

4
674.4677 .10 .407
51150.
1157.8
0.677
61748.
1766.7
1.072

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Author (S):
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\section*{5 \\ Summary of the results}

Good agreement with the reference solution (less than 1. \% of error on all the modes safe on first where the error is \(2.2 \%\) ).

This test could not be carried out with an element of beam because the calculation of the centrifugal matrix of rigidity
is not available for this type of element. In the same way, as it is not available for discrete elements, we could not use connection 3D-beam. In order to stage this problem, all the nodes of surface containing point A were bound by a solid connection.

\section*{Handbook of Validation}

V2.02 booklet: Linear dynamics of the beams
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
Date:
24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
V2.03.001-C Page:
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Organization (S): EDF/EP/AMV, EDFIMA/MNN
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates

\section*{V2.03.001 document}

SDLS01-Thin, free or embedded square plate at the edge

\section*{Summary:}

The applicability of this case test relates to the dynamics of the structures, and more particularly
modal calculation and the harmonic calculation of answer.
For modal calculation, it is a question of calculating the clean modes of inflection of a thin square plate in
two configurations:
- Plaque embedded on an edge,
- Free Plaque.

The plate is with a grid in triangular elements to which elements DKT are affected.
Four different modelings are tested:
- Modal Calcul Edges of the plate directed according to axes' of the reference mark,
- Modal Calcul unspecified Orientation of the plate and harmonic response for the plate embedded,
- Modal Calcul by traditional and cyclic dynamic sousstructuration,
- Modal Calcul following a condensation of Guyan.

The results of reference of modal calculations result from analytical calculations. They validate on the one hand
tools for creation of the matrices of mass and rigidity, as well as the operators of traditional and cyclic dynamic under-structuring implemented in Code_Aster. In addition, it case test validates modal calculation following a condensation of Guyan (condensation of the matrix of mass).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
Date:
24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
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1
Problem of reference

\subsection*{1.1 Geometry}

With
D
With
D
\(T\)
\(y, v\)
H
K
\(G\)
I
J
has
B
C
B
has
C

Dimensioned \(=1 . \mathrm{m}\) has
thickness \(T=0.01 \mathrm{~m}\)
Co-ordinates of the points (in m):

\section*{With}

B
C
D
G
H
I
\(J\)
K
\(X\)
0.
1.
1.
0.
0.5
0.25
0.75
0.75
0.25
\(y\)
0.
0.
1.
1.
0.5
0.25
0.25
0.75
0.75

Z
0.
0.
0.

0 .
0.

0 .
0.

0 .
0 .

\section*{1.2 \\ Material properties \\ E \\ \(=211011 \mathrm{~Pa}\) \\ \(=03\) \\ \(=7800 \mathrm{~kg} \mathrm{~m} 3\)}

\section*{1.3}

Boundary conditions and loadings
Case 1: dimensioned embedded \(A B\)
for any point \(P\) such as \(y\)
\(U=v=W=0\).
\(==\)
\(X\)
y
Z
0 .
Case 2: free plate

\subsection*{1.4 Conditions}
initial
Without object for the modal analysis
Handbook of Validation
\(V 2.03\) booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
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Titrate:
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2

\section*{Reference solution}
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLSO1/89 of the guide VPCS which presents method of calculation in the following way:
The formulation of M.V. BARTON, for a plate of with dimensions \(A\), conduit with:

And 2
F
\(=\)
2
\(I=\)
I
I
12
, ,..
2 A 2
12(1-2
)
with, for a Poisson's ratio \(=03\)
.:
\(1^{\circ}\) : Plate embedded on a side
\(2^{\circ}\) : Free plate
I
2
I
2
I
I
1
3.492

1 to 6
0.

2
8.525

7
13.49

3
21.43

8
19.79

4
27.33

9
24.43

5
31.11

10
35.02

6
54.44

11
35.02
( 6 modes of solid body at null frequency).
This reference solution applies to the thin sections such as: T/has <.
01 .
Coefficients I are establish by development limited on the modal deformations of a network of cross beams (beam encastréelibre and beam librelibre).

\section*{2.2}

\section*{Results of reference}

Case 1: the first 6 clean modes
Case 2: the first 11 clean modes

\section*{2.3}

Uncertainty on the solution
Semianalytic solution.

\subsection*{2.4 References \\ bibliographical}
[1]
Mr. V. BARTON Vibrations of rectangular and skew cantilever punts. Newspaper of Applied Mechanics, flight 18, p. 129134 (1951)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
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24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:

\section*{V2.03.001-C Page:}

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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling A
Modeling DKT
y
D
C
K
\(J\)
G
H
I
X
With
B
Names of the nodes:
Points
With \(=N 1\)
\(B=N 78\)
\(C=N 145\)
\(D=N 80\)
\(G=N 65\)
\(H=N 17\)
\(I=N 73\)
\(J=N 121\)
\(K=N 71\)

\section*{Limiting conditions:}

Case 1 in all the nodes on the side \(A B\) :
DDL_IMPO: (GROUP_NO: AB DX: 0. , DY: 0. , DZ: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Cases 2 none

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes: 145
A number of meshs and types: 256 TRIA3

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
HULL

\section*{ALL}
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MODELE
"MECHANICAL"
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
"BAND"
[U4.52.01]
CALC_ELEM
OPTION
"EPOT_ELEM_DEPL"
[U4.61.02]
"ECIN_ELEM_DEPL"
Handbook of Validation
\(V 2.03\) booklet: Linear dynamics of the hulls and the plates
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge

\section*{Date:}

24/08/99
Author (S):

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G. ROUSSEAU, C. VARE, J. PELLET
}

Key:
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\section*{Results of modeling A}

\subsection*{4.1 Values}
tested
Frequency (Hz)

\section*{Clean mode}

\section*{Reference}

Aster
\% difference

\section*{Tolerance}

\section*{\(1^{\circ}\) : Plate embedded on a side}

1
8.7266
8.6718
0.63

2
21.3042
21.2904
0.06

3
53.5542
53.0992
0.85
1. 102

4
68.2984
67.9269
0.54

5
77.7448
77.4294
0.40

6
136.0471
135.7635
0.21

Aster epot \(=\) ecin
1
1.4796104

2
1.7331104

3
4.3802104

4
3.7367104

5
5.4956104

6

\subsection*{4.2 Remarks}

MODE_ITER_SIMULT
the first 6 Eigen frequencies, clean vectors and modal parameters deformation energy and kinetic energy of the 6 modes.
\(2^{\circ}\) :
5 Eigen frequencies, clean vectors and modal parameters ( \(F>0\) ) deformation energy and kinetics of the 5 modes.

\author{
4.3 Parameters
}
of execution
Version: 3.04.05
Machine: CRAY C98
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
18.91 seconds

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Code_Aster \({ }^{\circledR}\)
Version
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Titrate:
SDLS01 Plates square thin, free or embedded at the edge
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24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
V2.03.001-C Page:
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5 Modeling
B
5.1

Characteristics of modeling B
Modeling DKT with grid identical to modeling A.
Rotation of the plate such as side \(A B\) is on the line \(3 y=4 x\)
C
y
J
I
B
G
D
K
H
With
X
Names of the nodes:
Points
With \(=\mathrm{N} 1\)
B \(=\) N78
C \(=\) N145
D = N80
G = N65
H = N17
I = N73
\(\mathrm{J}=\mathrm{N} 121\)
K = N71

\section*{Limiting conditions:}

Case 1 in all the nodes on the side AB :
DDL_IMPO: (GROUP_NO: AB DX: 0. , DY: 0. , DZ: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)

Case 2: none
Harmonic answer:
Nodal force point C (N145): Fz = 98100
Material: AMOR_ALPHA: 0.1 AMOR_BETA: 0.1

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 145
A number of meshs and types: 256 TRIA3

\subsection*{5.3 Functionalities}

\section*{tested}

Orders

\section*{Keys}

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MODELE
'MECHANICAL
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
AMOR_ALPHA
AMORBETA
[U4.23.01]
MODE_ITER_SIMULT
"BAND"
[U4.52.01]
DYNA_LINE_HARM
[U4.54.02]
CALC_ELEM
OPTION
EFGE_ELNO_DEPL
[U4.61.02]
SIGM_ELNO_DEPL
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
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24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
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Results of modeling B
6.1 Values
tested
The values of the Eigen frequencies are identical to those of modeling A.
Harmonic answer:
FREQ:
50 Hz
NODE:
N145
NET:
M255

\author{
Reference
}

Aster 3.03.15
Aster 3.05.16
\% difference
DEPL "DZ"
2.90290E02
2.90290E02
0.0
5.20606E02
5.20606 E 02

DEPL "DRX"
2.52920E02
2.52920 E 02
0.0
9.44717E02
9.44717 E 02

QUICKLY "DZ"
\(1.63553 \mathrm{E}+01\)
\(1.63553 \mathrm{E}+01\)
0.0
\(9.11973 \mathrm{E}+00\)
\(9.11973 \mathrm{E}+00\)
QUICKLY "DRX"
\(2.96792 \mathrm{E}+01\)
\(2.96792 \mathrm{E}+01\)
0.0
\(7.94573 \mathrm{E}+00\)
\(7.94573 \mathrm{E}+00\)
ACCE "DZ"
\(2.86505 \mathrm{E}+03\)
\(2.86505 \mathrm{E}+03\)
0.0
\(5.13817 \mathrm{E}+03\)
\(5.13817 \mathrm{E}+03\)
ACCE "DRX"
\(2.49622 \mathrm{E}+03\)
\(2.49622 \mathrm{E}+03\)
0.0
\(9.32398 \mathrm{E}+03\)
\(9.32398 \mathrm{E}+03\)
"EFGE_ELNO_DEPL" "MXX"
\(1.14053 \mathrm{E}+01\)
\(1.14053 \mathrm{E}+01\)
0.0
\(1.45539 \mathrm{E}+03\)
\(1.45539 \mathrm{E}+03\)
"EFGE_ELNO_DEPL" "MYY"
\(1.10224 \mathrm{E}+01\)
\(1.10224 \mathrm{E}+01\)
0.0
\(1.31441 \mathrm{E}+03\)
\(1.31441 \mathrm{E}+03\)
"EFGE_ELNO_DEPL" "MXY"
\(1.03148 \mathrm{E}+01\)
\(1.03148 \mathrm{E}+01\)
0.0
\(3.55382 \mathrm{E}+02\)
\(3.55382 \mathrm{E}+02\)
"EFGE_ELNO_DEPL" "QX"
\(3.66163 \mathrm{E}+02\)
\(3.66163 \mathrm{E}+02\)
0.0
```

3.77331E+03
3.77331E+03
"EFGE_ELNO_DEPL" "QY"
3.14676E+02
3.14676E+02
0 . 0
2.06813E+03
2.06813E+03
"SIGM_ELNO_DEPL" "SIXZ"
5.49245E+04
5.49245E+04
0.0
5.65997E+05
5.65997E+05
"SIGM_ELNO_DEPL" "SIYZ"
4.72014E+04
4.72014E+04
0.0
3.10219E+05
3.10219E+05

```

\subsection*{6.2 Remarks}
```

MODE_ITER_SIMULT
OPTIONS: "BAND"
FREQ: (8. , 140.)
CASE 1
FREQ: (32., 90.)
CASE 2

```

\section*{Contents of the file results:}
\(1^{\circ}\) :
the first 6 Eigen frequencies, clean vectors and modal parameters.
\(2^{\circ}\) :
the first 11 Eigen frequencies, clean vectors and modal parameters.
\(3^{\circ}\) :
displacement DZ DRX with the N145 node
efforts generalized and forced M255 mesh

\subsection*{6.3 Parameters}
of execution
Version: 3.03.10
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords

Time CPU To use:
15.35 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster ®}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
Date:
24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
V2.03.001-C Page:
8/12
7 Modeling
C
7.1

Characteristics of modeling C
G
In the 2 cases, the plate is cut out in 4 parts of equal size. Each sousstructure
considered is with a grid in triangles to which elements of plate DKT are affected.

\section*{Case 1: Plate embedded on an edge}

The structure is studied using the method of sousstructuration traditional with interfaces of the type
CRAIG_BAMPTON. The modal base used for each sousstructure is made up of 25 modes clean and of the constrained modes associated the interfaces.

\section*{Case 2: Free plate}

The structure is studied using the method of sousstructuration cyclic with interfaces of the type HARMONIC CRAIG_BAMPTON and taking into account of the specificity of the node of the axis (not G).
base modal used for the basic sector is made up of 25 clean modes and the modes harmonics associated with the interfaces.

\section*{7.2}

Characteristics of the grid
A number of nodes: 121
A number of meshs and types: 200 TRIA3

\subsection*{7.3 Functionalities}

\section*{tested}

Orders
Keys
SOUS_STRUC_1
SOUS_STRUC_2
[U4.55.06]
DDEFI_MODELE_GENE
CONNECTION
INTERFACE_1
INTERFACE_2
NUME_DDL_GENE
MODELE_GENE
[U4.55.07]
ASSE_MATR_GENE
NUME_DDL_GENE
[U4.55.08]
ASSE_MATR_GENE
OPTION
"MASS_GENE"
"RIGI_GENE"
DEFI_SQUELETTE
MODELE_GENE
[U4.75.01]
DEFI_SQUELETTE
SOUS_STRUC
NAME
REST_BASE_PHYS
SKELETON
[U4.64.01]
```

REST_BASE_PHYS
TOUT_ORDRE
MODE_ITER_CYCL
CONNECTION
CENTER
"YES"
MODE_ITER_CYCL
CALCULATION
TOUT_DIAM
"CENTER"
[U4.52.03]
MODE_ITER_CYCL
CALCULATION
OPTION
DEFI_SQUELETTE
MODE_CYCL
[U4.75.01]
DEFI_SQUELETTE
SECTOR
Handbook of Validation
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```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
Date:
24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
V2.03.001-C Page:
9/12
8
Results of modeling C
8.1 Values
tested
Order of
Frequency (Hz)
clean mode I
Reference
\begin{tabular}{l} 
Aster \\
\% difference \\
Tolerance \\
\(1^{\circ}:\) Plate embedded on a side \\
1 \\
8.7266 \\
8.6419 \\
0.97 \\
2 \\
21.3042 \\
21.2253 \\
0.37 \\
3 \\
53.5542 \\
52.9693 \\
1.09 \\
1.25102 \\
4 \\
68.2984 \\
67.5444 \\
1.10 \\
5 \\
77.7448 \\
77.3966 \\
0.45 \\
6 \\
136.0471 \\
134.5785 \\
1.08 \\
\(2^{\circ}:\) Free plate \\
7 \\
33.7119 \\
33.6808 \\
0.09 \\
8 \\
49.4558 \\
48.9785 \\
0.96 \\
9 \\
61.0513 \\
60.6739 \\
0.62 \\
1.102 \\
\hline
\end{tabular}
\% difference\(1^{\circ}\). Plate embedded on a side
1
8.7266
8.6419
0.97
2
21.3042
21.2253
0.37
3
53.5542
9693
1.09
1.25102
4
68.2984
67.5444
. 10
77.7448
77.3966
0.45
6
. 0471
134.5785
. 08
\(2^{\circ}\) : Free plate
33.7119
33.6808
0.09
8
49.4558
48.9785
9
60.6739
0.62
1. 102

\subsection*{8.2 Parameters}

\section*{of execution}

\author{
Version: 3.05.13
}

Machine: CRAY C98
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
112.81 seconds

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\section*{Code_Aster ®}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge

\section*{Date:}

24/08/99
Author (S):

\section*{G. ROUSSEAU, C. VARE, J. PELLET}

Key:
V2.03.001-C Page:
10/12
9 Modeling
D
9.1

Characteristics of modeling D
DKT + sousstructuration of GUYAN
y
D
NR
C

\section*{K}
J
O
G
M
H
I
With
B
L
X

Limiting conditions: Free plate
Condensation of the matrices of mass and rigidity on the nodes:
(A, B, C, D, G, H, I, J, K, L, M, NR, O).
9.2

Characteristics of the grid
A number of nodes: 145
A number of meshs and types: 256 TRIA3

\subsection*{9.3 Functionalities}
tested

\section*{Orders}

\section*{Keys}

AFFE_CARA_ELEM

\section*{HULL}

ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MODELE
'MECHANICAL
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
"BAND"
[U4.52.01]
MACR_ELEM_STAT
RIGI_MECA
[U4.44.01]
MACR_ELEM_STAT
MACR_MECA
[U4.44.01]
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V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
Date:
24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
V2.03.001-C Page:
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10
Results of modeling D
10.1 Values
tested

\section*{Order of}

Frequency (Hz)

\section*{clean mode I}

\section*{Reference}

\author{
Aster
}
\% difference

\section*{Tolerance}
\(2^{\circ}\) : Free plate
61.6240
0.94

\subsection*{10.2 Remarks}

One seeks to calculate the first 3 nonnull Eigen frequencies of the problem of the free plate on its edges.
If one condenses the matrices on the only nodes:
(A, B, C, D, G, H, I, J, K)
The precision of the frequencies is not whereas \(2 \%\).
To obtain the results wanted with the awaited precision (1\%), it is necessary to add the points (L, M,
NR, O).
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V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}

3
Titrate:
SDLS01 Plates square thin, free or embedded at the edge
Date:
24/08/99
Author (S):
G. ROUSSEAU, C. VARE, J. PELLET

Key:
V2.03.001-C Page:
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11

\section*{Summary of the results}
- Modélisations A and b:

Precision on the Eigen frequencies \({ }^{2} 1 \%\) until the sixth mode of inflection.
- Modélisation C:

In sousstructuration, the quality of the results could be improved by the use of a grid of finer sousstructure.
- Modélisation D:

To obtain an accuracy of \(1 \%\) on the Eigen frequencies, it is necessary to condense too on the 4 nodes mediums of the edges \(\mathrm{L}, \mathrm{M}, \mathrm{NR}\) and O .
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

\section*{Titrate:}

SDLS02 Plates mean rhombus embedded at the edge
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.002-C Page:
1/6
Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

\section*{V2.03 booklet: Linear dynamics of the hulls and the plates}

\section*{V2.03.002 document}

\section*{SDLS02 - Plate mean rhombus embedded at the edge}

\section*{Summary:}

This three-dimensional problem consists in seeking the frequencies of vibration of a mechanical structure composed of a parallelepipedic plate (nonrectangular), embedded on only one side. This test of mechanics of the structures corresponds to a dynamic analysis of a surface model having one linear behavior. It comprises only one modeling.
This problem makes it possible to test the element of plate DKT and the calculation of frequencies of vibration by the method of
Lanczos.
The results obtained on the first two Eigen frequencies are in concord with those of the guide VPCS.
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V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS02 Plates mean rhombus embedded at the edge
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.002-C Page:
2/6
1
Problem of reference
1.1 Geometry
```

Z, W
y
T
With
D
y, v
has
has
has
C
X
B
has
X, U
side has =1. m, thickness T = 0.01 m,=30
Co-ordinates of the points (in m):
With
B
C
D
X
0.
has
has (1+sin)
sin has
y
0.
0.
cos has
cos has
Z
0.
0.
0.
0 .
1.2
Properties of materials
E=2.1 1011 Pa
= 0.3
= 7.800. kg/m3

```

\section*{1.3}

\section*{Boundary conditions and loadings}

Embedded side AB:
for any point P such as \(y\)
\(=0\).
p
\(U=v=W=0\).
= =
\(X\)
y
Z
0.

\subsection*{1.4 Conditions}
initial
Without object for the modal analysis.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLS02 Plates mean rhombus embedded at the edge
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.002-C Page:
3/6
2
Reference solution
2.1

\section*{Method of calculation used for the reference solution}

The formula of reference is that given in card SDLS02/89 of the guide VPCS which presents method of calculation in the following way:
The formulation of M.V. BARTON, for a plate on side, led to:
1
ET2
F
```

I=
, ,..
2
I
12
2
has
12(1-2
)
2 =
where:
G
()
I
with, for a Poisson's ratio = 0.3 and = 30
=30
2
3.961
1
2
10.19
2

```
- M.V. Barton mentions the sensitivity of the result to the order of the mode and the angle.
- This reference solution applies to the thin sections such as: \(\mathrm{t} / \mathrm{a}<0.1\).
- Coefficients \(I\) were established with a limited development of an insufficient nature.
2.2

Results of reference
The first two clean modes given by:
- the formula of M.V. Barton,
- the average of 5 software packages of calculation by the finite element method.
2.3

Uncertainty on the solution
Semi-analytical solution \(<2 \%\).

\subsection*{2.4 References \\ bibliographical}
[1]
M.V. BARTON, Vibrations of rectangular and skew cantilever punts. Newspaper of Applied Mechanics, vol. 18, p. 129-134 (1951).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates

HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS02 Plates mean rhombus embedded at the edge
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.002-C Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling DKT

Cutting: 10 on each side of the rhombus 200 meshs TRIA3.

\section*{Limiting conditions:}
in all the nodes on the side AB :
DDL_IMPO: (GROUP_NO: AB DX: 0. , DY: 0. , DZ: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Name of the nodes:
Not \(\mathrm{A}=\mathrm{N} 1\)
Not C: N121
Bridge \(\mathrm{B}=\mathrm{N} 11\)
Not \(\mathrm{D}=\mathrm{N} 111\)
3.2

Characteristics of the grid
A number of nodes:
121
A number of meshs and types:
200 TRIA3
3.3 Functionalities
tested
Orders

\section*{Keys}

AFFE_CARA_ELEM
HULL
ALL
[U4.21.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
```

ELAS
[U4.23.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

```

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS02 Plates mean rhombus embedded at the edge
Date:
08/01/98
Author (S):
B. QUINNEZ

Key:
V2.03.002-C Page:
5/6
4
Results of modeling A
4.1 Values
tested
Order of the mode
Frequency (Hz)
proper I
Reference
Reference
Aster
\% difference
(Barton)
(average of
average codes
5 codes)
1
9.8987

\subsection*{4.2 Remarks}

Calculations carried out by:
MODE_ITER_SIMULT
METHOD: 'TRI_DIAG
OPTION: "PLUS_PETITE"
NMAX_FREQ: 2
4.3

Contents of the file results
the first 2 Eigen frequencies, clean vectors and modal parameters.

\subsection*{4.4 Parameters}

\section*{of execution}

Version: NEW 3.4
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
4.5 seconds

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V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLS02 Plates mean rhombus embedded at the edge
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.002-C Page:
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\section*{Summary of the results}

The results given by Code_Aster are comparable with the results given by other codes of using calculation of the formulations different for this plate in form from parallélélogramme.
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V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
1/8
Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

\section*{V2.03 booklet: Linear dynamics of the hulls and the plates}

\section*{V2.03.003 document}

\section*{SDLS03 - Thin rectangular plate simply}

\section*{pressed on the edges}

\section*{Summary:}

This three-dimensional problem consists in seeking the frequencies of vibration of a mechanical structure of type
plate. Two different configurations make it possible to test the modes of vibration in the plan of the plate (behavior out of membrane) with elastic supports on two opposed edges, and the modes of vibration in inflection of a plate supported on its contour.
This test of mechanics of the structures corresponds to a dynamic analysis of a surface model having one linear behavior. It comprises two modelings (grid in triangle or quadrangle).
This problem makes it possible to test the elements of transverse membrane and flexbeam and the calculation of
frequencies of vibration by the method of Lanczos or the method of Bathe and Wilson.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A
Code_Aster \({ }^{\circledR}\)
Version

\section*{4.0}

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
2/8
1
Problem of reference

\subsection*{1.1 Geometry}
y, v
B
C
has
With
D

X, U
Z, W
B
That is to say a plate whose characteristics are as follows:
length: \(=1.5 \mathrm{~m}\) have
width: \(\mathrm{B}=1 \mathrm{~m}\)
thickness: \(T=0.01 \mathrm{~m}\)
The points characteristic of the plate have as co-ordinates:
With
B
C
D
X
0.
0.
1.
1.
0.
1.5
1.5
0.

Z

\section*{Properties of materials}

The parameters characterizing the properties of material are:
\(\mathrm{E}=2.11011 \mathrm{~Pa}\)
\(=0.3\)
\(=7.800 \mathrm{~kg} / \mathrm{m} 3\)
1.3

\section*{Boundary conditions and loadings}

\subsection*{1.3.1 Problem of inflection}

The plate is in simple support on all its sides: for any point P of the edge one a : \(\mathrm{W}=0\).

\subsection*{1.3.2 Problem of membrane}

For all the points of the plate, one blocks displacement in Z and the three degrees of rotation, it be-with to say:
\(\mathrm{W}=0 . \mathrm{X}=\mathrm{y}=\mathrm{Z}=0\).
On sides AD and BC one blocks displacement in y : for \(\mathrm{y}=0\). or \(\mathrm{y}=\) have one has \(\mathrm{v}=0\).
To points A, B, C, D one attaches springs of stiffness K . The axis of these springs is direction X .

The numerical value of K is as follows: \(\mathrm{K}=25 \mathrm{~N} / \mathrm{m}\).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
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\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
3/8

\section*{2 \\ Reference solution \\ 2.1 \\ Method of calculation used for the reference solution}

\subsection*{2.1.1 Problem of inflection}

The reference solution of the problem of inflection is that given in card SDLS03/89 of the guide VPCS which presents the method of calculation in the following way.
The formulation of M.V. BARTON for a rectangular plate, posed on its four sides led for the modes of inflection with:
I 2
J 2
E 2 T4
\(F=\)
\(i j\)
2
has +
B

12 (1 2
)
with:
\(I=\) a number of half-length of wave according to y (dimension has),
\(J=\) a number of half-length of wave according to X (dimension b ).

\subsection*{2.1.2 Problem of membrane}

The problem dealt with out of membrane is equivalent for the research of the first frequency of vibration with the following unidimensional problem:
2 K
2 K
m
X
where:
\(K\) is the stiffness of the springs, \(m\) is the mass of the plate.
4k
The sought frequency is thus: \(F=1\)
2
m
2.2

\section*{Results of reference}

For the problem of inflection, one calculates the first six frequencies of vibration and for calculation in membrane, one calculates only the first frequency.

\section*{2.3}

\section*{Uncertainty on the solution}

The solutions being analytical, there is no uncertainty.

\subsection*{2.4 References \\ bibliographical}
[1]
M.V. BARTON "Vibrations of rectangular and skew cantilever punts" - Newspaper of Applied Mechanics, vol. 18, p. 129-134 (1951).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
4/8
3 Modeling

\section*{With}
3.1

\section*{Characteristics of modeling}

One cut out the plate in 200 meshs TRIA3. Two modelings for the plate are used: DKT and DST.
For the problem of inflection, the boundary conditions are as follows:
- in all the nodes of the edge: \(\mathrm{DZ}=0\)

For the problem of membrane, the boundary conditions are:
\(\cdot\) in all the nodes of the grid: \(\mathrm{DZ}=0 \mathrm{DRX}=\mathrm{DRY}=\mathrm{DRZ}=0\),
\(\cdot\) in all the nodes on the sides AB and \(\mathrm{BC}: \mathrm{DY}=0\)
- at the points A, B, C, D one adds discrete elements of rigidity (direction X).
y
111
113
115

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes: 121
A number of meshs and types: 200 TRIA3
The points characteristic of the grid are as follows:
Not \(\mathrm{A}=\mathrm{N} 1\)
Not \(\mathrm{C}=\mathrm{N} 121\)
Not \(B=\) N111
Not D = N11

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
ALL
```

[U4.24.01]
DISCRETE
GROUP_NO
"K_T_D_N"
CARA
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MODELE
"MECHANICAL"
'DKT' or "DST"
ALL
[U4.22.01]
"MECHANICAL"
"DIS_T"
MACRO_MATR_ASSE
MATR_ASSE
OPTION
"RIGI_MECA"
[U4.31.02]
"MASS_MECA"
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"BAND"
CALC_FREQ
OPTION
"PLUS_PETITE"
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98

\section*{Author (S):}

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
5/8
4
Results of modeling A

\subsection*{4.1 Values}
tested
For the modes of inflection:
Number
Frequencies
mode
Reference
Aster DKT
\% diff.
\% tolé.
4
35.63
35.46
0.477
0.5

5
68.51
67.82
1.003
1.1

6
109.62
108.67
0.867
0.9

7
123.32
121.90
1.150
1.2

8
142.51
139.99
1.761
1.8

9

\section*{\% diff.}
\% tolé.
35.45
0.492
0.5
67.80
1.030
1.1
108.62
0.910
1.
121.84
1.199
1.3
139.92
1.815
1.9
191.57
2.912
3.

For the problem out of membrane:

\section*{Reference}

Aster DKT
\% diff.
\% tolé.
0.14714
0.147136
0.002
0.1

DST aster
\% diff.
\% tolé.
0.147136
0.001
0.1

\subsection*{4.2 Remarks}

For the problem in inflection, the modal position of the first mode found in the band (5. , 200.) is fourth, because there are three modes of solid body at frequency zero:
- modes of translation U and v in the plan,
- mode of rotation around axis Z .

\subsection*{4.3 Parameters}
of execution
Version: 3.05.18
Machine: CRAY C98
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
25 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
6/8
5 Modeling

\section*{B}
5.1

\section*{Characteristics of modeling}

One cut out the plate in 100 meshs QUAD4.
Three modelings for the plate are used: Q4G, DKT (DKQ), DST (DSQ).
Compared to modeling A, the plate was turned in the plan ( \(\mathrm{O}, \mathrm{X}, \mathrm{y}\) ) of an angle of \(60^{\circ}\)
C121 y
33
x'
22D
11
115
113
y'

For the problem of inflection, the boundary conditions are as follows:
- in all the nodes of the edge: \(\mathrm{DZ}=0\)

For the problem of membrane, the boundary conditions are:
\(\cdot\) in all the nodes of the grid: \(\mathrm{DZ}=0 \mathrm{DRX}=\mathrm{DRY}=\mathrm{DRZ}=0\),
- with node A, one blocks displacement DY in the reference mark (A, x', y'),
- at the points A, B, C, D one adds discrete elements of rigidity (direction \(\mathrm{x}^{\prime}\) ).

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 121
A number of meshs and types: 100 QUAD4
The points characteristic of the grid are as follows:
Not \(\mathrm{A}=\mathrm{N} 1\)
Not B = N111
Not \(\mathrm{C}=\mathrm{N} 121\)
Not D = N11
5.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DISCRETE
GROUP_NO
CARA
"K_T_D_N"
LOCATE
"LOCAL"
ORIENTATION
"ANGL_NAUT"
AFFE_CHAR_MECA
DDL_IMPO

\section*{GROUP_NO}
[U4.25.01]
LIAISON_OBLIQUE
NODE
DY
ANGL_NAUT
AFFE_MODELE
"MECHANICAL"
'Q4G' or "DST"
ALL
[U4.22.01]
or "DKT"
"MECHANICAL"
"DIS_T"
MACRO_MATR_ASSE
MATR_ASSE
OPTION
"RIGI_MECA"
[U4.31.02]
"MASS_MECA"
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"BAND" or
"CENTER"
CALC_FREQ
OPTION
"PLUS_PETITE"
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
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6
Results of modeling B
6.1 Values
tested
For the modes of inflection:
Number
Frequencies
mode
Reference
Aster DKQ
\% difference
\% tolerance
4
35.63
35.359
0.760
0.8

5
68.51
67.491
1.427
1.5

6
109.62
108.563
0.964
1.

7
123.32
121.144
1.765
1.8

8
142.51
138.402
2.882
2.9

9
197.32
4.470
4.5

\section*{Aster DSQ}

4
35.63
35.351
0.782
0.8

5
68.51
67.464
1.527
1.6

6
109.62
108.494
1.027
1.1

7
123.32
121.060
1.832
1.9

8
142.51
138.291
2.961
3.

9
197.32
188.298
4.572
4.6

Aster Q4G
4
35.63
36.011
1.068
1.1

5
68.51
70.795

For the problem out of membrane:

\section*{Reference}

Aster
\% difference
\% tolerance
DKQ
0.14714
0.14713
0.003
0.1

DSQ
0.14714
0.14714
0.002
0.1

Q4G
0.14714
0.14714
0.
0.1

\subsection*{6.2 Remarks}

For the problem in inflection, the modal position of the first mode found in the band \((5 ., 200\).\() is\) fourth, because there are three modes of solid body at frequency zero:
- modes of translation U and v in the plan, - mode of rotation around axis Z .

\subsection*{6.3 Parameters}

\section*{of execution}

Version: 3.05.18
Machine: CRAY C98
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
40 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

Code_Aster ®
Version
4.0

Titrate:
SDLS03 Plates rectangular thin
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.003-A Page:
8/8
7
Summary of the results
The precision, for the modes of inflection, remains acceptable on the first six modes. Let us note however that the precision is worse than in the case of the free plate in space (test
SDLS01 [V2.03.001]).
It is noticed that the treatment of the modes of solid body is suitable.
For the test out of membrane, the results are very satisfactory.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster ©}

Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
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Organization (S): EDF/EP/AMV

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates
Document: V2.03.004
SDLS04 - Under-structuring cyclic:
Annular thin section embedded in its hub

\section*{Summary:}

The applicability of this test relates to the dynamics of the structures, and more particularly calculation
modal by cyclic dynamic under-structuring.
It is a question of calculating the clean modes of an axisymmetric structure (annular thin section embedded in sound
hub) by regarding it as a structure with cyclic repetitivity.
The model consists of an angular sector of \(20^{\circ}\) of the ring, with a grid in triangles to which are affected elements of the type plates: DKT. Two methods of calculation are tested:
- cyclic dynamic Under-structuring of Craig-Bampton
- cyclic dynamic Under-structuring of Mac Neal

The results of reference result from an analytical calculation. They validate the modal computational tools by
cyclic dynamic under-structuring implemented in Code_Aster.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
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1
Problem of reference
1.1 Geometry

Interior ray:
\(\mathrm{IH}=0.1 \mathrm{~m}\)
External ray:
\(\mathrm{R}=0.2 \mathrm{~m}\)
E
Thickness:
\(\mathrm{T}=0.001 \mathrm{~m}\)
1.2

Material properties
\(\mathrm{E}=2.1011 \mathrm{~Pa}\)
\(=0.3\)
\(=7.800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3}

\section*{Boundary conditions and loadings}

Embedding with the hub
For any point \(\mathrm{R}=\mathrm{IH}, \mathrm{U}=\mathrm{v}=\mathrm{W}=0\). and \(\mathrm{X}=\mathrm{y}=\mathrm{Z}=0\).

\subsection*{1.4 Conditions}

\section*{initial}

Without object for the modal analysis.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
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2

\section*{Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card SDLS04/89 of the guide VPCS which presents analytical solution in the following way:
The solution of the determinant of the frequencies established starting from the functions of Bessel leads to
formulate:
1
ET2
F
```

E
(
)
With:

```
- I = a number of nodal diameters
- \(\mathrm{J}=\mathrm{a}\) number of nodal circles
2
and such as:
\(i j\)
I
0
1
2
3
J
0
13.0
13.3
14.7
18.5
1
85.1
86.7
91.7
100.
Mode of inflection to 2 nodal diameters and 1 nodal circle: \(f 2,1=559,09 \mathrm{~Hz}\)
2.2
Results of reference
8 clean modes.

\section*{2.3}

\section*{Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
A.W. LEISSA, Vibration of punts, Document NASA SP160, 1969, p. 19-30.

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version

\title{
G. ROUSSEAU, C. VARE
}

Key:
V2.03.004-B Page:
4/8
3 Modeling
With

\section*{3.1}

\section*{Characteristics of modeling}

This structure with cyclic repetitivity is studied using the method of under-structuring cyclic dynamics of CRAIG-BAMPTON.
A basic sector, consisted an angular sector of \(20^{\circ}\), is with a grid in triangles to which are affected elements of plate DKT.
The modal base used for the sector is made up of 20 clean modes and the constrained modes associated the interfaces.
3.2

Characteristics of the grid
A number of nodes: 66 .
A number of meshs and types: 100 triangles with 3 nodes DKT
3.3 Functionalities
tested

\section*{Orders}

\section*{Keys}

DEFI_INTERF_DYNA
NUME_DDL
[U4.55.03]
INTERFACE
NAME
INTERFACE
TYPE
"CRAIGB"
INTERFACE
MASK
DEFI_BASE_MODALE
TRADITIONAL
INTERF_DYNA
[U4.55.04]
TRADITIONAL

\title{
MODE_MECA
}

TRADITIONAL
NMAX_MODE
MODE_ITER_CYCL
BASE_MODALE
[U4.52.03]
NB_MODE
NB_SECTEUR
CONNECTION
RIGHT-HAND SIDE
CONNECTION
LEFT
CALCULATION
NB_DIAM
CALCULATION
NMAX_FREQ
REST_BASE_PHYS
RESU_GENE
[U4.64.01]

\section*{SECTOR}

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
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4
Results of modeling A
4.1 Values
tested
Nb of
Nb of
Nume

\section*{diameters}
circles
Reference
Aster
\% difference
order
nodal
nodal
I
J
1
0
0
79.26
79.58
0.4

8
0
1
518.85
519.54
0.1
2.3

1
0
81.09
81.18
0.1
9.10

1
1
528.61
529.50
0.2
4.5

2
0
89.63
89.72
0.1
11.12

2
file:///Z|/process/valid/p840.htm (7 of 14)9/28/2006 4:31:10 PM

\subsection*{4.2 Remarks}

The modes with more than 1 modal diameter are double modes.

\subsection*{4.3 Parameters}

\section*{of execution}

Version: 3.02.15
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CUP To use:
18.17 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
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\section*{5 Modeling}

B

\section*{5.1}

\section*{Characteristics of modeling}

This structure with cyclic repetitivity is studied using the method of under-structuring cyclic dynamics of MAC-NEAL.
A basic sector, consisted an angular sector of \(20^{\circ}\), is with a grid in triangles to which are affected elements of plate DKT.
The modal base used for the sector is made up of 20 clean modes and the modes of fastener associated the interfaces.

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 66 .
A number of meshs and types: 100 triangles with 3 nodes DKT

\subsection*{5.3 Functionalities}
tested
Orders
Keys
DEFI_INTERF_DYNA
NUME_DDL
[U4.55.03]
INTERFACE
NAME
INTERFACE
TYPE
"MNEAL"
INTERFACE
DDL_ACTIF
DEFI_BASE_MODALE
TRADITIONAL
INTERF_DYNA
[U4.55.04]
TRADITIONAL
MODE_MECA
TRADITIONAL
NMAX_MODE
MODE_ITER_CYCL
BASE_MODALE
[U4.52.03]
NB_MODE
NB_SECTEUR
CONNECTION
RIGHT-HAND SIDE

\section*{CONNECTION}

LEFT
CALCULATION
NB_DIAM
CALCULATION
FREQ
CALCULATION
OPTION
CALCULATION
NMAX_FREQ
"BAND"
REST_BASE_PHYS
RESU_GENE
[U4.64.01]
SECTOR
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
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6
Results of modeling B

\subsection*{6.1 Values}
tested
Nb of
Nb of
Nume
diameter
circles
Reference
Aster
\% difference
nodal
I
J
1
0
0
79.26
79.58
0.4
8
0
1
518.85
519.53
0.1
2.3
1
0
81.09
81.18
0.1
9.10
1
1
528.61
529.50
0.2
4.5
2
0
89.63
89.72
0.1
11.12
2
1
559.09
559.48
0.07
6.7
3

\subsection*{6.2 Remarks}

The modes with more than 1 modal diameter are double modes.

\subsection*{6.3 Parameters}
of execution
Version: 3.02.15
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CUP To use:
17.50 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
Cyclic SDLS04 Under-structuring
Date:
01/12/98
Author (S):
G. ROUSSEAU, C. VARE

Key:
V2.03.004-B Page:
8/8
7
Summary of the results
The quality of the results seems to be able to be improved by the use of a grid of finer sector.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
1/14
Organization (S): EDF/IMA/MMN, SAMTECH

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates
Document: V2.03.007
SDLS07-Clean modes of an envelope
spherical thin
Summary:
This test from guide VPCS makes it possible to validate the algorithm of search for eigenvalues
MODE_ITER_SIMULT [U4.52.02] with the operators of rigidity and mass corresponding to modelings
following:
1) Three-dimensional hulls: finite elements DKT (mailllage of one \(1 / 8\) of sphere),
2) Elements
finished
2D axisymmetric TRIA6 and QUAD8 (grid of a section),
3) Three-dimensional hulls: axisymmetric finite elements isoparametric SEG3 (linear grid of the section),
4) Hulls
three-dimensional
COQUE_3D: finite element MEC3QU9H (grid \(1 / 8\) of sphere),
5) Hulls
three-dimensional
COQUE_3D: finite element MEC3TR7H.
The results obtained are compared with the analytical solution (HAYEK) and reveal for the six first modes of the lower deviations than:
\(\cdot 0,45 \%\) for the axisymmetric elements continuous mediums,
\(\cdot 0,20 \%\) for the elements of hull DKT,
\(\cdot 0,17 \%\) for the elements of axisymmetric hulls isoparametric,
- 0,17\% for elements COQUE_3D.

Handbook of Validation

\section*{V2.03 booklet: Linear dynamics of the hulls and the plates}

HI-75/98/040 - Ind X

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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1
Problem of reference
1.1 Geometry

Rm
T
It is of a thin sphere, average radius \(\mathrm{Rm}=2.5 \mathrm{~m}\), and about thickness \(\mathrm{T}=0.1 \mathrm{Mr}\).
1.2

\section*{Properties of materials}

The material is homogeneous, isotropic, elastic linear. The elastic coefficients are:
\(\mathrm{E}=200.000 \mathrm{MPa}\) and \(=0.3\).
The density is constant and is worth: \(=7.800 \mathrm{~kg} / \mathrm{m} 3\).
1.3

\section*{Boundary conditions and loadings}

The structure is free in space.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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2

\section*{Reference solution}

\section*{2.1}

\section*{Method of calculation used for the reference solution}

For the thin spheres (i.t \(\ll R\) with \(I\), order of the mode), the clean modes with displacement radial and tangential establish by a theory of membrane are given by [bib1] and [bib2]:
```

E
F
I
=
I
2R
(1-2)
= 1B\pmb2-4 1-2
2+-2
=2
with
and

+ +1+
I
(
)(I I) B I I 3
2

```
The theory presented by Hayek makes it possible to introduce a correction of the effect of inflection
(approximation of
general theory of Wilkinson) who leads to values of \(I\) function of
\(=T 2\) has \(/\)
R2
12
\(B=I(I+)\)
1
and solution of:
4
2
2
2
2
```

+     - 
+     - 
+ 

=
I[13A(1)B(1 A ab)] ab [B
B
4
5] (1)[B 2 (1 A)] 0
2.2

```
Results of reference
Eigen frequencies:
I
Eigen frequencies
2
237.25
3
282.85
4
305.24
5
324.17
6
346.76
7
376.68
8
416.
9
465.75
10
526.20
2.3
Uncertainty on the solution
Analytical solution.
2.4 References
bibliographical
[1]

Card-index VPCS SDLS 07/89 in the Guide of Validation of the Software packages of Calculation of

Structures/SFM AFNOR TECHNIQUE 1990.
[2]
S. HAYEK: "Vibrations of has spherical Shell in acoustic medium", Journal of the Acoustical Society of America, vol. 40, 2, 1996, p. 342-348
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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3 Modeling
With
3.1

Characteristics of modeling

\section*{Hulls DKT}

Z
2,5
(3 symmetry planes)
0
2,5
2,5
X
y
The discretized geometry is represented above. Elements DKT are plane facets with
3 nodes. The number of the nodes on the meridian line and the equator is: 34 .
The boundary conditions applied to the three borders correspond to the conditions of symmetry (displacements and blocked rotations).

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes: 1128
A number of meshs and types: 2125 TRIA3

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

\section*{Keys}

AFFE_MODELE
AFFE
MODELING: "DKT"
[U4.22.01]
AFFE_CARA_ELEM
HULL
THICK: 0.10
[U4.24.01]
CALC_MATR_ELEM
OPTION
"RIGI_MECA"
[U4.41.01]
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
BANDAGE FREQ: \((230 ., 530\).
[U4.52.02]
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
5/14
4
Results of modeling A
4.1 Values
tested
(Frequencies in Hertz)

\section*{Value of parameter I of \\ Reference \\ Aster}

\section*{\% difference \\ the reference solution}
237.25
237.24
0.005
237.24
0.003

2
3
282.85
not obtained [§4.2]
4
305.24
304.97
0.089
304.99
0.080
305.08
0.054

5
324.17
not obtained [§4.2]
6
346.76
346.11
0.186
346.12
0.185
346.30
0.133
346.38
0.108

7
376.68
not obtained [§4.2]
8
416.00
414.89
0.266
414.92
0.259
415.16
0.201
415.24
0.183
415.33
0.161

9
465.75
not obtained [§4.2]
10
526.20
524.34
0.353
524.43
0.337
524.71
0.283
524.94
0.240
524.97
0.234
525.12
0.205

\subsection*{4.2 Remarks}

The reference solution does not give the multiplicity of the modes. One observes with calculations of orders of multiplicity which grow with the value of the frequency.
Modes 3, 5, 7, 9 are not obtained because of the boundary conditions chosen for this model, with the three symmetry planes.

\subsection*{4.3 Parameters}
of execution
Version: 4.00.02
Machine: C90
System:

\section*{UNICOS 8.0}

Obstruction memory:
16 megawords
Time CPU To use:
393.7 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

\section*{Titrate:}

SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
6/14
5 Modeling
B
5.1

Characteristics of modeling
axisymmetric 2D
y
X
No boundary condition.

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 365
A number of meshs and types: 40 QUAD8 and 80 TRIA6

\subsection*{5.3 Functionalities}

\section*{tested}

Orders

\section*{Keys}

AFFE_MODELE
AFFE
MODELING: "AXIS"
[U4.22.01]
CALC_MATR_ELEM
OPTION
"RIGI_MECA"
[U4.41.01]
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
"BAND" FREQ: (230. , 530.)
[U4.52.02]
Handbook of Validation
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HI-75/98/040 - Ind X
Code_Aster \({ }^{\circledR}\)

\section*{Version \\ 4.0}

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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6
Results of modeling B
6.1 Values
tested
Frequencies in Hertz
Identification
Reference
Aster
\% difference
\(\mathbf{n}^{\circ}\) mode
237.25
237.24
0.036

2
3
282.85
282.78
0.023

4
305.24
304.85
0.125

5
324.17
323.32
0.262

6
346.76
345.22
0.443

7
376.68

\subsection*{6.2 Parameters} of execution
Version: 4.00.02
Machine: C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
8.88 seconds

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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7 Modeling
C
7.1

\section*{Characteristics of modeling}

Axisymmetric hulls 1D
y
\(+2,50\)
\(+2,50\)
X
- 2,50

No boundary condition.
One chooses the model of Coils-Kirchhoff to describe kinematics. With the element chosen, this kinematics is obtained by penalization: one puts a great value for coefficient A_CIS. By elsewhere, one neglects the correction of metric.

\section*{7.2}

\section*{Characteristics of the grid}

A number of nodes: 81
A number of meshs and types: 40 SEG3
7.3 Functionalities
tested
Orders
Keys
AFFE_MODELE
AFFE
MODELING: "COQUE_AXIS"
[U4.22.01]
AFFE_CARA_ELEM
HULL
THICK: 0.10,
[U4.24.01]
A_CIS: 1.E6
MODI_METRIQUE: "NOT"
CALC_MATR_ELEM
OPTION
"RIGI_MECA"
[U4.41.01]
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
"BAND" FREQ: (220. , 530.)
[U4.52.02]
MODE_ITER_INV
CALC_FREQ
FREQ: (220. , 530.)
[U4.52.01]
Handbook of Validation

V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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8
Results of modeling C
8.1 Values
tested
(Frequencies in Hertz)
Identification
Reference
Aster
Aster
\% difference
\(\mathrm{n}^{\circ}\) mode
ITER_SIMULT
ITER_INV
2
237.25
237.31
237.32
0.025/0.029

3
282.85
282.77
282.78
0.028/0.025

4
305.24
304.95
304.95
0.096

\section*{5}
324.17
323.68
323.68
0.150

6
346.76
346.23
346.23
0.154

\subsection*{8.2 Parameters}
of execution
Version: 4.00.02
Machine: C90
System:

\section*{UNICOS 8.0}

Obstruction memory:
8 megawords
Time CPU To use:
8.0 seconds

\subsection*{8.3 Remarks}

The purpose of this test with this modeling is only to test the matrix of mass. A satisfactory variation
being observed on the first six frequencies, one chose not to calculate the following ones.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates

\section*{HI-75/98/040 - Ind X}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
10/14
9 Modeling
D

\section*{9.1}

Characteristics of modeling

\section*{Hulls 3D MEC3QU9H}

Z
2,5
(3 symmetry planes)
0
2,5
2,5
X
y
The boundary conditions applied to the three borders correspond to the conditions of symmetry (displacements and blocked rotations).

\section*{9.2}

\section*{Characteristics of the grid}

A number of nodes: 331
A number of meshs and types: 75 QUAD9

\subsection*{9.3 Functionalities}

\section*{tested}

Orders

\section*{Keys}

AFFE_MODELE
AFFE
MODELING: "COQUE_3D"
[U4.22.01]
AFFE_CARA_ELEM
HULL
THICK: 0.10
[U4.24.01]
CALC_MATR_ELEM
OPTION
"RIGI_MECA"
[U4.41.01]
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
BANDAGE FREQ: \((230 ., ~ 530\).
[U4.52.02]
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HI-75/98/040 - Ind X

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

\section*{Titrate:}

SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
11/14
10
Results of modeling D
10.1 Values
tested
(Frequencies in Hertz)

\section*{Identification}

\section*{Reference}

Aster
\% difference
\(n^{\circ}\) mode
237.25
237.25

0
237.26
0.004

2
3
282.85
not obtained [§10.2]
4
305.24
305.18
0.019
305.19
0.017
305.20
0.011

5
324.17
not obtained [\$10.2]
6
346.76
346.17
0.169
346.19
0.165
346.25
0.147
346.36
0.114

7
376.68
not obtained [ \(\$ 10.2\) ]
8
416.00
413.81
0.525
413.84
0.520
413.84
0.518
414.02
0.476
414.09
0.46

9
465.75
not obtained [ \(\$ 10.2\) ]
10
526.20
520.57
1.071
520.62
1.06
520.64
1.056
521.28
0.935
521.29
0.933
521.31
0.929

\subsection*{10.2 Remarks}

Modes 3, 5, 7, 9 are not obtained because of the boundary conditions chosen for this model, with the three symmetry planes.

\subsection*{10.3 Parameters}
of execution

Version: 4.00.14
Machine: C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
41.3 seconds

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Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
12/14
11 Modeling
E
11.1 Characteristics of modeling

Hulls 3D MEC3TR7H
Z
2,5
(3 symmetry planes)
0
2,5
2,5
X
y

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 925
A number of meshs and types: 294 TRIA7
11.3 Functionalities
tested
Orders
Keys
AFFE_MODELE
AFFE
MODELING: "COQUE_3D"
[U4.22.01]
AFFE_CARA_ELEM
HULL
THICK: 0.10
[U4.24.01]
CALC_MATR_ELEM
OPTION
"RIGI_MECA"
[U4.41.01]
```

"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
BANDAGE FREQ: (230. , 530.)
[U4.52.02]
Handbook of Validation
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```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
13/14
12
Results of modeling \(\mathbf{E}\)
12.1 Values
tested
(Frequencies in Hertz)
Identification

\section*{Reference}

Aster
\% difference
\(\mathrm{n}^{\circ}\) mode
237.25
237.25
0.001
237.25
0.001

2
3
282.85
not obtained [ \(\$ 12.2\) ]
4
305.24
305.20

\subsection*{12.2 Remarks}

Modes 3, 5, 7, 9 are not obtained because of the boundary conditions chosen for this model, with the three symmetry planes.

\subsection*{12.3 Parameters}
of execution
Version: 4.00.14
Machine: C90
System:

\section*{UNICOS 8.0}

Obstruction memory:
16 megawords
Time CPU To use:
98.64 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster ©}

Version
4.0

Titrate:
SDLS07 clean Modes of a thin spherical envelope
Date:
07/12/98
Author (S):
P. MASSIN, B. QUINNEZ, A. LAULUSA

Key:
V2.03.007-C Page:
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\section*{13}

\section*{Summary of the results}
- Modeling hull DKT, here restricted with the modes having 3 symmetries compared to
plans \(\mathrm{X}=0, \mathrm{y}=0, \mathrm{Z}=0\), provide the Eigen frequencies with an error lower than \(0.4 \%\) on the 20 first modes.
- Modeling continuous medium axisymmetric 2D provides the Eigen frequencies with an error lower than \(2 \%\).
- Modeling COQUE_AXIS (quadratic isoparametric elements) provides the frequencies clean with an error lower than \(0.2 \%\) on the first 5 modes (space discretization
identical to the trace of the axisymmetric grid 2D).
- Degenerated modeling COQUE_3D (elements of thick hull MEC3QU9H, MEC3TR7H)
to provide the Eigen frequencies with an error lower than \(1.2 \%\) on the first 10 modes.
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V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind X

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates

\section*{V2.03.100 document}

\section*{SDLS100 - Study of grids on a plate}
square thin

\section*{Summary:}

This three-dimensional problem consists in seeking the frequencies of vibration of a mechanical structure composed of a thin square plate embedded on a side. One studies the influence of the distortion of the grid
on the results. This test of Mechanics of the Structures corresponds to a dynamic analysis of a model surface having a linear behavior. It comprises three modelings.
This problem makes it possible to test the element of plate DKT in transverse inflection and the calculation of the frequencies
clean, either by the method of Lanczos, or by the method of Bathe and Wilson. The first modeling consist in netting finely and regularly the plate by triangles. For the second modeling, it grid is coarser while for the third, it coarse and is distorted.
The first modeling is used as results of reference.
The results obtained are in concord between them and with those of a card NAFEMS. The effect of distortion of
grid does not appear on the first frequencies of vibration.
Handbook of Validation

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\section*{Code_Aster ©}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
y
8
7
6
has
9
5
```

y
Node
X
y
1
4 . 0
4 . 0
1
4 . 0
4 . 0
2
2.25
2.25
3
4 . 7 5
2.5
4
7.25
2.75
5
7.5
4 . 7 5
6
7.75
7.25
7
5 . 2 5
7.25
8
2.25
7.25
9
2.5
4 . 7 5
1.2
Properties of materials
E}=2.1011 Pa
=0.3
= 8.000. kg/m3
1.3
Boundary conditions and loadings
Any point P such as xp=0:(U=v=W = 0, y = 0).
1.4 Conditions

```

\section*{initial}

Without object for the modal analysis.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
3/10
2
Reference solution

\section*{2.1}

\section*{Method of calculation used for the reference solution}

The reference solution is that given in the card "Test 16 " of the tests of reference published by NAFEMS.
Card NAFEMS gives the results of reference as well as computation results carried out in using elements of the mean hull type of Kirchoff based on a formulation of displacement isoparametric quadratic (ddl of rotation and normal translation to the plate).

\section*{2.2}

Results of reference
the first 6 clean modes.

\subsection*{2.3 References \\ bibliographical}
[1]
F.

ABASSIAN, D.J.
DAWSWELL, N.C.
KNOWLES. Selected Benchmarks for Natural
Frequency Analysis. NAFEMS (1987).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster ®}

\section*{Version}
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
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3 Modeling
With
3.1

Characteristics of modeling
Fine grid for validation of the reference solution
y
Test 0
121
111
89
67
has
45
23
1
3
5
7
9
11
X
has
Cutting:
10 on each side of the rhombus 200 meshs TRIA3.
has
Twinge of the element
\(=20\).
\(10 T\)

\section*{Limiting conditions:}
in all the nodes P on the Xp side \(=0\). :
DDL_IMPO: (GROUP_NO: DIMENSION DX: 0. , DY: 0. , DZ: 0. , DRY: 0. )
Name of the nodes:

Point \(1=\mathrm{N} 1\)
Point \(121=\) N121

\section*{3.2}

Characteristics of the grid
A number of nodes:
121
A number of meshs and types:
200 TRIA3
3.3 Functionalities
tested
Orders

\section*{Keys}

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION
"PLUS_PETITE"
NMAX_FREQ
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HI-75/96/013 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
5/10
4
Results of modeling A
4.1 Values
tested
Order of the mode
Reference
Aster
\% difference
clean
Frequency (Hz)
Frequency (Hz)
1
0.421
0.4178
0.8

2
1.029
1.0255
0.3

3
2.582
2.5669
0.6

4
3.306
3.2733
-1.
5
3.753
3.7347
0.5

\subsection*{4.2 Remarks}

Calculations carried out by:
MODE_ITER_SIMULT
METHOD: "TRI_DIAG"
OPTION: "PLUS_PETITE"
NMAX_FREQ: 12
4.3

Contents of the file results
the first 12 Eigen frequencies, clean vectors and modal parameters.

\subsection*{4.4 Parameters}
of execution
Version: 3.03.25
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
7.2 seconds

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:

SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
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5 Modeling
B

\section*{5.1}

Characteristics of modeling
Element DKT coarser grid
y
Test 1
21
20
19
18
22
8
7
6
23
17

1
5
24
16
has
2

25
15

Twinge of the elements:
\(=50\).
4 T

\section*{Limiting conditions:}
in all the nodes P on the Xp side \(=0\). :
DDL_IMPO: (GROUP_NO: DIMENSION DX: 0. , DY: 0. , DZ: 0. , DRY: 0. )
Name of the nodes:
Point \(1=\mathrm{N} 1\)
Point \(25=\) N25
5.2

Characteristics of the grid
A number of nodes:
25
A number of meshs and types:
32 TRIA3
5.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
METHOD
"JACOBI"
[U4.52.02]
CALC_FREQ
```

OPTION
"PLUS_PETITE"
NMAX_FREQ
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

```

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
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6
Results of modeling B
6.1 Values
tested
Frequency (Hz)
Order of the mode
Reference
Test NAFEMS
Aster
\% difference
clean
NAFEMS
1
0.421
0.4174
0.4165
1.07

2
1.029
1.020
1.0301
0.11

3
2.582
5
3.753
3.769
3.7397
0.35
6
6.555
6.805
6.4544
1.54
Test 0
7
7.3756
7.2821
1.27
8
7.7332
7.6852
0.62
9
8.5567
8.3764
2.11
10
11.1199
10.7209
3.59
11
11.6474
11.2904
3.06
12
14.3531
13.7573
4.16

\subsection*{6.2 Remarks}

Calculations carried out by:
MODE_ITER_SIMULT
METHOD: "JACOBI"
OPTION: "PLUS_PETITE"
NMAX_FREQ: 12
6.3

Contents of the file results
the first 12 Eigen frequencies, clean vectors and modal parameters.

\subsection*{6.4 Parameters}
of execution
Version: 3.03.25
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5.36 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
8/10
7 Modeling
C
7.1

Characteristics of modeling
Element DKT with distorted grid
y
Test 2
21
20
19
18
22
8
23

Twinge of the element: between 50 and 75 .

\section*{Limiting conditions:}
in all the nodes P on the Xp side \(=0\). :
DDL_IMPO: (GROUP_NO: DIMENSION DX: 0. , DY: 0. , DZ: 0. , DRY: 0. )
Name of the nodes:
Point \(1=\mathrm{N} 1\)
Point \(25=\mathrm{N} 25\)
7.2

Characteristics of the grid
A number of nodes:
25
A number of meshs and types:
32 TRIA3
7.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM

\section*{HULL}

ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
```

OPTION
"PLUS_PETITE"
NMAX_FREQ
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
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```

\section*{Code_Aster ®}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
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8
Results of modeling C
8.1 Values
tested
Frequency (Hz)
Order of the mode
Reference
Test NAFEMS
Aster
\% difference
clean
NAFEMS
1
0.421
0.4174
0.4163
1.12

2
1.029
1.020
1.0340
0.49

3
2.582

\subsection*{2.571}
2.5644
0.68

4
3.306
3.317
3.2539
1.58

5
3.753
3.780
3.7433
0.26

6
6.555
6.883
6.4898
0.99

Test 0
7
7.3756
7.2119
2.22

8
7.7332
7.6026
1.69

9
8.5567
8.3232
2.73

10
11.1199
10.7735
3.12

11
11.6474
11.2607
3.32

12
14.3531
13.3008
7.34

\subsection*{8.2 Remarks}

Calculations carried out by:
MODE_ITER_SIMULT
METHOD: "TRI_DIAG"
OPTION: "PLUS_PETITE"
NMAX_FREQ: 12

\section*{8.3}

Contents of the file results
the first 12 Eigen frequencies, clean vectors and modal parameters.

\subsection*{8.4 Parameters}
of execution
Version: 3.03.25
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5.2 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS100 Study of grids on a thin square plate
Date:
08/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.03.100-C Page:
10/10
9
Summary of the results
\% differences/reference
Order of
Reference
Aster
Aster
Aster
```

% diff.
% diff.
% diff.
mode
NAFEMS

```
Test 0
Test 1
Test 2
Test 0
Test 1
Test 2
clean
1
0.421
0.4178
0.4165
0.4163
0.76
1.07
1.12
2
1.029
1.0255
1.0301
1.0340
0.34
0.11
0.49
3
2.582
2.5669
2.5793
2.5644
0.58
0.10
0.68
4
3.306
3.2733
3.2572
3.2539
0.99
1.47

\section*{Test 0}

Test 1
Test 2
Test 0
Test 1
Test 2
clean
7
7.3756
7.2821
7.2119
1.27
2.22

8
7.7332
- For tests 1 and 2, the quadrangles of card NAFEMS were cut out in triangles.
- Tests 3 and 4 can be carried out by Aster (not quadratic element of hull).
- Until the 9 th mode, the error on the frequency is \({ }^{2} 2.5 \%\).
- The effect of distortion of the grid appears really only on modes 7 and 12.

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/96/013 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS101 Plates rectangular thin on elastic mattress
Date:
01/12/98

Author (S):

\section*{B. QUINNEZ, L. VIVAN}

Key:
V2.03.101-A Page:
1/6
Organization (S): EDF/IMA/MMN, CISI

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates
Document: V2.03.101
SDLS101 - Thin rectangular plate on mattress

\section*{rubber band}

\section*{Summary:}

This test makes it possible to validate modeling "APPUI_REP" [U4.22.01] for elements of hull. It corresponds
with calculations of frequency of inflection of a thin rectangular plate which rests on an elastic mattress.
By
report/ratio with test SDLS03 [V2.03.003], which validates the frequencies of vibrations of a plate pressed on its edge,
this test validates modeling "APPUI_REP" and order CREA_MAILLAGE [U4.12.06]. The plate is with a grid with triangles and quadrangles.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster ®}

\section*{Version}
4.0

Titrate:
SDLS101 Plates rectangular thin on elastic mattress
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ, L. VIVAN}

Key:
V2.03.101-A Page:
2/6
1
Problem of reference

\subsection*{1.1 Geometry}

Z, W
B
With
y, v
TBD
C
has
X, U
Z
Rectangular plate:
length
\(=1.5 \mathrm{~m}\) have
width
\(\mathrm{B}=1\). m
thickness
\(\mathrm{T}=0.01 \mathrm{~m}\)
Co-ordinates of the points (in m)
With
BC
D
X
0.
0.
1.
1.

y
0.
0.
0.
0.
0.

\section*{1.2}
Properties of materials

Any point P such as:
\(\mathrm{xp}=0\) or \(\mathrm{xp}=1 \mathrm{~W}=0\)
\(\mathrm{YP}=0\) or \(\mathrm{YP}=1 \mathrm{~W}=0\)

\subsection*{1.4 Conditions}
initial
Without object.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS101 Plates rectangular thin on elastic mattress
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ, L. VIVAN}

Key:
V2.03.101-A Page:
3/6
2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The formulation for a rectangular plate thickness \(T\) posed on the four sides leads to:
I 2
J 2
And 2
\(F=\)
\(i j\)
2
has + B
\(I=\) a number of half-lengths of wave according to \(y\)
\(J=\) a number of half-lengths of wave according to X
The formulation for the plate posed on the four sides and resting on a led elastic mattress with:
/
12
E
\(F\)
\(F=F 2 /\) sans mattress +

2
4 T
with \(E F\) : density of stiffness per unit of area.
2.2

Results of reference
the first 6 clean modes of nonnull frequency (the system comprises 3 modes of rigid body).
2.3

\section*{Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
D. BLEVINS: Formulated for natural frequency and mode shape page 238-239.

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS101 Plates rectangular thin on elastic mattress
Date:
01/12/98
Author (S):

\section*{B. QUINNEZ, L. VIVAN}

Key:
V2.03.101-A Page:
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\section*{3 Modeling \\ With \\ 3.1 \\ Characteristics of modeling \\ Modeling DKT \\ Z \\ y \\ B \\ C \\ With \\ D \\ X}

\section*{Limiting conditions:}
in all the nodes located on the edges
DDL_IMPO: (GROUP_NO: EDGE DZ = 0.)
3.2

Characteristics of the grid
A number of nodes: 176
A number of meshs and types: 150 TRIA3, 75 QUAD4
3.3 Functionalities
tested
Orders

\section*{Keys}

DEFI_GROUP
CREA_GROUP_NO
GROUP_MA
[U4.12.03]
CREA_MAILLAGE
CREA_GROUP_MA
GROUP_MA
[U4.12.06]
DEFI_MATERIAU
APPUI_ELAS
[U4.23.01]
AFFE_MODELE
MODELING: "DKT"
[U4.22.01]
MODELING: "APPUI_REP"
MODELING: "DIS_T"
MODE_ITER_SIMULT
OPTION: "BAND"
[U4.52.02]
METHOD: "TRI_DIAG"

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS101 Plates rectangular thin on elastic mattress
Date:
01/12/98
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.03.101-A Page:
5/6
4
Results of modeling A

\subsection*{4.1 Values}
tested
Modeling
DKT
Identification
Reference
Aster
\% difference
Frequency 4
35.62638
35.5056
0.339

Frequency 5
68.51227
68.0411
0.688

Frequency 6
109.61964
109.0087
0.557

Frequency 7
123.32210
122.1914
0.917

Frequency 8
140.6399
1.309

Frequency 9
197.31535
193.3111
2.029

Modeling DKT and APPUI_REP

\section*{Identification}

\section*{Reference}

Aster
\% difference
Frequency 4
38.68542
38.8710
0.480

Frequency 5
70.15165
69.6943
0.652

Frequency 6
110.65166
110.0539
0.540

Frequency 7
124.24034
123.1200
0.902

Frequency 8
143.30092
141.4473
1.293

Frequency 9
197.89056
193.9004
2.016

Modeling DKT and DIS_T_N (to be able to make comparisons, one replaced the mattress rubber band by discrete springs positioned in each node of the grid).

\section*{Identification}

\section*{Reference}

Aster
\% difference
Frequency 4
38.68542
38.1366
1.419

Frequency 5
70.15165
69.4507
0.999

Frequency 6
110.65166
109.8944
0.684

Frequency 7
124.24034
122.9824
1.012

Frequency 8
143.30092
141.3281
1.377

Frequency 9
197.89056
193.8133
2.060

\subsection*{4.2 Parameters}
of execution
Version: 3.06.04
Machine: CRAY C90
System:
Obstruction memory:
8 MW
Time CPU To use:
22 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLS101 Plates rectangular thin on elastic mattress
Date:
01/12/98

Author (S):

\section*{B. QUINNEZ, L. VIVAN}

Key:
V2.03.101-A Page:
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5

\section*{Summary of the results}

The results obtained are correct compared to the results of reference.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Free SDLS102 Vibrations of a paddle of compression
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V2.03.102-A Page:
1/6
Organization (S): EDF/IMA/MMN, SAMTECH

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates
Document: V2.03.102
SDLS102 - Free vibrations of a paddle of compression

\section*{Summary:}

This test makes it possible to validate the calculation of the Eigen frequencies of a paddle of compression by using the order
MODE_ITER_INV [U4.52.01].
Modelings correspond to the use of elements "COQUE_3D" MEC3QU9H (modeling A) and MEC3TR7H (modeling B).
The reference solutions are experimental results. The variation enters the numerical results and them experimental values does not exceed \(4,5 \%\) for two modelings.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Free SDLS102 Vibrations of a paddle of compression
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V2.03.102-A Page:
2/6
1
Problem of reference
1.1 Geometry

B
D
L
With
C
H
Rm
X
Y
It is about a cylindrical panel:
- Longueur: \(\mathrm{L}=0.3048 \mathrm{~m}\),
- Average Rayon: \(\mathrm{Rm}=0.6096\),
- Longueur of arc: 0.3042 m ,
- Epaisseur: \(\mathrm{H}=0.003048 \mathrm{Mr}\).
1.2

Properties of material
The material is homogeneous, isotropic, elastic linear. The elastic coefficients are:
\(E=206.850\). MPa
\(=0.3\)
Density: \(=7857,2 \mathrm{~kg} / \mathrm{m} 3\)
Coefficient of the deformations of shearing action: A_CIS \(=0.8333\)
1.3

\section*{Boundary conditions and loadings}

The structure is embedded at end data base
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Free SDLS102 Vibrations of a paddle of compression
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V2.03.102-A Page:
3/6
2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution corresponds to the experimental measurements given in [bib1].
2.2

\section*{Results of reference}

The first six measured Eigen frequencies.
Number of the mode
Experimental values
1
85.6

2
134.5

3
259
4
351
5
395
6
531
2.3 References
bibliographical
[1]
J.L. BATOZ, G. DHATT: Modeling of the structures by finite elements - Volume 3 hulls, 1992 HERMES pp 467 to 470.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Free SDLS102 Vibrations of a paddle of compression
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V2.03.102-A Page:
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3 Modeling
With
3.1

Characteristics of modeling
B
Hull 3D MEC3QU9H
With
Z
X
D
C
Y
3.2

Characteristics of the grid
A number of nodes: 169 , a Number of meshs and types: 36 QUAD9

\subsection*{3.3 Functionalities}
tested
Order
Key word factor
Key word
Argument
Keys
AFFE_MODELE
AFFE
MODELING
"COQUE_3D"
[U4.22.01]
AFFE_CARA_ELEM
HULL
THICK
[U4.24.01]

\author{
A_CIS \\ AFFE_CHAR_MECA \\ DDL_IMPO \\ [U4.25.01] \\ MACRO_MATR_ASSE \\ MATR_ASSE \\ [U4.31.02] \\ MODE_ITER_INV \\ CALC_FREQ \\ [U4.52.01]
}

One seeks the frequencies in the interval (80. , 570.) by using the option "ADJUSTS" under the key word factor CALC_FREQ of order MODE_ITER_INV.

\section*{4}

Results of modeling A
4.1 Values
tested
(Frequencies in Hertz)
Identification

\section*{Reference}

Aster
\% difference
\(n^{\circ}\) mode
1
85.6
85.85
0.302

2
134.5
138.56
3.021

3
259
246.92
4.664

4
351
342.71
2.361

5
395
386.66
2.112

6

\section*{531}
531.59
0.112
4.2 Parameters
of execution
Version: 4.00.14
Machine: C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
15.8 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
Free SDLS102 Vibrations of a paddle of compression
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V2.03.102-A Page:
5/6
5 Modeling
B
5.1

Characteristics of modeling
B
Hull 3D MEC3TR7H
With
Z
X
D
C
Y

\section*{5.2}

Characteristics of the grid

\section*{A number of nodes: 913, a Number of meshs and types: 288 TRIA7}

\subsection*{5.3 Functionalities}

\section*{tested}

Order
Key word factor
Key word
Argument
Keys
AFFE_MODELE
AFFE
MODELING
"COQUE_3D"
[U4.22.01]
AFFE_CARA_ELEM
HULL
THICK
[U4.24.01]
A_CIS
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
MACRO_MATR_ASSE
MATR_ASSE
[U4.31.02]
MODE_ITER_INV
CALC_FREQ
[U4.52.01]
6
Results of modeling B
6.1 Values
tested
(Frequencies in Hertz)
Identification

\section*{Reference}

Aster
\% difference
\(\mathbf{n}^{\circ}\) mode
1
85.6
86.06
0.534

2
134.5

\subsection*{138.68}
3.112

3
259
248
4.246

4
351
344.52
1.845

5
395
390.62
1.108

6
531
533.2
0.415
6.2 Parameters
of execution
Version: 4.00.14
Machine: C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
108.7 seconds

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Free SDLS102 Vibrations of a paddle of compression
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V2.03.102-A Page:

\section*{Summary of the results}

Resulted are satisfactory. But the grid with elements MEC3TR7H must be fine to have the same level of error as that obtained with elements MEC3QU9H.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS103 - Coaxial hulls under annular flow
Date:
09/10/01
Author (S):
A. ADOBES, Key Mr. LAINET

V2.03.103-A Page:
1/6
Organization (S): EDF/MTI/MFTT, CS IF

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates

SDLS103 - Coaxial hulls under flow annular: inertial coupling between modes

\section*{Summary}

One considers a hardware configuration made up of two coaxial cylindrical hulls, in interaction with a fluid running out in annular space separating the hulls.

The goal of the case-test is to validate the model of coupling fluid-structure developed in the operator CALC_FLUI_STRU for this type of configuration.

One is interested here more particularly in the taking into account of the inertial coupling between modes, obtained out of water
at rest (mean velocity of flow null). The reference solution is provided by a calculation carried out with
Code_Aster implementing operator CALC_MATR_AJOU .
Handbook of Validation
\(V 2.03\) booklet: Linear dynamics of the hulls and plates
HI-86/01/02 1/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS103 - Coaxial hulls under annular flow
Date:
09/10/01
Author (S):
A. ADOBES, Key Mr. LAINET

V2.03.103-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\title{
The studied configuration consists of two 4 meters height coaxial cylindrical hulls:
}

The internal hull has an average radius of 1 meter and a thickness of 1 centimetre.
The external hull has an average radius of 1,10 meter and a thickness of 1 centimetre.

\subsection*{1.2 Properties}
material
The material constituting the two hulls is steel. Its physical characteristics are:
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)
\(E=2.1011 \mathrm{~Pa}\)
\(=0,3\)

\section*{1.3}

Boundary conditions and loadings
The conditions of self-supporting quality are the same ones for the two hulls: partly embedded ends lower \((Z=0)\) and free partly higher \((Z=4 \mathrm{~m})\).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-86/01/021/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS103 - Coaxial hulls under annular flow
Date:
09/10/01
Author (S):
A. ADOBES, Key Mr. LAINET

\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is provided by a calculation carried out by means of Code_Aster implementing operator CALC_MATR_AJOU.

With this intention, one uses a grid on which the elements of the structure (hulls internal are defined and external), voluminal elements of fluid (annular space) and elements of interface fluid-structure. This method is described completely in [bib1].
On the elements of interface, the condition normal speed limit null is imposed, translating condition of nonpenetration of the fluid in the hulls.
On the sections of entry and exit of annular space, the limiting condition of null potential is imposed, translating the condition of null disturbed pressure at the ends.

One carries out the first modal calculation of the structure in air. Operator CALC_MATR_AJOU allows
then to calculate the matrix of mass added by the fluid, projected on the basis of modal structure in air. One can then recombine this matrix of mass added with the matrix of mass generalized of the structure in air, then to solve a new modal problem which leads to characteristics of the water system at rest. Results obtained for the Eigen frequencies system out of water at rest constitute the reference solution.

Characteristics of the grid 17 nodes on a vertical generator

60 nodes on a crown

960 meshs QUAD4 on each hull
=> 1920 meshs for the interface fluid-structure

2880 meshs HEXA8 for the fluid field
180 meshs QUAD4 for the section of entry of annular space

180 meshs QUAD4 for the section of exit of annular space
2.2

Results of reference
\(N^{\circ}\)
mode
1234567891011
12
Freq
(Hz) 5,65 5,65 6,48 6,48 9,34 9,34 20,82 20,82 28,22 28,22 31,48 31,4
8
2.3

Uncertainty on the solution
In the studied case, the half-thicknesses of hull account for \(10 \%\) of the dimension of the annular play. In the calculation of reference realized by operator CALC_MATR_AJOU, the half-thicknesses are neglected for the definition of the fluid field. One thus awaits a systematic error about 5\% on the frequencies, because of this approximation.
Moreover, differences of modeling and resolution between the numerical method implemented in operator CALC_MATR_AJOU and the analytical model developed in the operator
CALC_FLUI_STRU induce an additional variation.
2.4 References
bibliographical
[1]
G. ROUSSEAU: "Schedule of conditions and principle of realization of the calculation of stiffness and
of damping added in Code_Aster '", HP-51/96/005/B.
[2]
L. PEROTIN, "Note of principle of model MOCCA_COQUE", HT-32/95/021/A.

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-86/01/021/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS103 - Coaxial hulls under annular flow

\section*{Date:}

09/10/01
Author (S):
A. ADOBES, Key Mr. LAINET
:
V2.03.103-A Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The geometry of the structures and the characteristics of material constituting the hulls were presented before.

Concerning the absolute roughness of wall of the structures, one takes a value of 105 meter.
The surrounding fluid is water. Values taken for the density and viscosity kinematics are respectively
\[
\begin{aligned}
& =106 \mathrm{~m} 2 / \mathrm{s} . \\
& F=1000 \mathrm{~kg} / \mathrm{m} 3 \mathrm{and} \\
& F
\end{aligned}
\]

One considers the flow not confined upstream and downstream from the structures.
The problem of coupling fluid-structure is solved by analytical model MOCCA_COQUE [bib2] integrated in Code_Aster (operator CALC_FLUI_STRU), for a mean velocity of flow null: one thus obtains the modal characteristics of the water system at rest, by catch in count effects of added mass.

\subsection*{3.2 Characteristics}
grid
Compared to the calculation of reference, the characteristics of the grid are similar, with the difference
close the fluid field is not represented any more. In order to make the two hulls interdependent one of the other, one adds a group of meshs connecting the nodes to the level of embedding.

One a: 17 nodes on a vertical generator,

60 nodes on a crown,

960 meshs QUAD4 on each hull,
60 meshs QUAD4 to solidarize the two hulls (bases embedded).

\section*{3.3}

Stages of calculation
The definition of the characteristics of a hardware configuration made up of two hulls cylindrical coaxial for a calculation of coupling fluid-structure is given via operator DEFI_FLUI_STRU key word factor COQUE_COAX.

The resolution of the coupling fluid-structure for a configuration of the type "coaxial hulls" and it calculation of the modal parameters (reduced frequencies and depreciation) and deformed modal out of water at rest are realized with operator CALC_FLUI_STRU .

\author{
3.4 Functionalities \\ tested \\ Orders \\ Key word factor \\ Key word \\ \title{
DEFI_FLUI_STRU COQUE_COAX
} \\ MASS_AJOU \\ "YES" \\ CALC_FLUI_STRU BASE_MODALE \\ MODE_MECA
}

\section*{Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HI-86/01/021/A}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS103 - Coaxial hulls under annular flow
Date:

\section*{Author (S):}
A. ADOBES, Key Mr. LAINET
:
V2.03.103-A Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
The comparisons relate to the water frequencies at rest of the first 12 modes of the system.
Numbers of the modes
CALC_FLUI_STRU
CALC_MATR_AJOU
variation
(H=9cm)
\((\mathrm{H}=10 \mathrm{~cm})\)
1 and 2
\(5,30 \mathrm{~Hz}\)
\(5,65 \mathrm{~Hz}\)
\(6,2 \%\)
3 and 4
\(6,12 \mathrm{~Hz}\)
\(6,48 \mathrm{~Hz}\)
\(5,5 \%\)
5 and 6
\(8,69 \mathrm{~Hz}\)
\(9,34 \mathrm{~Hz}\)
\(6,9 \%\)
7 and 8
\(21,96 \mathrm{~Hz}\)
\(20,82 \mathrm{~Hz}\)
\(-5,5 \%\)
9 and 10
\(29,34 \mathrm{~Hz}\)
\(28,22 \mathrm{~Hz}\)
\(-3,9 \%\)
11 and 12
\(33,75 \mathrm{~Hz}\)

\section*{31,48 Hz}
-7,2 \%
One gives, for information, the values of the frequencies of these modes in air:
Numbers of Hull in mvt
Order of hull
Order of beam
Frequency
modes
1 and 2
external
3
1
\(25,15 \mathrm{~Hz}\)
3 and 4
intern
3
1
\(26,12 \mathrm{~Hz}\)
5 and 6
external
4
1
31,91 Hz
7 and 8
intern
2
1
36,85 Hz
9 and 10
intern
4
1
37,42 Hz
11 and 12
external
2
1
39,49 Hz

\subsection*{4.2 Remarks}

The results are in conformity so that one could wait. One observes indeed:
- a systematic error of about 5\%, because of not taken into account thicknesses of hull for the definition of the fluid field in the calculation of reference;
- a residual variation due to the differences of modeling and resolution between the two operators CALC_MATR_AJOU and CALC_FLUI_STRU.

Modes 1 to 6 are modes for which the structure is strongly coupled with the fluid. In practical, these modes correspond to movements of the hulls internal and external overall in opposition of phase. For these modes, the terms of added mass are theoretically proportional

For these the first six modes, the results provided by CALC_FLUI_STRU lead to frequencies clean lower than those calculated by CALC_MATR_AJOU. Indeed, H being weaker, them terms of added mass are larger.

Modes 7 to 12 are modes for which the structure is slightly coupled with the fluid. In practical, these modes correspond to movements of the hulls internal and external almost in phase. Thus, the terms of added mass are theoretically proportional to the water mass involved, i.e. with
F
RH.
In this case, it is normal that the results provided by CALC_FLUI_STRU lead to Eigen frequencies higher than those calculated by CALC_MATR_AJOU. Indeed, H being more weak, the terms of added mass are smaller.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-86/01/021/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS103 - Coaxial hulls under annular flow
Date:
09/10/01
Author (S):
A. ADOBES, Key Mr. LAINET

V2.03.103-A Page:
6/6

\subsection*{4.3 Parameters \\ of execution}

Version: NEW 5.02.20

\author{
Machine: SGI ORIGIN 2000
}

Obstruction memory: 64 Mo
Time CPU To use: 50,63 seconds

\section*{5 \\ Summary of the results}

The comparison of the water frequencies at rest calculated with operators CALC_FLUI_STRU and CALC_MATR_AJOU is satisfactory. The variations met between these two operators are explained by the fact that they use a different modeling.

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-86/01/02 1/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

\section*{Titrate:}

SDLS106 - Modal calculation of plate in under-structuring
Date:
01/03/04
Author (S):
E. BOYERE Key

V2.03.106-A Page:
1/4
Organization (S): EDF-R \& D /AMA

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and the plates
Document: V2.03.106

SDLS106-Modal calculation of plate in under-structuring with base of Ritz

\section*{Summary:}

This test of the field of the modal analysis implements the calculation of Eigen frequencies of inflection in under-structuring of a plate pressed on its edges. The interface is of type CRAIG-BAMPTON. The reference solution is analytical.

\title{
Handbook of Validation
}

V2.03 booklet: Dynamics linear of the hulls and the plates
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLS106 - Modal calculation of plate in under-structuring
Date:
01/03/04
Author (S):
E. BOYERE Key
:
V2.03.106-A Page:
2/4

\section*{1}

Problem of reference

\subsection*{1.1 Geometry}
plate
L
SS1
substructure 1
SS2
I
interface
simple support
\(L=2 m\)
\(L=1,5 \mathrm{~m}\)

\section*{1.2}

Properties of the structure
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)
\(E=2.1011 \mathrm{~Pa}\)
\(=0.3\)
thickness 1 Misters.
\(S\)
1.3

Boundary conditions and loadings
The plate is in simple support on its four edges. The interface of each substructure is embedded.

\section*{Handbook of Validation}
\(V 2.03\) booklet: Dynamics linear of the hulls and the plates
HT-66/04/005/A

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLS106 - Modal calculation of plate in under-structuring
Date:
01/03/04
Author (S):
E. BOYERE Key
\(:\)
\(V\)
3
2
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Reference solution

\section*{2.1}

Reference solution of each substructure
Each substructure is a plate length \(1,5 \mathrm{~m}\) and width 1 m , supported on three dimensioned and embedded on the fourth, vibrating in inflection.

It is shown [bib1] that the Eigen frequencies are worth:
2
2
\(i j\)

\(E h\)
2
fij \(=\)

\section*{2 L2 121 \\ (-2)}
with 2
=
53
' 42
2
what gives for the first frequencies
```

F=
,
47 26Hz,
11
F
=
5 7
76
Hz,
21
F
=
129 24Hz,
31
F
=
13447
12
Hz

```

\section*{2.2 \\ Reference solution of the assembled problem}

According to [bib1], one has for the Eigen frequencies of vibration of a supported plate
```

I
L

```

\section*{That is to say}
\(F=\)
12
,
17
\(H z\),
11
\(F\)
\(=\)
61
35
\(H z\),
21
\(F\)
\(=\)
99

\section*{Hz}

\subsection*{2.3 Reference}
bibliographical
[1]
BLEVINS R.D: Formulated for natural frequency and shape mode. ED. Krieger 1984.

\section*{Handbook of Validation}
\(V 2.03\) booklet: Dynamics linear of the hulls and the plates
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLS106 - Modal calculation of plate in under-structuring
Date:
01/03/04
Author (S):
E. BOYERE Key
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\(4 / 4\)

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling \\ For each substructure: 600 meshs QUAD4.}

\subsection*{3.2 Functionalities}
tested
Orders
DEFI_BASE_MODALE OPTION
RITZ

\section*{MODE_STATIQUE \\ FREQ}

\section*{MODE_ITER_SIMULT \\ "REAL"}

\section*{4}

Results of modeling \(A\)
```

4.1
Values tested on the complete structure

```

\section*{Identification Reference}

Aster \%
difference
\(N^{\circ} 11\) mode
frequency
17.12 Hz
17.12 Hz
0.00
\(N^{\circ} 21\) mode
frequency
35.61 Hz
35.59 Hz
0.05
\(N^{\circ} 12\) mode
frequency 49.99 Hz
50.03 Hz
0.08

\title{
\(N^{\circ} 31\) mode
}
frequency 66.42
Hz
66.57 Hz
0.2
\(N^{\circ} 22\) mode
frequency 68.48
Hz
68.36 Hz
0.01

\section*{5 \\ Summary of the results}

Calculation in under-structuring with modal base of type "Ritz" was validated on the modes of inflection
of a plate pressed on its four edges.

\section*{Handbook of Validation}

V2.03 booklet: Dynamics linear of the hulls and the plates
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS108 Use of RIGI_PARASOL within the framework of a study EPR
Date:
30/08/01
Author (S):
G. DEVESA, Key P. LATRUBESSE

V2.03.108-A Page:

\author{
Organization (S): EDF/RNE/AMV, SAMTECH-France
}

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
V2.03.108 document

SDLS108 - Use of option RIGI_PARASOL within the framework of a study EPR

\section*{Summary:}

The goal of the test is to test the option RIGI_PARASOL which makes it possible to distribute stiffnesses of floor under a foundation raft of
building. This option is called successively in orders AFFE_CARA_ELEM and CALC_AMOR_MODAL.

In order AFFE_CARA_ELEM, it makes it possible to calculate then to affect by a modeling DIS_TR them
specific stiffnesses of a carpet of springs of floor under the foundation raft of building. In the order CALC_AMOR_MODAL, these values make it possible to calculate modal potential energy in the ground which contributes with the calculation of modal depreciation according to the rule of the RCC-G [bib1].

The studied case is an industrial structure of the nuclear small island of EPR. It includes/understands only one modeling. It
case test is thus used to test nonthe regression of the code as much as it constitutes documentation on its use
in oneself. The model of approximately 40000 degrees of freedom consists of 6700 nodes and 10300 elements approximately:
9500 elements DKT, 100 elements of beam and 700 discrete elements of connection.
This industrial case-test is representative of the studies led by SEPTEN/MS on modelings 3D of civil engineering subjected to the seism and makes it possible to reabsorb the tools used within this framework. The comparison relates to
frequencies and depreciation for the first modes calculated on carpet of springs of ground.
The values of reference for a ground of the type SA (soft ground) are given in note E.N.T.MS 96.052
A
[bib2].
The 6 values of total stiffnesses of ground to be distributed under the foundation raft are determined by PARASOL for
modulate ground of the type SA (444 MPa).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
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5.0

Titrate:
SDLS108 Use of RIGI_PARASOL within the framework of a study EPR
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1
Problem of reference

\subsection*{1.1 Geometry}

The studied case is the industrial structure of the nuclear small island of EPR. It includes/ understands only one modeling composed primarily of elements of hull. The connection of the foundation with the ground is

\footnotetext{
file:///Z|/process/valid/p890.htm (12 of 21)9/28/2006 4:31:13 PM
}
modelled
by specific discrete elements.
A number of nodes: 6732
A number of triangular meshs (TRIA3): 1848
A number of rectangular meshs (QUAD4): 7677
A number of POI1: 1119
A number of SEG2: 9047
Handbook of Validation
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Date:
30/08/01

\section*{Author (S):}
G. DEVESA, Key P. LATRUBESSE

\section*{:}

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1.2

Material properties
Reinforced concrete:
\(\boldsymbol{E}=\)
35000.

MPa
Naked =
0.2
\(A M O R \_A L P H A=4.95 E-04\)
\(A M O R \_B E T A=\)
3.91

Various distributions of the densities of reinforced concrete following the components:

To erase
\(R H O=2500 \mathrm{~kg} / \mathrm{m} 3\)
External walls
RHO \(=2500 \mathrm{~kg} / \mathrm{m} 3\)
Floors of the boxes \(\mathbf{R H O}=2500 \mathrm{~kg} / \mathrm{m} 3\)
Internal structures
\(R H O=2500,12500,9700,5850,5290,4500,4180,3900,3680,620 \mathrm{~kg} / \mathrm{m} 3\)
External enclosure
RHO \(=2500 \mathrm{~kg} / \mathrm{m} 3\)
Division 0
\(R H O=2500,5000,4500,4180,3750 \mathrm{~kg} / \mathrm{m} 3\)
Division 1
\(R H O=2500,4500,3410 \mathrm{~kg} / \mathrm{m} 3\)
Division 2-3
\(R H O=2500,4500,4180,3900 \mathrm{~kg} / \mathrm{m} 3\)
Division 4
\(R H O=42500,500,3410 \mathrm{~kg} / \mathrm{m} 3\)
Concrete prestressed:
\(E=\)
40000
MPa
Naked =
0.2

RHO =
2500
kg/m3
AMOR_ALPHA \(=3.54\)
E-04
AMOR_BETA =
2.79

Used for the internal enclosure.

\subsection*{1.3 Characteristics}
elementary
To erase
thicknesses of hull 8, 4.5, 4 m
External enclosure
thickness 1.3 m
Internal enclosure
thicknesses 2.5, 1.3, 0.9 m
External walls

Floors of the boxes thickness 0.6 m
Internal structures
thicknesses \(7.5,2.8,1.9,1.5,1.2,1.0,0.8,0.6,0.5\) m
Division 0
thicknesses 3, 2.5, 2.0, 1.5, 1, 0.8, 0.6, 0.5, 0.4, 0.3 m
Division 2-3
thicknesses 0.8, 0.7, 0.6, 0.5, 0.4, 0.3 m
Beams squares dig height \(=0.5 \mathrm{~m}\), thickness \(=0.25 \mathrm{~m}\)

\section*{Division 1}
thicknesses 1.1, 0.8, 0.7, 0.6, 0.5, 0.4, 0.3 m
Beams squares dig height \(=0.5 \mathrm{~m}\), thickness \(=0.25 \mathrm{~m}\)
Division 4
thicknesses 1.1, 0.8, 0.7, 0.6, 0.5, 0.4, 0.3 m
Beams squares dig height \(=0.5 \mathrm{~m}\), thickness \(=0.25 \mathrm{~m}\)
Stiffnesses of ground resulting from PARASOL:
Stiffnesses of the foundation raft:
\(K x=3.3 \mathrm{E} 10 \mathrm{~N} / \mathrm{m}\)
\(K y=3.3 \mathrm{E} 10 \mathrm{~N} / \mathrm{m}\)
\(K z=3.3 \mathrm{E} 10 \mathrm{~N} / \mathrm{m}\)
\(K r x=6.01 \mathrm{E} 13 \mathrm{~N} . \mathrm{m}\)
\(K r y=5.76 \mathrm{E} 13 \mathrm{~N} . \mathrm{m}\)
\(K r z=5.76 \mathrm{E} 13 \mathrm{~N} . \mathrm{m}\)

Co-ordinates of the center: 0.0 .11 .6 m

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
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SDLS108 Use of RIGI_PARASOL within the framework of a study EPR
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Author (S):
G. DEVESA, Key P. LATRUBESSE

\section*{2.1 \\ Method of calculation used for the reference solution}

The modal damping coefficients are calculated according to the rule of the RCC-G [bib1]. They are obtained by summoning depreciation of the substructures constitutive of a building and them depreciation structural and geometrical of the ground balanced by their respective rates of energy potential compared to total potential energy, and that for each mode. There are 2 methods and two key words of CALC_AMOR_MODAL to estimate the contribution of the ground with the potential energy according to
that one average modal efforts (key word "RIGI_PARASOL") or modal displacements (key word "DEPL") with the nodes of the foundation raft.
2.2

Results of reference
The results of reference, frequencies, masses effective and modal depreciation are provided in the note bench-mark datum [bib2]. They were calculated by Code_Aster. It should be noted that modal depreciation was also calculated by affecting a stiffness to each node of the foundation raft (instead of RIGI_PARASOL).

\section*{2.3}

Uncertainty on the solution
Results of the industrial study. The case-test is thus included/understood like a test of not-regression of code.

\subsection*{2.4 References \\ bibliographical}
[1]
RCC-G Rules of design and construction of the nuclear small islands REFERENCE MARK - EDF Direction
equipment - Edition July 1988
[2]
EPR BASIC Design dynamic analyses - modal Modelling and analysis of NR. I. buildings - note E.N.T.MS 96.052 A

\footnotetext{
file:///Z|/process/valid/p890.htm (16 of 21)9/28/2006 4:31:13 PM
}
3 ModelingWith
3.1
Characteristics of modeling
PHENOMENON formulation: "MECHANICAL", MODELING: DKT.
3.2
Characteristics of the grid
A number of nodes: 6732
A number of meshs and type: 7677 QUAD4 and 1848 TRIA3
3.3 Functionalities
tested
Orders
AFFE_CARA_ELEM RIGI_PARASOL
CARA
VALE
MODE_ITER_INV
CALC_FREQ FREQ
OPTION
"NEAR"
POST_ELEM ENER_POT
CALC_AMOR_MODAL ENERSOL
METHOD
"RIGI_PARASOL"
AMOR_INTERNE
"DEPL"
AMOR_SOL
Handbook of Validation

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS108 Use of RIGI_PARASOL within the framework of a study EPR

\begin{abstract}
Date:
\end{abstract}

30/08/01
Author (S):
G. DEVESA, Key P. LATRUBESSE
:
V2.03.108-A Page:
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4
Results of modeling \(A\)

\subsection*{4.1 Depreciation}

Frequencies modes
Depreciation (\%)
Results of
Variations reference -
(Hz)
with RIGI_PARASOL
reference (\%)
calculation with
threshold \(=30 \%\)
Code_Aster (\%)
11.17453
20.905
20.905

0
2
1.17782
21.052
21.052

0
31.35640
30.000

0
42.16990
30.000
30.000

0
5
2.27813
29.925
29.925

0

\section*{Modes Frequencies Depreciation (\%)}

Results of
Variations reference -
(Hz)
with RIGI_PARASOL
reference (\%)
calculation with
without threshold
Code_Aster (\%)
11.17453
20.905
20.9050

2
1.17782
21.052
21.052

0
31.35640
54.970
54.970

0
42.16990
30.223
30.223

0
5
2.27813
29.925
```

Modes Frequencies Depreciation(%)
Results of
Variations reference -
(Hz)
with DEPL
reference (%)
calculation with
threshold = 30%
Code_Aster (%)
1 1.17453
21.033
21.033
0
2 1.17782
21.183
21.183
O
31.35640
30.000
30.000
0
4.16990
30.000
30.000
0
5.27813
30.000
30.000
0

```

\subsection*{4.2 Parameters}
```

of execution

```

Version: 5.02

\section*{Machine: SGI ORIGIN 2000}

\section*{Obstruction memory:}

400 megawords
Time CPU to use:
258 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
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:
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\section*{5 \\ Summary of the results}

Results of the depreciation obtained by CALC_AMOR_MODAL with the method of the forces (key word "RIGI_PARASOL") with or without truncation of threshold to \(30 \%\) are exactly the same ones as
those given in the reference [bib2]. They are very close to those obtained by CALC_AMOR_MODAL with the method of displacements (key word "DEPL"), also identical to those given in reference [bib2].

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
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Version
7.2

Titrate:
SDLS109-Eigen frequencies of a thick cylindrical ring
Date:
06/02/04
Author (S):
J.M. PROIX, Key S. CAILLAUD
:
V2.03.109-B Page:
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Organization (S): EDF-R \& D /AMA

SDLS109-Eigen frequencies of a ring cylindrical thick

\section*{Summary:}

This test is inspired by a vibratory study carried out on collector VVP of the N4 sections. This collector is
thick and a maximum report/ratio thickness presents on average radius of 0,13. This value, being able to be typical
of an industrial structure, is slightly higher than the limiting value of usually recognized validity for the plates and hulls. In this study, the modeling of the collector in hulls is then evaluated by comparison with a voluminal model on a ring.

This test makes it possible to evaluate the algorithm of search for eigenvalues MODE_ITER_SIMULT [U4.52.03] with operators of rigidity and mass corresponding to following modelings:
1) plates of the type DKQ (finite element MEDKQU4) and DSQ (finite element MEDSQU4), 2) plates of the type DKT (finite element MEDKTR3) and DST (finite element MEDSTR3) with an ear grid
and in star grid,
3) three-dimensional hulls of type COQUE_3D (finite elements MEC3QU9H and MEC3TR7H),
4) sections of hull in plane constraints of type COQUE_C_PLAN (finite element METCSE3), 5) telegraphic elements with kinematics of beam and modes of Fourier PIPE (finite element METUSEG3) and
TUYAU_6M (finite element MET6SEG3).
The results obtained are compared with the solution resulting from a voluminal modeling of the ring (element finished MECA_HEXA8) revealing the modes of Fourier of order 2 (ovalization) and 3 (trifoliate) like 2 modes except plan. The variations on the frequencies of the modes of ovalization and trifoliate are close to:
\(0.4 \%\) in DSQ and COQUE_3D in QUAD9,
0.7\% in COQUE_3D in TRIA7, COQUE_C_PLAN, PIPE and TUYAU_6M,
\(1 \%\) for the DKQ, DKT and DST.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SDLS109- Eigen frequencies of a thick cylindrical ring
Date:
06/02/04
Author (S):
J.M. PROIX, Key S. CAILLAUD
:
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\section*{1}

Problem of reference
1.1 Geometry

Rm
L

It is of a cylindrical ring, average radius \(\mathrm{Rm}=0.369 \mathrm{~m}\), about thickness \(T=0.048 \mathrm{~m}\) and of length \(L=0.05 \mathrm{Mr}\).

\section*{1.2 \\ Properties of material}

The material is homogeneous, isotropic, elastic linear. The elastic coefficients are:
\(E=185.000 \mathrm{MPa}\) and \(=0.3\).
The density is constant and is worth: \(=7800 \mathrm{~kg} . \mathrm{m3}\).

\section*{1.3 \\ Boundary conditions and loadings}

The structure is free in space.
1.4

Order of magnitude of the Eigen frequencies
The required clean modes correspond to the modes of Fourier of order 2 and 3 of the ring. frequencies of a ring can be estimated starting from an analytical model of curved beam of Euler [bibl]. For a mode of Fourier of order N, the frequency is worth:
```

N(N2 -)
I
E I
F
y
=
N
2
R2
2
m
m(N+)
I
Lt3

```
where: \(I=\)
=
y
and \(m\)

For the modes of ovalization \((N=2)\) and trifoliate \((N=3)\), the corresponding frequencies are worth respectively 211.65 Hz and 598.64 Hz . The search for clean modes is carried out on the tape \(200-800 \mathrm{~Hz}\) in order to collect these 2 modes of Fourier.

\subsection*{1.5 Reference \\ bibliographical}
[1]
Blevins R.D., Formulated for natural frequency and shape mode, N.Y.: Van Nostrand Reynhold Company, 1979, 492 p.
Handbook of Validation
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7.2

Titrate:
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Date:
06/02/04
Author (S):

\author{
J.M. PROIX, Key S. CAILLAUD
}

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\section*{2}

\section*{Modeling of reference}

\section*{2.1 \\ Characteristics of the modeling of reference}

Voluminal elements \(3 D\)

The discretized geometry is represented above. The elements 3D are voluminal with 8 nodes of type HEXA8. The number of nodes on the circumference is 600, on thickness 9 and length 9.

\section*{2.2 \\ Characteristics of the grid}

A number of nodes: 48600
A number of meshs and type: 38400 HEXA8

\author{
2.3 Functionalities \\ tested \\ Orders \\ AFFE_MODELE \\ AFFE \\ MODELING = " 3 D " \\ CALC_MATR_ELEM OPTION "RIGI_MECA" \\ "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ BANDAGE FREQ \(=(200 ., 800\). \\ Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HT-66/04/005/A
}

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SDLS109-Eigen frequencies of a thick cylindrical ring
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\section*{3 \\ Results of the modeling of reference}

\section*{3.1 \\ Values of reference tested}

Frequencies of the clean modes of ovalization, trifoliate and except plan.

\author{
Mode \\ Eigen frequencies (Hz) \\ ovalization 210.55 \\ 210.55 \\ trifoliate 587.92 \\ 587.92 \\ except plan \\ 205.89 \\ 205.89 \\ 588.88 \\ 588.88
}

\subsection*{3.2 Remarks}

The axisymmetric problem has double modes in the plan and except plan.
The modes except plan have the following deformations:
205.89 Hz :
588.88 Hz :

\subsection*{3.3 Uncertainties}

Uncertainty results from the analysis of convergence of the grid where Eigen frequencies with grid of reference \(600 \times 8 \times 8\) are compared with those of the grids 500x7x9.

\section*{A number of elements}

Eigen frequencies (Hz)
Circumference Thickness
Length
Ovalization Trifoliate
100
3
3
232.03
648.62

200
3
3
225.20
628.74

400
5

Uncertainty is \(0.05 \%\) on the frequency.

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\section*{4 Modeling \\ With}
4.1

\section*{Characteristics of modeling}

Plates \(D K Q\) and \(D S Q\).

The discretized geometry is represented above. Elements DKQ and DSQ are facets plane with four nodes of the type QUAD4. The number of nodes on the circumference is 100 and on length 5.

\section*{4.2 \\ Characteristics of the grid}

A number of nodes: 500
A number of meshs and type: 400 QUAD4

\subsection*{4.3 Functionalities}
tested

\author{
Orders
}

\author{
AFFE_MODELE \\ AFFE \\ MODELING = "DKT" or "DST" \\ AFFE_CARA_ELEM \\ HULL \\ THICK \(=0.048\) \\ CALC_MATR_ELEM OPTION \\ "RIGI_MECA" \\ "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ BANDAGE FREQ \(=(200 ., 800\). \\ Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HT-66/04/005/A
}

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\section*{5 \\ Results of modeling \(A\)}

\subsection*{5.1 Values}
tested
(Frequencies in Hertz)

\section*{Reference mode}

Aster
DKQ \%
difference Aster
DSQ \%
difference
ovalization 210.55211 .48
0.44
209.57
-0.46
210.55
211.48
0.44
209.57
-0.46
trifoliate 587.92
598.23
1.75
586.30
-0.27
587.92
598.23
1.75
586.30
-0.27
except plan
205.89
234.70
13.99
234.70
13.99

\subsection*{5.2 Remarks}

Modelings in plates DKQ and DSQ do not make it possible to represent the modes correctly except plan. One can think that that is due to the number of meshs over the too low length (4 with the place
from 8 in the modeling of reference). The first frequency except plan in plates DSQ must be lower than 200 Hz .

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\section*{6 Modeling}

The geometry discretized on the average radius \(R m=0.369 \mathrm{~m}\) is represented above. Elements DKT and DST are plane facets with three nodes of the type TRIA3 laid out out of ears. The number of nodes on the circumference is 100 and over length 5.

\section*{6.1 \\ Characteristics of the grid}

A number of nodes: 500
A number of meshs and type: 800 TRIA3

\subsection*{6.2 Functionalities}
tested
Orders
AFFE_MODELE
AFFE
MODELING \(=\) "DKT" or "DST"
AFFE_CARA_ELEM
HULL
THICK \(=0.048\)
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
BANDAGE FREQ \(=(200 ., 800\).
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\section*{7 \\ Results of modeling B}

\subsection*{7.1 Values}
tested
(Frequencies in Hertz)
Reference mode
Aster
DKT \%
difference Aster
DST \%
difference
ovalization 210.55211 .54
0.47
203.69
-3.25
210.55
211.54
0.47
203.69
-3.25
trifoliate 587.92
598.64
1.82
568.70
-3.27
587.92
598.64
1.82
568.70
-3.27
except plan
205.89
254.89
23.80

\subsection*{7.2 Remarks}

Modeling in plates DKT does not make it possible to represent the modes except plan correctly.
The errors on the frequencies in DST plates are relatively important.
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\section*{8 Modeling}

The geometry discretized on the average radius \(R m=0.369 \mathrm{~m}\) is represented above. Elements DKT and DST are plane facets with three nodes of the type TRIA3 laid out out of stars. The number nodes on the circumference is 100 and over length 5.

\section*{8.1 \\ Characteristics of the grid}

A number of nodes: 500
A number of meshs and type: 800 TRIA3

\author{
8.2 Functionalities \\ tested \\ Orders \\ AFFE_MODELE \\ AFFE \\ MODELING = "DKT" or "DST" \\ AFFE_CARA_ELEM \\ HULL \\ THICK \(=0.048\) \\ CALC_MATR_ELEM OPTION \\ "RIGI_MECA" \\ "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ BANDAGE FREQ \(=(200 ., 800\).
}

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\section*{9}

Results of modeling \(C\)

\subsection*{9.1 Values}
tested
(Frequencies in Hertz)

\section*{Reference mode}

Aster
DKT \%
difference Aster
DST \%
difference
ovalization 210.55211 .54
0.47
208.20
-1.11
210.55
211.54
0.47
208.20
-1.11
trifoliate 587.92
598.58
1.81
581.00
-1.18
587.92
598.58
1.81
581.00
-1.18
except plan
205.89
284.38
38.12
225.23
9.39
205.89
284.38
38.12
225.23
9.39
588.88
797.24
35.38
690.73
17.29
588.88
797.24
35.38
690.73
17.29

\subsection*{9.2 Remarks}

Modelings in plates DKT and DST do not make it possible to represent the modes correctly except plan.

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10 Modeling
D
10.1 Characteristics of modeling

COQUE_3D grid in QUAD9.

The geometry discretized on the average radius \(\mathrm{Rm}=0.369 \mathrm{~m}\) is represented above. Elements COQUE_3D are meshs with 9 nodes of the type QUAD9 making it possible to take into account the ray of
curve of the ring. The nodes mediums are well on the average circumference. The number of nodes on the circumference is 40 and over length 5.
10.2 Characteristics of the grid

A number of nodes: 200
A number of meshs and type: 40 QUAD9

\subsection*{10.3 Functionalities}
tested
Orders
CREA_MAILLAGE
MODI_MAILLAGE
OPTION = "QUAD8_9"
AFFE_MODELE
AFFE
MODELING = "COQUE_3D"
AFFE_CARA_ELEM
HULL
\(A_{-} C I S=0.833333\)
COEF_RIGI_DRZ = 1.E-5
THICK \(=0.048\)
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"

\title{
MODE_ITER_SIMULT
}

CALC_FREQ
BANDAGE FREQ = (200. , 800.)
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\section*{11}

Results of modeling \(D\)
11.1 Values
tested
(Frequencies in Hertz)
Reference mode
Code_Aster \%
difference
ovalization 210.55
209.91
-0.30
210.55
209.91
-0.30
trifoliate 587.92
586.51
-0.24
587.92
586.51

\subsection*{11.2 Remarks}

All the frequencies are correctly estimated.

\section*{Handbook of Validation}

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\section*{12 Modeling \\ E}

\subsection*{12.1 Characteristics of modeling}

The geometry discretized on the average radius \(\mathrm{Rm}=0.369 \mathrm{~m}\) is represented above. Elements COQUE_3D are meshs with 7 nodes of the type TRIA7 making it possible to take into account the ray of curve of the ring. The nodes mediums are well on the average circumference. The number of nodes on the circumference is 40 and over length 5.

\subsection*{12.2 Characteristics of the grid}

A number of nodes: 280
A number of meshs and type: 160 TRIA7

\subsection*{12.3 Functionalities}
tested
Orders
CREA_MAILLAGE
MODI_MAILLAGE
OPTION = "TRIA6_7"
AFFE_MODELE
AFFE
MODELING = "COQUE_3D"
AFFE_CARA_ELEM
HULL
\(A_{-} C I S=0.833333\)
COEF_RIGI_DRZ \(=1 . E-5\)
THICK \(=0.048\)
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
BANDAGE FREQ \(=(200 ., 800\).
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\section*{13 \\ Results of modeling \(E\)}

\subsection*{13.1 Values \\ tested}
(Frequencies in Hertz)

\section*{Reference mode}

Code_Aster \%
difference
ovalization 210.55211 .19
0.30
210.55
211.19
0.30
trifoliate 587.92
590.98
0.52
587.92
590.98
0.52
except plan
205.89
205.81
-0.04
205.89
205.81
-0.04
588.88
595.38
1.10
588.88
595.38

\subsection*{13.2 Remarks}

The frequencies are less better estimated that with elements QUAD9.

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\section*{14 Modeling \\ F}

\subsection*{14.1 Characteristics of modeling}

COQUE_C_PLAN.

The geometry discretized on the average radius \(\mathrm{Rm}=0.369 \mathrm{~m}\) is represented above. Elements COQUE_C_PLAN are meshs with 3 nodes of the type SEG3 making it possible to take into account the ray
of curve of the ring. The nodes mediums are well on the average circumference. The number of nodes on the circumference is 100.

\subsection*{14.2 Characteristics of the grid}

A number of nodes: 100
A number of meshs and type: 50 SEG3

\subsection*{14.3 Functionalities}
tested
Orders
```

AFFE_MODELE
AFFE
MODELING = "COQUE_C_PLAN"
AFFE_CARA_ELEM
HULL
$A_{-} C I S=0.833333$
MODI_METRIQUE = "NOT"
THICK $=0.048$
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
BANDAGE FREQ = (200. , 800. $)$
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```

Code_Aster \({ }^{\circledR}\)
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15
Results of modeling \(F\)

\subsection*{15.1 Values}
tested
(Frequencies in Hertz)
Reference mode
Code_Aster \%
difference
ovalization 210.55
209.14
-0.67
210.55
209.14
-0.67
trifoliate 587.92
583.28
-0.79
587.92
583.28
-0.79
except plan
205.89
205.89
588.88
588.88

\subsection*{15.2 Remarks}

Modeling in plane constraints does not make it possible to reveal the modes except plan of the ring.

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\section*{16 Modeling}

G

\subsection*{16.1 Characteristics of modeling}

PIPE and TUYAU_6M.
CENTER in SEG3

The geometry discretized on the axis of the ring is represented above. The elements PIPE and TUYAU_6M are meshs with 3 nodes of the type SEG3 representing the axis of the ring. The number of nodes on the axis is 5.

\subsection*{16.2 Characteristics of the grid}

A number of nodes: 5
A number of meshs and type: 2 SEG3

\subsection*{16.3 Functionalities}
tested
Orders

AFFE_MODELE
AFFE
MODELING = "PIPE" or "TUYAU_6M"
AFFE_CARA_ELEM
BEAM
SECTION = "CIRCLE"
MODI_METRIQUE = "NOT"
CARA = ("R" "EP")
\(V A L E=(0.395,0.048)\)
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"

MODE_ITER_SIMULT

CALC_FREQ
BANDAGE FREQ \(=(200 ., 800\).

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\section*{17 \\ Results of modeling \(G\)}

\subsection*{17.1 Values}
tested
(Frequencies in Hertz)
Reference mode
Aster
PIPE \%
difference Aster
TUYAU_6M \%
difference
ovalization 210.55209 .02
-0.72
209.02
-0.72
210.55
209.02
-0.72
209.02
-0.72
trifoliate 587.92
591.00
0.52
591.00
0.52
587.92
591.00
0.52
591.00
0.52
except plan
205.89
259.74
26.15
259.74
26.15
205.89
259.74
26.15
259.74
26.15
588.88
649.57
10.31
649.57
10.31
588.88
649.57
10.31
649.57
10.31

\subsection*{17.2 Remarks}

Modelings in PIPE (limited by construction to 3 modes of Fourier) and TUYAU_6M do not allow to represent the modes except plan correctly. On the other hand, they provide them same results on the outline views, close to the reference.

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18 Modeling
H

\subsection*{18.1 Characteristics of modeling}

Modeling SHB8

The discretized geometry is represented above. Elements SHB8 are pressed on meshs HEXA8. The number of elements on the circumference is 100 and over length 5.

\subsection*{18.2 Characteristics of the grid}

A number of nodes: 1000
A number of meshs and type: 400 HEXA8, 400 QUAD4

\subsection*{18.3 Functionalities}
tested
Orders
AFFE_MODELE
AFFE
MODELING = "SHB8"
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"

\title{
MODE_ITER_SIMULT
}

CALC_FREQ
BANDAGE FREQ = (200. , 800.)
CREA_MAILLAGE COQU_VOLU

\author{
Handbook of Validation
}

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19
Results of modeling \(\boldsymbol{H}\)

\subsection*{19.1 Values}
tested
(Frequencies in Hertz)

\section*{Reference mode}

Aster
DKQ \%
difference
ovalization 210.55210 .71
0.08
210.55
210.71
0.08
trifoliate 587.92
590.84
0.5
587.92
590.84
0.5
except plan
205.89
208.05
1.05
205.89
208.05

\section*{Summary of the results}

Even if the results obtained are honourable, models DKQ, DSQ, DKT, DST, COQUE_C_PLAN, PIPE and TUYAU_6M do not make it possible to estimate the modes except plan of the ring. Only model COQUE_3D with a grid in QUAD9 provides a good estimate frequential of these modes with an error lower than \(0.4 \%\).

Models DSQ and COQUE_3D in QUAD9 give an error close to \(0.3 \%\) for the modes of ovalization and trifoliate. For models COQUE_3D in TRIA7, COQUE_C_PLAN, PIPE and TUYAU_6M, this error border \(0.7 \%\) and are higher than \(1 \%\) for the \(D K Q, D K T\) and DST.

The best results of the DSQ compared to the DKQ confirm than the effect of shearing transverse is not negligible in the hulls notable thickness.

On a developable geometry like that of the cylinder studied here, finite elements quadrangular provide better results which triangular finite elements. DST, tested on an ear grid and a symmetrical star grid, are much less goods that DSQ. The same remark is also valid for the COQUE_3D in TRIA7 by report/ratio with the COQUE_3D in QUAD9.

The performances of the COQUE_3D are good quantitatively and in computing times. richness of the interpolation, the taking into account of the curve of the cylinder in the elements and metric correction carried out in the thickness of the hull seem to explain these good results.

Elements SHB8 make it possible to about obtain all the modes with a maximum change \(1 \%\).

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

Titrate:
SDLS114 Calculation of the modal stress intensity factors
Date:
07/1 1/05
Author (S):
E. GALENNE, Key S. DI DOMIZIO
:
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Organization (S): EDF-R \& D /AMA

\section*{Handbook of Validation}

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SDLS114 Calculation of the factors of intensity of constraint of a plate fissured by recombination modal

\section*{Summary}

This test aims at validating the calculation of the factors of intensity of a plate fissured by modal recombination.
modal factors of intensity, i.e associated each clean mode of vibration of the structure, are calculated with
operators CALC_G_THETA_T (option K_G_MODA) in 2D and CALC_G_LOCAL_T (option \(\left.K_{-} G_{-} M O D A\right)\) in 3D.

This test contains a modeling 2D and a modeling 3D. The reference solution results from one direct temporal resolution of the transitory problem.

Two modelings illustrate the possibility of recombining the modal factors of intensity directly in the command file by instructions python.
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

One considers a plate height \(\boldsymbol{H}=0,1 \mathrm{~m}\), dispatcher \(L=0,05 \mathrm{~m}\) and thickness \(E=0.005 \mathrm{Mr}\). Une crack is positioned in the middle of the height of the beam, with a depth of 0,1 L .

One considers the traditional properties of a steel:
Young modulus:
\(E=2.10+5 \mathrm{MPa}\)
Poisson's ratio:
\(=0.3\)
Density
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3}

Boundary conditions and loadings
The plate is:
- embedded on S1 surface;
- subjected to a force \(\boldsymbol{F}(T)\) on S2 surface.

The evolution of the standard of \(\boldsymbol{F}(T)\) is traced on the figure above. One takes \(=0,001 S\). the direction of
the force \(\boldsymbol{F}(T)\) is as follows:
- \(\boldsymbol{F}(T)=F(T)\).ex for modeling \(A\);
\(\cdot \boldsymbol{F}(T)=(a e x+\) bey \(+c e z) F(T)\) for modeling \(B\), with \(B=2 a\) and \(C=0.4 \mathrm{~A}\).
For modeling A, one blocks displacements in direction Z (plane problem).
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is that obtained by a direct temporal resolution of the problem transient. Operator DYNA_TRAN_EXPLI is used to identify the fields of displacement, with a diagram of integration in times of Newmark.
The evolution of the stress intensity factors according to time is then calculated by interpolation of the jumps of displacements (operator POST_K1_K2_K3).

\section*{2.2 \\ Result of reference Modeling A}

For modeling \(A\), the plate is requested by a force in the plan \((O, X, y)\) and displacements in direction \(Z\) are blocked. The result of reference, calculated by direct temporal resolution on a grid 2D, is traced on the following figure. The horizontal displacement top of the plate and it factor of intensity of the constraints oscillate with a frequency corresponding to the first clean mode structure.

\section*{2.3 \\ Result of reference Modeling B}

The evolution of the three factors of intensity of the constraints is traced on the following figure for the node
located in the middle of the bottom of crack. The oscillations of the factors of intensity of the constraints show
dominating contribution of the first mode of inflection of the plate in direction X and the first mode of inflection in direction Z .
1,6E+06
KI
1,4E+06
KII
1,2E+06
KIII
1,0E+06
\[
\begin{aligned}
& 8,0 E+05 \\
& 6,0 E+05 \\
& 4,0 E+05 \\
& 2,0 E+05 \\
& 0,0 E+00 \\
& 0 \\
& 0,001 \\
& 0,002 \\
& 0,003 \\
& 0,004 \\
& 0,005 \\
& \text { Time }(\mathbf{S})
\end{aligned}
\]

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.2

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SDLS114 Calculation of the modal stress intensity factors
Date:
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Author (S):

\author{
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\section*{2.4 \\ Uncertainty on the solution}

The explicit direct resolution of the transitory problem can be regarded as exact. Uncertainty on the identification of the factors of intensity of the constraints by interpolation of the jumps of displacements
is about 5\%.

\subsection*{2.5 References \\ bibliographical}
E. GALENNE, S. DI DOMIZIO: Method theta in breaking process: development bilinear form \(G\) in 3D and application to the case of dynamics low frequency,
Note EDF HT-65/05/024/A, 2005
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\section*{3 Modeling}

With

\section*{3.1}

\section*{Characteristics of modeling}

It is about a modeling 2D plane deformations. The calculation of the evolution of the factors of intensity constraints according to time is carried out in several stages:
- calculation of the first 15 clean modes of the structure;
- calculation of the modal factors of intensity of the constraints associated these modes by two methods;
- resolution of the transitory dynamic problem by projection on modal basis;
- recombination of \(K\) modal.

\section*{3.2 \\ Characteristics of the grid}

The grid is composed of quadratic elements. It comprises 2000 nodes and 700 meshs and is refined around the bottom of crack.
3.3 Functionalities
tested
Orders
MODE_ITER_SIMULT
POST_K1_K2_K3
CALC_G_THETA_T
Option K_G_MODA
DYNA_TRAN_MODA
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4
Results of modeling \(A\)
4.1 Values
tested
Modal values: case test of not-regression
Number of
\(\boldsymbol{K I}(\) POST_K1_K2_K3)
\(\boldsymbol{K I}\left(K_{-} G \_M O D A\right)\)
mode

Temporal values \(\mathrm{K} 1(\mathrm{~T}):\) comparison with the explicit resolution
Moment Reference Aster
\% difference
0,0005 24055,6
24337,0
1,2
0,001 44676,8
45159,3
1,1
0,002 90592,3
91679,4
1,2
0,003 134065,3
135633,9
1,2
0,004 181113,3
183286,7
1,2

\subsection*{4.2 Notice}

The difference between the modal values calculated by interpolation of the jumps of displacement or by method theta is weak and coherent with that observed on the static problems.

The value of \(\operatorname{KI}(T)\) is calculated starting from \(K\) I modal (method K_G_MODA) and of the coefficients of the resolution about modal base directly in the case test by lines of order in python:
```

M
K(T)=(T).I

```
\(K\)
where the coefficients (T
I) are the coefficients of modal participation, extracted the result from the operator DYNA_TRAN_MODA, and I
\(K\) are the modal factors of intensity of the constraints.
I
The precision obtained is satisfactory taking into account the number of elements retained in the base modal. The precision increases quickly with the number of modes [bib1].

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\section*{5 Modeling \\ B}
5.1

Characteristics of modeling
It is about a modeling 3D. The calculation of the evolution of the factors of intensity of the constraints in
function of time is fulfilled in several stages:
- calculation of the first 50 clean modes of the structure;
- calculation of the modal factors of intensity of the constraints associated these modes by two
methods;
- resolution of the transitory dynamic problem by projection on modal basis;
\(\cdot\) recombination of \(K\) modal.

\section*{5.2}

Characteristics of the grid
The grid is composed of linear elements. It comprises 8200 nodes and 8900 meshs and is refined around the bottom of crack.

\author{
5.3 Functionalities \\ tested \\ Orders \\ MODE_ITER_SIMULT \\ DEFI_FOND_FISS \\ POST_K1_K2_K3 \\ DEFI_FISS_XFEM \\ CALC_G_LOCAL_T \\ Option K_G_MODA \\ DYNA_TRAN_MODA \\ Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HT-66/05/005/A
}

Code_Aster \({ }^{\circledR}\)
Version
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Titrate:
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E. GALENNE, Key S. DI DOMIZIO

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\section*{Results of modeling B}

\subsection*{6.1 Values}
tested
The values indicated are those found with the node which is in the middle of the bottom of crack.
Modal values: case test of not-regression
```

Number of
KI (POST_K1_K2_K3)
KI (K_G_MODA)
\% difference
mode
$15,631 E+09$
4,790E+09
14,9
$28,599 E+09$
7,291E+09
15,2
3 6,940E+10
$5,897 E+10$
15,0
4-2,702E+11
$-2,897 E+11$
-7,2
$5-9,637 E+10$
$-8,165 E+10$
15,3

```

Temporal values KI (T): comparison with the explicit resolution
Moment (S)
Reference (Pa.m)
Aster (Pa.m)
\% difference
0.0005 696752,4
721825,9
3,6
0.001 1153703,3
1239061,8 7,4
0.002 997675,6
1110569,6

\section*{1364524,8}

4,5
\(0.004870347,2\)
1004735,2
15,4

\subsection*{6.2 Notice}

The difference between the modal values calculated by interpolation of the jumps of displacement or by method theta is high: that is explained by the linear grid very little refined in the thickness of the plate.

The value of \(K I(T)\) is calculated starting from \(K\) I modal (method \(\left.K_{-} G_{-} M O D A\right)\) and of the coefficients of
the resolution about modal base directly in the case test by lines of order in python:

\section*{M}
\(K(S, T)=(T) . I\)
\(K(S)\)
I
I
I
I 1
\(=\)
where the coefficients (T
I) are the coefficients of modal participation, extracted the result from the operator DYNA_TRAN_MODA, and I
\(K(S)\) are the modal factors of intensity of the constraints.
I
The precision obtained is satisfactory taking into account the number of elements retained in the base modal (50) and cuts it grid. The precision increases quickly with the number of modes [bib1].

\section*{Handbook of Validation}

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\section*{7}

Summary of the results
This test makes it possible to validate the calculation of the modal factors of intensity by the operators CALC_G_LOCAL_T and CALC_G_THETA_T (option K_G_MODA) and illustrate their use for the resolution
of a problem of breaking process in dynamics low frequency by modal recombination.
The relationship between the computing times of the resolution clarifies and of the resolution about modal base are ranging between 10 and 50 according to the type of grid, and the precision of the method of recombination
modal is fully satisfactory.

\section*{Handbook of Validation}

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\section*{Titrate:}

SDLS300 - Air cooler subjected to an excitation ground
Date

25/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
V2.03.300 document

SDLS300 - Air cooler subjected to an excitation ground

\section*{Summary:}

The applicability of this test is the seismic analysis. The studied structure is an air cooler subjected to an excitation ground.
Displacements are calculated along meridian in plan XZ.
The objective is to test axial, tangential and normal displacements for modal recombinations CQC and SRSS, for elements hull 3D.
Displacements of reference come from results obtained by several computer codes.
One is interested more particularly in calculation of the Eigen frequencies of this system. Values of reference
are obtained on a model of Fourier with the computation software of structures the SAMCEF software.

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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{The structure is defined by two arcs of hyperbole.}

\section*{Dimensions:}
\[
\begin{aligned}
& \text { ruffle }=55.587 \mathrm{~m} \\
& R b=37.515 \mathrm{~m} \\
& r c=35.532 \mathrm{~m} \\
& h 1=30.16 \mathrm{~m} \\
& H 2=107.01 \mathrm{~m} \\
& e p=0.305 \mathrm{~m} \\
& 1.2 \\
& \text { Properties of materials } \\
& \text { E }=2.76 \mathrm{E} 10 \mathrm{~Pa} \\
& =0.166 \\
& =2244 \mathrm{Kg} / \mathrm{m} 3
\end{aligned}
\]

\section*{1.3 \\ Boundary conditions and loading}

\section*{Boundary conditions}

Embedding of the air cooler on the level of the ground.

\section*{Loading:}

The air cooler is subjected to an excitation in direction \(X\) on the level of embedding. Calculation is realized starting from the spectrum of answer of speed [Figure 1.3-a].
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Appear 1.3-a: Spectrum of answer of speed

\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The spectral analysis is carried out with code CASTEM 2000.

\section*{2.2}

Results of reference
- Fréquences calculated with the SAMCEF software.
- Axial, tangential and normal Déplacements along the wall located in the \(x O z\) plan ( \(X\) and \(Z\) positive) for modal recombinations CQC and SRSS. (Calculated while taking into account 4 modes).

In order to improve the identification of the modes, one evaluates the spectrum with a model of Fourier. This modeling was carried out with software the SAMCEF software. The spectrum being very dense, the following table present that the frequencies lower than \(\mathbf{4 H z}\).

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Harmonic \(N^{\circ}\)
Frequencies
41,1451,17
3 1,24
41,3561,45
51,50
2 1,53
61,62
7 1,70
3 1,73
51,93
8 1,967 2,03
6 2,0992,26
7 2,28
6 2,428 2,45
4 2,47
7 2,55
10 2,60
8 2,6712,80

9 2,82
112,98
53,01
83,04
8 3,10
73,10
23,20
103,21
93,29
9 3,37
123,40
6 3,56
11 3,65
9 3,70
103,80
3 3,81
83,83
13 3,86
9 3,91
2.3

Uncertainty on the solution
Comparisons between codes.

\subsection*{2.4 References}
bibliographical
The case test is inspired by the following reference:
[1]
P.L. GOULD and S.H. ABU SITTA: Dynamic response of structures to wind and earthquake loading PENTECH PRESS
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\section*{3 Modeling}

With

\author{
3.1 \\ Characteristics of modeling
}

\section*{3.2 \\ Characteristics of the grid}

The grid consists of 1860 nodes and 1800 elements hull DKT.

\author{
3.3 \\ Functionalities tested \\ Orders
}

AFFE_MODELE AFFE MODELING
"DKT"
AFFE_CHAR_MECA DDL_IMPO
THICK AFFE_CARA_ELEM

POST_ELEM MASS_INER

MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
```

CALC_FREQ
OPTION
"BAND"
NORM_MODE NORMALIZES
"TRAN_ROTA"

```

\section*{Handbook of Validation}
```

V2.03 booklet: Linear dynamics of the hulls and plates
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```

Code_Aster \({ }^{\circledR}\)
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```

\section*{4}
```

Results of modeling $A$

```

\subsection*{4.1 Values}
```

tested
Eigen frequencies (Hz) below.
Number of the mode
Réf
Code_Aster
Difference between the SAMCEF software and
(The SAMCEF software)
Code_Aster in \%
12.80058
2.800650 .003
22.80058
2.800650 .003
35.92549
5.92490 -0.010
45.92549
5.92490 -0.010

```

Only the effective frequencies having a mass higher than 0.1\% are indicated in the table

These four modes make it possible to obtain a good representation of the mass in the direction of excitation since the effective mass according to \(X\) amounts to \(83.2 \%\)

Horizontal displacement \(\boldsymbol{X}(\boldsymbol{m})\) :
Altitude CASTEM

\section*{2000}

Code_Aster
Variation (\%)
(m) CQC SRSS CQC SRSS CQC

SRSS
13.3 1.318E-03
1.316E-03
1.538

E-03 1.432
E-03 16.6538 .740
26.8 1.484E-03
1.485E-03
2.853

E-03 2.669
E-03 92.15579 .744
40.3 1.898E-03
1.898E-03
4.456

E-03 4.249
E-03 134.714123 .819
49.4 2.4448E-03
2.442E-03 5.693

E-03 5.495
E-03 132.872124 .948
63.08 3.278E-03 3.275E-03 7.822

E-03 7.657
E-03 138.603133 .751
76.8 4.570E-03
4.568E-03
1.026

E-02 1.014
E-02 124.510121 .860
90.7 5.918E-03
5.918E-03
1.293

E-02 1.283
E-02 118.516116 .802
100 7.023E-03
7.024E-03
1.477

E-02 1.467
E-02 110.294108 .862
109.3 7.677E-03 7.677E-03 1.658

E-02 1.648
E-02 115.998114 .614
127.9 9.053E-03 9.054E-03 1.990

E-02 1.975
E-02 119.818118 .117

Vertical displacement Z (m):

\section*{Altitude CASTEM}

2000
Code_Aster
Variation (\%)
(m) CQC SRSS CQC SRSS CQC

SRSS

\subsection*{13.3 4.534E-04}
4.540E-04
8.297

E-04 8.101
E-04 82.95578 .407
26.8 6.832E-04
6.832E-04
1.712

E-03 1.674
E-03 150.528145 .025
40.3 1.091E-03
1.091E-03
2.464

E-03 2.408
E-03 125.872 120.680
49.4 1.510E-03
1.510E-03
2.892

E-03 2.823
E-03 91.52186 .877
63.08 1.794E-03 1.795E-03 3.424

E-03 3.337
E-03 90.76985 .848
76.8 1.944E-03
1.945E-03
3.840

E-03 3.737
E-03 97.437 92.035
90.7 2.024E-03

\subsection*{4.2 Remarks}

These four modes make it possible to obtain a good representation of the mass in the direction of excitation since the effective mass according to \(X\) amounts to 83.2\%.
Calculation is carried out without the taking into account of the neglected modes.
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Summary of the results
One is interested in two the first hundred modes of the structure: those are located in band 0 7Hz.
One finds well these frequencies with Code_Aster with a maximum change compared to the solution of reference of \(1.7 \%\).

Four modes make it possible to obtain an effective mass cumulated according to \(X\) and there of 83.2\%. Mass
effective cumulated according to \(Z\) is null. These effective weight breakdowns are identical to those obtained with the SAMCEF software.

For seismic calculation, displacements obtained are far away from the reference solution. However, one can express some doubts on the validity of these results of reference, being data the bad representation of the mass: indeed, for this calculation of reference, the mass effective cumulated in direction \(X\) accounts for \(43 \%\) of the total mass. This is why this part of calculation was put in comment in the case-test.

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\author{
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS501-Free vibrations of a corrugated sheet

\section*{Date:}

29/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.501-A Page:
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\author{
Organization (S): EDF/MTI/MMN, DeltaCAD
}

\title{
Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ Document: V2.03.501
}

SDLS501-Free vibrations of a corrugated sheet

\section*{Summary:}

This test represents a calculation in dynamic modal analysis of an iron corrugated into free-free. This test allows to validate modeling finite elements COQUE_D_PLAN. There is a test of the same structure in statics non-linear material (SSNV115) [V6.04.115].

The frequencies and the modes obtained are compared with a reference solution obtained with Code_Aster starting from a modeling D_PLAN.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS501 - Free vibrations of a corrugated sheet

Date:
29/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key

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1
Problem of reference

\subsection*{1.1 Geometry}
\(y\)
B
D
With
C
X
\(X\)
\(Z\)
\(y\)
sight of "crosses"

\section*{R}

R
With
C
\(X\)
\(X\)
R
R

\section*{Characteristics of the hull:}
- thickness \(\boldsymbol{H}=0.05 m\),
- radius of curvature \(R=1 . m\)
- width \(=A B=C D=0.1 \mathrm{~m}\),
\(\cdot\) the angle is selected so that the surface upper of the hull than item \(X\) is with \((y=0)\), i.e. aligned with \(A\) and \(C\).
1 H
\(\cos =1\)
\(4 R\)

\subsection*{1.2 Properties}
material
The properties of material constituting the plate are:
\(E=2 . E+11 \mathrm{~Pa}\)
Young modulus
\(=0.3\)
Poisson's ratio
\(=7800 . \mathrm{Kg} / \mathrm{m} 3\) Masses
voluminal

\section*{1.3}

Boundary conditions and loadings
No boundary condition: analyze dynamic into free-free
1.4 Conditions
initial
Without object
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Version
5.0

Titrate:
SDLS501-Free vibrations of a corrugated sheet

\section*{Date:}

29/10/01

\section*{Author (S):}

\section*{P. MASSIN, F. LEBOUVIER Key}

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution \\ Modeling A (D_PLAN) is used as reference for modeling COQUE_D_PLAN.}

\section*{2.2}

Results of reference
The first three nonnull Eigen frequencies.
Frequency mode 4:
658.24 Hz

Frequency mode 5:
1749.35 Hz

Frequency mode 6:
3225.42 Hz

\section*{2.3}

Uncertainties on the solution
\(\cdot\) not regression for modeling \(A\)
- < 2\% for modeling B

\author{
2.4 References \\ bibliographical \\ None. \\ Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HI-75/01/010/A
}

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

\section*{Titrate: \\ SDLS501-Free vibrations of a corrugated sheet}

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P. MASSIN, F. LEBOUVIER Key
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\section*{3 Modeling}

With

\author{
3.1 \\ Characteristics of modeling \\ Modeling D_PLAN \\ \(y\) \\ 4 \\ 0 \\ \(X\) \\ 20
}

\subsection*{3.2 Characteristics}
grid
A number of nodes: 289
A number of meshs and types: 80 QUAD8

\subsection*{3.3 Functionalities}
tested
Orders Key word
factor
Key word

\author{
AFFE_MODELE \\ AFFE \\ MODELING: "D_PLAN" \\ CALC_MATR_ELEM \\ OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT CALC_FREQ \\ METHOD: \\ "TRI_DIAG" \\ OPTION: "BAND" \\ FREQ: (500. 5000.)
}

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Moments Reference
Aster \%
difference
Frequency mode 4
658.24
658.24
0.

Frequency mode 5
1749.35
1749.35
0.

Frequency mode 6
3225.42
3225.42
0.

\subsection*{4.2 Remarks}

\subsection*{4.3 Parameters \\ of execution}

Version:

\title{
NEW 5.04.17 \\ Machine: \\ SGI-Origin2000 R12000 \\ Obstruction memory: 16 megabytes \\ Time CPU To use: 2.19 seconds \\ Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS501-Free vibrations of a corrugated sheet

Date:
29/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
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\section*{5 Modeling}

B
5.1

Characteristics of modeling
Modeling COQUE_D_PLAN
With
5
C
5
5
5
5.2

Characteristics of the grid

\section*{A number of nodes: 41}

\section*{A number of meshs and types: 20 SEG3}

\author{
5.3 Functionalities \\ tested \\ Orders Key word \\ factor Key word \\ AFFE_MODELE AFFE \\ MODELING
}

\author{
: \\ "COQUE_D_PLAN" \\ AFFE_CARA_ELEM HULL \\ THICK \\ MODI_METRIQUE: "YES"
}

CALC_MATR_ELEM

\author{
OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ OPTION: "BAND" \\ FREQ: (500. 5000.)
}

\section*{6}

Results of modeling \(B\)
6.1 Values
tested

\section*{Identification Moments Reference}

Aster \%
difference
Frequency mode 4
658.24
660.51
0.345

Frequency mode 5
1749.35
1759.09
0.557

Frequency mode 6

\subsection*{6.2 Remarks}

\subsection*{6.3 Parameters}
of execution
Version:
NEW 5.04.17
Machine:
SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 1.68 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS501 - Free vibrations of a corrugated sheet

\section*{Date:}

29/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
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7
Summary of the results
This case-test made it possible to test modeling COQUE_D_PLAN. The results obtained compared

\section*{has}
a solution resulting from a modeling D_PLAN are very good, the maximum change observed is \(\mathbf{0 . 5 \%}\).
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:
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Organization (S): EDF/AMA, DeltaCAD

Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
Document: V2.03.502

SDLS502 - Square plate "solid" simply
supported

\section*{Summary:}

This test simply represents a calculation in dynamic modal analysis of a thick square plate supported. This test makes it possible to validate:
\(\cdot\) modelings finite elements DST, DKT, COQUE_3D with meshs QUAD4 and TRIA3, QUAD8 and TRIA6, and 3D with meshs HEXA20, \(\cdot\) the taking into account of rigidity in transverse shearing.

The frequencies and the modes obtained are compared with a reference solution, suggested by NAFEMS, obtained with a calculation finite elements of voluminal type.

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{B}

C
H
```

Z,W
L
y,v
L=10.0 m
H=1.0m
With
D
X,U

```

\section*{1.2}

Properties of material
The properties of material constituting the plate are:
\(E=2.1011\)
Pa Modulus Young
\(=0.3\)
Poisson's ratio
= 8000. Kg/m3 Masses
voluminal

\section*{1.3 \\ Boundary conditions and loadings}
- C.L. : plate simply supported on its contour

\subsection*{1.4 Conditions}
initial

Without object
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates HT-66/02/001/A
\(\square\)
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported

\section*{Date:}

04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}
the reference solution suggested by NAFEMS [bib1] was obtained to leave a calculation finite elements
3D with elements bricks with 20 nodes and with a grid \(4 x 4\) (xy plan) and 1 following element the thickness.

\section*{2.2}

Results of reference
the first 7 nonnull frequencies and the associated clean modes, the first three modes are those of rigid bodies:
- Fréquence (mode 4 except plan)
: 44.762
Hz
- Fréquence (modes 5 \& 6 except plan)
: 110.52
Hz
- Fréquence (mode 7 except plan)
: 169.08
Hz
- Fréquence (Mode 8 in the plan)
: 193.93
Hz
- Fréquence (mode 9 \& 10 in the plan): 206.64

Hz
mode 4 except plan
\(5 \& 6\) mode except plan
mode 7 except plan
mode 8 in the plan
\(9 \& 10\) mode in the plan

\section*{2.3 \\ Uncertainties on the solution \\ < \(2 \%\) for a grid identical to that of [\$2.1], i.e. with few elements.}

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

NAFEMS: Standard The NAFEMS Benchmarks, TNSB, rev. October 3, 1990.

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported

\section*{Date:}

04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{\(\boldsymbol{Y}\) \\ C \\ Modeling DST (TRIA3) \\ - The plate is located in the plan \(\mathrm{Z}=2.3\) \\ B}
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
\(15^{\circ} 5\)
O

\section*{\(X\)}
- Dimensioned AB, BC, CD, DA: w=0
- To validate modeling in
a reference mark different from the reference mark total, the plate is turned of
15,5 degrees. This does not have
D
to change the Eigen frequencies obtained.
With

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: 122 \\ A number of meshs and types: 200 TRIA3}

\subsection*{3.3 Functionalities}

\section*{Orders Key word \\ factor}

Key word

\author{
AFFE_MODELE \\ AFFE \\ MODELING: `DST \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ CALC_MATR_ELEM \\ OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ METHOD: \\ "SORENSEN" \\ OPTION: "BAND"
}

4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Moments
Reference Aster \%
difference
Frequency
44.762
44.989
0.507
(mode 4 except plan)
Frequency
110.52
107.608
-2.634
(modes 5 \& 6 except plan)
107.880
-2.388
Frequency

\title{
1.114
}
(Mode 8 in the plan)
Frequency
206.64
211.658
2.428
(mode 9 \& 10 in the plan)
212.000
2.594

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:
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\subsection*{4.2 Remarks}
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection enters modes 8 and 9 of reference: these modes are them modes 6 and 7 of Code_Aster.

In the table below we deferred the first 14 found Eigen frequencies.
\(N^{\circ}\) mode
Frequency (Hz)
144,98
2107.61
3107.88
4165.45
5196.09
6202.80
7203.54
8211.66
9212.00
10222.53
11254.7412255.62
13264.73
14289.85
4.3 Parameters
of execution
Version:
NEW 5.04.17
Machine:
SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 4.62 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\section*{5 Modeling \\ B}

\section*{5.1}

\section*{Characteristics of modeling}

Y
C
Modeling DST (QUAD4)
- The plate is located in the plan \(\mathrm{Z}=2.3\)

B
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
\(15^{\circ} 5\)
O

\section*{\(X\)}
- Dimensioned \(A B, B C, C D, D A: w=0\)
- To validate modeling in
a reference mark different from the reference mark total, the plate is turned of
D
15,5 degrees. This does not have
to change the Eigen frequencies obtained.
With

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 122
A number of meshs and types: 100 QUAD4

\subsection*{5.3 Functionalities}
tested

\author{
Orders Key word \\ factor \\ Key word \\ AFFE_MODELE \\ AFFE \\ MODELING: `DST \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ CALC_MATR_ELEM \\ OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ METHOD: \\ "SORENSEN" \\ OPTION: "BAND"
}

\author{
6 \\ Results of modeling B \\ 6.1 Values \\ tested \\ \section*{Identification Moments} \\ Reference \\ Aster \% \\ difference \\ Frequency \\ 44.762 \\ 44.64 \\ -0.273 \\ (mode 4 except plan) \\ Frequency
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported

\section*{Date:}

04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

\subsection*{6.2 Remarks}
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection enters modes 8 and 9 of reference: these modes are them modes 6 and 7 of Code_Aster.

In the table below we deferred the first 14 found Eigen frequencies.
\(N^{\circ}\) mode
Frequency
144.64
2108.04
3108.26
4162.86
5195.70
6203.97
7206.08
8208.89
9208.89
10220.92
11248.12
12250.10
13252.49
14289.79

\subsection*{6.3 Parameters}
of execution
Version:
NEW 5.04.17

\author{
Machine: \\ SGI-Origin2000 R12000 \\ Obstruction memory: 16 megabytes \\ Time CPU To use: 4.81 seconds \\ Handbook of Validation \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\section*{7 Modeling}

C

\section*{7.1 \\ Characteristics of modeling}

\section*{Y}

C
Modeling DKT (TRIA6)
- The plate is located in the plan \(Z=2.3\)

B
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
\(15^{\circ} 5\)
O
X
- Dimensioned \(A B, B C, C D, D A: w=0\)
- To validate modeling cf page 4

D
With

\section*{7.2}

Characteristics of the grid
A number of nodes: 122
A number of meshs and types: 200 TRIA3

\subsection*{7.3 Functionalities \\ tested \\ Orders Key word factor}

Key word

\author{
AFFE_MODELE \\ AFFE \\ MODELING: "DKT" \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ CALC_MATR_ELEM \\ OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ METHOD: \\ "SORENSEN" \\ OPTION: "BAND"
}

\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested

\section*{Identification Moments}

Reference
Aster \%
difference
Frequency
44.762
47.358
5.799
(mode 4 except plan)
Frequency
110.52
118.029
6.795
(modes 5 \& 6 except plan)
118.059
6.822

Frequency
169.08
187.504
10.897

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\subsection*{8.2 Remarks}
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection after mode 11 of reference (mode 8 of Code_Aster), of the same form as those appeared in DST modeling enters modes 8 and 9 of reference (modes 6 and 7 of Code_Aster).

In the table below we deferred the first 14 found Eigen frequencies.
\(N^{\circ}\) mode
Frequency ( Hz )
147.358
2118.03
3118.06
4187.50
5196.09
6211.66
7212.00
8222.53
9235.41
10235.56
11264.73
12289.85
13302.84
14303.15

\subsection*{8.3 Parameters \\ of execution}
Version:
NEW 5.04.17
Machine:
SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 3.97 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

\section*{9 Modeling}

D

\title{
9.1 \\ \\ Characteristics of modeling
} \\ \\ Characteristics of modeling
}

\section*{Y}

C
Modeling DKT (QUAD4)
- The plate is located in the plan \(Z=2.3\)

B
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
\(15^{\circ} 5\)
O
X
- Dimensioned \(A B, B C, C D, D A: w=0\)
- To validate modeling in
a reference mark different from the reference mark total, the plate is turned of
D
15,5 degrees. This does not have to change the Eigen frequencies obtained.
With

\section*{9.2 \\ Characteristics of the grid}

A number of nodes: 122
A number of meshs and types: 100 QUAD4

\subsection*{9.3 Functionalities}
tested

\section*{Orders Key word}
factor
Key word

\author{
AFFE_MODELE \\ AFFE \\ MODELING: "DKT" \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ CALC_MATR_ELEM \\ OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ METHOD: \\ "SORENSEN" \\ OPTION: "BAND"
}

\section*{10 Results of modeling \(D\)}
10.1 Values
tested

\section*{Identification Moments}

Reference
Aster \%
difference
Frequency
44.762
47.182
5.408
(mode 4 except plan)
Frequency
110.52
117.463
6.283
(modes 5 \& 6 except plan)
Frequency
169.08
184.746
9.266
(mode 7 except plan)
Frequency
193.93
195.699
0.912
(Mode 8 in the plan)
Frequency
206.64
208.8871.088
(mode \(9 \& 10\) in the plan)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
SDLS502 - Square plate "solid" simply supported
Date:04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key:V2.03.502-A Page:11/18
10.2 Remarks
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection after mode 11 of reference (mode 8 of Code_Aster), of the same form as those appeared in DST modeling enters modes 8 and 9 of reference (modes 6 and 7 of Code_Aster).

In the table below we deferred the first 14 found Eigen frequencies.

\section*{\(N^{\circ}\) mode \\ Frequency (Hz)}
147.183
2117.46
3117.46
4184.75
5195.70
6208.89
10.3 Parameters
of execution
Version:
NEW 5.04.17
Machine:
SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 4.02 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:
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11 Modeling E
11.1 Characteristics of modeling

Modeling COQUE_3D (TRIA6)
- The plate is located in the plan \(\mathrm{Z}=2.3\)

B
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
\(15^{\circ} 5\)
O

\section*{\(X\)}
- Dimensioned AB, BC, CD, DA: w=0
- To validate modeling in
a reference mark different from the reference mark total, the plate is turned of

\section*{D}

15,5 degrees. This does not have
to change the Eigen frequencies obtained.
With

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 122
A number of meshs and types: 200 TRIA6

\subsection*{11.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE
AFFE
MODELING:
"COQUE-3D"
AFFE_CARA_ELEM
HULL
THICK
CALC_MATR_ELEM
OPTION: "RIGI_MECA"
OPTION: "MASS_MECA"

\section*{MODE_ITER_SIMULT}

CALC_FREQ
METHOD:
"SORENSEN"
OPTION: "BAND"

\section*{12 Results of modeling \(E\)}

\subsection*{12.1 Values}
tested
Identification Moments
Reference
Aster \%
difference
Frequency
44.762
43.867
2.00
(mode 4 except plan)
Frequency
110.52
106.058
-4.037
(modes 5 \& 6 except plan)
106.066
-4.029
Frequency
169.08
160.010
-5.305
(mode 7 except plan)
Frequency
193.93
193.600
-0.170
(Mode 8 in the plan)
Frequency
206.64
206.209
0.208
(mode \(9 \& 10\) in the plan) 206.211

Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:

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\subsection*{12.2 Remarks}
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection enters modes 8 and 9 of reference: they are modes 6 and 7 of Code_Aster.

In the table below we deferred the first 14 found Eigen frequencies.
\(N^{\circ}\) mode
Frequency (Hz)
1 43,867
2106.06
3106.07
4160.11

5 186,72
6193.60
7199.76
8200.23
9206.21
10206.21
11219.28
12245.91

\subsection*{12.3 Parameters}
of execution

\author{
Version: \\ NEW 5.04.17
}

\section*{Machine:}

SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 10.97 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0}

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported

\section*{Date:}

04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:
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\section*{13 Modeling \\ F}

\subsection*{13.1 Characteristics of modeling}

\section*{Y \\ C}

Modeling COQUE_3D (QUAD8)
- The plate is located in the plan \(\mathrm{Z}=2.3\)

B
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
\(15^{\circ} 5\)
O
X
- Dimensioned AB, BC, CD, DA: w=0
- To validate modeling in
a reference mark different from the reference mark
total, the plate is turned of
15,5 degrees. This does not have
D
to change the Eigen frequencies obtained.
With

\subsection*{13.2 Characteristics of the grid}

A number of nodes: 122
A number of meshs and types: 100 QUAD8

\subsection*{13.3 Functionalities}
tested

Orders Key word
factor
Key word

\section*{AFFE_MODELE}

AFFE
MODELING:
"COQUE_3D"
AFFE_CARA_ELEM
HULL
THICK
CALC_MATR_ELEM
OPTION: "RIGI_MECA"
OPTION: "MASS_MECA"
MODE_ITER_SIMULT
CALC_FREQ
METHOD:
"SORENSEN"
OPTION: "BAND"
14 Results of modeling \(F\)
14.1 Valuestested
Identification Moments
Reference
Aster \%
difference
Frequency
44.762
43.870
-1.993
(mode 4 except plan)
Frequency
110.52106.041
-4.052
(modes 5 \& 6 except plan)
Frequency
169.08
160.055
-5.337
(mode 7 except plan)
Frequency
193.93
193.588
-0.176
(Mode 8 in the plan)
Frequency
206.64
206.192
-0.216
(mode 9 \& 10 in the plan)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:
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\subsection*{14.2 Remarks}
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection enters modes 8 and 9 of reference: they are modes 6 and 7 of Code_Aster.

In the table below we deferred the first 14 found Eigen frequencies.

\author{
\(N^{\circ}\) mode \\ Frequency (Hz) \\ 143.87 \\ 2106.04 \\ 3106.04 \\ 4160.06 \\ 5193.59 \\ 6199.64 \\ 7200.13 \\ 8206.19 \\ 9206.19 \\ 10219.26 \\ 11245.68 \\ 12245.68 \\ 13249.20 \\ 14287.99 \\ \subsection*{14.3 Parameters} \\ of execution \\ Version: \\ NEW 5.04.17
}

\author{
Machine: \\ SGI-Origin2000 R12000 \\ Obstruction memory: 16 megabytes \\ Time CPU To use: 9.99 seconds \\ \section*{Handbook of Validation} \\ V2.03 booklet: Linear dynamics of the hulls and plates \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\section*{15 Modeling}

G

\subsection*{15.1 Characteristics of modeling}

Y
C
Modeling 3D (HEXA20)
- The average plan of the plate is located in the plan \(Z=2.3\) B
- Not O: (0. ; 0. ; 2.3)

Boundary conditions:
O
X
- Dimensioned \(A B, B C, C D, D A: w=0\)

Z
D
X
With
O

\subsection*{15.2 Characteristics of the grid}

A number of nodes: 1266
A number of meshs and types: 200 HEXA20

\subsection*{15.3 Functionalities}

\section*{Orders Key word \\ factor}

Key word

\author{
AFFE_MODELE \\ AFFE \\ MODELING: "3D" \\ CALC_MATR_ELEM \\ OPTION: "RIGI_MECA" \\ OPTION: "MASS_MECA" \\ MODE_ITER_SIMULT \\ CALC_FREQ \\ METHOD: \\ "SORENSEN" \\ OPTION: "BAND" \\ \section*{16 Results of modeling \(G\)} \\ \subsection*{16.1 Values} \\ tested \\ \section*{Identification Moments} \\ Reference \\ Aster \% \\ difference \\ Frequency \\ 44.762 \\ 43.862 \\ -2.009 \\ (mode 4 except plan) \\ Frequency \\ 110.52 \\ 105.953 \\ -4.132 \\ (modes 5 \& 6 except plan) \\ Frequency \\ 169.08 \\ 159.749 \\ -5.518 \\ (mode 7 except plan) \\ Frequency
}

\title{
193.93
}
193.590
\[
-0.175
\]
(Mode 8 in the plan)

\section*{Frequency}
206.64
199.410
-3.498
(mode 9 \& 10 in the plan)
199.903
-3.260
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported

\section*{Date:}

04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:

\section*{17/18}

\subsection*{16.2 Remarks}
- In Aster, the calculated modes are those of rigid body: the fourth mode of reference is it first mode calculated by Code_Aster.
- Apparition of two modes of inflection enters modes 8 and 9 of reference: they are modes 6 and 7 of Code_Aster.

In the table below we deferred the first 14 found Eigen frequencies.
\(N^{\circ}\) mode
Frequency (Hz)
143.86
```

2 105.95
3105.95
4159.75
5193.59
6}199.4
7}199.9
8206.16
906.16
10219.27
11245.07
12245.07
13249.13
14287.75

```

\subsection*{16.3 Parameters}
of execution
Version:
NEW 5.04.17

\section*{Machine:}

SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 24.77 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLS502 - Square plate "solid" simply supported
Date:
04/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.502-A Page:
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17 Summary of the results
Taking into account the nature of the numerical solution (voluminal finite elements), the results obtained are satisfactory for:
\(\cdot\) modeling \(A\) and \(B(D S T)\) the maximum change is less than \(4 \%\) for the first 5 modes, \(\cdot\) modeling \(E\) and \(F\left(C O Q U E \_3 D\right)\), the maximum change is approximately \(5 \%\) for the 5 first modes,
- modeling \(G(3 D)\), the maximum change is approximately \(5 \%\) for the first 5 modes,
- the modes of reference \(5 \& 6\) except plan have symmetry different from those met in modelings \(E, F\) and \(G\), but they are equivalent because it is about modal recombinations.

Modelings C and D (DKT) are less satisfactory with relative variations reaching 10\% on mode 7 except plan, this due to is not taken into account of transverse shearing for this plate relatively thick.
Moreover one observes the appearance of modes of inflection and membrane for all these modelings,
\(y\)
included/understood voluminal modeling 3D G. When one refines sufficiently the grids, this tendency is confirmed and the relative variations regress. Calculation 3D in addition showed that in on this side
of a grid \(6 x 6\) in plan (XY), the modes of inflection and membrane were not detected.

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.503-A Page:
1/12
Organization (S): EDF/MTI/MMN, DeltaCAD

SDLS503 - Vibrations of inflection of a beam sandwich

\section*{Summary:}

This test represents a calculation in modal analysis of a sandwich beam simply supported. This test allows to validate:

\section*{modeling finite elements DKT with meshs QUAD4 and TRIA3,} modeling finite elements DST with meshs QUAD4 and TRIA3, the taking into account of rigidity in transverse shearing, the taking into composite material account.

The frequencies and the modes obtained are compared with an analytical reference solution. Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLS503 - Vibrations of inflection of a sandwich beam

\section*{Date:}

01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.03.503-A Page:
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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}
\(Z, W\)
\(y, v\)
L
D
\(X, U\)
C
B
Heart
h1
H2
\(L=1.0 \mathrm{~m}\)
\(D=0.1 m\)
With
\(H=0.1 m\)
1
D
\(H=0.05 m\)
2

\section*{Properties of material}

\section*{Coatings:}
\(E=4.1010 \mathrm{~Pa}\)
\(G=4.109 \mathrm{~Pa}\)
\(=0.3\)
\(=2000 \mathrm{~kg} / \mathrm{m} 3\)
\(X\)
\(x z\)
\(\boldsymbol{x} z\)
1
Heart:
\(E=4.107 \mathrm{~Pa}\)
\(G=1.5 .107 \mathrm{~Pa}\)
\(=0.3\)
\(=50 \mathrm{~kg} / \mathrm{m} 3\)
\(X\)
\(x z\)
\(x z\)
2
Coefficient of shearing K: \(1 / K=110.8\)
The Poisson's ratios are identical: ==
\(x z\)
\(x y\)
\(y z\)

\section*{1.3}

Boundary conditions and loadings
C.L. : the beam rests simply on the with dimensions \(A B\) and CD.

\subsection*{1.4 Conditions}
initial
Without object
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version

\section*{5.0}

\section*{Titrate:}

SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.503-A Page:
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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

Calculation is carried out starting from the relations of dynamic balance and behavior [bib2] pointed out
hereafter:
M
2
\(T\)
2
\(X\)
\(y\)
\(v\)
\(+T=\)
\(=\)

I
\(S\)
\(X\)
\(y\)
T 2 X
T 2
\(v\)
\(M\)
I.E.(internal excitation)
transverse \(v(X, T)\). One
1
(+has)
1 -has 2
2
1
1
\(\sin (X)=0\) with \(X\)
2
2
2
\(=\)
\(+\)
2
2
2
\(2+r 2\)
and

These relations make it possible to write the equation of the movement of dynamic inflection
the equation at the Eigen frequencies obtains after having associated the boundary conditions. The equation at the Eigen frequencies is written:
\(I 22 l\)
\(I\)
S I.E.(INTERNAL EXCITATION)
\(2=\)
\(; R 2=\)
; has \(=\)
I.E.(internal excitation)
S l2
K I GS

\title{
The solutions of the equation at the Eigen frequencies are written then: \(X=N(n=1,2,3, \ldots)\)
}
2
2.2

Results of reference
the first 5 frequencies and clean modes of inflection associated.

\section*{Frequency mode 1:}
64.476 Hz

Frequency mode 2: 131.918 Hz

\section*{Frequency mode 3:}
198.734 Hz

\section*{Frequency mode 4:} 265.383 Hz

\section*{Frequency mode 5:}
331.963 Hz

\section*{2.3}

Uncertainties on the solution
The reference solution is calculated within the framework of the assumptions of the theory of the beams [bib2]:
\(=0\).
```

y
Z

```

\subsection*{2.4 References bibliographical}

\section*{[1]}

VPCS: Software package of composite structural analysis; Examples of validation. Review of composites and of advanced materials, Volume 5-number except series 1995-Hermes Edition. [2]
CIEAUX J.M.: Dynamic inflection of the composite beams with orthotropic phases; Validity of quasi-static field, thesis of the university Paul Sabatier Toulouse III, 1988.
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.03.503-A Page:
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3 Modeling
With

\section*{3.1}

Characteristics of modeling
Z, W
Modeling DKT (TRIA3)
C
B
D
\(y, v\)
With
\(9.48^{\circ}\)
\(X, U\)
- Plate located in the plan \(x=0.33\)
- Boundary conditions: Dimensioned AB and CD: u=0

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: 22 \\ A number of meshs and type: 20 TRIA3}

\subsection*{3.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE
AFFE
MODELING: "DKT"
AFFE_CARA_ELEM HULL
THICK
DEFI_MATERIAU
ELAS_ORTH
E_L E_T G_LT G_LN
NU_LT NU_LN RHO
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_INV CALC_FREQ
OPTION
: "ADJUSTS"
FREQ: (5. 8500.)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\title{
Titrate: \\ SDLS503 - Vibrations of inflection of a sandwich beam
}

\section*{Date:}

01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.03.503-A Page:
5/12

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Reference Aster \%
difference
Frequency mode 1
64.476
277.449
330.

Frequency mode 2
131.918
1105.83
738.

Frequency mode 3
198.734
2473.80
1.14E3

Frequency mode 4
265.383
4363.97
1.54E3

Frequency mode 5
331.963
6753.904
1.93E3
4.2 Remarks

In the table of results, we deferred the frequencies whose modes are identical to modes of reference.
the effects of transverse shearing are neglected in modeling "DKT",
the Aster results are much higher than the results of reference,
appearance of a mode of membrane enters modes 2 and 3 and between modes 5 and 6 of reference.
4.3 Parameters
of execution
Version:
NEW 5.04.17
Machine:
SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 3.37 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.503-A Page:
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\section*{5 Modeling}

B
5.1Characteristics of modeling
Z, W
Modeling DKT (QUAD4)
C
B
D
\(y, v\)
With
\(9.48^{\circ}\)
\(\boldsymbol{X}, \boldsymbol{U}\)
- Plate located in the plan \(x=0.33\)
- Boundary conditions: Dimensioned AB and CD: \(u=0\)
5.2
Characteristics of the grid
A number of nodes: 22
A number of meshs and type: 10 QUAD4
5.3 Functionalities
tested
Orders Key word
factorKey word
AFFE_MODELE
AFFE
MODELING: "DKT"
AFFE_CARA_ELEM HULL
THICK
DEFI_MATERIAU
ELAS_ORTH
\(\boldsymbol{E}_{-} L E_{-} T G_{-} L T G_{-} L N\)
NU_LT NU_LN RHO
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"

\section*{MODE_ITER_INV CALC_FREQ OPTION}

\section*{: "ADJUSTS"}

FREQ: (5. 8500.)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.503-A Page:
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\section*{6}

Results of modeling B

\subsection*{6.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
Frequency mode 1
64.476
277.788

331
Frequency mode 2
131.918
1111.225
742.

Frequency mode 3
198.734
2500.930
1.16E3

\section*{Frequency mode 4}
\[
265.383
\]
4449.073
1.52E3

Frequency mode 5
331.963
6960.324
2.00E3

\subsection*{6.2 Remarks}

In the table of results, we deferred the frequencies whose modes are identical to modes of reference.
the effects of transverse shearing are neglected in modeling "DKT",

the Aster results are much higher than the results of reference,
appearance of a mode of membrane enters modes 2 and 3 and between modes 5 and 6 of reference.

\subsection*{6.3 Parameters \\ of execution}

Version:
NEW 5.04.17

\section*{Machine: \\ SGI-Origin2000 R12000}

Obstruction memory: 16 megabytes
Time CPU To use: 3.53 seconds

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A

\section*{7 Modeling}

C

\section*{7.1}

\section*{Characteristics of modeling}

Modeling DST (TRIA3)
Z, W
C
B
D
\(y, v\)
With
\(9.48^{\circ}\)
\(X, U\)
- Plate located in the plan \(x=0.33\)
- Boundary conditions: Dimensioned \(A B\) and \(C D: u=0\)

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 22
A number of meshs and type: 20 TRIA3

\subsection*{7.3 Functionalities}
tested
```

Orders Key word
factor
Key word
AFFE_MODELE
AFFE
MODELING:"DST"
AFFE_CARA_ELEM HULL
THICK
DEFI_MATERIAU
ELAS_ORTH
E_LE_T
G_LT G_LN
NU_LT NU_LN
RHO
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_INV CALC_FREQ OPTION
: "ADJUSTS"
FREQ: (5. 500.)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A

```
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key

V2.03.503-A Page:
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8
Results of modeling \(C\)

\subsection*{8.1 Values \\ tested}

\section*{Identification Reference}

Aster \% difference
Frequency mode 1
64.476
64.573
0.150

Frequency mode 2
131.918
133.987
1.568

Frequency mode 3
198.734
206.046
3.679

Frequency mode 4
265.383
282.875
6.591

Frequency mode 5
331.963
365.919
10.229

\subsection*{8.2 Parameters \\ of execution}

Version:
NEW 5.04.17
Machine:
SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 2.84 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates
HI-75/01/010/A

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.503-A Page:
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\section*{9 Modeling}

D
9.1

Characteristics of modeling
Modeling DST (QUAD4)
Z, W
C
B
D
\(y, v\)
With
\(9.48^{\circ}\)
\(X, U\)
- Plate located in the plan \(x=0.33\)
- Boundary conditions: Dimensioned \(A B\) and \(C D: u=0\)

\section*{9.2 \\ Characteristics of the grid}

A number of nodes: 22
A number of meshs and type: 10 QUAD4

\subsection*{9.3 Functionalities}
tested
```

Orders Key word
factor
Key word
AFFE_MODELE
AFFE
MODELING:"DST"
AFFE_CARA_ELEM HULL
THICK
DEFI_MATERIAU
ELAS_ORTH
E_LE_T
G_LT G_LN
NU_LT NU_LN
RHO
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_INV CALC_FREQ
OPTION
: "ADJUSTS"
FREQ: (5. 500.)
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates

```
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
SDLS503 - Vibrations of inflection of a sandwich beam

Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V2.03.503-A Page:
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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
Frequency mode 1
64.476
64.595
0.184

Frequency mode 2
131.918
131.495
-0.320
Frequency mode 3
198.734
196.861
-0.942
Frequency mode 4 265.383
260.247
-1.935
Frequency mode 5
331.963
320.409
-3.480

\author{
10.2 Parameters \\ of execution \\ Version: \\ NEW 5.04.17 \\ Machine: \\ SGI-Origin2000 R12000
}

Obstruction memory: 16 megabytes
Time CPU To use: 3.21 seconds
Handbook of Validation
V2.03 booklet: Linear dynamics of the hulls and plates HI-75/01/010/A

Modeling DKT is not adapted to model this case-test, the errors are very important. Formulation DKT does not take into account transverse shearing contrary to modeling DST. For this type of example, where the structure makes up of a composite material and relatively thick ( \(h / L=0.1\) ), it is preferable to use DST modeling.

The results obtained with DST are:
satisfactory for the first 3 frequencies with mesh TRIA3 and for the 5 first frequencies for mesh QUAD4 with a better precision for mesh QUAD4,
the error of \(10 \%\) for 4th and 5th frequency with mesh TRIA3 is significant. One finer grid should make it possible to improve the results by having the best representation of the last modes.

\section*{Handbook of Validation}

V2.03 booklet: Linear dynamics of the hulls and plates

\section*{HI-75/01/010/A}

Code_Aster \({ }^{\circledR}\)
Version
4.0

\section*{Titrate:}

SDLV100 Vibration of a slim beam of rectangular section
Date:
31/01/00
Author (S):

\title{
D. Key GIRARDOT
}
:
V2.04.100-A Page:
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Organization (S): EDF/EP/AMV

Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
Document: V2.04.100

SDLV100 - Vibration of a slim beam
of variable rectangular section (embed-free)

\section*{Summary:}

The studied structure is a beam out of free steel embedded with rectangular variable section modelled by
voluminal elements. One is interested in his Eigen frequencies in inflection. The same problem is dealt with in
modeling beam in the case test SDLL09.
This problem makes it possible to test voluminal elements MECA_HEXA20 and MECA_PENTA15 in modal analysis.
It also makes it possible to test option MASS_MECA_DIAG of calculation of the matrices of mass diagonalized for voluminal modelings.

The reference solution is a numerical solution obtained using the computer code by finite elements The SAMCEF software for similar modelings. The results obtained are also in concord with semi-analytical results given in guide VPCS.

Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLV100 Vibration of a slim beam of rectangular section
Date:
31/01/00
Author (S):
D. Key GIRARDOT
:
V2.04.100-A Page:
2/8

1
Problem of reference

\subsection*{1.1 Geometry}
\(y, v\)
\(L\)
\(y\)
\(y\)
bo
b1
B
H
H
O
h1
H
With
1
Z
```

O
X,U
Z
Z,W
b1
bo
Rectangular sections

```

\section*{Length of the beam:}
\(L=1 \mathrm{~m}\)
Rectangular section:

\section*{Initial cross-section}

Final cross-section
height:
\(H o=0.04 \mathrm{~m}\)
\(h 1=0.01 \mathrm{~m}\)
width:
bo \(=0.04 \mathrm{~m}\)
\(b 1=0.01 \mathrm{~m}\)
surface:
\(A o=1.6103 \mathrm{~m} 2\)
\(A 1=1.104 \mathrm{~m} 2\)
inertia:
\(l z o=2.1333107 \mathrm{~m} 4 \mathrm{Izl}=8.33331010 \mathrm{~m} 4\)
Co-ordinates of the points (in meters)

\section*{WITH B}
\(X\)
0. 1.
\(y\)
0. 0 .

Z
0. 0.

\section*{1.2}

Properties of steel

\section*{1.3 \\ Boundary conditions and loadings}

Not a: embedded \(U=v=Z=0\)
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLV100 Vibration of a slim beam of rectangular section
Date:
31/01/00
Author (S):
D. Key GIRARDOT
:
V2.04.100-A Page:
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is obtained using the computation software by finite elements the SAMCEF software for
identical modelings but with elementary matrices of mass coherent.
One points out the analytical solution given in card SDLL09/89 of guide VPCS. The equation differential in inflection of the beam considered, in theory of Euler-Bernoulli is written (Theory of Euler-Bernoulli):
where Iz and \(A\) vary with the \(X\)-coordinate.

The Eigen frequencies are then of the form:

H
B
1
1
For this value of and, the first values of the continuation (I) are:

\section*{2.2}

\section*{Results of reference}

The results of reference selected are the first 5 Eigen frequencies of the modes of inflection.

\section*{2.3}

Uncertainty on the solution
Analytical solution in theory of beam of Bernoulli, and numerical solution the SAMCEF software.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}
H.H. MABIE, C.B. ROGERS, Transverse vibrations of double-tapered cantilever beams -

Newspaper of the Acoustical Society of America, \(n^{\circ}\) 51, p. 1771-1774 (1972).
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures

\section*{HT-62/01/012/A}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SDLV100 Vibration of a slim beam of rectangular section

Date:
31/01/00
Author (S):

\section*{D. Key GIRARDOT}

V2.04.100-A Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Elements of volume MECA_HEXA20

Discretization:
beam AB: 30 meshs HEXA20
(1 mesh in the section)
Boundary conditions:
in all the nodes
DDL_IMPO: (ALL: "YES" DZ: 0.)
at end \(A\) (group of G_1 nodes) (GROUP_NO: G_1 DX: 0. , DY: 0)

\section*{3.2 \\ Characteristics of the grid}

Grid:
A number of nodes: 368
A number of meshs and type: 30 HEXA20

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

\author{
Keys \\ AFFE_CHAR_MECA DDL_IMPO \\ [U4.25.01] \\ "MECHANICAL" AFFE_MODELE \\ "3D" \\ [U4.22.01] \\ DEFI_MATERIAU ELAS \\ [U4.23.01] \\ CALC_MATR_ELEM OPTION \\ "MASS_MECA_DIAG" \\ [U4.41.01] \\ MODE_ITER_INV
}
[U4.52.01]

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLV100 Vibration of a slim beam of rectangular section
Date:
31/01/00
Author (S):

\section*{D. Key GIRARDOT}

V2.04.100-A Page:
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\section*{4}

Results of modeling \(A\)

\author{
4.1 Values \\ tested \\ \section*{Identification Solution} \\ beam \\ Reference \\ Aster \% \\ difference \\ analytical \\ The SAMCEF software \\ The Aster-SAMCEF software \\ frequency \\ in HZ \\ in \(H Z\)
}
coherent matrix
inflection 1
54.18
56.84
56.85
\(0.0176 \%\)
inflection 2
171.94
180.0
180.08
\(0.0444 \%\)
inflection 3
384.40
401.0
401.23
\(0.0574 \%\)
inflection 4
697.24
723.2

\subsection*{4.2 Parameters of execution}

Version: NEW4.03.06

\section*{Machine: CRAY C90}

Obstruction memory: 8 MW, time CPU To use: 32.08 seconds.

\author{
Handbook of Validation \\ V2.04 booklet: Linear dynamics of the voluminal structures HT-62/01/012/A
}

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Titrate:
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31/01/00
Author (S):
D. Key GIRARDOT

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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

Elements of volume MECA_PENTA15

Discretization:
beam AB: 60 meshs PENTA15
( 2 meshs in the section)
Boundary conditions:
```

in all the nodes
DDL_IMPO: (ALL: "YES" DZ: 0.)
at end $A$ (group of $G_{-} 1$ nodes) (GROUP_NO: G_1 DX: 0. , DY: 0)

```

\section*{5.2 \\ Characteristics of the grid}

Grid:
A number of nodes: 368
A number of meshs and type: 60 PENTA15

\author{
5.3 Functionalities \\ tested \\ \section*{Orders} \\ Keys \\ AFFE_CHAR_MECA DDL_IMPO \\ [U4.25.01] \\ "MECHANICAL" AFFE_MODELE \\ "3D" \\ [U4.22.01] \\ DEFI_MATERIAU ELAS \\ [U4.23.01] \\ CALC_MATR_ELEM OPTION \\ "MASS_MECA_DIAG" \\ [U4.41.01] \\ MODE_ITER_INV
}
[U4.52.01]

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4.0

Titrate:
SDLV100 Vibration of a slim beam of rectangular section
Date:
31/01/00
Author (S):

\section*{D. Key GIRARDOT}

\author{
6 \\ Results of modeling B
}

\author{
6.1 Values
}
tested

\section*{Identification Solution}
beam
Reference
Aster \%
difference
semi-analytical
The SAMCEF software
ASTER-SAMCEF
frequency
in HZ
in HZ
inflection 1
54.18
56.84
56.82
-0.038\%
inflection 2
171.94
180.00
179.96
-0.022\%
inflection 3
384.40

\title{
6.2 Parameters of execution
}

Version: NEW4.03.06
Machine: CRAY C90
Obstruction memory: 8 MW, time CPU To use: 47.09 seconds.

\section*{Handbook of Validation}

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31/01/00
Author (S):
D. Key GIRARDOT

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\section*{7}

Summary of the results
The differences between the results of calculations Aster and the SAMCEF software with coherent masses are lower than
\(0.2 \%\).

Differences between the computation results Aster with diagonal masses and the SAMCEF software with masses
coherent remain lower than \(1 \%\).
These results are in conformity so that one could wait, and validate in a reliable way them calculations of Eigen frequencies in Aster by MODE_ITER_INV and operator CALC_MATR_ELEM in coherent masses as in diagonal masses.

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Titrate:
SDLV111 Homogenisation of a network of beams
Date:
08/01/98
Author (S):
B. QUINNEZ, H. HADDAR

Key:
V2.04.111-A Page:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures

\section*{V2.04.111 document}

\section*{SDLV111-Homogenisation of a network}

\section*{beams in an incompressible fluid}

\section*{Summary:}

Test in modal analysis, being used to validate the elements of modeling 3D_FAISCEAU: héxaèdre with 8 nodes
or héxaèdre with 20 nodes. These elements represent the homogenized medium of a bathing network of beams
in an incompressible fluid, initially at rest.
One tests the Eigen frequencies of the beams of the medium homogenized without or with fluid.
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\section*{Titrate:}

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Date:
08/01/98
Author (S):
B. QUINNEZ, H. HADDAR

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1
Problem of reference

\subsection*{1.1 Geometry}

One considers a periodic network of \(4 X 4\) beams [1.1-a]. The period of the field is \(Y\). appear [1.1-b] represents an enlarging of 1 of the period. Each beam is right of square section.
Surface top: HS
\(L\)
Z
\(y\)
Surface bottom: Sb

\section*{X}

Appear 1.1-a: Geometry of the heterogeneous medium-Beams without fluid E
Y2
Y1
has
1
Appear 1.1-b: Cell of reference \(\boldsymbol{Y}\) - Enlarging from \(=10\)
- Caractéristiques of the period:
- Dimensions
:
\(Y=(0.21 \mathrm{~m}, 0.21 \mathrm{~m})\)
\(=1.5 \mathrm{~m}\) have
\(E=0.3 \mathrm{~m}\)
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- Caractéristiques of each beam:
- Section

With \(=(X\) has \() 2=(0.1 \times 1.5) 2=0.0225 \mathrm{~m} 2\)
- Length
:
\(L=4.1 \mathrm{~m}\)
Moment of inertia of inflection:
\(I x=I y=(X\) has \() 4 / 12 m 4\)

\section*{1.2}

\section*{Material properties}

Isotropic linear elastic material:
\(E=109 \mathrm{~Pa}\)
NAKED \(=0.3\)
Densities:
Beam:
Rho \(=7641 \mathrm{~kg} / \mathrm{m} 3\)
Fluid:
Rho \(=0 \mathrm{~kg} / \mathrm{m} 3\) (case without fluid)
Rho \(=1000 \mathrm{~kg} / \mathrm{m} 3\) (case with fluid)

\subsection*{1.3 Terms}
correctors
The correct terms are calculated on the cell of reference Y [1.1-b]. B_T
\(=\)
0.79 m 2

B_N
\(=\)
0.79 m 2

B_TN
\(=\)
0 m2
A_FLUI
\(=\)
2.16 m 2

\section*{A_CELL}
2.25 m 2

COEF_ECHELLE
=
10
1.4

\section*{Boundary conditions and loadings}

\section*{Case without fluid:}

Surface low Sb: embedding
All the DDL are blocked.
Surface high HS: embedding
All the DDL are blocked.

\section*{Case with fluid:}

Surface low Sb: embedding
All the DDL are blocked.
Surface high HS: plane support (bilateral connection)
All rotations are blocked.
Longitudinal displacement DZ is blocked.
All the nodes of HS have same transverse displacement DX and same normal displacement \(D Y\).
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2
Reference solution
2.1

\section*{Method of calculation used for the reference solution}

\section*{Case without fluid:}

Let us consider the heterogeneous field described with [§1] in absence of the fluid. It is supposed that

\section*{the beams}
respect the assumptions of modeling of a right beam of Euler Bernoulli. Since conditions in extreme cases applied to the whole of the beams are the same ones as for each one of them, one can bring back the research of the Eigen frequencies of the unit to that of only one beam.
The following problem is thus studied:
That is to say a beam Bi embedded [2.1-a] in the same way characteristic geometrical and material that them
beams of the heterogeneous medium. One notes A the surface of the section, \(L\) his length, and I the moment of inertia of inflection.
By the method of rigidity dynamic one shows that such a beam admits frequencies double the form:
1
2 I.E.(internal excitation) 2
\(F=\)
I
I
2 L2
With
I
\((2 i+1) / 2 I=1,2, \ldots\) for the second case of boundary conditions: [2.1-a].
Embedding
Z
\(L\)
X
Embedding

\section*{Appear 2.1-a}

The field contains NR beams independent between them (not of boundary conditions which couples displacements of two different beams), It results from it that the multiplicity of the frequencies is equal with \(2 N\) ( 2 modes of inflections by beams).
For the homogenized medium discretized by the finite elements héxaèdre with 8 nodes or héxaèdre with 20
nodes, the number NR must be replaced by the number of straight lines parallel with the axis of the beams.

\section*{Case with fluid:}

The case with fluid is more difficult to solve analytically: no analytical result was found until the drafting of this case test. The results of reference which one has benches thus come from one numerical resolution by finite elements of the complete heterogeneous problem. One used for this fact version 3.6.2 of Code_Aster.
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures

\section*{Code_Aster \({ }^{\circledR}\)}

\author{
Version
}
4.0

Titrate:
SDLV111 Homogenisation of a network of beams
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Each beam is represented in the grid by its average fibre modelled by \(P O U_{-} D_{\_} E\) (beam
right-hand side of Euler). For all the beams, one binds each node of average fibre to the nodes of side surface, located in the same transverse section as the node in question, by
LIAISON_SOLIDE. The fluid interface beam is modelled by FLUI_STRU which translates the continuity
of
normal speeds with the walls. The fluid, was to be perfect incompressible, one deduced his modeling
of that of the compressible true fluid \(3 D \_F L U I D E\) by removing the contribution of the pressure.
Boundary conditions imposed on the fields [§1.3], and especially the relation which couples it
displacement of all the beams on the level of HS, reveal two kinds of clean modes of
the structure:
Modes of sets: all the beams become deformed in the same way and top surfaces it
a displacement not no one admits.
Local modes: they correspond to modes of embed-embedded beams.
surfaces top thus admits a null displacement. None of these modes can correspond to an overall mode.
The action of the fluid results in an effect of added mass and thus a lowering of the frequencies by report/ratio with the case without fluid. It also causes, in the case of the local modes, to spread out the spectrum of
associated frequencies. In the case without fluid one saw that this spectrum was concentrated in only one frequency of vibration.
2.2

\section*{Results of reference}

Value of the Eigen frequencies.

\subsection*{2.3 References \\ bibliographical}
[1]
Walter D. Pilkey: "Formulated for Stress, Strain and Structural Matrices", A Wiley-Interscience
Publication JOHN WILEY \& SOUNDS, Inc. Edition 1994.
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Modeling 3D_FAISCEAU
Boundary conditions:

\section*{Case without fluid:}

DDL_IMPO: (
GROUP_MA: Sb DX: 0
DY: 0
DZ: 0
DRX: 0 DRY: 0 DRZ: 0
GROUP_MA: HS DX: 0
DY: 0
DZ: 0
DRX: 0 DRY: 0 DRZ: 0
NI NODE: PHI: 0
)
Case with fluid:
DDL_IMPO: (
GROUP_MA: Sb DX: 0
DY: 0
DZ: 0
DRX: 0 DRY: 0 DRZ: 0
GROUP_MA: HS DZ: 0
DRX: 0 DRY: 0 DRZ: 0
NI NODE: PHI: 0

\section*{3.2}

\section*{Characteristics of the grid}

Grid of the homogenized medium used, for the two cases of figure: with or without fluid, is represented by [3.2-a].
It comprises 48 meshs HEXA8.
The grid contains 9 straight lines parallel with average fibre of each beam.
4 meshs in cross section
HS
12
meshs
in
height
Sb

\section*{Appear 3.2-a: grid}

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\subsection*{3.3 Functionalities}
tested
Case without fluid:
Orders
Keys
AFFE_MODELE
3D_FAISCEAU
[U4.22.01]
AFFE_CHAR_MECA

\section*{Case with fluid:}

\section*{Orders}

\section*{Keys}

AFFE_MODELE
3D_FAISCEAU
[U4.22.01]
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
LIAISON_UNIF
AFFE_CARA_ELEM
BEAM
SECTION:
"RIGHT-ANGLED"
[U4.24.01]
ORIENTATION
ANGL_NAUT
POUTRE_FLUI
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION:

\section*{"BAND"}

4

\section*{Results of modeling \(A\)}
4.1 Values
tested
Case without fluid:
Number
Size and
Reference
Aster
\% difference
of order
unit
1, 2 and 6
frequency ( Hz )
3.3333
3.3183
-0.45\%
19 and 20
frequency (Hz)
9.2584
9.1480
-1.19\%
Case with fluid:
Number
Size and
Reference
Aster
\% difference
of order
unit
1 and 2
frequency ( Hz )
0.6908
0.6932
0.35\%

19 and 20
frequency ( Hz )
3.7871
3.7984
0.30\%
4.2 Parameters
of execution
Version: 3.6.2
Machine: CRAY C90

\section*{Obstruction memory:}

16 MW
Time CPU To use:
100 S
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Date:
08/01/98
Author (S):

\section*{B. QUINNEZ, H. HADDAR}

Key:
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5 Modeling
B
5.1

Characteristics of modeling
Modeling 3D_FAISCEAU
Boundary conditions:
Case with fluid:
DDL_IMPO: (
GROUP_MA: Sb DX: 0
DY: 0
DZ: 0
DRX: 0 DRY: 0 DRZ: 0
GROUP_MA: HS DZ: 0
DRX: 0 DRY: 0 DRZ: 0
N1 NODE: PHI: 0
)
LIAISON_UNIF: (GROUP_MA: HS
DDL: "DX")
LIAISON_UNIF: (GROUP_MA: HS
DDL: "DY")

\section*{5.2}

\section*{Characteristics of the grid}

Grid of the homogenized medium used, for the two cases of figures: with or without fluid, is represented by [5.2-a].
It comprises 48 meshs HEXA20.
The grid contains 9 straight lines parallel with average fibre of each beam.
4 meshs in section

\section*{HS}

12
meshs
in

\section*{height}

Sb
Appear 5.2-a: grid

\subsection*{5.3 Functionalities}
tested

\section*{Case with fluid:}

Orders
Keys
AFFE_MODELE
3D_FAISCEAU
[U4.22.01]
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
LIAISON_UNIF
AFFE_CARA_ELEM
BEAM
SECTION:
"RIGHT-ANGLED"
[U4.24.01]
ORIENTATION
ANGL_NAUT
POUTRE_FLUI
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.02]
CALC_FREQ
OPTION:
"BAND"
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\section*{Code_Aster \({ }^{\circledR}\)}

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Key:
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6
Results of modeling B
6.1 Values
tested
Case with fluid:
Number
Size and
Reference
Aster
\% difference
of order
unit
1 and 2
frequency (Hz)
0.6908
0.6932
0.35\%

19 and 20
frequency (Hz)
3.7871
3.7984
0.23\%

\subsection*{6.2 Parameters}
of execution
Version: 3.6.2
Machine: CRAY C90
Obstruction memory:
16 MW
Time CPU To use:
100 S
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Titrate:
SDLV111 Homogenisation of a network of beams
Date:

\section*{B. QUINNEZ, H. HADDAR}

Key:
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7
Summary of the results
The results show the good modal behavior of the elements of modeling 3D_FAISCEAU in inflection, absence of the fluid. It show also a very good agreement of the frequencies of the modes overall with calculation Aster into heterogeneous, when there is fluid.
For the frequencies of the overall modes, one does not observe differences between a grid HEXA8 and HEXA20 (this is not true for the other modes).
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Version
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\section*{Titrate:}

SDLV120 Absorption of a wave of compression in an elastic bar Dates:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key

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Organization (S): EDF/RNE/AMV

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures
Document: V2.04.120

SDLV120-Absorption of a wave of compression in an elastic bar

\section*{Summary}

One tests the elastic paraxial elements of order 0 intended to apply conditions absorbing to border of a grid finite elements to simulate the infinite one in direct transitory calculations. Are used they to model an infinite elastic bar, in 3D or 2D, in which one creates a wave of pressure by imposing a displacement on the one of the ends. One is interested in nonthe reflexion of the wave in
the "infinite" end of the bar.
One tests successively the two direct transitory operators of Code_Aster, namely DYNA_LINE_TRAN and
DYNA_NON_LINE.
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Version
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\section*{Titrate:}

SDLV120 Absorption of a wave of compression in an elastic bar Dates:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key
:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

The system considered in the case 3D is that of an elastic bar with square section. One is imposed displacement according to \(X\) on one of the vertical faces and one observes the propagation of a wave of
compression. The side surface of the bar is left free. One places the elements absorbents on face opposed to the face of excitation to simulate the infinite character of the bar in this direction. In the case 2D, the principle is identical with a very broad supposed bar which one does not model that a vertical section (see diagram).

\section*{Z}
\(X\)

Imposed displacement
Elastic solid
Surface absorbing
section
Section case 3D:
Section case 2D:

\section*{Z}
\(y\)

\subsection*{1.2 Properties}
materials
Bar: concrete
Density:

\section*{2400 kg.m3}

Young modulus:

\author{
3,6.1010 Pa
}

Poisson's ratio: 0,48

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\section*{1.3}

Boundary conditions and loadings
One imposes on all the nodes of the face of the piston in contact with the fluid a displacement according to \(X\)
with the function of following temporal excitation:
Displacement of the piston according to \(X\)
10-3
\(1,00 E+00\)
9,00E-01
8,00E-01
7,00E-01
6,00E-01
5,00E-01
4,00E-01
3,00E-01
Displacement (m) 2,00E-01
1,00E-01
0,00E+00
-0,1
0,1

\subsection*{1.4 Conditions}
initial

Displacement is null in all the bar at the initial moment.

\section*{2}

Reference solution
The solution must show the absorption of a wave of compression by absorbing surface. imposed displacement is a uniform translation according to the \(x\) axis. One must obtain a field of identical displacement according to this direction in all the plans \(X=\) Cte. Moreover, the border absorbing is orthogonal with this axis. One thus studies the absorption of plane waves of compression under normal incidence. The theory [bib1] known as that with a solid paraxial border of order 0, this absorption is perfect. It is what one must check with this reference solution.

One thus goes, by observing the evolution of displacement in a given point of the grid, to stick to to find in the signal obtained the duration of excitation and the return at rest after the passage of the wave, characteristic of its absorption.
2.1

Results of reference
One gives in this paragraph the results obtained with Code_Aster in this configuration. One check that they are satisfactory and one takes them as reference for the future.
They concern, for the case 3D, the bar being 200 m length, the evolution of displacement in \(X\) in a point of the bar located at 150 m of the face excited in direction \(X\) and at the center of the section in the yz plan. For the case 2D, the bar being 50 m length, the point is located at 40 m of face according to \(X\) and in the middle of the section in the direction \(y\) (in 2D, one takes a shorter grid and
refined).
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\footnotetext{
file:///Z|/process/valid/p990.htm (8 of 15)9/28/2006 4:31:20 PM
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV120 Absorption of a wave of compression in an elastic bar Dates:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key

\section*{:}

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Displacement in X in the bar - case 3D
1,00E-03
9,00E-04
8,00E-04
7,00E-04
6,00E-04
5,00E-04
4,00E-04
3,00E-04
Displacement (m) 2,00E-04
1,00E-04
0,00E+00
-1,00E
-1,
-04
00E-01 1,00E-01 3,00E-01 5,00E-01 7,00E-01 9,00E-01 1,10E+00 1,30E+00 1,50E+00
Time (S)

Displacement in the bar - case 2D
1,00E-03
9,00E-04
8,00E-04
7,00E-04
6,00E-04
5,00E-04
4,00E-04
3,00E-04

As envisaged, the width of the signal measured in both cases is identical to that of the function of excitation. Physically, one observes the wave propagation well of compression. The signal is little modified in its propagation and one thus finds well the maximum amplitude of 1 Misters One notes
also clearly the return at rest immediately after the passage of the wave and the absence of signal thought of the end of the grid.

\subsection*{2.2 Uncertainties}

It is about a numerical result of the study. The qualitative forecasts are found. Values numerical are related to the precision of calculation. Only the return at rest is precisely given by analysis.

\subsection*{2.3 References \\ bibliographical}
[1]
H. MODARESSI "numerical Modeling of the wave propagation in the mediums porous rubber bands. "Thesis doctor-engineer, Central School of Paris (1987).
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV120 Absorption of a wave of compression in an elastic bar Dates:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key

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```

3
Modeling a: case 3D

```
3.1
Characteristics of modeling
Bar: PHENOMENON: "MECHANICAL" MODELING: "3D"
3.2 Characteristics
grid
A number of nodes: 45
A number of meshs and types: 16 HEXA8
8 QUA4 (faces of HEXA8)
Node 43
200 m
Node 16
50 m
Node 18
3.3 Functionalities
tested
Orders
AFFE_MODELE AFFE
MODELING
3D_ABSO
DYNA_LINE_TRAN
DYNA_NON_LINE
3.4 Values
tested
One tests the values of displacement in \(X\) to nodes 16, 18 and 43 (see grid). For node 16,
one tests the maximum and the return at rest. For nodes 18 and 43, one tests the maximum.
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Version
5.0

Titrate:
SDLV120 Absorption of a wave of compression in an elastic bar Dates:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key
:

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-DYNA_LINE_TRAN:
Node
Moment (S)
Calculation with
Results of
Variations reference -
Code_Aster
reference
calculation with
(displacement
(displacement
in m)
in m)
Code_Aster (\%)
N16 5.39500E-01
9.91869E-04
1.00000E-03 0.81

RELATIVE
1.20000E +00
1.7E-8
0. 1.7E-6

ABSOLUTE
N18 5.40000E-01
9.91393E-04
1.00000E-03 0.86

RELATIVE
N43 5.00000E-01
1.00000E-03
1.00000E-03 0.

RELATIVE
- DYNA_NON_LINE:

Node
Moment (S)
Calculation with
Results of
Variations reference -
Code_Aster
reference
calculation with
(displacement
(displacement
in \(m\) )
in m)
Code_Aster (\%)
N16 5.40000E-01
9.92640E-04
9.92640E-04 0.74

RELATIVE
1.20000E +00
3.0E-8
0. 3.0E-6

ABSOLUTE
N18 5.40000E-01
9.92182E-04
9.92182E-04 0.78

RELATIVE
N43 5.00000E-01
1.00000E-03
1.00000E-03 0.

RELATIVE

\subsection*{3.5 Parameters \\ of execution}

Version: 5.2.16
Machine: SGI ORIGIN 2000
Time CPU: 600
Memory: 64 Mo
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5.0

Titrate:
SDLV120 Absorption of a wave of compression in an elastic bar Dates:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key
:

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\section*{4}

Modeling b: case 2D

\section*{4.1}

Characteristics of modeling
Bar: PHENOMENON: "MECHANICAL"
MODELING: "D_PLAN"
4.2 Characteristics
grid
\(25 m\)

Node 3
50 m
Node 14
Node 32

A number of nodes: 36

A number of meshs and types: 30 QUA4

12 SEG2 (faces of QUA4)

\author{
4.3 Functionalities \\ tested \\ Orders \\ AFFE_MODELE AFFE \\ MODELING \\ D_PLAN_ABSO \\ \(D Y N A \_L I N E \_T R A N\)
}

\section*{DYNA_NON_LINE}

\subsection*{4.4 Values}
tested
One tests the values of displacement in X to nodes 32, 14 and 3 (see grid). For node 32, one tests the maximum and the return at rest. For nodes 14 and 3, one tests the maximum.

Note:
Node 3 is on vis-a-vis imposed displacement. One thus has exactly the values of excitation in this point.
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV120 Absorption of a wave of compression in an elastic bar Dates: 09/10/01
Author (S):
G. DEVESA, V. TO MOW Key

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- DYNA_LINE_TRAN:

Node
Moment (S)
Calculation with
Results of
Variations reference -
Code_Aster
reference
calculation with
(displacement
(displacement in m)
in m)
Code_Aster (\%)
N32 5.09500E-01
9.99536E-04 1.00000E-03 0.046

RELATIVE
1.20000E+00
6.3E-10
0.
6.3E-8

ABSOLUTE
N14 5.09500E-01
9.99536E-04 1.00000E-03 0.046

RELATIVE
N3 5.00000E-01
1.00000E-03 1.00000E-03 0.

RELATIVE

\author{
Node \\ Moment (S) \\ Calculation with \\ Results of \\ Variations reference - \\ Code_Aster \\ reference \\ calculation with \\ (displacement \\ (displacement in m) \\ in m) \\ Code_Aster (\%) \\ N32 5.09500E-01 \\ 9.99867E-04 9.99867E-04 0.013 \\ RELATIVE \\ 1.20000E +00 \\ -3.8E-9 \\ 0. \\ 3.8E-7 \\ ABSOLUTE \\ N14 5.09500E-01 \\ 9.99867E-04 9.99867E-04 0.013 \\ RELATIVE \\ N3 5.00000E-01 \\ 1.00000E-03 1.00000E-03 0. \\ RELATIVE
}

\subsection*{4.5 Parameters}
of execution
Version: 5.2.16
Machine: SGI ORIGIN 2000
Time CPU: 1200
Memory: 300 Mo

\section*{5 \\ Summary of the results}

One finds by calculation with two modelings quantitatively, the maximum of displacement equal to the maximum amplitude of the signal and qualitatively, the return at rest after the passage of the wave.
The results obtained with operators DYNA_LINE_TRAN and DYNA_NON_LINE are very close.
The difference comes from obtaining to each step in time from the state from balance from the efforts from the second
member with operator DYNA_NON_LINE, which explains why its results are a little bit better even with a step of larger time. This difference remains however tiny because the step time used with DYNA_LINE_TRAN is sufficiently small.

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Version
5.0

Titrate:
SDLV121 Onde planes shearing in an elastic column
Date:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key
:
V2.04.121-A Page:
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Organization (S): EDF/RNE/AMV

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures
Document: V2.04.121

SDLV121 - Wave planes shearing in one elastic column

\section*{Summary}

One tests the application of a loading in transient in the form of a plane wave thanks to the elements paraxial rubber bands of order 0, in 3D and 2D. One applies this loading to an elastic solid mass occupying one
half space and which one models a column. This column is supposed to be infinite in its lower part and
level in its part higher than the level of the surface of the free half space left. One observes propagation of the incidental wave, its reflexion on the free face of the solid mass and its absorption by the elements
paraxial at the lower end of the column.
One tests successively the two direct transitory operators of Code_Aster, namely DYNA_LINE_TRAN and
DYNA_NON_LINE.
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

The system considered in the case 3D is that of a homogeneous elastic ground occupying the half space
\(Z<0\). The plan \(Z=0\) is left free. One models of this ground a vertical column, presumedly infinite in its lower part and levelling than the free face at its higher end. The elements are placed paraxial on lower surface, to translate the infinite character of the column and to apply it loading by plane wave. In the case 2D, the principle is identical, with a very broad column which one models only one vertical section (see diagram).
Moreover, the direction of vibration is the \(y\) axis in the case 3D. It is about the \(x\) axis in the case 2D.

\section*{Locate:}

Z
Case 3D
Free face
\(y\)
( \(Z=0\) )
\(X\)
50 m
\(y\)
Case 2D
Elastic solid
\(X\)
Paraxial surface
Section case 3D:
Section case 2D:
\(X\)
\(y\)
\(y\)

\subsection*{1.2 Properties}
materials
Elastic solid mass: floor covering
Density:
1900 kg.m3
Young modulus:
4,44.108 Pa

Poisson's ratio:
0,2
1.3

Boundary conditions and loadings
One is interested in the movement 1D of the column under the exiting action of a wave planes vertical.
To identify this movement, one forces all the nodes of the same horizontal section to have it even displacement.
In this configuration, the loading by plane wave comprises the following characteristics:
Direction: (0.0.1.)
Type_d' wave: "HS"
Outdistance initial origin: 150 m
Signal: function given below (with its derivative which is used as entry with calculation):
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\section*{:}

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Signal of the incidental wave
1,00E-03
9,00E-04
8,00E-04
7,00E-04
6,00E-04
5,00E-04
4,00E-04
3,00E-04

\section*{2,00E-04}

Displacement tranversal (m) 1,00E-04
0,00E+00
0,00E+00
\(2,00 E+01\)
4,00E+01
6,00E+01
8,00E+01
1,00E+02
1,20E+02
-1,00E-04
length in the direction of the wave (m)

The maximum is obtained for a value of 49,5 m of the parameter.
Derived from the signal
8,00E-05
6,00E-05
4,00E-05
2,00E-05
0,00E+00
0,00E+00
2,00E+01
4,00E+01
6,00E+01
8,00E+01
1,00E+02
\(1,20 E+02\)
-2,00E-05
Derived (without dimension) -4,00E-05
-6,00E-05
-8,00E-05
Length in the direction of the wave ( m )

\subsection*{1.4 Conditions \\ initial}

Displacement is null in all the column at the initial moment.
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2
Reference solution
The propagation 1D of the signal of the incidental wave in the column is known analytically [bib1].
One
can for example determine the moment of passage of the maximum of the incidental wave with middle height, that is to say with
a 25 m depth, and that of the maximum of the wave thought of the same point.
Taking into account the signal given previously and position of its source to \(Z=150 \mathrm{~m}\), the maximum signal is \(Z=105.5 \mathrm{~m}\) (i.e. 150 - value of \(49,5 \mathrm{~m}\) of the parameter corresponding) that is to say to \(50,5 \mathrm{~m}\) of the paraxial surface \((Z=-50 \mathrm{~m})\) of the column in direction \(Z\) (that of the wave) with the initial moment. To arrive to 25 m , it will thus have to traverse \(75,5 \mathrm{Mr}\). the speed of the waves of shearing being of 281 m.sl for the ground considered, one can thus await the maximum of displacement with middle height in the column for time 0,27 S. Moreover, at the time of the passage of the wave
reflected, the signal will have traversed 50 m moreover, therefore one can await it for time \(0,44 \mathrm{~S}\). the value
maximum measured at these moments must be 1 Misters Ce are these analytical values which one will test
in calculation.
2.1

Results of reference
One gives in this paragraph the results obtained with Code_Aster in this configuration. One check that they are satisfactory qualitatively and quantitatively.
They concern, for the case 3D, the evolution of displacement in the three directions in a point of column located at middle height, is to 25 m of the free face in direction Z . The measurement of
displacement is identical in the case 2D.
Moreover, the direction of vibration is the \(y\) axis in the case 3D. It is about the \(x\) axis in the case 2D.

Transverse displacement in the column - case 3D
1,20E-03
1,00E-03
8,00E-04
6,00E-04
4,00E-04
2,00E-04
Displacement according to \(y\) (m)
0,00E+00
0,00E+00 1,00E-01 2,00E-01 3,00E-01 4,00E-01 5,00E-01 6,00E-01 7,00E-01 8,00E-01
-2,00E-04
Time (S)

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Code_Aster \({ }^{\circledR}\)
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5.0

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Date:
09/10/01
Author (S):
G. DEVESA, V. TO MOW Key

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Transverse displacement in the column - case 2D
2,00E-04
0,00E+00
\(0,00 E+0\) 1,00E-
2,00E-
3,00E-
4,00E-
5,00E-

The inversion of the sign of displacement in the case 2D is due only to the orientation of the reference mark.

\subsection*{2.2 Uncertainties}

It is about a numerical result of the study. One finds the qualitative and quantitative forecasts. numerical values are related to the precision of calculation.

\subsection*{2.3 References \\ bibliographical \\ [1] \\ H. MODARESSI "numerical Modeling of the wave propagation in the mediums}

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G. DEVESA, V. TO MOW Key
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3
Modeling a: case 3D

\author{
3.1 \\ Characteristics of modeling \\ Bar: PHENOMENON: "MECHANICAL" MODELING: "3D"
}

\subsection*{3.2 Characteristics}
grid
A number of nodes: 44
A number of meshs and types: 10 HEXA8
2 QUA4 (faces
Z
HEXA8)
50 m
\(y\)
\(X\)
Not measurement of displacement (node 22)

\author{
3.3 Functionalities \\ tested \\ Orders \\ AFFE_MODELE AFFE MODELING \\ 3D_ABSO \\ \(A F F E \_C H A R \_M E C A \_F O N D E \_P L A N E\)
}

\section*{DYNA_LINE_TRAN}

\section*{DYNA_NON_LINE}

\subsection*{3.4 Values}
tested

One tests the values of displacement in the three directions with the node 22 (see grid). For direction \(y\), one tests the value of the two maximum ones and the return at rest after the passage of the wave. For
the two other directions, one tests the nullity of displacement, for example at the moment of the first maximum in \(Y\).
-DYNA_LINE_TRAN:

Direction Moment
(S) Calculation with

Results of
Variations reference -
Code_Aster
reference
calculation with
(displacement
(displacement
in m)
in m)
Code_Aster (\%)
Y 2.65600E01

\title{
RELATIVE
}
4.38400E01
9.94716E04
1.E03 0.53

RELATIVE
8.00000E01
-5.8E6
0. 5.8E4

ABSOLUTE
X 2.65600E01 0.
0.
0.

ABSOLUTE
Z 2.65600E01 0.
0.
0.

ABSOLUTE
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SDLV121 Onde planes shearing in an elastic column

\section*{Date:}

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Author (S):
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\section*{-DYNA_NON_LINE:}

Direction Moment
(S) Calculation with

Results of
Variations reference -

\author{
Code_Aster
}
reference
calculation with
(displacement
(displacement in
in m)
m)

Code_Aster (\%)
Y 2.67200E01
1.00396E04
1.E03 0.40

RELATIVE
4.40000E01
9.94928E04
1.E03 0.51

RELATIVE
7.20000E01
5.1E6
0. 5.1E4

ABSOLUTE
X 2.67200E01 0.
0.
0.

ABSOLUTE
Z 2.67200E01 0.
0.
0.

ABSOLUTE
3.5 Parameters
of execution
Version:
5.2.16

\section*{Machine: \\ SGI ORIGIN 2000}

\section*{Time CPU: \\ 300 \\ Memory: \\ 64 Mo}

4
Modeling b: case 2D
4.1
Characteristics of modeling
Bar: PHENOMENON: "MECHANICAL"
MODELING: "D_PLAN"

\subsection*{4.2 Characteristics}
grid
A number of nodes: 22
A number of meshs and types: 10 QUA4
2 SEG2 (faces of QUA4)
\(y\)
\(X\)
\(X\)
50 m
Not measurement ofdisplacement (node 11)
5 m
4.3 Functionalitiestested
Orders
AFFE_MODELE AFFE
MODELING
D_PLAN_ABSO
AFFE_CHAR_MECA_F ONDE_PLANE
DYNA_LINE_TRAN
DYNA_NON_LINE

\title{
Handbook of Validation \\ V2.04 booklet: Linear dynamics of the voluminal structures \\ HT-62/01/012/A
}

Code_Aster \({ }^{\circledR}\)
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Author (S):
G. DEVESA, V. TO MOW Key
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4.4 Values
tested
One tests the values of displacement in the three directions with node 11 (see grid). For direction \(X\), one tests the value of the two maximum ones and the return at rest after the passage of the wave. For
the direction y, one tests the nullity of displacement, for example at the moment of the first maximum in \(Y\).
-DYNA_LINE_TRAN:

\section*{Direction Moment}
(S) Calculation with

Results of
Variations reference -
Code_Aster
reference
calculation with
(displacement
(displacement
in m)
in m)
Code_Aster (\%)
X 2.65600E01

\section*{- DYNA_NON_LINE:}

Direction Moment
(S) Calculation with

Results of
Variations reference -
Code_Aster
reference
calculation with
(displacement
(displacement
in \(m\) )
in m)
Code_Aster (\%)
X 2.65600E01
1.00319E03
1.E03 0.32

RELATIVE
4.38400E01
9.93554E04
1.E03 0.64

RELATIVE
8.00000E01
3.0E6
0. 3.0E4

ABSOLUTE
Y 2.65600E01 0.
0.
0.

ABSOLUTE
4.5 Parameters
of execution

\section*{Version:}
5.2.16

Machine:
SGI ORIGIN 2000

Time CPU:
300
Memory:
64 Mo

\section*{5}

Summary of the results
One finds by calculation with two modelings quantitatively, the values of maximum of displacement equal to the maximum amplitude of the signal and the values of the corresponding moments and
qualitatively, the return at rest after the passage of the considered wave.
The results obtained with operators DYNA_LINE_TRAN and DYNA_NON_LINE are very close. The difference comes from obtaining to each step in time from the state from balance from the efforts from the second member with operator DYNA_NON_LINE, which explains why its results are in general small not very better even with a step larger time. This difference remains however tiny because it no the time used with DYNA_LINE_TRAN is sufficiently small.

\author{
Handbook of Validation
}

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:

SDLV122- Extrapolation of local measurements on a complete model (3D)
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN
:
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Organization (S): EDF/RNE/AMV, CS IF

Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
V2.04.122 document

SDLV122-Extrapolation of local measurements
on a complete model (3D)

\section*{Summary:}

It is about a linear test of dynamics 3D.
The goal is to test order PROJ_MESU_MODAL in the case of a system 3D. This order allows to project experimental dynamic transitory answers in a certain number of points on a basis modal of a numerical modeling.

This test contains 2 modelings:
projection (of constraints) is done on a basic concept modal of type [mode_meca],
projection (of constraints) is done on a basic concept modal of type [base_modale].
For 2 modelings, provided experimental measurements are identical and make it possible to test seek nodes in opposite and the taking into account of a local orientation.

In both cases, the reference solution is obtained by a direct calculation with Code_Aster; projection is realized in the successful outcome where the number of modes is equal to the number of measurements. Answers in constraint obtained after projection are identical to the constraints of reference provided in data.

For modeling A, the answers in displacements and deformation obtained after projection are in perfect adequacy with the reference solutions. Values speeds and the accelerations deduced from identified modal contributions are close to those obtained by direct calculation. Weak variations noted are due to the errors of approximation generated by the determination via a linear diagram in time speeds and accelerations.
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1
Problem of reference

\subsection*{1.1 Geometry}

Let us consider the embed-free slim cylindrical bar described below:

R

\section*{L}

Length
: \(L=4\) m
Ray
: \(R=0.1 \mathrm{~m}\)
End 1: embedded ( \(x=0\) )
End 2: free ( \(x=l\) )

\section*{1.2 \\ Properties of materials}

The characteristics of material are as follows:
Young modulus: \(\mathrm{E}=2.11011 \mathrm{~Pa}\)
Poisson's ratio: \(=0.3\)
Density: \(=7800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loading}

The boundary condition is the embedding of end 1 of the bar. This embedding is of type beam to allow the effects Poisson on the section.

The loading applied for the calculation of answer is a thrust load, constant in traction, distributed on the section of end 2:
\(P(T)=0\)
if \(T<0\)
\(P=106\) NR
if \(T 0\)
0
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Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

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S. AUDEBERT, Key P. HERMAN

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\section*{2 \\ Reference solutions}

\section*{2.1 \\ Method of calculation used for the reference solution}

Analytical solution:
An analytical solution of this problem exists. It is described in:

In this case, the solution by modal superposition of this problem is written:
```

s-1
(
8PI

```
0
0
- 1
\(X\)
C
\(\boldsymbol{U} \boldsymbol{X}, \boldsymbol{T})\)
()
\(=\)
\(X-2\)
\(\sin (2 s-)\)
1
\(\cos (2 s\)
)
1
\(T\)
EA
EA
(2s-) 2
1
\(2 I\)
2I
\(s=1\)

\section*{EA}
with: \(C=\)
: propagation velocity of wave in the bar
m

Adopted reference solution:
The analytical solution utilizing an infinite sum on the modes, it is preferable that reference solution and the solution with projection corresponds to the same configuration, with the same number of modes.
Moreover, to avoid problems involved in the discretization of the numerical grid, the solution of reference selected is the answer provided by the direct calculation carried out with Code_Aster with order DYNA_TRAN_MODAL.

\section*{2.2 \\ Results of reference}

For modeling A, the comparison of the results relates to displacements, speeds, accelerations, strains and stresses along axis \(X\), of the nodes N2 and NR 4 to 3 moments different. NR 4 corresponds to a node of measurement and \(N 2\) is not one.
For modeling B, the comparison of the results relates to the constraints of the nodes NR 3 and NR 4 at 3 different moments.

\section*{2.3}

Uncertainty on the solution
The selected reference makes it possible to draw aside uncertainties related to the discretization of the numerical grid.
The number of modes of the base of projection is equal to the number of measurements, therefore the solution of
the inversion is exact (in opposition to an approximate solution of a generalized opposite problem).
If projection is done on a concept of the type [mode_meca], modal bases of reference solution and solution obtained by projection are identical, the answers in displacements, strains and stresses obtained must thus be similar to the answers of reference. Some errors of approximation can appear on speeds and accelerations which are determined by a linear diagram in time.
If projection is done on a concept of the type [base_modale], modal bases of reference solution and solution obtained by projection contain the same number of modes but are different. The calculation of reference not being possible on a concept of the type [base_modale], the comparison of the results relates only to answers corresponding to provided measurements.

\subsection*{2.4 Bibliography}

Mr. GERADIN, D. RIXEN: Theory of the vibrations - Application to the dynamics of the structures Edition MASSON 1993
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV122- Extrapolation of local measurements on a complete model (3D)
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN
:
V2.04.122-A Page:
\(4 / 8\)

\section*{3 Modeling \\ With}

\section*{3.1}

Characteristics of modeling and the grids

\section*{Numerical grid:}

The numerical grid is carried out with I-DEAS version Master Series 5. It comprises 2667 nodes and 3328 meshs of the linear type 3D.

\section*{Experimental grid:}

The grid of measurement includes/understands only 5 specific elements and 5 nodes positioned like indicates the following figure:

N1 (0. 0. 0.)
N2 (l/4=1. 0. 0.)
N3 (l/2=2. 0.0.)
N4 (3l/4=3. 0. 0.)
N5 (l=4. 0.0.)
\(P(T)\)

\section*{3.2}

Characteristics of measurements
Provided experimental measurements are:

With the nodes NR 3, NR 4 and NR 5:
The data are the axial stresses, applied in direction \(X\).
The sampling of time is constant: initial time is 0 S, the step of times is 105 S and it a many moments are 1001 (i.e until a final time of 0.01 S).

The values result from the direct calculation carried out with Code_Aster.

\section*{3.3}

Characteristics of the modal base
The modes are stored in a concept of the type [mode_meca], containing the first three modes dynamic of traction. These modes are obtained by blocking transverse displacements (i.e according to DY and DZ) of the nodes of fibre of neutral and the nodes of the higher line ( \(X=0\). to 4.
\(y=0.1\) and \(Z=0\).\() . Their Eigen frequencies ( 326.5 \mathrm{~Hz}, 980.0 \mathrm{~Hz}\) and 1634.5 Hz ) are close to Eigen frequencies of traction analytically calculated ( \(324.3 \mathrm{~Hz}, 972.9 \mathrm{~Hz}\) and 1621.5 Hz ). Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV122- Extrapolation of local measurements on a complete model (3D)
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN
:
V2.04.122-A Page:
5/8
```

4
Results of modeling A

```

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Code_Aster difference

\author{
with \(T=9.104 \mathrm{~s}\) \\ 2.686 10-4 2.686 \\ 10-4 0.00 \\ \% \\ DEPL_X \\ with the node \(N 2\) with \\ \(T=17\). 104 s \\ 3.074 10-4 3.074 \\ 10-4 0.00 \\ \% \\ (m) \\ with \(T=25.104 s\) \\ 1.446 10-5 1.446 \\ 10-5 0.01 \\ \%
}
with \(T=9.104 s\)
5.793 10-4 5.793

10-4 0.00
\%
DEPL_X
with the N4 node with
\(T=17.104 s\)
9.160 10-4 9.160

10-4 0.00
\%
(m)
with \(T=25.104 s\)
3.095 10-4 3.095

10-4 0.00
\%
with \(T=9.104 \mathrm{~s}\)
6.221 10-1 5.855

10-1-5.88
\%
VITE_X
with the node \(N 2\) with
\(T=17.104 s\)
-4.683 10-2 -4.094
10-2-12.59
\%
( \(\mathrm{m} / \mathrm{s}\) )
with \(T=25.104 s\)
-3.542 10-1 -3.168
10-1 -10.56
\%
with \(T=9.104 \mathrm{~s}\)
8.056 10-1 8.140

10-1 1.04
\%
VITE_X
with the N4 node with
\(T=17\). 104 s
-3.556 10-1 -3.800
10-1 6.87
\%
( \(\mathrm{m} / \mathrm{s}\) )
with \(T=25.104 s\)
-8.638 10-1 -8.708
10-1 0.82
\%
with \(T=9.104 \mathrm{~s}\)
-3.633 10+3-3.653
10+3 0.55
\%
ACCE_X
with the node \(N 2\) with
\(T=17.104 s\)
6.337 10+2 7.550

10+2 19.14
\%
```

(m/s2)

```
with \(T=25.104 s\)
3.801 10+3 3.778

10+3-0.62
\%
with \(T=9.104 s\)
\(8.65510+29.636\)
\(10+211.34\)
\%
\(A C C E \_X\)
with the N4 node with
\(T=17.104 \mathrm{~s}\)
-2.387 10+3-2.371
10+3-0.69
\%
(m/s2)
with \(T=25.104 s\)
\(-6.35510+2-5.019\)
10+2-21.02
\%
with \(T=9.104 s\)
1.957 10-4 1.957

10-4 0.00
\(\%\)
EPXX
with the node N2 with
\(T=17.104 \mathrm{~s}\)
3.015 10-4 3.015

10-4 0.00
\(\%\)
(m)
with \(T=25.104 \mathrm{~s}\)
5.422 10-5 5.422

10-5 0.00
\%
with \(T=9.104 \mathrm{~s}\)
1.822 10-4 1.822

10-4 0.00

\section*{\%}

EPXX
with the N4 node with
\(T=17.104 \mathrm{~s}\)
2.611 10-4 2.611

10-4 0.00
\%
(m)
with \(T=25.104 s\)
1.681 10-4 1.681

10-4 0.00
\%
with \(T=9.104 s\)
\(5.01210+75.012\)
\(10+70.00\)
\%
SIXX
with the node \(N 2\) with
\(T=17.104 \mathrm{~s}\)
7.717 10+7 7.717

10+7 0.00
\%
(Pa)
with \(T=25.104 s\)
\(1.39010+71.390\)
10+7 0.00
\%
with \(T=9.104 s\)
\(4.65010+74.650\)
\(10+70.00\)
\(\%\)
\(S I X X\)
with the \(N 4\) node with
\(T=17.104 s\)
\(6.67110+76.671\)
\(10+70.00\)
\(\%\)
\((P a)\)
with \(T=25.104 s\)
\(4.29310+74.293\)

\subsection*{4.2 Parameters \\ of execution}

Version: NEW5 (5.04)
Machine: CLASTER
Obstruction memory: \(\mathbf{3 0 0}\) Mo
Time CPU To use: 146.84 seconds
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV122- Extrapolation of local measurements on a complete model (3D)
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN

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\section*{5 Modeling}

B
5.1

Characteristics of modeling and the grids

\section*{Numerical grid:}

The numerical grid is identical to that used in the preceding case. It is carried out with I-DEAS version Master Series 5 and comprises 2667 nodes and 3328 meshs of the linear type 3D. One
group comprising only one node is added to be used as interface.

\section*{Experimental grid:}

The grid of measurement includes/understands only 5 specific elements and 5 nodes positioned like indicates the following figure:

N1 (0. 0.1 0.)
N2 (l/4=1. 0.10.\()\)
N3 ( \(l / 2=2.0 .10\).
N4 (3l/4=3. 0.1 0.)
N5 (l=4. 0.1 0.)
\(P(T)\)
N3INF (3l/4=3. -0.1 0.)

\section*{5.2}

Characteristic of measurements
Provided experimental measurements are:

With the nodes NR 3, NR 4 and N5:
The data are the axial stresses, applied in direction \(X\).
The sampling of time is constant: initial time is 0 S, the step of times is 105 S and it a many moments are 1001 (i.e until a final time of 0.01 S).

The values result from the direct calculation carried out with Code_Aster.

\section*{5.3}

Characteristics of the modal base
The modes are stored in a concept of the type [base_modale], containing the two first dynamic modes of traction and static mode with the node NR 3INF for the degree of freedom DX . The interface is of Craig-Bampton type. The base thus contains on the whole 3 modes.

Note:
The number of modes being very reduced, the solution depends on the modal base. However, modes determined for this modeling are not the same ones as those of the modal base of reference, and it is not possible to carry out direct calculation with Code_Aster on one concept [base_modale]. Only the answers corresponding to provided measurements can
thus to be validated. No comparison can be carried out on the other answers.
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV122- Extrapolation of local measurements on a complete model (3D)
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN
:
V2.04.122-A Page:
7/8

\section*{6}

Results of modeling B

\subsection*{6.1 Values}
tested
Identification Reference
Code_Aster difference
```

with T = 9. 104s
3.416 10+7 3.416
10+7 0.00
%
SIXX
with the N3 node with
T=17. 104s
8.046 10+7 8.046
10+7 0.00
%
(Pa)
with T=25.104s
4.251 10+7 4.251
10+7 0.00

```

Machine: CLASTER

Obstruction memory: 300 Mo
Time CPU To use: 147.42 seconds

\author{
Handbook of Validation
}

V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLV122- Extrapolation of local measurements on a complete model (3D)
Date:
04/03/02
Author (S):
S. AUDEBERT, Key P. HERMAN

\section*{7}

Summary of the results
For two modelings, the answers in constraints obtained after projection are identical with the constraints of reference obtained by direct calculation with Code_Aster and provided in data.

For modeling A, the answers in displacements and deformation are in perfect adequacy with the reference solutions. Values speeds and accelerations obtained afterwards projection are close to those obtained by direct calculation. The weak noted variations are due with the errors of approximation generated by the determination by a linear diagram in time of speeds and accelerations.

The cases where the number of modes is not equal to the number of measurements are not tested (problem
opposite generalized); in particular, the method of regularization of Tikhonov is not tested.

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures
HT-62/01/012/A
Code_Aster \({ }^{\circledR}\)
Version
7.3

Titrate:
SDLV401-Free-free full sphere
Date

25/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key
:
V2.04.401-A Page:
1/8
Organization (S): EDF-R \& D /AMA, SAMTECH

\author{
Handbook of Validation \\ V2.04 booklet: Linear dynamics of the voluminal structures \\ Document: V2.04.401
}

SDLV401-Free-free full sphere

\section*{Summary:}

This three-dimensional test of modal analysis consists in calculating the Eigen frequencies of a full sphere in
free-free.
The interest of this test is to evaluate the robustness and the performance of Code_Aster in the detection of the modes
rigid and of the multiple frequencies, during the use of voluminal elements.
The reference solution is numerical and is obtained using the computation software of structures by elements
stop the SAMCEF software.
In this test, one compares the results obtained with two types \(D\) `elements:

\section*{- element HEXA8 (modeling A)}
- element HEXA20 (modeling B).

In order to determine the clean elements of this system, the method known as of SORENSEN is used.

\section*{Handbook of Validation \\ V2.04 booklet: Linear dynamics of the voluminal structures}

Code_Aster \({ }^{\circledR}\)
Version
7.3

Titrate:
SDLV401 - Free-free full sphere

\section*{Date}

\section*{25/10/04}

Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key
:
V2.04.401-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

Full sphere:

\section*{\(\boldsymbol{R}\)}

Interior ray \(R=0.01 \mathrm{~m}\)

\section*{1.2 \\ Material properties}

The presumedly elastic material linear A following characteristics:
\[
\begin{aligned}
& E=1 . E 8 \mathrm{~Pa} \\
& =0.3 \\
& =10000 \mathrm{~kg} / \mathrm{m} 3
\end{aligned}
\]
1.3
Boundary conditions and loadings
The studied structure is free in space.
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.3
Titrate:
SDLV401-Free-free full sphere
Date
25/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key
V2.04.401-A Page:3/8
2
Reference solution
2.1
Method of calculation used for the reference solution
The reference solution is numerical: it is obtained with the computation software of structures byfinite elements the SAMCEF software.The method of calculation of the Eigen frequencies is the method known as of SORENSEN (methodbydefect in operator MODE_ITER_SIMULT).

\section*{2.2 \\ Results of reference}

The structure presenting six rigid modes, one is interested in the first 10 Eigen frequencies not null.
The following table shows the values of the frequencies obtained (in Hz ) according to the degree of the elements voluminal.

\author{
Mode
}

Degree 1
Degree 2
1
2.54231 E3
2.47035 E3

2
2.54231 E3
2.47035 E3

3
2.60747 E3
2.47101 E3

4
2.60747 E3
2.47101 E3

5
2.60747 E3
2.47101 E3

6
2.74095 E3
2.61296 E3

7
2.74095 E3
2.61296 E3

8
2.74095 E3
2.61430 E3

9
2.76313 E3
2.61430 E3

10
2.76313 E3
2.61430 E3

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures HT-66/04/005/A

Code_Aster \({ }^{\circledR}\)
Version
7.3

Titrate:
SDLV401 - Free-free full sphere

\section*{Date}

25/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key

V2.04.401-A Page:
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\section*{3 Modeling}

With

\section*{3.1}

\section*{Characteristics of modeling \(A\)}

Grid made up of elements HEXA8

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 417 \\ A number of meshs and type: 160 HEXA8
}

\author{
3.3 Functionalities \\ tested \\ \section*{Orders} \\ AFFE_MODELE AFFE MODELING \\ "3D" \\ CALC_MATR_ELEM OPTION \\ "RIGI_MECA" \\ "MASS_MECA" \\ MODE_ITER_SIMULT METHOD \\ "SORENSEN" \\ CALC_FREQ \\ OPTION \\ BANDAGE \\ FREQ \\ (1, 3000 Hz ) \\ Handbook of Validation \\ V2.04 booklet: Linear dynamics of the voluminal structures \\ HT-66/04/005/A
}

Code_Aster \({ }^{\circledR}\)
Version
7.3

Titrate:
SDLV401 - Free-free full sphere
Date

Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key :
V2.04.401-A Page:
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested
(Frequencies in Hertz)

\section*{Identification}

Reference
Code_Aster
\% difference
\(n^{\circ}\) mode
1
2.54231 E3
2.54231 E3
~ 1 E-6
2
2.54231 E3
2.54231 E3
~ 1 E-6
3
2.60747 E3
2.60747 E3
~ 1 E-6
4
2.60747 E3
2.60747 E3
~ 1 E-6
5
2.60747 E3
2.60747 E3
~ 1 E-6
6
2.74095 E3
2.74095 E3
~ 1 E-6
7
2.74095 E3
2.74095 E3
~ 1 E-682.74095 E3
2.74095 E3
~ 1 E-6
9
2.76313 E3
\(2.76313 \mathrm{E3}\)
~ 1 E-6
10
2.76313 E3
2.76313 E3
~ 1 E-6

\subsection*{4.2 Remarks}
Code_Aster detects well the 6 modes of rigid body.

\section*{Handbook of Validation}
V2.04 booklet: Linear dynamics of the voluminal structures HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.3

\section*{Titrate:}
SDLV401-Free-free full sphere

\section*{Date}
25/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key

\section*{:}
V2.04.401-A Page:

\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling B}

Grid made up of elements HEXA20

\author{
5.2 \\ Characteristics of the grid \\ A number of nodes: 815 \\ A number of meshs and type: 160 HEXA20
}

\subsection*{5.3 Functionalities}
tested

\author{
Orders
}
AFFE_MODELE AFFE
MODELING
"3D"
CALC_MATR_ELEM OPTION
"RIGI_MECA"
"MASS_MECA"
MODE_ITER_SIMULT METHOD
"SORENSEN"
CALC_FREQ
OPTION ..... BANDAGE
FREQ
(1, 3000)
Handbook of Validation
V2.04 booklet: Linear dynamics of the voluminal structures ..... HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version ..... 7.3
Titrate:
SDLV401 - Free-free full sphere
Date
:
25/10/04
Author (S):
E. BOYERE, G. ROBERT, F. SOULIE Key
V2.04.401-A Page: ..... 7/8
6
Results of modeling B
6.1 Values
tested
(Frequencies in Hertz)

\section*{Identification}

Reference
Code_Aster
\% difference
\(n^{\circ}\) mode
1
2.47035 E3
2.46964
-0.029
22.47035

E3
2.46964-0.029
32.47101

E3
2.47084-0.007
42.47101

E3
\(2.47084-0.007\)
52.47101

E3
2.47084-0.007
62.61296

E3
2.613440 .019
72.61296

E3
2.613440 .019
82.61430

E3
2.61345 -0.033
92.61430

E3
2.61364-0.025
102.61430

E3
2.61364-0.025

\subsection*{6.2 Remarks}

Code_Aster detects well the \(\mathbf{6}\) modes of rigid body.

\section*{Handbook of Validation \\ V2.04 booklet: Linear dynamics of the voluminal structures \\ HT-66/04/005/A}

\author{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 7.3 \\ Titrate: \\ SDLV401-Free-free full sphere \\ Date \\ : \\ 25/10/04 \\ Author (S): \\ E. BOYERE, G. ROBERT, F. SOULIE Key \\ : \\ V2.04.401-A Page: \\ 8/8
}

\section*{7}

Summary of the results
The results obtained are excellent since the instantaneous frequency deviations with the reference solution are lower than 0.03\%. Moreover, Code_Aster detects well the 6 modes of rigid body, which one does not have not asked to calculate.

To calculate the rigid modes indeed, two possibilities exist. With the option "BANDAGES" on frequency band ( \(0,3000 \mathrm{~Hz}\) ), one can employ either the method of "SORENSEN", or the method LANCZOS ("TRI_DIAG") by specifying option "MODE_RIGIDE".

\section*{Handbook of Validation}

V2.04 booklet: Linear dynamics of the voluminal structures
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

\section*{Titrate:}

SDLX01 Inflection of a symmetrical gantry
Date:
09/01/98
Author (S):
B. QUINNEZ

Key:
V2.05.001-C Page:
1/6
Organization (S): EDF/IMA/MMN
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
V2.05.001 document
SDLX01-Inflection of a symmetrical gantry
Summary:
This plane problem consists in seeking the frequencies of vibration of a mechanical structure made up of one
assembly of beams with rectangular section (symmetrical gantry). This test of mechanics of the structures
corresponds to a dynamic analysis of an assembled structure having a linear behavior. It includes/ understands
only one modeling.
Via this problem, one tests the element of beam of Timoshenko as well as the calculation of frequencies of vibration by the method of the iterations opposite.
The results obtained are in very good agreement with those of guide VPCS. The error on the thirteen first
frequencies of vibration is lower than 0,2\%.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLX01 Inflection of a symmetrical gantry
Date:
09/01/98
Author (S):
B. QUINNEZ

Key:
V2.05.001-C Page:
2/6
```

I
Problem of reference
1.1 Geometry
E
F
Rectangular cross-sections:
thickness
H=0.0048 m
y,v
C
D
width
B=0.029 m
surface
To = 1.392 10-4 m2
inertia
lz=2.673 10-10 m4
With
B
X,U
y
z'
section:
z'
H
B
y'
B
posts
cross-pieces
H
Co-ordinates of the points (in meters):
With
B
C
D
E
F
X
0 . 3 0
0 . 3 0

```

Material properties
\(E=2.11011 \mathrm{~Pa}\)
\(=0.3\)
\(=7.800 . \mathrm{kg} / \mathrm{m} 3\)
1.3

Boundary conditions and loadings
Embedded points \(A\) and \(b:(U=v=0,=0)\).

\subsection*{1.4 Conditions}
initial
Without object for the modal analysis.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLX01 Inflection of a symmetrical gantry
Date:
09/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.05.001-C Page:
3/6
2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SDLX01/89 of the guide VPCS which presents
method of calculation in the following way:
Method of the dynamic stiffness (Theory of the slim beams)
2.2

Results of reference
the first 13 Eigen frequencies.
2.3

Uncertainty on the solution
\((f / f)<0.5 \%\).
2.4 References
bibliographical
[1]
J. PIRANDA. Run and Directed Work of vibrations of the structures. Mechanical option. School

Higher main road of Mechanics and Micromechanics. Laboratory of Mechanics
Applied. Besancon (France) (1983).
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004-Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLX01 Inflection of a symmetrical gantry
Date:
09/01/98
Author (S):
B. QUINNEZ

Key:
V2.05.001-C Page:
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3 Modeling
With
3.1

Characteristics of modeling
\(P O U \_D \_T\)
\(y\)
E
F
C
D
With
B
\(X\)

\section*{Cutting:}

AC and data base
6 meshs SEG2
EC and DF
9 meshs SEG2
\(C D\) and \(E F\)
10 meshs SEG2
Limiting conditions:
plane problem
DDL_IMPO:
(ALL: "YES" DZ: 0., DRX: 0. , DRY: 0. )
embedded nodes \(A\) and \(B\)
(GROUP_NO: AB DX: 0. , DY: 0. , DRZ: 0. )
Name of the nodes:
Not \(A=\) N100
Not \(B=N 600\)
Not \(C=N 200\)
Not \(D=N 500\)
Not \(E=N 300\)
Not \(\boldsymbol{F}=\mathbf{N} 400\)

\section*{3.2}

Characteristics of the grid
A number of nodes:
50
A number of meshs and types:
50 SEG2
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"RIGHT-ANGLED"
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
ALL
[U4.25.01]
GROUP_NO
AFFE_MATERIAU
ALL
[U4.23.02]
```

AFFE_MODELE
"MECHANICAL"
"POU_D_T"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
MODE_ITER_INV
CALC_FREQ
OPTION
"ADJUSTS"
[U4.52.01]
FREQ
NMAX_FREQ
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

```
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
SDLX01 Inflection of a symmetrical gantry
Date:
09/01/98
Author (S):
B. QUINNEZ
Key:
V2.05.001-C Page:
5/6
4
Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference
1 anti
8.8
8.7802
0.23
0.03
13 sym
335.0
335.23000.07
4.2 Remarks
Calculations carried out by:
MODE_ITER_INV
OPTION: LIST_FREQ "ADJUSTS": (5. , 350.) NMAX_FREQ: 13
4.3
Contents of the file results
the first 13 Eigen frequencies (clean vectors and modal parameters).
4.4 Parameters
of execution
Version: 3.4.10
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
8.7 seconds
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SDLX01 Inflection of a symmetrical gantry
Date:
09/01/98
Author (S):

\section*{B. QUINNEZ}

Key:
V2.05.001-C Page:
6/6
5
Summary of the results
Precision \(<0.2 \%\) on all the Eigen frequencies until the 13th mode.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
1/12
Organization (S): EDF/IMA/MMN, CISI
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
Document: V2.05.002
SDLX02 - Piping: Problem of Hovgaard.
Spectral analysis

\section*{Summary:}

The three-dimensional problem consists firstly, to seek the modes of vibration of a mechanical structure composed of a curved beam embed-embedded (problem of Hovgaard), secondly, to analyze the answer of this structure subjected to a spectrum of acceleration. This test of mechanics of the structures corresponds to one
analyze dynamic of a linear model (assembled structure) having a linear behavior. It includes/

\section*{understands}
three modelings.
Via this problem, one tests the element of beam of Timoshenko (right beam or curve) in inflection, the calculation of the clean modes by the method of Lanczos, the calculation of the static modes and calculation
of a spectral response of a structure subjected to a spectrum of acceleration (one tests also the interpolation of spectrum).
The results obtained are in concord with the results of reference (compilation of results obtained by other software packages).
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

\author{
Version
}

3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
2/12
1 Problem of reference
1.1 Geometry
- 0.
- 0.922
2.75

With - 1.828
B - 0.922
=
Z
- 0.922
- 0 .
=
4
35
6
=

\subsection*{1.3 Boundary conditions and loadings}

Items 1 and 15 embedded \((\mathrm{U}=\mathrm{v}=\mathrm{W}=\mathrm{X}=\mathrm{y}=\mathrm{Z}=0)\).
Loading: without object for the modal analysis.
For the spectral analysis: definition of a spectrum of acceleration to the supports for a damping of \(2 \%\).
0.1
1.
1.
0.1
0.1 according to Z
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
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2 Reference solution
2.1 Method of calculation used for the reference solution

Averages of codes: Lice, ADL, TITUS-T.
Guide validation of the Software packages of structural analysis - AFNOR - 1990 (for modal calculation).
values provided in the card under are estimated and were corrected thereafter in 1992. However, they were preserved for calculations with matrix of diagonal mass.

\subsection*{2.2 Results of reference}

Modal calculation:
the first 9 Eigen frequencies.
Spectral answer:
displacement of the nodes N3 N5 and N7, N9, N11.
Reaction of supports to the nodes N1, N15.
Generalized efforts of the nodes N3, N7, N11.

\subsection*{2.3 Uncertainty on the solution}

About \(1 \%\) on the first 5 modes.
Between 1 and \(2,5 \%\) for modes 6 to 9 .

\subsection*{2.4 Bibliographical references}
[1] Guide Technical VPCS AFNOR - 1990
[2] W. HOVGAARD "dimensional Stress in three pipe bens", Trans of ASME vol. 57, FSP 7512 P 401-416.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
4/12
3 Modeling A

\subsection*{3.1 Characteristics of modeling}

The curved elements are modelled by elements "POU_C_T" (2 elements per elbow).
The right elements are modelled by elements "POU_D_T".
2.75
=
Z
\(=\)
N400
N500
=
N600
N300
=
=
With
N700N800
N900
3.69

B N1000
N200

\subsection*{3.2 Functionalities tested}

\section*{Orders}

Keys
AFFE_MODELE
GROUP_MA
"MECHANICAL"
"POU_D_T"
[U4.22.01]
"POU_C_T"
CALC_MATR_ELEM
OPTION
"MASS_MECA_DIAG"
[U4.41.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.01]
CALC_FREQ
DIM_SOUS_ESPACE
NMAX_FREQ
NORM_MODE
NORMALIZES
"TRAN_ROTA"
[U4.64.02]
CALC_ELEM
OPTION
"SIELF_ELGA_DEP"
[U4.61.02]
"EFGE_ELNO_DEPL"

\subsection*{3.3 Characteristics of the grid}

A number of nodes:
15
A number of meshs and types:
10 POU_D_T
4 POU_C_T

\subsection*{3.4 Remarks}

The modes are normalized in the following way: larger component (degree of freedom of translation or rotation) with one.
The total answer is obtained by quadratic combination of the directions of the excitations.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
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\section*{4 Results of modeling A}
4.1 Values tested

Frequencies of the structure (matrix of complete mass).

\section*{Eigen frequencies}

\section*{Reference}

\section*{Aster}
\% difference
Mode 1
10.39
10.4488
0.566

2
20.02
19.9067
0.566

3
25.45
25.3369
0.444

4
48.32
46.8806
2.979

5
52.60
51.7708
1.576

6
84.81
82.4334
2.802

7
87.16
84.1737
3.426

8
129.31
130.138
0.640

9
131.69
131.691

0
Frequencies of the structure (matrix of mass diagnonale).

\section*{Eigen frequency}

Reference
```

Aster
% difference
10.18
10.198
0 . 1 8
2
19.54
19.544
0 . 0 2
3
25.47
25.368
0 . 4 0
4
4 8 . 0 9
7 4 . 7 6 3
0 . 6 8
5
52.86
52.509
0 . 6 6
6
75.94
7 5 . 1 3 1
1.07
7
80.11
7 9 . 5 1 2
0 . 7 5
8
122.34
120.785
1.27
9
123.15
121.480
1.36
Spectral answer: one does not take account of the correction of the frequencies due to damping
(option CORR_FREQ with not in operator COMB_SISM_MODAL)
Displacement
Identification
Reference
Aster

```

\section*{\% difference}

DEPL
N300
DX
4.847103
4.8449103
0.04

DY
2.192103
2.1914103
0.03

DZ
2,735 106
2.7341106
0.03

N500
DX
4.808103
4.8062103
0.04

DY
2.914103
2.131103
0.03

DZ
6.507104
6.5048104
0.03

N700
DX
3.588103
3.5866103
0.04

DY
2.914103
2.9124103
0.05

DZ
8.599104
8.5959104
0.03

N900
DX

\section*{DZ}
3.364104
3.3621104
0.06

Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}

3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
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Nodal reaction

\section*{Identification}

\section*{Reference}
\begin{tabular}{l} 
Aster \\
\% difference \\
REAC \\
N100 \\
DX \\
2132 \\
2130.81 \\
0.06 \\
DY \\
1241 \\
1240.24 \\
0.06 \\
DZ \\
564.6 \\
564.33 \\
0.05 \\
DRX \\
2352 \\
2351.27 \\
0.03 \\
DRY \\
4746 \\
4744.27 \\
0.04 \\
DRZ \\
937.3 \\
936.92 \\
0.04 \\
N1500 \\
DX \\
1653 \\
1652.24 \\
0.05 \\
DY \\
3354 \\
3352.19 \\
0.05 \\
DZ \\
893.7 \\
893.34 \\
0.04 \\
DRX \\
170.8 \\
\hline
\end{tabular}
\% difference
REAC
N100
DX
2132
2130.81
0.06
DY
1241
1240.24
0.06
DZ
564.6
564.33
0.05
DRX
2352
2351.27
0.03
DRY
4746
4744.27
DRZ
937.3
936.92
0.04
N1500
DX
1652.24
0.05
DY
3354
3352.19
DZ
893.7
893.34
0.04
DRX
170.8
0.03

DRY
1668
1667.71
0.02

DRZ
4903
4900.62
0.05

Generalized efforts

\section*{Identification}

Reference
Aster
\% difference

\section*{EFGE}

N300
NR
559.9
559.86
0.01

VY
430.8
430.75
0.01

VZ
914.9
914.88

0 .
MT
932.5
932.50

0 .
MF
587.3
587.35
0.01

Y
MFZ
620.4
620.36
0.01

N700

\section*{NR}
162.5
1624.83
0.01

VY
1367.
1367.04
0.

VZ
225.4
225.38
0.01

MT
170.6
170.64
0.03

MF
924.7
924.69
0.

Y
MFZ
2150
2150.29
0.01

Spectral answer: one takes account of the correction of the frequencies due to damping (option
CORR_FREQ with yes in operator COMB_SISM_MODAL)
Displacement and nodal Reaction

\section*{Identification}

\section*{Reference}

Aster
\% difference

\section*{DEPL}

N3
DX
4.847103
4.8469103

0
DY
2.192103
2.1922103
0.01

N7

\section*{DX}
3.58810
3.588110

0
DY
2.914103
2.913510
0.01

DRY
1.43610
1.436110
0.01

REAC_NOD
N1

\section*{DX}
2132.
2131.66
0.02

With
DY
1241.
1240.94
0.02

DZ
564.6
564.56

0
N15
DRX
170.8
170.81

0
DRY
166.8
1668.38
0.02

DRZ
4903.
4902.58

0
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
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\subsection*{4.2 Remarks}

Values of the spectrum (interpolation).

\section*{Mode}

1, 2, 3
4
5
6
7
8, 9
Following Acclération
19.620
8.06148
6.72586
3.38994
3.04168
1.9620

X and y
Acceleration according to Z
9.810
4.03074
3.36293
1.69497
1.52084
0.9810

\subsection*{4.3 Parameters of execution}

Version: 3.04.7
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
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\section*{5 Modeling B}

\subsection*{5.1 Characteristics of modeling}

The curved elements are modelled by elements "POU_C_T" (2 elements per elbow).
The right elements are modelled by elements "POU_D_T_G".
2.75
=
Z
N400
N500
=
N600
N300
=
=
With
N700N800
N900
3.69

B N1000
N200
N1100
N1200
y
=

N1300
=
N1400

\subsection*{5.2 Functionalities tested} Orders

\section*{Keys}

\author{
AFFE_MODELE
}

GROUP_MA
"MECHANICAL"
"POU_D_T_G"
[U4.22.01]
"POU_C_T"
CALC_MATR_ELEM
OPTION
"MASS_MECA"
[U4.41.01]
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.01]
CALC_FREQ
DIM_SOUS_ESPACE
NMAX_FREQ

\subsection*{5.3 Characteristics of the grid}

A number of nodes:
15
A number of meshs and types:
10 POU_D_T_G
4 POU_C_T

\subsection*{5.4 Remarks}

The modes are normalized in the following way: larger component (degree of freedom of translation or rotation) with one.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
\(9 / 12\)
6 Results of modeling \(B\)
6.1 Values tested

Frequencies of the structure (matrix of complete mass).

\section*{Eigen frequency}

\section*{Reference}

Aster
\% difference
Mode 1
10.39
10.4488
0.566

2
20.02
19.9067
0.566

3
25.45
25.3369
0.444

4
48.32
46.8806
2.979

5
52.60
51.7708
1.576

6
84.81
82.4334
2.802
0

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
10/12
7 Modeling C
7.1 Characteristics of modeling

The curved elements are modelled by elements "POU_C_T" (10 elements per elbow).
The right elements are modelled by elements "POU_D_T_G" (10 elements per right beam).
2.75

N3
=
Z

N2
\(=\)
\(=\)
\(=\)
With
N4
3.69

B
NS
y
=
=
P1500 = N6
=
=
=
1.96

X
N1 = P100

\subsection*{7.2 Functionalities tested}

\section*{Orders}

Keys

AFFE_MODELE
GROUP_MA
"MECHANICAL"
"POU_D_T_G"
[U4.22.01]
"POU_C_T"
CALC_MATR_ELEM
OPTION
"MASS_MECA"
[U4.41.01]
"MASS_MECA_DIAG"
MODE_ITER_SIMULT
METHOD
"TRI_DIAG"
[U4.52.01]
CALC_FREQ
DIM_SOUS_ESPACE
NMAX_FREQ

\subsection*{7.3 Characteristics of the grid}

A number of nodes:
51
A number of meshs and types:
30 POU_D_T_G
20 POU_C_T

\subsection*{7.4 Remarks}

The modes are normalized in the following way: larger component (degree of freedom of translation or rotation) with one.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
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\section*{8 Results of modeling C}
8.1 Values tested

Frequencies of the structure (matrix of complete mass).

\section*{Eigen frequency}

\section*{Reference}

Aster
\% difference
Mode 1
10.39
10.41
0.170

2
20.02
20.02
0.002

3
25.45
25.52
0.289

4
48.32
48.47
0.314

5
52.60
52.68
0.152

6
84.81
84.89
0.1

7
87.16
87.29
0.153

8
129.31
130.22
0.704

Frequencies of the structure (matrix of diagonal mass).
Reference
Aster
\% difference

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SDLX02 Piping: Problem of Hovgaard. Spectral analysis
Date:
20/09/99
Author (S):
B. QUINNEZ, L. VIVAN

Key:
V2.05.002-C Page:
12/12
9 Summary of the results and remarks general Modal calculation:
The results are in conformity with the card of validation.

By refining the grid (modeling C) one obtains correct results.
Spectral answer:
The results are in conformity with the results of reference (the error is lower than the thousandths).
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/96/004 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX03 Assembly of braced thin rectangular plates
Date:
23/01/00
Author (S):
B. QUINNEZ Key
:
V2.05.003-D Page:
1/8
Organization (S): EDF/IMA/MMN

Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
Document: V2.05.003

SDLX03 - Assembly of rectangular plates
thin braced

\section*{Summary:}

This three-dimensional problem consists in seeking the frequencies of vibration of a mechanical structure composed of an assembly of plates where one simulated an effect of stiffening. This test of Mechanics of
Structures corresponds to a dynamic analysis of an assembled structure having a linear behavior. It includes/understands two modelings.

Via this problem, one tests the element of plate DKT as well as the calculation of the frequencies of vibration by the method of Lanczos with detection of the modes of rigid body.

In the second modeling, one tests in more the connection between hulls (key word LIAISON_COQUE of order \(\left.A F F E \_C H A R \_M E C A\right)\).

The results obtained are in concord with the results given in guide VPCS (average of results obtained by various computer codes). The six modes of rigid body were indeed detected. A comparison with experimental results is also satisfactory.

\section*{Handbook of Validation}

V2.05 booklet: Linear dynamics of the assembled structures

\section*{HI-75/01/010/A}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX03 Assembly of braced thin rectangular plates
Date:
23/01/00
Author (S):
B. QUINNEZ Key
:
V2.05.003-D Page:
2/8

\section*{1}

Problem of reference

\subsection*{1.1 Geometry}
0.005
0.2
0.05
0.05
(lengths
0.375
0.025
0.075
in meters)
1/2 cross section

Thickness of all the plates:
\(T=0.005 \mathrm{~m}\)
Plates higher and lower:
length has \(=0.375 \mathrm{~m}\)
dispatcher \(B=0.2 \mathrm{~m}\)

\section*{Vertical plates:}
length has \(=0.375 \mathrm{~m}\)
width \(B=0.05 \mathrm{~m}\)

\section*{1.2}

Properties of materials
\(E=2.11011 \mathrm{~Pa}\)
\(=0.3\)
\(=7.800 . \mathrm{kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

Free structure in any point.

\subsection*{1.4 Conditions}
initial
Without object for the modal analysis.
Handbook of Validation

V2.05 booklet: Linear dynamics of the assembled structures

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX03 Assembly of braced thin rectangular plates
Date:
23/01/00
Author (S):
B. QUINNEZ Key

2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card SDLX03/89 of guide VPCS.
The reference solution was obtained by experimental study of the frequencies and clean modes free structure on a model produced with welded sheets.
The structure suspended by flexible connections is put in vibration by a discharger electrodynamics. The statement of the Eigen frequencies is obtained starting from an accelerometer.

In addition, of the digital simulations, carried out by various computer codes, allowed to establish "results of reference" for the models finite elements.
2.2

Results of reference
the first 6 nonnull Eigen frequencies.

\section*{2.3}

Uncertainty on the solution
Lower than 4\%.
2.4 References
bibliographical
[1]
Tests carried out by Company METRAVIB (64 Way of Dampings - BP 182-69132 EcullyCedex - France). Report/ratio METRAVIB R.D.S. \(n^{\circ}\) 1.604.50 (1987).
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
SDLX03 Assembly of braced thin rectangular plates
Date:
23/01/00
Author (S):
B. QUINNEZ Key

:
V2.05.003-D Page:
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\section*{3 Modeling}

With
3.1
Characteristics of modeling
Elements of hull DKT
Z509508
\(X\)

\section*{Cutting:}

Plates higher and lower
10 in length
==> 160 meshs TRIA3
8 in width

\author{
Vertical plates \\ 10 in length \\ ==> 20 meshs TRIA3 \\ 1 in width \\ Names of the nodes: \\ Lower plate: \\ N101,..., N109 \\ N111,..., N119 \\ N201,..., N209 \\ Higher plate: \\ N401,..., N409 \\ N411,..., N419 \\ N501,..., N509 \\ Vertical plates: \\ N102, N112,..., N202 \\ N402, N412,..., N502 \\ N108, N118,..., N208 \\ N408, N418,..., N508 \\ \section*{3.2} \\ Characteristics of the grid
}

\author{
A number of nodes: \\ 198 \\ A number of meshs and types: \\ 360 TRIA3 \\ \subsection*{3.3 Functionalities} \\ tested \\ Orders \\ Keys \\ AFFE_CARA_ELEM HULL \\ ALL [U4.24.01] \\ AFFE_MATERIAU ALL \\ [U4.23.02] \\ "MECHANICAL"AFFE_MODELE \\ "DKT" \\ ALL \\ [U4.22.01] \\ DEFI_MATERIAU ELAS
}
[U4.23.01]
MODE_ITER_SIMULT METHOD
"TRI_DIAG"
[U4.52.01]
CALC_FREQ
OPTION
"BAND"

FREQ

NMAX_FREQ

Handbook of Validation

\section*{V2.05 booklet: Linear dynamics of the assembled structures}

HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SDLX03 Assembly of braced thin rectangular plates
Date:
23/01/00
Author (S):
B. QUINNEZ Key
:
V2.05.003-D Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Order of
Reference
Model reference
Aster
\% differencel
experimental clean mode
elements finis*
model elements
finished
7
606.
\(584 . \pm 1 \%\)
590.0310
1.03

8
760.
826. \(\pm 1.5 \%\)
829.4009
0.41

9

\section*{865.}
\(855 . \pm 1.7 \%\)
848.1548
-0.80
10
944.
\(911 . \pm 2 \%\)
908.8566
-0.23
11
1113.
\(1113 . \pm 3.6 \%\)
1097.6578
-1.38
12
1144.
\(1136 . \pm 4 \%\)
1164.0088
2.46
* average of 5 computer codes

\subsection*{4.2 Remarks}

Calculations carried out by:

\section*{MODE_ITER_SIMULT}

METHOD: "TRI_DIAG"
OPTION
"BAND"
LIST_FREQ:
(1.,1200.)

DIM_SOUS_ESPACE:
12

\section*{4.3}

Contents of the file results
The first 6 nonnull Eigen frequencies (clean vectors and modal parameters).

\subsection*{4.4 Parameters}
of execution

Version: 5.02

Machine: SGI Origin 2000

\section*{Obstruction memory:}

64 Mo
Time CPU To use: 2.42 seconds

Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate: \\ SDLX03 Assembly of braced thin rectangular plates}

Date:
23/01/00
Author (S):
B. QUINNEZ Key
:
V2.05.003-D Page:
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5 Modeling
B
5.1

Characteristics of modeling
One uses elements of plate DKT
P16
P15
Z
\(P\)
\begin{tabular}{|c|c|}
\hline & \(y\) \\
\hline & \(\boldsymbol{P}\) \\
\hline & 23 \\
\hline & 8 \\
\hline & P22 \\
\hline & P14 \\
\hline & \(P\) \\
\hline & \(\boldsymbol{P}\) \\
\hline & 7 \\
\hline & 24 \\
\hline & \(P\) \\
\hline & \(P\) \\
\hline & \(X\) \\
\hline & 21 \\
\hline & 13 \\
\hline & \(P\) \\
\hline & \(P 10\) \\
\hline & \(P\) \\
\hline & \(P\) \\
\hline & \(P\) \\
\hline & 9 \\
\hline & 11 \\
\hline & P12 \\
\hline & 6 \\
\hline & 9 \\
\hline & \(P\) \\
\hline & \(P\) \\
\hline & 5 \\
\hline & \(X\) \\
\hline & \(X\) \\
\hline & \(2.510-3\) \\
\hline & \(P\) \\
\hline & \(P\) \\
\hline & 10 \\
\hline & 20 \\
\hline & 19 \\
\hline & P1 \\
\hline & P20 \\
\hline & P17 \\
\hline & \(P\) \\
\hline & P11 \\
\hline & \(2.510-3\) \\
\hline & P18 \\
\hline
\end{tabular}

\begin{abstract}
\(P\)17P
\(P\)

2
12
19
P
P2
\(P\)
\(P\)
1
4
\(P\)
18
0.025
0.15
0.025
3
P4
front view
Cutting is done in the following way:
There is 1
element between P1 and P2
6
elements between P2 and P3
1
element between P3 and P4
1
element between P17 and P20
10 elements according to sides' parallel with y (P4 P5 for example)
2 calculations are passed:
\end{abstract}
in the first calculation, one establishes connections of solid body between the lines:
- P2 P7 and P17 P22

\section*{- P3 P6 and P18 P21}
- P10 P15 and P20 P23
- P11 P14 and P19 P24
via the key word factor "LIAISON_COQUE" of order AFFE_CHAR_MECA.
in the second calculation, one establishes connections of solid body between the nodes in with respect to
above mentioned line couples via the key word factor "LIAISON_SOLIDE" of the order AFFE_CHAR_MECA.

\section*{5.2}

Characteristics of the grid
A number of nodes:
242
A number of elements TRIA3: 360
5.3 Functionalities
tested
Orders

Keys
AFFE_CHAR_MECA
LIAISON_SOLIDE LIAISON_COQUE [U4.25.01]
AFFE_CARA_ELEM HULL
2
ALL
[U4.24.01]
AFFE_MATERIAU ALL
[U4.23.02]
"MECHANICAL" AFFE_MODELE
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU ELAS

\author{
[U4.23.01] \\ MODE_ITER_SIMULT METHOD \\ "TRI_DIAG" \\ [U4.52.01] \\ CALC_FREQ \\ OPTION \\ "BAND"
}

FREQ

\author{
NMAX_FREQ \\ \section*{Handbook of Validation} \\ V2.05 booklet: Linear dynamics of the assembled structures \\ HI-75/01/010/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SDLX03 Assembly of braced thin rectangular plates
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Author (S):
B. QUINNEZ Key
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\section*{6 \\ Results of modeling B}

The results of 2 calculations (one with LIAISON_COQUE, the other with LIAISON_SOLIDE) are identical. One will mention only calculation with LIAISON_COQUE.

\subsection*{6.1 Values}
tested
Order of

\section*{Reference}

Model reference
Aster
\% difference/ experimental clean mode
elements finis*
model elements
finished
7
606.
\(584 . \pm 1 \%\)
610.2
4.5

8
760.
\(826 . \pm 1.5 \%\)
852.4
3.2

9
865.
\(855 . \pm 1.7 \%\)
864.8
1.1

10
944.
\(911 . \pm 2 \%\)
923.9
1.4

11
1113.
\(1113 . \pm 3.6 \%\)
1110.8
-0.2
12
1144.
\(1136 . \pm 4 \%\)
1179.5
3.8
* average of 5 computer codes
6.2 Remarks

\section*{Calculations carried out by:}
```

MODE_ITER_SIMULT
METHOD: "TRI_DIAG"
OPTION
"BAND"
LIST_FREQ:
(1.,1200.)
DIM_SOUS_ESPACE:
1 2

```

\section*{6.3}
```

Contents of the file results
The first 6 nonnull Eigen frequencies (clean vectors and modal parameters).

```

\subsection*{6.4 Parameters}
of execution
Version: 5.02

Machine: SGI Origin 2000

Obstruction memory:
28 Mo
Time CPU To use: 8.7 seconds

\section*{Handbook of Validation}

V2.05 booklet: Linear dynamics of the assembled structures
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX03 Assembly of braced thin rectangular plates
Date:
23/01/00
Author (S):
B. QUINNEZ Key

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\section*{7}

\section*{Summary of the results}

For the modeling A, the results provided by Code_Aster are in the interval of dispersion of codes which made it possible to establish the reference solution VPCS.

For modeling B, the two ways of writing the connection between the hulls give the same ones results.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLX100 - Transitory response of a plate to a field of pressure
Date:
17/02/04
Author (S):
E. BOYERE Key

V2.05.100-A Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
Document: V2.05.100

SDLX100-Transitory response of a plate with a field of pressure resulting from ENSIGHT

\section*{Summary:}

The applicability of this test relates to the dynamics of the structures, and more particularly the calculation of
linear transitory response direct to a field of pressure evolving/moving in time, defined in files with format ENSIGHT. It includes/understands a modeling.

It is a question of calculating the response of an element of hull supported in its 4 nodes out of 4 stiffnesses and shock absorbers, with a sinusoidal pressure defined in the four nodes.

This test makes it possible to validate the tools of definition of loading of pressure starting from files to the format
ENSIGHT and of calculation of direct transitory response to this loading.
The results obtained are in very good agreement with the analytical reference solution. Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
```

6.4

```

Titrate:
SDLX100-Transitory response of a plate to a field of pressure
Date:
17/02/04
Author (S):
E. BOYERE Key
:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{C}

N2
K
C
N1
P
\(K\)
C
\(X, U\)
has
N3
K
C
N4
K

Plate side has \(=1 . \mathrm{m}\), thickness \(E=1.282 \mathrm{E}-04 \mathrm{~m}\)
Stiffnesses: K
Viscous depreciation: C

\section*{1.2 \\ Material properties}

Material of the plate:
\(E=2.1 x 1011 P a\)
\(=0.3\)
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)
Comes out from linear elastic translation one-way: \(K=9.8696 E+4 \mathrm{~N} / \mathrm{m}\)
One-way viscous damping: \(C=3.1416 \mathrm{~N}(\mathrm{~m} / \mathrm{s})\)
These values correspond to a reduced damping of \(1 \%\) on the first mode of the structure.

\section*{1.3}

Boundary conditions and loadings
Sinusoidal uniform pressure applied to the plate along axis \(X\), defined by a value in the four nodes:
\(P=P 0 \sin T\)
\(=2 F\),
\(F=100 \mathrm{~Hz}=\) first Eigen frequency of the system.
P0
\(==1 \mathrm{~N} / \mathrm{m} 2\) is a total force on the F 0 plate \(=a 2 \mathrm{P}=1 \mathrm{~N}\)
constant

\subsection*{1.4 Conditions \\ initial}

Structure initially at rest.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SDLX100-Transitory response of a plate to a field of pressure
Date:
17/02/04
Author (S):
E. BOYERE Key

\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
The simple oscillator checks the following equation:
```

mu\&+cu\&+ku=F 涪(T

```
)
0
with \(\boldsymbol{U}()\)
\(0=0\) and \(u \&\) ()
\(0=0\)
\(K\)
: own pulsation of the oscillator \(=\)
m

\section*{Critical damping is \(C\)}
m
critical \(=2\).

\section*{C}

The solution for a subcritical damping such as = is:
ccritic

\section*{\(F\) \\ F \\ F}
\(U(T)=\) and \(0 \cos (T)+\)
0
\(\sin (T)-0 \cos (T)\)

\section*{2 K}

D
\(2 k\)
D
```

2K
D
with
2
D=1 -

```

\section*{2.2}

Results of reference
Displacement according to \(X\) of the N1 point.

\section*{2.3}

Uncertainty on the solution
Analytical solution.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures HT-66/04/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.4

Titrate:
SDLX100-Transitory response of a plate to a field of pressure
Date:
17/02/04
Author (S):
E. BOYERE Key

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Element of DST hull

\section*{Discrete elements:}

DISCRETE: with matrices of rigidity in translation \(K_{-} T_{-} D_{-} N\)
and matrices of damping
A_T_D_N
Names of the nodes: N1, N2, N3 and N4

Loading of pressure contained in the repertory sdlx100a.ensi:
carre_m.result:
- a number of moments of definition of the pressures: 200
\(\cdot\) urgent of definition of the pressures: \(n * t(T=2.5 E-04, N=0,199)\)
- name of the file containing the affected nodes of a pressure: carre_measured.geom
- root of the names of the files containing the value of the pressures for each step of time:
pressure. ***
carre_measured.geom: coordinated nodes N1, N2, N3 and N4 where the pressure is defined pression.n (pression. 000 with pression.199):
file containing the values of pressure to the 4 nodes with \(T=n * t\).

Direct transitory calculation:
No time used T = 1.E-4 S
Integration NEWMARK \(=0.25,=0.5\)

\section*{3.2}

Characteristics of the grid

A number of nodes \(=4\)
A number of meshs and type \(=1\) QUAD4

\section*{3.3}

Functionalities tested

Orders

\section*{LIRE_RESU FORMAT "ENSIGHT"}

\section*{TYPE_RESU}
"EVOL_CHAR"

\author{
NOM_CHAM "CLOSE" \\ AFFE_CHAR_MECA PRES_CALCULEE
}
AFFE_CARA_ELEM HULL
NET
DISCRETE
GROUP_NO ..... "K_TR_D_N"
"A_TR_D_N"
DYNA_LINE_TRAN EXCIT ..... CHARGE

4

Results of modeling \(A\)

\author{
4.1 Values
}
tested
Time (S)
Reference (m)
Aster (m) Difference
(\%)
0.005 3,917E06
3.906E06
-0.28
0.015 1,139E05
1.136E05
-0.26
0.025 1,841E05
1.836E05
-0.27
0.035 2,500E05
2.493E05

\title{
0.045 3,119E05
}

\subsection*{3.111E05}
-0.25

\section*{Handbook of Validation}

V2.05 booklet: Linear dynamics of the assembled structures
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
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Titrate:
SDLX100-Transitory response of a plate to a field of pressure
Date:
17/02/04
Author (S):
E. BOYERE Key
\(:\)
V
\(5 /\)
5

V2.05.100-A Page:
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\section*{5}

Summary of the results
The results of analytical reference are found with a very good precision (less than 0.3\% of variation).

\section*{Handbook of Validation}

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Code_Aster \({ }^{\circledR}\)
Version
6.4

\section*{Titrate:}

SDLX100-Transitory response of a plate to a field of pressure
Date:
17/02/04
Author (S):
E. BOYERE Key

\title{
V2.05 booklet: Linear dynamics of the assembled structures
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
8.0

\section*{Titrate:}

SDLX301-Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
: V2.05.301-B Page:
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Organization (S): EDF-R \& D /AMA, CS IF, SAMTECH

\author{
Handbook of Validation \\ V2.05 booklet: Linear dynamics of the assembled structures \\ Document: V2.05.301 \\ SDLX301 - Building with floor-columns \\ dissymmetrical subjected to a horizontal excitation
}

\section*{Summary:}

Applicability: seismic calculation. Type of analysis: spectral and combinations. A number of modelings: 1.
Description: three-dimensional study of a building with 3 floors out of 9 columns, embedded at the base of
columns, with unbalance, subjected to a horizontal seismic excitation in displacement. The distribution
offset of the masses of the floors allows to break symmetry, to couple the geometrical directions principal and to generate an effect of torsion. The values of reference are obtained with the code CASTEM 2000 and the SAMCEF software, which have slightly different methods. The columns are modelled by beams, and the floors by elements of plane hull. Eights first clean modes are preserved for calculations of modal recombination. Objective: to test them displacements, interior efforts, and reactions to the embedding of a column for the recombinations modal CQC, SRSS, DSC. Precision of the results: comparison between codes. Strong tolerances are allowed for certain computed fields whose values are several weaker orders of magnitude. Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures
HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
8.0

Titrate:
SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
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\section*{1}

Problem of reference
This case test is inspired by the report/ratio referred to [bib1].

\subsection*{1.1 Geometry}

The studied building is composed of 3 floors and 9 columns embedded in the floors.
\(Z\)
\(y\)
0.40 m
\(y\)
G
H
I
\(X\)
0.20 m

E
F
D
section of the columns

Section of columns A with I
: \(0.20 \mathrm{~m} \mathrm{X} \mathrm{0.40m}\)
Surface of columns A with I
: 8.00102 m 2
With

\section*{B}

C
Inertias of columns A with I : Ix = \(2.667104 \mathrm{m4}\) (in the total reference mark)

\section*{\(I y=1.066103 m 4\)}

\section*{PFI}

\section*{\(J=7.45104 \mathrm{m4}\)}

Coefficients of reduced section
\(A Y=A Z=1.2\)
Thickness of the floors
: 0.2 m
\(X\)

PFF

PFA
PFB
PFC
\(y\)
\(Z\)
\(B\)
\(E\)
\(H\)
\(G\)
\(H\)
\(I\)
\(P M 6\)
\(3 m\)
\(P M 5\)
\(C m\)
\(4 m\)
\(P M 4\)
\(D\)
```

E
F
X
3m
PM3
4m
PM2
With
B
C
3m
PM1
PM0
y
4m
4m
Appear 1.1-a
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```

Code_Aster \({ }^{\circledR}\)
Version
8.0

Titrate:
SDLX301-Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
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\section*{1.2 \\ Properties of materials}

In order to obtain the centre of mass Cm, excentré compared to the geometrical center of 0,3071 m, one
material of density \(2=1,8481\) is affected with \(1 / 4\) surface of each floor (PLAN21, PLAN22
and PLAN23).

Columns and left PLAN11, PLAN21 and PLAN31 of the floors:

Young modulus:
E1 = 4,0 1010 Pa Poisson's ratio:
\(1=0,15\)
Density:
\(1=2500 \mathrm{~kg} / \mathrm{m} 3\)
Parts PLAN12, PLAN22 and PLAN32 of the floors:
Young modulus:
E2 = 4,0 1010 Pa Poisson's ratio:
\(2=0,15\)
Density:
\(2=4620 \mathrm{~kg} / \mathrm{m} 3\)
\(y\)
H
I
G
PLANi2
Cm
X
MAT2
D
E
F
MAT1
PLANil
With
B
C
Floor \(n^{\circ}\) I (groups of meshs PLANi1 and PLANi2)
Appear 1.2-a
1.3

Boundary conditions and loading
Boundary condition
The columns are embedded on the level of the foundation.

Loading

The seism is applied in direction \(X\).
The spectrum of response of oscillators in displacement is obtained by superposition of four spectra of displacement. Each one of these spectra of displacement corresponds to the response of an oscillator to
a degree of freedom to the sinewave excitations defined in table [Table 1.3-a] Ci below:
SD (
I
Amplitude K (m)
I

\section*{Damping}
sine 1
1.51
0.15
0.05
sine 2
2.05
0.25
0.05
sine 3
2.34
0.25
0.05
sine 4
4.86
0.30
0.05

\author{
Table 1.3-a
}

The neglected modes are represented by a pseudo-mode.
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures HT-66/05/005/A

Titrate:
SDLX301 - Dissymmetrical building subjected to a seismic excitation
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Author (S):
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\section*{2 \\ Reference solutions}

\section*{2.1 \\ Method of calculation used for the reference solutions}

The calculations taken for reference are carried out with codes CASTEM 2000 and the SAMCEF software. The solution
of reference is not given by the results of [bib1] because it missed in this reference some geometrical characteristics and of material to remake with identical the model of the structure studied. Certain data retained in this case test are thus different from those of the report/ratio [bib1], what does not allow a comparison of the results.

\section*{2.2 \\ Results of reference}

Frequencies calculated with CASTEM 2000 and the SAMCEF software,
Spectrum of response in displacement for a damping from \(=5 \%\), - Déplacements by modal recombination CQC, SRSS, DSC for the column B (calculated in taking into account the first 8 modes primarily torsion of the building and inflection of columns, but the floors are bent little),

Dynamic and pseudo fashion for the static correction,
Efforts with the embedding of the column B and the central column E,
Interior efforts along the column B.

\section*{2.3 \\ Uncertainty on the solution}

Comparison between codes

\subsection*{2.4 References \\ bibliographical}
[1]
Mr. MONTAY: Dynamic calculation of the structures in seismic zone. Free university of Brussels, 1982.

\section*{Handbook of Validation}

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Code_Aster \({ }^{\circledR}\)
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Titrate:
SDLX301 - Dissymmetrical building subjected to a seismic excitation
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F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
: V2.05.301-B Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{3.2 \\ Characteristics of the grid}

The grid of the model calculated with Code_Aster consists of 3357 nodes and 3387 meshs of which 135 elements of right beam of Timoshenko (including 12 SEG2 by column, is 108 for the columns) and 3072 elements plates DKT (1024 per floor). In order to ensure the continuity of ddl DRZ of rotation clean of the beams with rotation around the normal of the plates (uninsured automatically by Code_Aster) of the elements of beams are added locally at the edge of plates DKT, with
level of the 27 connections column-floor, to ensure the transmission of rotations DRZ related to plane movement of the plate in rotation in the plan \((X, y)\).

The grid of the model calculated with CASTEM 2000 consists of 3765 nodes and 7368 elements including 108 elements right beam of Timoshenko and 6960 elements of hull DKT.
The grid of the model calculated with the SAMCEF software consists of 3360 nodes and 3180 elements of which
108 elements right beam of Mindlin and 3072 elements of hull of Mindlin.

\author{
3.3 Functionalities
}
tested

\section*{Orders}

\section*{COMB_SISM_MODAL COMB_MODE}

Version
8.0

\section*{Titrate:}

SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
: V2.05.301-B Page:
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Remarks}

For a given node I, effort generalized for element I-1 and for element I is compared respectively in tables "low" element and "high" element.
The efforts are given in the local reference mark of the elements of beam (principal reference mark of inertia).

\section*{4.2 \\ Calculation of the modal base}

Eigen frequencies ( Hz )
Mode
Code_Aster
CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
11.512
1.5120 .036
1.495
1.11
22.052
2.0500 .125
2.014
1.93
32.365
2.3430 .916
2.291
3.24
44.848
4.8590 .237
4.823
0.522
57.488
7.5210 .448
7.415
0.99
68.388
8.4260 .456
8.355
0.392
78.547
8.5430 .037
8.438
1.30
815.185
15.4051 .428
15.186
0.004

Visualization: Mode \(n^{\circ} 1\);
Mode n \({ }^{\circ}\) 3;
Mode \({ }^{\circ} 7\).
Effective modal masses ( \(k g\) )
Mode and Code_Aster direction
CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
1 X
2,129E+01
2.300E+01
-7,451
2.070E+01

2,846
Y
1,115E+05
1.113E+05

0,127

Z
5,203E02
\(6.698 E 02\)
-22,319
5.816E02
-10,531
2 X
9,559E+04
9.365E+04

2,068
\(9.294 E+04\)
2,847
Y
1,532E+02
\(1.817 E+02\)
-15,689
1.683E+02
-8,967
Z
1,002E02
1.440E02
-30,405
1.500E02
-33,186
\(3 X\)
1,063E+04
\(1.238 E+04\)
-14,202
1.201E+04
-11,509
Y
\(4,954 E+02\)
5.181E+02
-4,399
5.010E+02
-1,119
Z
6,074E03
9.450E03
-35,736
8.390E03
Z
3,026E02
4.386E02 31,012 4.668E02 35,178
6 X
1,829E+02
\(3.9466 E+02\)
-53,662
1.901E+03
-90,380
Y
3,771E+03
\(3.622 E+03\)

4,112
\(1.300 E+03\)
190,089
Z
1,282E01
1.809E01
-29,145
1.336E01
-4,028
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8.0

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\(7 X\)
2,064E+03
\(2.1461 E+03\)
-3,842
\(5.331 E+02\)
287,105
Y
\(9,264 E+01\)
\(2.7942 E+02\)
-66,846
\(2.627 E+03\)
-96,474
Z
1,449E02
1.222E02

18,522
2.709E02
-46,519

\section*{8 X}

4,932E+03
\(4.948 E+03\)
-0,346
4.974E+03
-0,851
Y
1,130E+00
1.121E+00

0,752
\(1.035 E+00\)
9,143
Z
5,731E+01
\(1.5420 E+02\)
-62,836
\(5.098 E+01\)
12,411
Office plurality \(X\)
1,2948E+05 1,2936E+05
0,092\% 1,2832E+05 0,905\%
Y
\(1,3037 E+051,3030 E+05\)
0,053\% 1,2919E+05 0,911\%
Z
5,7706E+01 1,5473E+02
\(-62,706 \% 5,1448 E+0112,164 \%\)
Note:
The standard of error of the modes calculated by the method of Sorensen de Code_Aster is always lower
at 10-9.
Note:
The total mass of the building is 132552 kg ; the strong orientation according to of the modes is due to the relative one there
less inertia according to there of the columns. Effective modal mass cumulated in direction \(X\) of the seism
obtained by Code_Aster \(\mathbf{9 7 . 6 7 8 \%}\) of the total mass represent.
Note:

The differences between modelings and software are rather strong in direction Z, because it is little solicited in these modes.
4.3Spectral answer - method CQC
Displacements - column B (in meter)
Altitude Z (m)
Component Code_Aster CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
PM1: 1.5
X
1.829E03
1.717E036.466
1.641E0311.439N982 Y
2.303E04
2.276E04
1.190
1.730E03
-86.686
Z
1.882E06
1.763E06
6.728
2.112E05
-91.087
PM2: 3.0
X
5.411E03
5.108E03
5.935
5.255E03
2.968
1st floor
Y
5.709E04
\(5.679 E 04\)
0.526

\author{
3.304E03 \\ -82.722 \\ N1245 Z \\ 3.764E06 \\ 3.526E06 \\ 6.729 \\ 4.223E05 \\ -91.087 \\ PM3: 4.5 \\ \(X\) \\ 9.762E03 \\ 9.243E03 \\ 5.608 \\ 9.551E03 \\ 2.209 \\ N1530 Y \\ 9.277E04 \\ 9.246E04 \\ 0.331 \\ 8.594E03 \\ -89.205 \\ Z \\ 4.750E06 \\ 4.452E06 \\ 6.671 \\ 5.540E05 \\ -91.426 \\ PM4: 6.0 \\ X \\ 1.409E02 \\ 1.336E02 \\ 5.462 \\ 1.381E02 \\ 2.047 \\ 2nd floor \\ Y \\ 1.259E03 \\ 1.255E03 \\ 0.296 \\ 1.229E02 \\ -89.756 \\ N1815 Z \\ 5.736E06
}

\author{
5.379E06 \\ 6.633 \\ 6.857E05 \\ -91.634 \\ PM5: 7.5 \\ \(X\) \\ 1.780E02 \\ 1.689E02 \\ 5.352 \\ 1.747E02 \\ 1.890 \\ N2106 Y \\ 1.486E03 \\ 1.482E03 \\ 0.224 \\ 1.539E02 \\ -90.347 \\ Z \\ 6.014E06 \\ 5.642E06 \\ 6.598 \\ 7.376E05 \\ -91.846 \\ PM6: 9.0 \\ X \\ 2.085E02 \\ 1.980E02 \\ 5.319 \\ 2.057E02 \\ 1.383 \\ 3rd floor \\ Y \\ 1.661E03 \\ 1.657E03 \\ 0.223 \\ 1.789E02 \\ -90.713 \\ N2355 Z \\ 6.293E06 \\ 5.905E06 \\ 6.567 \\ 7.896E05 \\ -92.029
}

Reaction (NR) and Moment (N.m) with the embedding of the column B (N758 node)

\section*{Reaction or Code_Aster moment \\ CASTEM 2000}

Variation in \%
The SAMCEF software
Variation in \%
Fx
\(3.445 E+04\)
\(3.325 E+04\)
3.590
\(3.362 E+04\)
2.460

Fy
\(1.644 E+03\)
\(1.629 E+03\)
0.916
\(2.265 E+03\)
-27.405
Fz
\(4.015 E+03\)
\(3.761 E+03\)
6.729
\(5.000 E+03\)
-19.694
MX
\(2.986 E+03\)
\(2.975 E+03\)
0.348
\(4.145 E+03\)
-27.967
My
\(8.488 E+04\)
\(8.135 E+04\)
4.336
\(8.225 E+04\)
3.208

Mz 1.8460E03
1.772E01
-98.958
\(2.165 E+01\)
-99.99

\author{
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}

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Reaction (NR) and Moment (N.m) with the embedding of the central column E (N885 node)

\section*{Reaction or Code_Aster moment}

CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
Fx
\(5.799 E+04\)
\(5341 E+04\)
8.552
5.056E+04
14.686

Fy
\(2.080 E+03\)
\(2.071 E+03\)
0.428
\(2.849 E+03\)
-26.994
Fz
\(2.471 E+02\)
\(4.067 E+02\)
-39.247
1.978E+03
-87.504
MX

\author{
\(3.419 E+03\) \\ \(3.417 E+03\) \\ 0.044 \\ \(4.728 E+03\) \\ -27.691 \\ My \\ \(1.202 E+05\) \\ \(1.116 E+05\) \\ 7.705 \\ \(1.074 E+05\) \\ 11.913 \\ Mz \\ 1.842E03 \\ 1.770E01 \\ -98.959 \\ \(2.591 E+01\) \\ -99.99
}

Generalized efforts of the column B (in local reference mark)
Table "low" element (see remark [§ 4.1])

\author{
Altitude Z (m) \\ Component Code_Aster CASTEM 2000 \\ Variation in \% \\ The SAMCEF software \\ Variation in \% \\ PM1: 1.5 \\ NR (NR) \\ \(4.015 E+03\) \\ \(3.7618 E+03\) \\ 6.728 \\ \(5.000 E+03\) \\ -19.702 \\ M3158 N982 \\ Vy (NR) \\ 1.640E+03 \\ \(1.627 E+03\) \\ 0.770 \\ \(2.260 E+03\) \\ -27.445 \\ Vz \\ (NR) \\ \(3.441 E+04\)
}
```

3.323E+04
3.528
3.320E+04
3.634
MT
(N.m)
1.846E03
1.756E01
-98.949
2.160E+01
-99.99
Mfy
(N.m)
3.325E+04
3.151E+04
5.522
3.320E+04
0.154
Mfz
(N.m)
5.215E+02
5.333E+02
-2.227
7.650E+02
-31.835
PM2: 3.0
NR (NR)
4.015E+03
3.761E+03
6.727
4.999E+03
-19.689
1st floor
Vy (NR)
1.618E+03
1.610E+03
0.482
2.230E+03
-27.450
M3160 N1245
Vz (NR)
3.420E+04
3.308E+04

```

\author{
\(2.993 E+04\) \\ 2.265 \\ MT \\ (N.m) \\ 1.594E03 \\ 1.402E01 \\ -98.863 \\ \(2.570 E+01\) \\ -99.99 \\ Mfy \\ (N.m) \\ \(1.434 E+03\) \\ 1.390E+03 \\ 3.161 \\ 1.440E+03 \\ -0.385 \\ Mfz \\ (N.m) \\ 1.295E+02 \\ 1.342E+02 \\ -3.554 \\ 1.890E+02 \\ -31.483 \\ PM4: 6.0 \\ NR (NR) \\ \(2.104 E+03\) \\ 1.976E+03 \\ 6.450 \\ \(2.636 E+03\) \\ -20.187 \\ 2nd floor \\ Vy (NR) \\ 1.324E+03 \\ \(1.315 E+03\) \\ 0.618 \\ 1.850E+03 \\ -28.458 \\ M3164 N1815 \\ Vz (NR) \\ \(2.993 E+04\) \\ \(2.941 E+04\) \\ 1.751 \\ \(2.931 E+04\)
}
(N.m)
1.594E03
1.049E01
-98.481
\(2.570 E+01\)
-99.99
Mfy
(N.m)
\(4.583 E+04\)
\(4.471 E+04\)
2.480
\(4.430 E+04\)
3.445

Mfz
(N.m)
\(2.157 E+03\)
\(2.126 E+03\)
1.456
\(2.990 E+03\)
-27.858
PM5: 7.5
NR (NR)
\(5.956 E+02\)
\(5.629 E+02\)
5.817
\(7.749 E+02\)
-23.133
M3166 N2106
Vy (NR)
\(7.279 E+02\)
\(7.312 E+02\)
-0.453
1.040E+03
-30.006
Vz
(NR)
\(1.935 E+04\)
\(1.934 E+04\)
0.039
\(1.934 E+04\)
0.039

\section*{MT}
(N.m)
9.470E04
6.137E02
-98.457
2.660E+01
-99.99
Mfy
(N.m)
1.234E+04
1.184E+04
4.184
1.210E+04
2.006

Mfz
(N.m)
\(2.511 E+02\)
\(2.578 E+02\)
-2.607
3.800E+02
-33.921

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Table "high" element (see remark [§ 4.1])
Altitude Z (m)
Component Code_Aster CASTEM 2000

\section*{Variation in \%}

The SAMCEF software
Variation in \%
PMO: 0.0
NR (NR)
\(4.015 E+03\)
\(3.762 E+03\)
6.729

M3157 N758
Vy (NR)
1.644E+03
\(1.629 E+03\)
0.928
.
Vz
(NR)
\(3.444 E+04\)
\(3.325 E+04\)
3.593

MT
(N.m)
1.846E03
1.770E01
-98.957

Mfy
(N.m)
8.488E+04
8.135E+04
4.336
--
\(M f z\)
(N.m)
2.986E+02
\(2.975 E+02\)
0.348

PM1: 1.5
NR (NR)
\(4.015 E+03\)
\(3.762 E+03\)
6.728
\(5.000 E+03\)
-19.702
M3159 N982
Vy (NR)
1.640E+03
\(1.621 E+03\)
1.149
\(2.250 E+03\)
-27.123
Vz
(NR)
\(3.440 E+04\)
\(3.318 E+04\)
3.683
\(3.300 E+04\)
4.258

MT
(N.m)
1.846E03
1.717E01
-98.925
\(2.160 E+01\)
-99.99
Mfy
(N.m)
\(3.325 E+04\)
\(3.150 E+04\)
5.529
\(3.260 E+04\)
1.997
\(M f z\)
(N.m)
\(5.215 E+02\)
\(5.331 E+02\)
-2.183
\(7.640 E+02\)
-31.746
PM2: 3.0
NR (NR)
\(2.104 E+03\)
\(1.976 E+03\)
6.454
```

2.640E+03
-20.289
1 st floor
Vy (NR)
1.419E+03
1.386E+03
2.319
1.950E+03
-27.233
M3161 N1245
Vz (NR)
3.103E+04
3.020E+04
2.733
3.010E+04
3.094
MT
(N.m)
1.595E03
1.540E01
-98.965
2.570E+01
-99.99
Mfy
(N.m)
4.591E+04
4.485E+04
2.354
4.500E+04
2.022
Mfz
(N.m)
1.976E+03
1.935E+03
2.095
2.730E+03
-27.610
PM3: 4.5
NR (NR)
2.104E+03
1.976E+03
6.453
2.640E+03

```

M3163 N1530
Vy (NR)
1.381E+03
\(1.344 E+03\)
2.739
1.900E+03
--27.309
Vz
(NR)
\(3.061 E+04\)
\(2.974 E+04\)
2.917
\(2.970 E+04\)
3.062

MT
(N.m)
1.594E03
1.237E01
-98.712
2.570E+01
-99.99
Mfy
(N.m)
\(1.434 E+03\)
1.391E+03
3.109
1.440E+03
-0.385
Mfz
(N.m)
1.295E+02
\(1.345 E+02\)
-3.741
1.900E+02
-31.843
PM4: 6.0
NR (NR)
\(5.960 E+02\)
5.630E+02
5.847
7.750E+02
-23.103
```

2nd floor
Vy (NR)
7.978E+02
7.660E+02
4 . 1 4 7
1.080E+03
-26.130
M3165 N1815
Vz(NR)
2.023E+04
1.976E+04
2.358
1.970E+04
2.696
MT
(N.m)
9.477E04 8.407E02 98.873
2.670E+01
-99.99
Mfy
(N.m)
1.751E+04
1.762E+04
-0.636
1.790E+04
-2.179
Mfz
(N.m)
8.967E+02
8.675E+02
3 . 3 6 3
1.280E+03
-29.945
PM5: 7.5
NR (NR)
5.956E+02
5.627E+02
5.842
7.750E+02
-23.143
M3167 N2106
Vy (NR)
7.279E+02

```

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\section*{4.4 \\ Spectral answer - method SRSS}

Displacements column B (in m)
Altitude Z (m)
Component Code_Aster CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
PM1: 1.5
\(X\)
1.593E03
1.4749E03
7.998
1.40E03
13.779

N982 Y
2.767E04
\(2.795 E 04\)
-1.050
1.47E03
-81.179
Z
\(2.253 E+06\)
2.156E06
4.469
1.95E05
-88.447
PM2: 3.0
\(X\)
4.714E03
4.386E03
7.475
4.46E03
5.707

1st floor
Y
PM3: 4.5
\(X\)
\(8.508 E 03\)
7.939E03
7.169
8.13E03
4.655
N1530 Y
1.113E03
1.133E03
-1.797
7.32E03
-84.798
Z
5.684E06
5.443E06
4.412
5.12E05
-88.898

PM4: 6.0
X
1.229E02
1.148E02
7.043
1.18E02
4.153
2nd floor
Y
1.510E03
1.538E03
-1.842
1.05E02
```

-85.621

```
N1815 Z
\(6.862 E 06\)
\(6.574 E 06\)
4.374
\(6.33 E 05\)
-89.159
PM5: 7.5
\(X\)
1.552E02
1.451E02
6.956
1.49E02
4.187
N2106 Y
1.780E03
1.815E03
-1.902
\(1.31 e 02\)
-86.410
Z
7.195E06
6.896E06
4.338
6.80E06
-89.419
PM6: 9.0
X
1.820E02
1.701E02
6.944
1.75E02
3.981
3rd floor
Y
1.990E03
2.028E03
-1.902
1.52E02
-86.908
N2355 Z
7.528E06
7.217E06

Reaction (NR) and Moment (N.m) with the embedding of the column B (N758 node)

\section*{Reaction or Code_Aster moment}

CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
Fx
\(2.999 E+04\)
\(2.854 E+04\)
5.040
\(2.883 E+04\)
4.006

Fy
1.977E+03
\(2.006 E+03\)
-1.434
\(1.914 E+03\)
3.336

Fz
4.806E+03
\(4.600 E+03\)
4.468
\(4.254 E+03\)
12.973

MX
\(3.587 E+03\)
\(3.657 E+03\)
-1.902
\(3.510 E+03\)
2.203

My
7.393E+04
6.985E+04
5.830
\(7.011 E+04\)
5.448

Mz
2.240E03
1.772E01
-98.736
1.989E+01
-99.99
Reaction (NR) and Moment (N.m) with the embedding of the central column E (N885 node)

\section*{Reaction or Code_Aster moment}

CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
Fx
5.591E+04
\(5.094 E+04\)
9.754
\(4.797 E+04\)
16.558

Fy
\(2.499 E+03\)
\(2.545 E+03\)
-1.818
\(2.413 E+03\)
3.571

Fz
2.472E+02
\(4.068 E+02\)
-39.240
1.972E+03
-87.462
MX
4.106E+03
\(4.196 E+03\)
-2.161
4.008E+03
2.454

My
1.159E+05
\(1.064 E+05\)
8.897
\(1.019 E+05\)
13.769

Mz

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Generalized efforts of the column \(B\)
Table "low" element (see remark [§4.1])

\section*{Altitude Z (m)}

Component Code_Aster CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
PM1: 1.5
NR (NR)
\(4.806 E+03\)
\(4.600 E+03\)
4.469
\(4.250 E+03\)
13.082

M3158 N982
Vy (NR)
1.972E+03
\(2.003 E+03\)
\(1.910 E+03\)
3.233

Vz
(NR)
\(2.995 E+04\)
\(2.853 E+04\)
4.979
\(2.830 E+04\)
5.832

MT
(N.m)
2.240E03
1.756E01
-98.725
1.990E+01
-99.99
Mfy
(N.m)
\(2.898 E+04\)
\(2.706 E+04\)
7.088
\(2.780 E+04\)
4.261

Mfz
(N.m)
\(6.254 E+02\)
\(6.536 E+02\)
-4.319
\(6.500 E+02\)
-3.781
PM2: 3.0
NR (NR)
\(4.806 E+03\)
\(4.600 E+03\)
4.467
\(4.250 E+03\)
13.078

1st floor
Vy (NR)
\(1.944 E+03\)
1.980E+03
-1.839
```

2.703

```

Vz
(NR)
\(2.670 E+04\)
\(2.578 E+04\)
3.551
\(2.560 E+04\)
4.291

MT
(N.m)
1.935E03
1.402E01
-98.621
2.570E+01
-99.99
Mfy
(N.m)
1.246E+03
1.152E+03
8.141
\(1.120 E+03\)
11.269

Mfz
(N.m)
\(1.564 E+02\)
1.656E+02
-5.535
1.600E+02
-2.217
PM4: 6.0
NR (NR)
\(2.514 E+03\)
\(2.413 E+03\)
4.183
\(2.240 E+03\)
12.253

2nd floor
Vy (NR)
1.592E+03
1.626E+03
-2.151
1.560E+03
2.033

\section*{M3164 N1815}

Vz (NR)
2.611E+04
\(2.528 E+04\)
3.308
\(2.500 E+04\)
4.461

MT
(N.m)
1.934E03
1.049E01
-98.156
\(2.570 E+01\)
-99.99
Mfy
(N.m)
3.993E+04
\(3.840 E+04\)
3.962
\(3.780 E+04\)
5.635

Mfz
(N.m)
\(2.598 E+03\)
2.630E+03
-1.242
\(2.520 E+03\)
3.085

PM5: 7.5
NR (NR)
7.130E+02
\(6.904 E+02\)
3.270
\(6.590 E+02\)
8.197

M3166 N2106
Vy (NR)
8.779E+02
\(9.099 E+02\)
-3.521
8.730E+02
0.564

Vz
(NR)
\(1.693 E+04\)
\(1.663 E+04\)
1.802
\(1.650 E+04\)
2.635

MT
(N.m)
1.150E03
6.137E02
-98.128
\(2.500 E+01\)
-99.99
Mfy
(N.m)
\(1.075 E+04\)
\(1.017 E+04\)
5.675
\(1.030 E+04\)
4.382
\(M f z\)
(N.m)
\(3.034 E+02\)
\(3.201 E+02\)
-5.243
\(3.200 E+02\)
-5.199
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
8.0

Titrate:
SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
: V2.05.301-B Page:

\title{
Table "high" element (see remark [§ 4.1])
}

Altitude Z (m)
Component Code_Aster CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
PM0: 0.0
NR (NR)
4.806E+03
\(4.600 E+03\)
4.468
--
M3157 N758
Vy (NR)
1.977E+03
\(2.006 E+03\)
-1.429

Vz
(NR)
\(2.998 E+04\)
\(2.854 E+04\)
5.043

MT
(N.m)
2.240E03
1.770E01
-98.735
- -

Mfy
(N.m)
7.393E+04
\(6.986 E+04\)
5.830
\(M f z\)
(N.m)
\(3.587 E+03\)
\(3.657 E+03\)

PM1: 1.5
\(N R(N R)\)
\(4.806 E+03\)
\(4.600 E+03\)
4.469
\(4.250 E+03\)
13.082

M3159 N982
Vy (NR)
\(1.972 E+03\)
\(1.995 E+03\)
-1.176
\(1.900 E+03\)
3.776

Vz
(NR)
\(2.995 E+04\)
\(2.848 E+04\)
5.140
\(2.810 E+04\)
6.580

MT
(N.m)
2.240E03
1.717E01
-98.696
1.990E+01
-99.99
Mfy
(N.m)
\(2.898 E+04\)
\(2.706 E+04\)
7.096
\(2.780 E+04\)
4.261

Mfz
(N.m)
\(6.254 E+02\)
\(6.533 E+02\)
-4.276
\(6.500 E+02\)
-3.781
PM2: 3.0
NR (NR)
\(2.515 E+03\)
\(2.414 E+03\)
4.191
\(2.240 E+03\)
12.280

1 st floor
Vy (NR)
\(1.709 E+03\)
\(1.716 E+03\)
-0.403
1.640E+03
4.231

M3161 N1245
Vz (NR)
\(2.706 E+04\)
\(2.595 E+04\)
4.269
\(2.570 E+04\)
5.304

MT
(N.m)
1.935E03
1.540E01
-98.744
\(2.570 E+01\)
-99.99
Mfy
(N.m)
\(4.009 E+04\)
\(3.856 E+04\)
3.969
\(3.850 E+04\)
4.131
\(M f z\)
(N.m)
\(2.380 E+03\)
\(2.397 E+03\)
-0.703
\(2.300 E+03\)
3.481

\author{
PM3: 4.5 \\ NR (NR) \\ \(2.515 E+03\) \\ \(2.413 E+03\) \\ 4.193 \\ \(2.240 E+03\) \\ 12.268 \\ M3163 N1530 \\ Vy (NR) \\ 1.664E+03 \\ \(1.664 E+03\) \\ -0.025 \\ 1.600E+03 \\ 3.987 \\ Vz \\ (NR) \\ 2.670E+04 \\ \(2.555 E+04\) \\ 4.469 \\ \(2.530 E+04\) \\ 5.534 \\ MT \\ (N.m) \\ 1.935E03 \\ 1.237E01 \\ -98.437 \\ \(2.570 E+01\) \\ -99.99 \\ Mfy \\ (N.m) \\ 1.246E+03 \\ 1.153E+03 \\ 8.084 \\ \(1.120 E+03\) \\ 11.269 \\ Mfz \\ (N.m) \\ \(1.564 E+02\) \\ \(1.659 E+02\) \\ -5.734 \\ 1.610E+02 \\ -2.824 \\ PM4: 6.0
}

\author{
NR (NR) \\ 7.134E+02 \\ \(6.906 E+02\) \\ 3.299 \\ \(6.590 E+02\) \\ 8.255 \\ 2nd floor \\ Vy (NR) \\ \(9.610 E+02\) \\ \(9.506 E+02\) \\ 1.087 \\ \(9.100 E+02\) \\ 5.607 \\ M3165 N1815 \\ Vz (NR) \\ 1.769E+04 \\ 1.700E+04 \\ 4.099 \\ 1.680E+04 \\ 5.325 \\ MT \\ (N.m) \\ 1.150E03 \\ 8.407E02 \\ -98.632 \\ 2.500E+01 \\ -99.99 \\ Mfy \\ (N.m) \\ \(1.537 E+04\) \\ \(1.516 E+04\) \\ 1.330 \\ 1.540E+04 \\ -0.197 \\ Mfz \\ (N.m) \\ 1.092E+03 \\ 1.1021E+03 \\ -0.959 \\ 1.060E+03 \\ 2.974 \\ PM5: 7.5 \\ NR (NR)
}
7.130E+02
\(6.902 E+02\)
3.297
\(6.580 E+02\)
8.362

M3167 N2106
Vy (NR)
\(8.779 E+02\)
\(8.644 E+02\)
1.561
8.310E+02
5.647

Vz
(NR)
1.693E+04
1.622E+04
4.345
1.610E+04
5.174

MT
(N.m)
1.149E03
3.736E02
-96.925
2.490E+01
-99.99
Mfy
(N.m)
\(1.075 E+04\)
1.017E+04
5.654
\(1.030 E+04\)
4.382
\(M f z\)
(N.m)
\(3.034 E+02\)
\(3.202 E+02\)
-5.282
3.210E+02
-5.494
Handbook of Validation
V2.05 booklet: Linear dynamics of the assembled structures

Code_Aster \({ }^{\circledR}\)
Version
8.0

Titrate:
SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
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\section*{4.5 \\ Spectral answer - method ROSENBLUETH DSC}

For this method, we used a time of 30 seconds simulation.
Displacements - column B (in m)
Altitude Z (m)
Component Code_Aster CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
PM1: 1.5
\(X\)
1.858E03
1.746E03
6.396
1.643E3
13.110

N982 Y
2.230E04
2.197E04
1.493
1.732E3
-87.124
Z
1.823E06
1.703E06

X
5.499E03
5.194E03
5.864
5.241 E3
4.917

1st floor
Y
\(5.528 E 04\)
5.4825E04
0.827
\(4.845 E 3\)
-88.590
N1245 Z
3.646E06
3.406E06
7.048
4.225E5
-91.370
PM3: 4.5
X
9.919E03
9.398E03
5.534
9.560E3
3.751

N1530 Y
8.983E04
8.927E04
0.631
8.603E3 -89.558
Z
4.601E06
4.300E06
6.991
5.453E5
-91.699
PM4: 6.0
\begin{tabular}{l}
\(X\) \\
\(1.432 E 02\) \\
\(1.359 E 02\) \\
5.386 \\
\(1.383 E 2\) \\
3.548 \\
\(2 n d\) floor \\
\(Y\) \\
\(1.219 E 03\) \\
\(1.212 E 03\) \\
0.596 \\
\(1.23 E 2\) \\
-90.087 \\
\(N 1815 Z\) \\
\(5.557 E 06\) \\
\(5.195 E 06\) \\
6.953 \\
\(6.861 E 5\) \\
-91.901 \\
\(P M 5: 7.5\) \\
\(X\) \\
\(1.808 E 02\) \\
\(1.717 E 02\) \\
5.273 \\
\(1.748 E 2\) \\
3.434 \\
\(N 2106 Y\) \\
\(1.439 E 03\) \\
\(1.431 E 03\) \\
0.525 \\
\(1.54 E 2\) \\
-90.657 \\
\(Z\) \\
\(5.827 E 06\) \\
\(5.450 E 06\) \\
6.918 \\
\(7.381 E 5\) \\
-92.105 \\
\(P M 6: 9.0\) \\
\(2.119 E 02\) \\
\(2.013 E 02\) \\
5.239 \\
\hline
\end{tabular}

\section*{Reaction or moment}

Code_Aster

\section*{CASTEM 2000}

Variation in \%
The SAMCEF software
Variation in \%
Fx
\(3.501 E+04\)
\(3.381 E+04\)
3.524
\(3.368 E+04\)
3.938

Fy
1.592E+03
1.572E+03
1.223
2.270E+03
-29.885
Fz
\(3.889 E+03\)
\(3.633 E+03\)
7.050
\(5.007 E+03\)
-22.330
MX
```

2.891E+03
2.872E+03
0.647
4.154E+03
-30.410
My
8.626E+04
8.273E+04
4 . 2 6 7
8.236E+04
4 . 7 3 8
Mz
1.787E03
1.772E01
-98.992
2.170E+01
-99.99
Reaction (NR) and Moment (N.m) with the embedding of the central column E (N885 node)

```

\section*{Reaction or moment}
```

Code_Aster
CASTEM 2000
Variation in \%
The SAMCEF software
Variation in \%
Fx
$5.827 E+04$
$5.374 E+04$
8.432
$5.061 E+04$
15.148
Fy
$2.014 E+03$
1.999E+03
0.724
$2.855 E+03$
-29.471
Fz
$2.471 E+02$
$4.067 E+02$
-39.252
$1.979 E+03$

```

\section*{Handbook of Validation}

V2.05 booklet: Linear dynamics of the assembled structures HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
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SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
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Generalized efforts of the column B
Table "low" element (see remark paragraph [§4.1])
Altitude Z (m) Component Code_Aster CASTEM 2000 Variation in \%
The SAMCEF software
Variation in \%
PM1: 1.5
NR (NR)
\(3.889 E+03\)
3.633E+03
7.048
\(5.007 E+03\)
-22.327
Vy
(NR)
\(1.588 E+03\)
\(1.570 E+03\)
1.070
\(2.266 E+03\)
-29.938
Vz
(NR)
\(3.497 E+04\)
\(3.380 E+04\)
3.461
\(3.325 E+04\)
5.172

MT
(N.m)
1.786E03
1.756E01
-98.983
\(2.169 E+01\)
-99.99
Mfy
(N.m)
\(3.378 E+04\)
\(3.204 E+04\)
5.445
\(3.264 E+04\)
3.507

Mfz
(N.m)
\(5.050 E+02\)
\(5.149 E+02\)
-1.933
7.659E+02
-34.067
PM2: 3.0
NR (NR)
\(3.889 E+03\)
\(3.632 E+03\)
7.047
5.007E+03
-22.330
1st floor
Vy (NR)
1.566E+03
\(1.554 E+03\)
0.785
\(2.235 E+03\)
-29.911
M3160
Vz (NR)
\(3.476 E+04\)
\(3.364 E+04\)
3.304
\(3.290 E+04\)
5.646

N1245
MT
(N.m)
1.786E03
1.646E01
-98.915
2.167E+01
-99.99
Mfy
(N.m)
1.861E+04
\(1.858 E+04\)
0.181
\(1.752 E+04\)
6.243

Mfz
(N.m)
1.863E+03
\(1.825 E+03\)
2.075
\(2.623 E+03\)
-28.960
PM3: 4.5
NR (NR)
\(2.039 E+03\)
1.910E+03
6.770
\(2.641 E+03\)
-22.799
M3162 N1530
Vy (NR)
1.338E+03
\(1.321 E+03\)
1.255
\(1.934 E+03\)
-30.834
Vz
(NR)
3.109E+04
\(3.050 E+04\)
1.931
\(2.997 E+04\)
3.751

MT
(N.m)
1.543E03

\section*{-98.53}
2.573E+01
-99.99
Mfy
(N.m)
4.656E+04
\(4.546 E+04\)
2.413
\(4.436 E+04\)
4.969
\(M f z\)
(N.m)
\(2.089 E+03\)
\(2.053 E+03\)
1.759
\(2.999 E+03\)
-30.340
PM5: 7.5
NR (NR)
\(5.775 E+02\)
\(5.441 E+02\)
6.129
\(7.773 E+02\)
-25.708
M3166 N2106
Vy (NR)
7.058E+02
\(7.068 E+02\)
-0.143
1.045E+03
-32.457
Vz
(NR)
1.964E+04
\(1.965 E+04\)
-0.056
\(1.937 E+04\)
1.389

MT
(N.m)
9.165E04
6.137E02
-98.51
\(2.669 E+01\)
-100.00
Mfy
(N.m)
1.254E+04
\(1.205 E+04\)
4.116
1.210E+04
3.652

Mfz
(N.m)
\(2.431 E+02\)
\(2.488 E+02\)
-2.314
\(3.815 E+02\)
-36.283

\section*{Handbook of Validation}

V2.05 booklet: Linear dynamics of the assembled structures
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Code_Aster \({ }^{\circledR}\)
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8.0

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SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05
Author (S):
F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
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Table "high" element (see remark paragraph [\$4.1])
Altitude Z (m)
Component Code_Aster CASTEM 2000 Variation in \%
The SAMCEF software
Variation in \%
PMO: 0.0
NR (NR)
3.889E+03
```

3.633E+03
7.050
M3157 N758
Vy (NR)
1.592E+03
1.573E+03
1.229
Vz
(NR)
3.501E+04
3.382E+04
3.527
--
MT
(N.m)
1.787E03
1.770E01
-98.991

```
Mfy
(N.m)
8.626E+04
8.273E+04
4.267
Mfz
(N.m)
2.891E+03
\(2.872 E+03\)
0.647
PM1: 1.5
NR (NR)
\(3.889 E+03\)
\(3.633 E+03\)
7.048 5.007E+03
-22.327
M3159 N982
Vy (NR)
1.587E+03
\(1.564 E+03\)

\subsection*{1.451 2.255E+03}
-29.596
Vz
(NR)
\(3.497 E+04\)
\(3.375 E+04\)
\(3.6203 .306 E+04\)
5.773

MT
(N.m)
1.786E03
1.718E01
98.96 2.168E+01
-99.99
Mfy
(N.m)
\(3.378 E+04\)
\(3.204 E+04\)
\(5.4523 .263 E+04\)
3.539

Mfz
(N.m)
\(5.050 E+02\)
\(5.147 E+02\)
1.891 7.655E+02
-34.033
PM2: 3.0
NR (NR)
\(2.039 E+03\)
1.910E+03
\(7.3582 .641 E+0322.791\)
M3161 N1245
Vy (NR)
\(1.374 E+03\)
\(1.339 E+03\)
2.625 1.374E \(+03-29.711\)

Vz
(NR)
3.153E+04
\(3.071 E+04\)
2.659 3.017E+04 4.493

MT
(N.m)
```

1.543E03
1.540E01
98.99 2.572E+01
-99.99
Mfy
(N.m)
4.663E+04
4.559E+04
2.271 4.51E+04 3.363
Mfz
(N.m)
1.914E+03
1.869E+02
2.396 2.742E+03 30.190
PM3: 4.5
NR (NR)
2.039E+03
1.909E+03
6.775 2.641E+03 22.799
M3163 N1530
Vy (NR)
1.338E+03
1.298E+03
3.049 1.902E+03 29.670
Vz
(NR)
3.110E+04
3.023E+04
2.843 2.969E+04 4.734
MT
(N.m)
1.543E03
1.237E01
97.75 2.573E+01 99.99
Mfy
(N.m)
1.442E+03
1.398E+03
3.095 4.440E+04 0.110
Mfz
(N.m)
1.253E+02
1.298E+02

```
3.464 1.904E+02 34.174

PM4: 6.0
NR (NR)
\(5.778 E+03\)
\(5.442 E+02\)
\(6.1587 .775 E+0225.689\)
2nd floor
Vy (NR)
7.731E+03
7.402E+02
\(4.4441 .087 E+0328.875\)
M3165 N1815
Vz (NR)
\(2.054 E+04\)
\(2.008 E+04\)
2.267 1.977E+04 3.887

MT
(N.m)
9.172E04
8.407E02
98.909 2.670E+01 100.00

Mfy
(N.m)
\(1.776 E+04\)
\(1.789 E+04\)
0.745 1.797E+04 1.176

Mfz
(N.m)
8.696E+02
8.388E+02
\(3.6631 .284 E+0332.273\)
PM5: 7.5
NR (NR)
\(5.775 E+02\)
\(5.440 E+02\)
6.153 7.771E+02 25.689

M3167 N2106
Vy (NR)
\(7.058 E+02\)
\(6.704 E+02\)
5.279 9.968E+02 29.191

Vz
(NR)
Code_Aster \({ }^{\circledR}\)
Version8.0
Titrate:
SDLX301 - Dissymmetrical building subjected to a seismic excitation
Date:
26/09/05Author (S):F.VOLDOIRE, E.LECLERE, P.LACLERGUE, Y.PONS Key
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\section*{5}

Summary of the results

\section*{Comparison with CASTEM 2000:}

The variations on the Eigen frequencies calculated with CASTEM 2000 and Aster are lower than 1,4\%.
The double mode was separate in two close modes ( 6 and 7) of which one is a dominating mode according to the axis \(y\) (mode 6) and the other according to \(X\) (mode 7); the variation on the effective
modal masses (in \%)
very high according to \(X\) for mode 6 and according to there for mode 7, is not relevant being given the weak one
weight of these directions in the modes considered.
The variations obtained on calculation with the spectral method, for displacements remain broadly lower than \(8 \%\), the variations on the reactions to the embedding of the columns B and E are broadly lower than \(11 \%\) (without taking account of the moment of reaction according to Z ), and variations on
generalized efforts remain overall lower than \(7 \%\) (without taking account of the torque). Strong tolerances are allowed for certain computed fields whose values are of several weaker orders of magnitude.

\section*{Comparison with the SAMCEF software:}

The method of resolution adopted in the SAMCEF software is based on the method known as of the ground node.
This method consists in binding to a single node all the nodes which are interdependent of the foundation. It
node is affected of a mass in translation which is worth 1000 times the mass of the structure. displacements deferred in the tables are not corrected effects of residual masses which are results also available.

The variations on the Eigen frequencies calculated with the SAMCEF software and Aster are lower than 3,2\%.
type of element of hull used (deformable or not with the sharp effort) influence the result, it goes from there from
even of the smoothness of the grid of the floors. Variations on the Eigen frequencies going until \(10 \%\) were observed by initially taking a coarser grid for the floors, made up
of 345 nodes and 516 elements including 108 elements of right beam of Timoshenko and 408 elements hull DKT. Modes 6 and 7 represent a mode doubles of which percentage of modal mass effective \(4 \%\) in the direction \(X\) and \(2 \%\) in direction \(Y\). do not exceed.

Variations obtained on calculation with the spectral method, for displacements in the direction excitation remain overall lower than 10,5\%. For the reactions to the embedding of column B, these variations are overall lower than \(30 \%\). They reach \(80 \%\) for the column \(E\), however for the reaction according to axis \(X\) and the moment according to the axis \(y\), they remain lower than \(18 \%\).
reaction of torsion of the columns is not null. Variations in connection with the efforts generalized in direction of the excitation remain overall lower than \(26 \%\). On the other hand, a different coupling between the directions of the excitation introduces important variations on the efforts into the directions transverses with the excitation.
Strong tolerances are allowed for certain computed fields whose values are of several weaker orders of magnitude.

\section*{Note:}
the form of the function describing the spectrum in displacement strongly depends on Eigen frequencies fi for which the peaks of displacement are given. In consequence, a shift of the calculated Eigen frequencies disturbs the answer seismic in entry of the data and an effective comparison of calculations does not allow,
the results of generalized efforts are expressed in the local reference mark of the beams and corrected static effects.

\section*{Handbook of Validation}

V2.05 booklet: Linear dynamics of the assembled structures
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX302 fixed Beam and masses concentrated subjected to a force
Date:
30/08/01
Author (S):
J. PIGAT, P. GUIHOT, T. FRIOU, E. LECLERE Key

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Organization (S): EDF/RNE/AMV, CS IF

\section*{Handbook of Validation}

\section*{V2.05 booklet: Linear dynamics of the assembled structures}

V2.05.302 document

\section*{SDLX302-Fixed beam and masses concentrated subjected to a transverse random force}

\section*{Summary: \\ A beam fixed with a concentrated mass is subjected to a random effort in the direction transverse.}

This test validates, using a comparison between codes, the calculation of the clean modes of inflection and that of displacement within the framework of a stochastic approach.

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX302 fixed Beam and masses concentrated subjected to a force
Date:
30/08/01
Author (S):
J. PIGAT, P. GUIHOT, T. FRIOU, E. LECLERE Key

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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{1.2}

Properties of materials
Density of the tube
\(=7850 \mathrm{~kg} . \mathrm{m}-3\)
Young modulus
\(E=210 . E+9\) N.m-2
Poisson's ratio
\[
=0 .
\]

\section*{1.3}

Boundary conditions and loading
The tube is embedded at the base. The mass is free. The movement is authorized in a vertical plane (DX, DRZ).

A random effort \(\boldsymbol{F}(\mathbf{T})\), applied to the concentrated mass is compared to a random process stationary Gaussian, centered, of white vibration type to band limited of 1.Hz to 101.Hz. It is characterized
by a standard deviation \(F=1 \mathrm{kN}\), and a unilateral spectral concentration in frequency \(S\) ( \(F\) F
) such as:
F [H
1 Z,
H
101 Z]

\title{
\(S(F)=F=104 \mathrm{NR} 2 \mathrm{~s}\) \\ F \\ 100 \\ Handbook of Validation \\ V2.05 booklet: Linear dynamics of the assembled structures \\ HT-62/01/012/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SDLX302 fixed Beam and masses concentrated subjected to a force
Date:
30/08/01
Author (S):
J. PIGAT, P. GUIHOT, T. FRIOU, E. LECLERE Key
:
V2.05.302-A Page:
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\(S(F\)
F)

1
101
F (Hz)

\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
Calculations are carried out with 8 computer codes, for 10 modelings. The different ones modelings are presented below.

\section*{Castem:}

20 elements of beam without shearing;

Dynam2D:
20 elements of beam without shearing;
PERMAS (1): 20 elements of beam without shearing;
Nastran:
20 elements of beam without shearing;
SYSTUS (1):
20 elements of beam without shearing;

ABAQUS:
20 elements of beam with shearing;
MECHANICA: 5 elements of beam with shearing, convergence with degree 7;
BEAVER:
10 elements of beam with shearing;
SYSTUS (2):
40 elements of beam with shearing;
PERMAS (2):
20 elements of beam with shearing;
Reduced damping is worth \(1 \%\) on all the modes.

\section*{2.2}

Results of reference
Frequencies.
Value RMS, for displacement at the loose lead of the beam.
2.3

Uncertainty on the solution
Comparison between codes.

\subsection*{2.4 References}
bibliographical
[1]
IPSI - Day 2 .AS - Flight XVIII, \(n^{\circ}\) 2. Damping in the structural analyses. June 21

SDLX302 fixed Beam and masses concentrated subjected to a force
Date:
30/08/01
Author (S):
J. PIGAT, P. GUIHOT, T. FRIOU, E. LECLERE Key

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}
\(y\)
\(X\)

Along the beam, we have \(D Y=D Z=D R X=D R Y=0\).
At the base, all the ddls are blocked.

\section*{3.2}

Characteristics of the grid
The grid consists of 21 nodes and 20 elements Timoshenko beam.

\section*{3.3 \\ Functionalities tested}

\author{
Orders
}

\section*{DEFI_FONCTION}

DEFI_INTER_SPEC PAR_FONCTION

\author{
DYNA_ALEA_MODAL EXCIT \\ INTERSPECTRE
}

\author{
CHAM_NO \\ ANSWER \\ \section*{REST_SPEC_PHYS} \\ POST_DYNA_ALEA GAUSS \\ \section*{Handbook of Validation} \\ V2.05 booklet: Linear dynamics of the assembled structures \\ HT-62/01/012/A
}
```

Code_Aster ${ }^{\circledR}$
Version
5.0
Titrate:
SDLX302 fixed Beam and masses concentrated subjected to a force
Date:
30/08/01
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J. PIGAT, P. GUIHOT, T. FRIOU, E. LECLERE Key
:
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4
Results of modeling $A$

```

\subsection*{4.1 Values}
tested
Eigen frequencies of inflection

\section*{Number}
mode 12345678910
Castem 0.70
5.04
14.69
28.78
46.1966 .8292 .74125 .15163 .91208 .81

DYNAM2D/DRAM 0.705 .0414 .67
28.7546 .1866 .8692 .77125 .05163 .38208 .81

PERMAS (1)
0.70
5.04
14.69
28.78
46.1966 .8292 .74125 .15163 .91208 .81

Nastran 0.70
5.04
14.67
28.75
46.1866 .8692 .78125 .05163 .48207 .70

SYSTUS (1)
0.72
5.13
14.91
29.19
46.8367 .6593 .64125 .95164 .39208 .61

ABAQUS 0.70
5.02
14.57
28.42
45.4065 .1789 .26118 .50152 .33

MECHANICA 0.70
5.03
14.62
28.54
45.6365 .6090 .21120 .43155 .88196 .01

BEAVER 0.70
5.03
45.4865 .3589 .88120 .24154 .40194 .06

SYSTUS
(2)
0.70
5.03
14.59
28.42
45.35

PERMAS (2)
0.70
5.03
14.60
28.4845 .5065 .2989 .59119 .41138 .42

Average values
0.70
5.04
14.66
28.6645 .8966 .2791 .51122 .77157 .79204 .69

ASTER 0.70
5.03
14.59
28.43
45.3865 .0789 .17118 .67153 .24192 .40

Variation Aster
0.000 .260 .480 .791 .121 .812 .563 .342 .886 .00

Average values
in \%
Displacement (m): with \(F x=1000\)

\title{
Value \\ Variation (\%) Value \\ CASTEM DYNAM2D \\ (1) \\ (2) \\ (1) \\ Average ABAQUS ASTER moyenne/ASTER \\ 0.039 0.038 0.041 0.038 0.035 0.039 0.038 0.0344 \\ -9.4
}

\subsection*{4.2 Remarks}

Problems of definition between effort intern and forced do not allow to compare them results of ASTER with the other codes (bending moment and sharp effort).

\subsection*{4.3 Parameters}
of execution
Version: 5.02

Machine: SGI ORIGIN 2000
Obstruction memory:
16 MW
Time CPU To use:
5.98 seconds
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Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
SDLX302 fixed Beam and masses concentrated subjected to a force
Date:
30/08/01
Author (S):

\title{
J. PIGAT, P. GUIHOT, T. FRIOU, E. LECLERE Key
}

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\section*{5}

Summary of the results
The results of the modal base are good since the maximum change is to the maximum of \(6 \%\) on last frequency.

For the results in displacement we obtain a variation of 9.4.
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Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLL100 Response of a bar per under-structuring
Date:
29/05/96
Author (S):
G. ROUSSEAU, C. VARE Key
:
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Organization (S): EDF/EP/AMV

\section*{Handbook of Validation}

V2.06 booklet: Harmonic response of the linear structures
Document: V2.06.100

SHLL100-Harmonic response of a bar
by dynamic under-structuring

\section*{Summary:}

The applicability of this test relates to the dynamics of the structures, and more particularly the calculation of
harmonic response by dynamic under-structuring.
It is a question of calculating the harmonic response in traction and compression of a embed-free beam modelled by elements of the type "bars". The modelled structure is deadened (damping of Rayleigh by elements).

The results of reference result from a direct harmonic calculation. This test thus makes it possible to validate the tools of calculations of harmonic response per under-structuring established in Code_Aster and more particularly:
the catch in depreciation account by element,
the calculation of the second member including the harmonic loading,
restitution of the harmonic response on a grid skeleton, including the fields of displacement, speed and of acceleration.
Handbook of Validation
V2.06 booklet: Harmonic response of the linear structures
HP-51/96/024/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
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Date:
29/05/96
Author (S):
G. ROUSSEAU, C. VARE Key

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1
Problem of reference

\subsection*{1.1 Geometry}
ep
With
F
X
\(D\)
L
\(L=1 \mathrm{~m}\)
\(D=0,2 m-\) circular Section

\section*{1.2}

Material properties
\[
E=1.1010 \mathrm{~Pa}
\]
\[
=0.3
\]
\(=1.104 \mathrm{~kg} / \mathrm{m} 3\)
Damping of Rayleigh per element: \(=0.1\)
E
\(E=0.1\)

\title{
1.3 \\ Boundary conditions and loadings
}

Embedding in end a: \(U(0)=N(0)=W(0)=0\).
For any point \(M(X): N(0)=W(0)=0\).
Harmonic loading in time, at the loose lead:
orientation: according to \(X\),
amplitude: 100 NR ,
frequency: 100 Hz .

\subsection*{1.4 Conditions initial}

Without object for a harmonic calculation of answer.
Handbook of Validation
V2.06 booklet: Harmonic response of the linear structures HP-51/96/024/A

Code_Aster \({ }^{\circledR}\)
Version
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

There is an analytical solution detailed in the reference [bib2].
Let us use the following notations:

\section*{E}
: Young modulus
\(L\)
: length of the bar
With
: section of the bar
NR
: normal effort directed according to axis \(X\)
: damping coefficients of Rayleigh
: frequency
of excitation
and let us pose
\(1+2\)
/2
\(R=\)
\(1+22\)
\(1+22\)
\(1+22\)

Displacement in a point \(M(X)\) unspecified is given by:

1
shpxcosqx+ ichpxsinqx
\(V X)=\)
\(E A(p+i q)(1+I) c h L \cos q L+i \operatorname{shp} L \operatorname{sinq} L\)

\section*{Displacement (m)}

Speed (m/s)
Acceleration (m/s2)
Real part
-7.00 10-11 -3.18
10-6 2.76
10-5
Imaginary part
5.07 10-9 -4.40

10-8-2.00
10-3

\section*{2.2}

\section*{Results of reference}

Fields of displacement, speed and acceleration of the loose lead of the bar.

\section*{2.3 \\ Uncertainty on the solution}

Numerical solution.

\subsection*{2.4 References \\ bibliographical}
[1] \(T\).
KERBER
"harmonic Under-structuring in Code_Aster", Report/ratio EDF,
HP-61/93-104.
[2]
G. ROBERT, analytical Solutions in dynamics of the structures, Report/ratio Samtech \(n^{\circ} 121\), March 1996.
[3]
P. RICHARD, Methods of under-structuring in Code_Aster, Internal report EDF-DER, HP-61/92-149.
Handbook of Validation

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLL100 Response of a bar per under-structuring
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G. ROUSSEAU, C. VARE Key
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

\section*{F}

L/2
L/2
The bar is cut out in 2 parts of equal size. Each substructure considered is with a grid in segments to which are affected elements "bars".

The structure is studied using the method of the harmonic under-structuring with interfaces of HARMONIC type CRAIG-BAMPTON.

The modal base used is made up of 4 clean modes for the substructure of right-hand side, of 5 clean modes for the substructure of left to which are added the constrained modes harmonics associated with the interfaces (calculated to 300 Hz . This value of the pulsation does not have any influence on the result, it is arbitrary [bib3]).

\subsection*{3.2 Functionalities \\ tested}

Orders

\author{
Keys \\ DEFI_INTERF_DYNA INTERFACES \\ TYPE \\ "CB_HARMO" [U4.55.03] \\ DEFI_INTERF_DYNA FREQ \\ 300 \\ [U4.55.03] \\ MACR_ELEM_DYNA OPTION \\ "TRADITIONAL" \\ [U4.55.05] \\ MACR_ELEM_DYNA MATR_AMOR \\ [U4.55.05] \\ ASSE_MATR_GENE OPTION \\ AMOR_GENE \\ [U4.55.08] \\ ASSE_VECT_GENE NUME_DDL_GENE \\ [U4.55.09] \\ ASSE_VECT_GENE CHAR_SOUS_STRUC \\ SOUS_STRUC \\ [U4.55.09] \\ ASSE_VECT_GENE CHAR_SOUS_STRUC \\ VECT_ASSE \\ [U4.55.09]
}

\section*{3.3 \\ Characteristics of the grid}

A number of nodes: 5
A number of meshs and types: 5 SEG 2
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)

\section*{Version}

3

Titrate:
Harmonic SHLL100 Response of a bar per under-structuring
Date:
29/05/96
Author (S):
G. ROUSSEAU, C. VARE Key

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Displacement}
(m)

\section*{Reference}

Aster
\% difference
Tolerance
Real part
-7.00 10-11-7.00
10-11
-0.007
2.10-3

Imaginary part
5.07 10-9 5.07

10-9
-0.097
2.10-3

Speed (m/s)

Real part
-3.18 10-6 -3.18

\section*{Acceleration}
(m/s2)

Real part
2.76 10-5 2.76

10-5
0.133
2.10-3

Imaginary part
-2.00 10-3-2.00
10-3
-0.019
2.10-3

\subsection*{4.2 Parameters \\ of execution}

Version: STA3.0.9
Machine: CRAY C90
System:
UNICOS 6.0
Obstruction memory:
8 megawords
Time CPU To use:
8.6 seconds

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\(\qquad\)
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\section*{5 \\ Summary of the results}

Precision on the complex co-ordinates of the fields of displacement speed and acceleration is lower than 0,1\%.

This test thus validates the operators of harmonic under-structuring.

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Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SHLL101 right Beam. Analyze harmonic
Date:
01/09/99
Author (S):

\section*{B. QUINNEZ, G. DEVESA}

Key:
V2.06.101-B Page:
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Organization (S): EDF/IMA/MMN, EP/AMV
Handbook of Validation
V2.06 booklet: Harmonic response of the linear structures
Document: V2.06.101
SHLL101-Right beam. Analyze harmonic

\section*{Summary:}

This two-dimensional problem consists in calculating the efforts present in a beam subjected to a

\section*{traction or}
with an inflection during a harmonic analysis. The reference solution is obtained starting from the equations
discretized.
This test comprises two modelings.
For the first modeling, four requests are tested:
- force of traction,
- force of traction and material presenting a damping,
- strength flexural,
- strength flexural and material presenting a damping.

For the second modeling, two requests are tested:
- force of traction,
- force of traction and material presenting a damping.

The second modeling makes it possible to test the complex loadings imposed by the order
AFFE_CHAR_MECA_C.
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

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Titrate:
SHLL101 right Beam. Analyze harmonic
Date:
01/09/99
Author (S):

\section*{B. QUINNEZ, G. DEVESA}

Key:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(y\)
\(y\)
\(y, v\)
\(X, U\)
With
B
X
Z

The geometrical characteristics of the beam constituting the mechanical model are as follows:
Length: \(L=10 \mathrm{~m}\)
Cross section
Surface
\(I Z=I Y\)
\(J X\)
3.439103 m 2
1.377105 m 4
2.754105 m 4

The co-ordinates (in meters) of the points characteristic of the beam are:
With
B
\(X\)
0 .
10.
y
0 .
0.

\section*{1.2}

\section*{Material properties}

The properties of material constituting the beam are:
\(E=1.6581010 \mathrm{~Pa}\)
\(=0.3\)
= \(1.3404106104 \mathrm{~kg} / \mathrm{m} 3\)
\(=\) Amor_alpha \(=0.001\)
\(=A\) mor_beta \(=0\).
1.3

\section*{Boundary conditions and loadings}

The boundary condition which characterizes this problem is the embedding of point A and is written:
\(U=v=0\).
\(=0\).
For the loading one \(a\) :
\(F x=3000\). NR
\(F y=F z=0\).
(tractive effort)
\(F x=0\).
\(F y=3000 . N R\)
\(F z=0\).
(bending stress)
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Code_Aster \({ }^{\circledR}\)
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2
Reference solution
2.1

Method of calculation used for the reference solution
If the beam is modelled by a beam of Euler-Bernoulli and only one finite element, the problem harmonic can be written in the following way:

\section*{problem in traction:}
(
ES
SL
\(1+I)\)
\(B=E S\)

\section*{12th I 1}

2
3
\(11 L\)
\(L\)

3
\(L\)
- L

L()

B
0

105
2
3

Note:
If the material does not present damping, one has then: Amor_alpha \(==0\).
The efforts at the point \(B\) are calculated in the following way:
problem in traction:
ES
\(2 S L\)
NR ()
\(B=\)
(
)
\(B\)
\(L\)
6
problem in inflection:

13L
-11L2
```

-L
12
l
VY()
B
E I
2
y
=-235
210
2
MFZ()
B
-11L2
L3
L3
-L
L()
B

```

One analytically solves the systems \(2 \times 2\) to obtain the solution.

\section*{2.2}

\section*{Results of reference}

The results of reference are displacements, speeds, accelerations and the efforts
generalized obtained at the point B during the harmonic analysis.

\section*{2.3 \\ Notice for modeling B}

For modeling B, one wants to test the problem in traction in the case of key word FORCE_POUTRE who allows to apply efforts distributed. To obtain the same solution as the beam subjected to nodal force in its end, the relation between the effort distributed constant and the nodal force is:
FL
F ()
\(B=\)
X
2
With the values given to the 1.3 , one a: \(F=600 \mathrm{~N} / \mathrm{m}\)
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\section*{Code_Aster \({ }^{\circledR}\)}

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Key:
V2.06.101-B Page:
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2.4

\section*{Uncertainty on the solution}

If the assumptions are checked (beam of Euler-Bernoulli), the solution is analytical.

\subsection*{2.5 References}

\section*{bibliographical}
[1]
Reference material of Code_Aster: Elements of beams "exact" (right and curves) - [R3.08.01].
Handbook of Validation

\section*{Code_Aster \({ }^{\circledR}\)}

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\section*{3 Modeling}

With
3.1

\section*{Characteristics of modeling}
\(y, v\)
With
B
\(X, U\)
The beam consists of only one mesh.
The modeling used for the beam is that of Euler-Bernoulli (POU_D_E).
End A is embedded:
\(D X=D Y=D Z=0\).
\(D R X=D R Y=D R Z=0\).

\section*{3.2}

Characteristics of the grid
A number of nodes: 2
A number of meshs and types: 1 mesh of the type SEG 2
The points characteristic of the grid are as follows:
Not \(A=A\)
Not \(B=B\)

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"GENERAL"
ALL
```

[U4.24.01]

```
AFFE_CHAR_MECA
DDL_IMPO
NODE
[U4.25.01]
FORCE_NODALE
NODE
FX
FY
DEFI_MATERIAU
ELAS
E, RHO, NAKED
[U4.23.01]
AMOR_ALPHA
AMOR_BETA
CALC_MATR_ELEM
OPTION
"MASS_MECA"
[U4.41.01]
'AMOR_MECA
"RIGI_MECA"
CALC_VECT_ELEM
OPTION
"CHAR_MECA"
[U4.41.02]
DYNA_LINE_HARM
MATR_MASS
[U4.54.02]
MATR_RIGI
MATR_AMOR
EXCIT
VECT_ASSE
CALC_ELEM
OPTION
"EFGE_ELNO_DEPL"
[U4.61.02]
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Author (S):
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4
Results of modeling A
4.1

Values tested (reality-imaginary form)
Problem 1: traction
Not/Size
Reference
Aster
\% difference
displacement
B
DX
(5.318 10-5, 0.)
(5.318 10-5, 0.)

0 .
speed
B
DX
(0., 3.341 10-3)
(0., 3.341 10-3)

0 .
acceleration
B
DX
(-2.099 10-1, 0.)
(-2.099 10-1, 0.)
0 .
generalized effort
B
NR
(3000., 0.)
(3000., 0.)
0.

\section*{Problem 2: inflection}

Not/Size
Reference
Aster
\% difference
displacement
B
DY
(1.828 10-2, 0.)
(1.828 10-2, 0.)

0 .
DRZ (1.82 102, 0.)
(1.82 10-2, 0.)

0 .
speed
B
DY
(0., 1.1489)
(0., 1.1489)

0 .
DRZ (0. , 1.1438)
(0., 1.1438)
0.
acceleration
B
DY
(-7.219 10-1)
(-7.219 10-1, 0.)
0.

DRZ (7.186 101, 0.)
(-7.186 10-1, 0.)
0.
generalized effort
B
VY
(3000., 0.)
(3000., 0.)
0.

MFZ (0. , 0.)
(-1.164 10-10, 0.)
0 .
```

Problem 3: traction + damping
Not/Size
Reference
Aster
% diff
displacement
B D
(5.296 10-5, -3.363 10-3)
(5.296 10-5, -3.363 10-3)
0.
X
speed
B D
(2.113 10-4, 3.327 10-3)
(2.113 10-4, 3.327 10-3)
0.
X
acceleration
B D
(-2.091 10-1, 1.327 10-2)
(-2.091 10-1, 1.327 10-2)
0.
X
generalized effort
B NR
(2.987 103, -1.8975 102)
(2.987 103, -1.8975 102)
0.

```
Problem 4: inflection + damping
Not/Size
Reference
Aster
\%
diff
displacement
B DY
(1.746 10-2, -4.469 10-3)
(1.746 10-2, -4.469 10-3)
0 .
DRZ (1.757 102, 3.402 103)
(1.757 10-2, -3.402 10-3)
0 .
speed

\section*{B DY}
(2.808 10-1, 1.097)
(2.808 10-1, 1.097)

0 .
DRZ (2.138 101, 1.104)
(2.138 10-1, 1.104)

0 .
acceleration
B DY
(-6.895 10-1, 1.764 10-1)
(-6.895 10-1, 1.764 10-1)
0.

DRZ (6.94 101, 1.343 101)
(-6.94 10-1, 1.343 10-1)
0 .
generalized effort
B VY
(3.021 103, 1.212 102)
(3.021 103, 1.212 102)
0.

MFZ (1.567 102, 8.583 102)
(-1.567 102, -8.583 102)
0 .

\subsection*{4.2 Parameters}
of execution
Version: NEW 3.06
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
5.9 seconds

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\section*{Code_Aster © \({ }^{\circledR}\)}

Version
4.0

Titrate:
SHLL101 right Beam. Analyze harmonic
Date:
01/09/99
Author (S):

\section*{B. QUINNEZ, G. DEVESA}

Key:
V2.06.101-B Page:
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5 Modeling
B
5.1

Characteristics of modeling
y, v
With
B
X, U
The beam consists of only one mesh.
The modeling used for the beam is that of Euler-Bernoulli (POU_D_E).
End A is embedded:
\(\mathrm{DX}=\mathrm{DY}=\mathrm{DZ}=0\).
\(\mathrm{DRX}=\mathrm{DRY}=\mathrm{DRZ}=0\).

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 2
A number of meshs and types: 1 mesh of the type SEG 2
The points characteristic of the grid are as follows:
Not \(\mathrm{A}=\mathrm{A}\)
Not B = B

\subsection*{5.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
"GENERAL"
ALL
[U4.24.01]
AFFE_CHAR_MECA_C
DDL_IMPO
NODE
[U4.25.01]
FORCE_POUTRE

\section*{NODE}

FX

\section*{DEFI_MATERIAU}

ELAS
E, Rho, Naked
[U4.23.01]
Amor_alpha
Amor_Beta
CALC_MATR_ELEM
OPTION
"MASS_MECA"
[U4.41.01]
'AMOR_MECA
"RIGI_MECA"
DYNA_LINE_HARM
MATR_MASS
[U4.54.02]
MATR_RIGI
MATR_AMOR
EXCIT
CHARGE
FONC_MULT_C
CALC_ELEM
OPTION
"EFGE_ELNO_DEPL"
[U4.61.02]
EXCIT
CHARGE
FONC_MULT_C
Handbook of Validation
V2.06 booklet: Harmonic response of the linear structures
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SHLL101 right Beam. Analyze harmonic
Date:
01/09/99
Author (S):
B. QUINNEZ, G. DEVESA

Key:
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6
Results of modeling B
6.1
```

Values tested (reality-imaginary form)
Problem 1: traction (effort distributed real: null imaginary part)
Not/Size
Reference
Aster
% difference
displacement
B
DX
(5.318 10-5, 0.)
(5.318 10-5, 0.)
0.
speed
B
DX
(0., 3.341 10-3)
(0., 3.3414 10-3)
0.
acceleration
B
DX
(-2.099 10-1, 0.)
(-2.0994 10-1, 0.)
0.
generalized effort
B
NR
(3000., 0.)
(3000., 0.)
0.
Problem 2: traction (effort distributed complex: null rélle part)
Not/Size
Reference
Aster
% difference
displacement
B
DX
(0., 5.318 10-5)
(0., 5.318 10-5)
0.
speed
B

```

\section*{DX}
(-3.341 10-3, 0.)
(-3.3414 10-3 ,0.)
0 .
acceleration
B
DX
(0., -2.099 10-1)
(0., -2.0994 10-1)

0 .
generalized effort
B
NR
(0., 3000.)
(0., 3000.)
0.

Problem 3: traction + damping (effort distributed real: null imaginary part)
Not/Size

\section*{Reference}

Aster
\% diff
displacement
B
DX
(5.296 10-5, -3.363 10-3)
(5.2966 10-5, -3.3637 10-3)

0 .
speed
B
DX
(2.113 10-4, 3.327 10-3)
(2.1135 10-4, 3.3279 10-3)
0.
acceleration
B
DX
(-2.091 10-1, 1.327 10-2)
(-2.091 10-1, 1.3279 10-2)
0 .
generalized effort
B
NR
(2.9879 103, -1.897 102)
(2.987 103, -1.8975 102)
0.

Problem 4: inflection + damping (effort distributed complex: null real part)
Not/Size

\section*{Reference}

Aster
\% diff
displacement
B
DX
(3.363 10-3 , 5.296 10-5)
(5.296 10-5, -3.363 10-3)

0 .
speed
B
DX
(-3.327 10-3 , 2.113 10-4)
(-3.3279 10-3 , 2.1135 10-4)
0 .
acceleration
B
DX
(-1.327 10-2, -2.091 10-1)
(-1.3279 10-2, -2.091 10-1)
0.
generalized effort
B
NR
(1.897 102, 2.9879 103)
(1.8975 102, 2.98794 103)
0.

When the effort distributed is applied as an imaginary part of the loading, the reference solution is obtained from that of real modeling A while exchanging left and imaginary part and in changing the sign of the new real parts.

\subsection*{6.2 Parameters}
of execution
Version: NEW 4.03
Machine: CRAY C90
Obstruction memory:
16 MW
Time CPU To use:
7.9 seconds

7

\section*{Summary of the results}

The analytical results well are found.
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Code_Aster \({ }^{\circledR}\)
Version
8.1

Titrate:
SHLS200 - Nonparametric probabilistic model in harmonic
Date:
04/05/06
Author (S):
S. CAMBIER, A. BATOU Key
:
V2.06.200-A Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V2.06 booklet: Harmonic response of a linear system
Document: V2.06.200

\section*{SHLS200 - Nonparametric probabilistic model:}

Harmonic response of a under-structured plate

\section*{Summary:}

This case-test treats taking into account of random uncertainties for the calculation of a harmonic answer by under-structuring of the type Craig Bampton. We take again here an example of the literature made up of one plate supported at its ends.
The functionality of operator GENE_MATR_ALEA consisting in is tested taking for entering concept one concept macr_elem_dyna of a given substructure and to produce a random macr_elem_dyna (mass, stiffness and damping). The concepts macr_elem_dyna random products then make it possible to calculate
for example the field of confidence of the harmonic response of the plate.

\section*{Handbook of validation}

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HT-62/06/005/A
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Version
8.1

Titrate:
SHLS200 - Nonparametric probabilistic model in harmonic
Date:
04/05/06
Author (S):
S. CAMBIER, A. BATOU Key

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{charge}

\section*{1}

2

\section*{\(0.5 m\) \\ Z \\ y \\ Specific masses}

X
\(0.6 m\)
\(0.4 m\)

The model is a thin section thickness \(0.410-3\) m rectangular separate in two substructures including/understanding each one a concentrated mass.

\section*{1.2 \\ Material properties}

The materials is homogeneous and isotropic.
Mass density: \(7800 \mathrm{~kg} / \mathrm{m} 3\),
11
Young modulus: \(2.110 \mathrm{~N} / \mathrm{m} 2\)
Concentrated stiffness: \(2.388107 \mathrm{~N} / \mathrm{m}\),
Coordinated stiffnesses concentrate: \((0.28,0.22),(0.54,0.33)\) and \((0.83,0.44)\),
Mass concentrated: 3 kg for substructure 1 and 4 kg for substructure 2,
Coordinated masses concentrate: \((0.4,0.2)\) and \((0.75,0.35)\).

\section*{Damping}

The matrix of damping [D] is defined as being a linear combination of the matrices averages of mass [M] and stiffness [K]:
2 maxmin

\section*{2}
\([D]=a[M]+B[K]\) with \(A=\)
and
\(B=\)
\(\max +\min\)
\(\max +\min\)
where \(=0.04, \mathrm{~min}=5.2 \mathrm{rad} / \mathrm{s}\) and \(\mathrm{max}=212.8 \mathrm{rad} / \mathrm{s}\).

\section*{1.3}

Boundary conditions and loadings
The flexbeam is in simple support on its four edges.
Substructure 1 is subjected to an effort external with the item \((0.24,0.24)\) equal to \(1 N\) on the tape of analysis [0, 2 .100] rad/s according to direction \(Z\) and null on other DDLs and for the others frequencies.

\subsection*{1.4 Conditions initial}

The dynamic system is initially at rest.
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\section*{Titrate:}

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The method used is the method published in particular in [bib1] (cf [\$2.3]). Uncertainties random of the dynamic system are modelled by using the probabilistic model said not parametric of uncertainties due to Soize (cf [bibl]). Statistics on the harmonic answer of linear dynamic system are obtained by the method of Monte Carlo. The procedure is identical with that presented to [§ 3.3], but developed under Matlab.
In the standard commodity, 500 pullings are carried out and the envelopes lower and higher of these 500 pullings are presented.

\section*{2.2 \\ Results of reference}

The ddls of observation correspond to direction DZ of the node of co-ordinates (0.39, 0.31) for first substructure and of the node of co-ordinates \((0.79,0.24)\) for the second substructure. The results of reference are given in the form of the graphs below drawn from [bibl].

Node (0.39, 0.31), DZ
Node (0.79, 0.24), DZ

\section*{Inter-quantiles fields (milked thin) of displacements to the ddls of observation \\ Results of reference}
(the fatty features are approximations, not used here for the comparison)

\subsection*{2.3 Reference \\ bibliographical}
[1]
C. SOIZE and H. CHEBLI: "Random Uncertainties Model in Dynamic Substructuring Using has Nonparametric Probabilistic Model, ASCE Newspaper of Engineering Mechanics, 0733-9399(2003)129:4(449).
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Date:
04/05/06
Author (S):

\section*{S. CAMBIER, A. BATOU Key}

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\section*{3 Modeling}

\section*{With}

\section*{3.1 \\ Characteristics of modeling}

\section*{Modeling: DKT}

The average model with the finite elements of the plate consists of a regular rectangular grid whose step is constant and is worth 0.01 m in the directions X1 and X2. Consequently, all elements stop are identical and each one is an element plates with 4 nodes.

\section*{Modeling: DIS_T}

The concentrated masses and the concentrated stiffness are modelled by elements DIST_T.

\section*{3.2 \\ Characteristics of the grid}

The average model finite elements comprises 14849 active degrees of freedom, including 8840 for substructure, 1, 5860 for substructure 2 and 149 for the interface.

A number of degrees of freedom: \(8840+5860+149\)
A number of finite elements: 6000 QUA4 et3 DIS_T

\section*{3.3 \\ Method of calculation}

\section*{Small-scale model}

As for the reference, we take for each substructure 20 modes in order to have good convergence of the calculated response with respect to the number of modes.

\section*{Achievements of the random matrices of the nonparametric probabilistic model by under structure}

For each under structures, reduced matrices of masses, stiffnesses and dissipation of model means are replaced by achievements of the random matrices of mass, stiffnesses and of dissipation according to the nonparametric probabilistic model. For that, we use the generator random matrices GENE_MATR_ALEA which generates a concept macr_elem_dyna from one macr_elem_dyna average.

One can thus allot to each substructure a level of uncertainty by fixing the parameters of dispersion for each substructure. We fixed them at 0.1 for each matrix of masses, of stiffnesses and generalized damping and for each substructure.

\section*{Resolution of the probabilistic linear dynamic system.}

Operator DYNA_LINE_HARM is used to build the harmonic response of the plate for each random realization of the matrices of mass, stiffness and dissipation.

The frequential interval of the study is \(B=[0,100] \mathrm{Hz}\), with a step of 0.5 Hz .

\section*{Construction of the statistical estimates.}

After each call to DYNA_LINE_HARM, we have a realization of the fields of displacement. With each pulling (iteration of Monte Carlo) we build the statistical estimates with the assistance only of operator CALC_FONCTION and key words WRAPS, POWER and COMB.
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\subsection*{3.4 Functionalities \\ tested}

\section*{Orders}

GENE_MATR_ALEA

The functionality tested of GENE_MATR_ALEA is the possibility of producing a concept of the type macr_elem_dyna starting from an other concept of the macr_elem_dyna type.

\section*{3.5}

\section*{Sizes tested and results}

The initial validity of the case test was established by graphic comparison with the bibliographical reference data in [§2.2]. The answers are calculated into 1 point for each substructure; these points have for co-ordinates: \((0.39,0.31)\) and \((0.79,0.24)\).

As for the reference, the envelopes lower and higher of 500 pullings are calculated. For each frequency, the inter-quantile field corresponding corresponds, for example with a level of confidence of 0.95 , with a probability of 0.994 for the maximum value and 0.006 for the value minimal.

The results obtained with Code_Aster are represented on the curves below:
Node (0.39, 0.31), DZ
Node (0.79, 0.24), DZ

\author{
Fatty features: inter-quantiles fields of displacements to the ddls of observation (thin features: averages) \\ Code_Aster results \\ Handbook of validation \\ V2.06 booklet: Harmonic response of a linear system \\ HT-62/06/005/A
}

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Author (S):
S. CAMBIER, A. BATOU Key

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One tests the following values in nonregression (cf comments):
Statistics on the values in displacement with 30 Hz with the ddl of observation of the first substructure
Parameters References Aster \%
Difference
Higher envelope
6.7338296870618D-05
\(6.7338296870618 D-05\)

\section*{0}

Lower envelope
5.1116761251425D-05
5.1116761251425D-05

0
Estimate of the average
6.0802671417375D-05
\(6.0802671417375 D-05\)
0
Estimate of the moment
3.7457833680156D-09 3.7457833680156D-09

0
of order 2

Statistics on the values in displacement with 30 Hz with the ddl of observation of the second substructure
Parameters References Aster \%

\section*{Difference}

Higher envelope
4.3459496115461D-04
\(4.3459496115461 D-04\)
```

O
Lower envelope
2.8511128677169D-04
2.8511128677169D-04
O
Estimate of the average
3.5186242151806D-04
3.5186242151806D-04
O
Estimate of the moment
1.2765909447885D-07 1.2765909447885D-07
O
of order 2

```

\subsection*{3.6 Comments}

The various statistical estimates are not converged here. Only 3 simulations of Monte Carlo were made drastiquement to reduce time CPU of the case test. Convergences having been validated on the study supplements (after convergence, the statistical estimates calculated with
to start from Code_Aster correspond to the results given by the cf graphs, standard commodity), case test is satisfied with nonthe regression.

\section*{4 \\ Summary of the results}

The results obtained are completely in conformity with those of the bibliographical reference [\$2.2] obtained entirely in Matlab.

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\section*{Code_Aster}

Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V2.07 booklet: Harmonic response of the voluminal structures

\section*{V2.07.100 document}

SHLV 100-Harmonic response of a cylinder

\section*{hollow in plane deformations}

\section*{Summary:}

This axisymmetric three-dimensional test makes it possible to validate calculations of the matrices of rigidity, mass and of
vectors of pressure on all the elements 3D and 2D plane and axisymmetric deformations (10 modelings).
Displacements are imposed:
- is by ddl,
- is by face of element.

For four modelings 3D, the pressures applied are provided with the minus sign, because the faces elements 3D are badly directed in the files of grid used.
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Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
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\section*{1 Problem of reference}
1.1 Geometry
y
F
E

C
\(45^{\circ}\)
D
P
With
B
Z
face blocked according to Z
X
face blocked according to Z
internal ray has \(=0.1 \mathrm{~m}\)
external ray \(\mathrm{B}=0.2 \mathrm{~m}\)
Co-ordinates of the points:
With
B
C
D
E
F
X
0.100
0.200
0.1 COS (22.5)
\(0.2 \operatorname{COS}\) (22.5)
0.
0.1 SIN (22.5)
0.2 SIN (22.5)

2/2
2
Z
0.
0.
0.

0 .
0.

\section*{0.}

\subsection*{1.2 Material properties}
\(\mathrm{E}=26 \mathrm{~N} / \mathrm{m} 2\)
\(=0.3\)
\(=35 \mathrm{Kg} / \mathrm{m} 3\)
The very low value of the Young modulus does not have anything physics.

\subsection*{1.3 Boundary conditions and loadings}

Pressure interns \(P=p E j t\)
with \(\mathrm{p}=1 \mathrm{Mpa}\)
and
\(=0.2 \mathrm{rad} / \mathrm{s}\)

\subsection*{1.4 Initial conditions}
- without initial conditions,
- direct calculation of the harmonic solution.

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X. DESROCHES

Key:

\section*{2 Reference solution}
2.1 Method of calculation used for the reference solution
\(U=A J\)
\(+\)
\(=\)
```

) B
Y0 (K R
L
)
Y1 (K R
L
)
K R
KR

```
\(L\)
\(N\)
\(=\)
1
1
\(2 \mu K\)
With
2
2
- 1
\(+\)
2
2
+
\(L\)
+
+
```

Jl (K R
L
) B
Y0 (K R
L
)
Y1 (K R
L
KR
KR

```
\(L\)
\(=2 \mu\)
2
\(z z\)
\(K L(\)
2
-) 1 [ \(A\) J0 (kL R) + B Y0 (kL R)]
= =
R
\(r z\)
Z
0
\(+\mu\)
with: 2
2
=
K
\(4 \mu\)
\(2(\)
\(=\)
\(=\)
\(=\)
\(1-)\)
\(L\)
2
\(C\)
+
4
\(2 \mu\)
\(L\)
\(J, J, Y, Y\)
: Functions of Bessel.
1
0
1
0
Constant \(A\) and \(B\) are calculated by solving the linear system obtained while writing:
(A) \(=-p\)
(b) \(=\)
rr
\(r r\)
0
One obtains:
-3
-3
for \(R=0.1 U=73398\)

10
for \(R=0.2\)
\(U=\)
\(R\)
\(R\)
46816
10
= 1
\(=\)
rr
\(r r\)
0.
\(=16685\)
\(=066738\)
\(=020055\)

020031

Passage in the system of Cartesian axes:
\(=\cos 2+\sin 2-2 \sin \cos\)
\(x x\)
\(r r\)
\(R\)
\(=\sin 2+\cos 2+2 \sin \cos\)
yy
\(r r\)

\section*{\(R\)}
\(=\sin \cos -\sin \cos -2\)
cos2
2
xy
\(r r\)
\(R(\)
\(\sin\) )
with
\(=0^{\circ}\) at points A and \(\mathrm{B}=\)
22.5 at the points C and D
\(={ }^{\circ}\)
45 at the points E and F
2.2 Results of reference

Displacements ( \(\mathrm{U}, \mathrm{v}\) ) and forced at the points \(\mathrm{A}, \mathrm{B}, \mathrm{C}, \mathrm{D}, \mathrm{E}, \mathrm{F}\).

\subsection*{2.3 Uncertainty on the solution}

Precision of the calculation of the Functions of Bessel.

\subsection*{2.4 Bibliographical references}
[1] Mr. BONNET: Methods of the integral equations regularized into elastodynamic - Bulletin DER - Series C - N \({ }^{\circ} 1 / 2\) - (1987).
[2] ERINGEN - SUHUBI - Elastodynamics, Vol.2: linear theory Academic Press (1975).
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

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\section*{3 Modeling A}

\subsection*{3.1 Characteristics of modeling}

Elements 3D (PENTA6 and HEXA8) (resulting from the grid 2D below).
F
Normally blocked face
y
D
E
C
\(45^{\circ}\)
X
With
B
Face with imposed pressure
Face blocked out of Dy
along axis Z: 2 layers of elements total thickness: 0.01
Limiting conditions:
DDL_IMPO:
(All: "yes"
Dz: 0. )
face AB
(Group_no: BordAB
Dy: 0. )
face EF
FACE_IMPO:
(Group_ma: FaceEF
Dnor: 0. )
pressure on face AE
PRES_REP:
(Group_ma: FaceAE
Near: -1. )
Names of the nodes:
A=No1
B=No119
C=No36
D=No166
E=No41
\(\mathrm{F}=\mathrm{No} 171\)
3.2 Characteristics of the grid

A number of nodes: 513
A number of meshs and types: 400 PENTA6 100 HEXA8 40 QUAD4
3.3 Functionalities tested Orders

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"

ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
3.4 Remarks

The pressure has a negative sign (instead of positive) because the faces of the elements 3D are badly directed.
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Author (S):

\section*{X. DESROCHES}

Key:
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4 Results of modeling A
4.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3243103
0.21

102
```

With
v
0.

```
eps
0.2
\(x x\)
-1.
0.8789
12.1
yy
1.6685
1.6241
2.66
Z.
0.20055
0.2235
11.75
0.2
xy
0 .
0.0922
\(U\)
6.78109103
6.7670103
0.21
102
C
\(v\)
2.80882103
2.8012103
0.27
0.3
0.3
\(x x\)
0.60921
0.5121
15.94
yy
1.27771
```

1.3300
4 . 0 9
0.3
0.3
zz
0.20055
0.2454
22.39
xy
0.94346
0.8567
9.20
0.3
U
5.19002 103
5.1784103
0.22
102
E
v
5.19002 103
5.1784103
0.22
0.6
0.6
xx
0.33425
0.4319
29.23
yy
0.33425
0.5315
5 9 . 0 4
0.6
0.6
zz
0.20055
0.289
44.50
xy
1.33425

```
0.56964
0.5728
0.56
0.3
\(z z\)
0.20021
0.1941
3.05
0.3
xy
0.23595
0.2348
0.49
0.3
\(U\)
3.31039103
3.2974103
0.39
102
F
\(v\)
3.31039103
3.2974103
0.39
0.2
\(x x\)
0.33369
0.2977
10.78
0.2
yy
0.33369
0.3245
2.75
0.2
\(z z\)
0.20021
0.1866

\subsection*{4.2 Remarks}

The grid is insufficient for linear elements.

\subsection*{4.3 Parameters of execution}

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 10.23 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
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\section*{5 Modeling B}

\subsection*{5.1 Characteristics of modeling}

Elements 3D (PENTA15 and HEXA20) (resulting from the grid 2D below).

Face blocked in dx
F
With
Normally blocked face

Face with imposed pressure
X
along axis Z: 2 layers of elements total thickness: 0.01
Limiting conditions:
DDL_IMPO:
(All: "yes"
Dz: 0. )
face AB
(Group_no: BordAB
Dx: 0.)
face EF
FACE_IMPO:
(Group_ma: FaceEF
Dnor: 0. )
pressure on face AE
PRES_REP:
(Group_ma: FaceAE
Near: -1. )
Names of the nodes:
\(\mathrm{A}=\mathrm{No} 2\)
B=No361
\(\mathrm{C}=\mathrm{No} 121\)
D=No584
E=No155
F=No503

\subsection*{5.2 Characteristics of the grid}

A number of nodes: 2115
A number of meshs and types: 400 PENTA15 100 HEXA20 40 QUAD8

\subsection*{5.3 Functionalities tested \\ Orders}

Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
```

AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
NUME_DDL
RENUM
"RCMK"
"EXTRACTION"
[U4.42.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
5.4 Remarks
The pressure has a negative sign (instead of positive) because the faces of the elements 3D are badly
directed.
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

```
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES
Key:
V2.07.100-C Page:
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6 Results of modeling B
6.1 Values tested
Localization

\section*{Sizes}

\section*{Reference}

\section*{Aster}
\% difference

\section*{tolerance}

\section*{\(U\)}

0 .
eps
With
\(v\)
7.3398103
7.3326103
0.10

102
\(x x\)
1.6685
1.6669
0.09

102
yy
-1.
0.9959
0.41

102
\(z z\)
0.20055
0.2013
0.37

102
xy
0 .
3.3234103

102
\(U\)
2.80882103
2.8063103
0.09

102
C
\(v\)
6.78109103
6.7745103
0.10

102
\(x x\)
1.27771
1.278
0.02

102
yy
0.60921
0.6078
0.23

102
z.
0.20055
0.20107
0.26

102
xy
0.94346
0.94027
0.34

102
\(U\)
5.19002103
5.1851103
0.09

102
E
\(v\)
5.19002103
5.1851103
0.10

102
\(x x\)
0.33425
0.3346
0.10

102
yy
0.33425
0.3340
0.07
0.
```

B

```
\(v\)
4.6716103
4.6682103
0.07
102
xx
0.66738
0.6675
0.02
102
yy
0.
3.2779104
102
zz
0.20021
0.2003
0.04
102
xy
0.
5.0918104
102
\(U\)

\title{
1.7864103
}
0.29

102
D
v
4.32523103
4.3129103
0.29

102
xx
0.56964
0.56957
0.01

102
yy
0.09774
0.09803
0.30

102
zz
0.20021
0.20027
0.03

102
xy
0.23595
0.23623
0.12

102
U
3.31039103
3.3009103
0.29

102
F
\(v\)
3.31039103
3.3009103
0.29

102
\(x x\)
0.33369
0.3337

\subsection*{6.2 Parameters of execution}

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 94.09 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
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7 Modeling C

\subsection*{7.1 Characteristics of modeling}

Elements 3D (TETRA4)
along axis Z: 2 layers of elements total thickness: 0.01
Limiting conditions:
DDL_IMPO:
(All: "yes"
Dz: 0. )
face AB
(Group_no: BordAB
Dy: 0. )
face EF
FACE_IMPO:
(Group_ma: FaceEF
Dnor: 0. )
pressure on face AE
PRES_REP:
(Group_ma: FaceAE
Near: -1. )
Names of the nodes:
\(\mathrm{A}=\mathrm{No} 3\)
\(\mathrm{B}=\mathrm{No} 7\)
\(\mathrm{C}=\mathrm{No} 4\)
D=No8
\(\mathrm{E}=\mathrm{No} 154\)
F=No156
plan \(\mathrm{Z}=0.005\)
A2 \(=\) No1
B2=No5
C2 \(=\) No2
D2=No6
E2=No153
F2=No155
plan \(\mathrm{Z}=0.01\)
A3 \(=\) No283
B3=No285
C3=No284
D3=No286
E3=No359
F3=No360

\subsection*{7.2 Characteristics of the grid}

A number of nodes: 423
A number of meshs and types: 1416 TETRA4 72 TRIA3

\subsection*{7.3 Functionalities tested}

\section*{Orders}

\section*{Keys}

\section*{AFFE_CHAR_MECA}

DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
[U4.61.01]

\subsection*{7.4 Remarks}

The pressure has a negative sign (instead of positive) because the faces of the elements 3D are badly directed.

\section*{Handbook of Validation}

V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:

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\section*{8 Results of modeling C}
8.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3331103
0.10

102
With
\(v\)
0.
eps
\(x x\)
-1.
0.9000
+10.00
0.02
yy
1.6685
1.6809
0.74
0.02
zz
0.20055
0.2343
16.83
0.02
xy
0 .
0.1016
0.02
\(U\)
6.78109103
6.7783103
0.04
```

yy
0.33425
0 . 4 9 2 0
4 7 . 1 9
0.5
0.5
zz
0.20055
0 . 2 3 4 3
16.83
xy
1.33425
1.2905
3.28
0.5
U
4 . 6 7 1 6 1 0 3
4 . 6 6 3 4 1 0 3
0 . 1 8
102
B
v
0.
eps
xx
0.
0 . 0 1 4 6
yy
0 . 6 6 7 3 8
0 . 6 5 7 0
1.55
5.102
Z,
0.20021
0 . 1 9 7 6
1.30
5 . 1 0 2
xy
0.
0 . 0 1 5 9

```

\author{
5.102 \\ \(U\) \\ 4.32523103 \\ 4.2960103 \\ 0.68 \\ 102 \\ D \\ \(v\) \\ 1.79157103 \\ 1.7795103 \\ 0.67 \\ \(x x\) \\ 0.09774 \\ 0.0824 \\ 15.69 \\ 0.2
}
0.2
yy
0.56964
0.5809
1.97
0.2
zz
0.20021
0.1921
4.05
xy
0.23595
0.2378
7.84
0.2
\(U\)
3.31039103
3.2976103
0.39

102
F
\(v\)
3.31039103
3.2975103

\subsection*{8.2 Remarks}

One notes a variation ( \(<0.24 \%\) ) displacements for the points of the \(\mathrm{z}=0.005\) plan.
The grid is insufficient for linear elements.

\subsection*{8.3 Parameters of execution}

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 18.55 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll

\section*{Date:}

20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:
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\section*{9 Modeling D}

\subsection*{9.1 Characteristics of modeling}

Elements 3D (TETRA10)
along axis Z: 2 layers of elements total thickness: 0.01
Limiting conditions:
DDL_IMPO:
(All: "yes"
Dz: 0.)
face AB
(Group_no: BordAB
Dy: 0. )
face EF
FACE_IMPO:
(Group_ma: FaceEF
Dnor: 0. )
pressure on face AE
PRES_REP:
(Group_ma: FaceAE
Near: -1. )
Names of the nodes:
A=No3
\(\mathrm{B}=\mathrm{No} 7\)
\(\mathrm{C}=\mathrm{No} 4\)
D=No8
E=No1228
\(\mathrm{F}=\mathrm{No} 230\)
plan \(\mathrm{Z}=0.005\)
A2 \(=\) No1
B2=No5
C2=No2
D2=No6
E2=No227
F2=No229
plan \(\mathrm{Z}=0.01\)
A3 \(=\) No420
B3 \(=\) No4 42

C3=No421
D3=No423
E3=No573
F3=No574

\subsection*{9.2 Characteristics of the grid}

A number of nodes: 703
A number of meshs and types: 356 TETRA10 36 TRIA6
9.3 Functionalities tested

Orders

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]

\subsection*{9.4 Remarks}

The pressure has a negative sign (instead of positive) because the faces of the elements 3D are badly directed.
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES
Key:
V2.07.100-C Page:
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10 Results of modeling D
10.1 Values tested
Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3522103
0.10
102
With
\(v\)
0.
eps
102
\(x x\)
-1.
0.9925
0.75
5.102
yy
1.6685
1.6725
0.24
5.102
\(z z\)
0.20055
0.2040
0 .
0.0365
5.102
\(U\)
6.78109103
6.7836103
0.04
102
C
v
2.80882103
2.8099103
0.04
102
\(x x\)
0.60921
0.5977
1.89
5.102
yy
1.27771
1.294
1.28
5.102
\(z z\)
0.20055
0.2088
4.11
5.102
xy
0.94346
0.9457
0.24
5.102
\(U\)
5.19002103
5.1988103
0.17
102
```

E
v
5.19002 103
5 . 1 9 8 8 1 0 3
0 . 1 7
102
0 . 1 5
XX
0.33425
0 . 3 0 3 5
9 . 2 0
yy
0.33425
0 . 3 7 6 6
12.67
0 . 1 5
0 . 1 5
zz
0.20055
0 . 2 0 4 0
1.72
xy
1.33425
1.332
0 . 1 7
0 . 1 5
U
4 . 6 7 1 6 1 0 3
4 . 6 7 1 1 1 0 3
0 . 0 1
102
B
0.
eps
1 0 2
xx
0.
8.597 104

```
0.66738
0.6679
0.08

102
\(z z\)
0.20021
0.2006
0.19

102
xy
0.
1.0181103

102
\(U\)
4.32523103
4.3134103
0.28

102
D
1.79157103
1.7867103
0.28

102
\(x x\)
0.09774
0.09418
3.64
5.102
yy
0.56964
0.5652
0.78
5.102
zz
0.20021
0.1978
1.20
5.102
\(x y\)

\title{
0.23595
}
0.2355
0.19
5.102
\(U\)
3.31039103
3.3029103
0.23

102
F
\(v\)
3.31039103
3.3029103
0.23

102
\(x x\)
0.33369
0.3357
0.60

102
yy
0.33369
0.3334
0.09

102
zz
0.20021
0.2007
0.24

102
xy
0.33369
0.3336

\section*{102}

\subsection*{10.2 Remarks}

One notes a variation ( \(<0.23 \%\) ) displacements for the points of the \(\mathrm{z}=0.005\) plan.

\subsection*{10.3 Parameters of execution}

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords

Time CPU To use: 21.05 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
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\section*{11 Modeling E}
11.1 Characteristics of modeling

Elements D_PLAN (TRIA3 + QUAD4)
F
Normally blocked face

\section*{y}

D
E
C
\(45^{\circ}\)
X
With
B
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO:
(Group_no: GRNM11
Dy: 0. )
side EF
FACE_IMPO:
(Group_ma: GRMA12
Dnor: 0. )
pressure on AE
PRES_REP:
(Group_ma: GRMA13
Near: 1. )
Names of the nodes:
A=N1
\(\mathrm{B}=\mathrm{N} 119\)
\[
\begin{aligned}
& C=N 36 \\
& D=N 166 \\
& E=N 41 \\
& F=N 171
\end{aligned}
\]

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 171
A number of meshs and types: 200 TRIA3 50 QUAD4
11.3 Functionalities tested

Orders

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
Handbook of Validation
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HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:

Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
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12 Results of modeling \(\mathbf{E}\)
12.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3243103
0.21

102
With
\(v\)
0.
eps
0.15
\(x x\)
-1.
0.8790
12.10
yy
1.6685
1.6241
2.66
0.15
0.15
zz
0.20055
0.2235
11.44
```

0.15
xy
0.
0.0922
U
6.78109 103
6.7670 103
0.21
102
C
v
2.80882 103
2.8012 103
0.27
0.3
xx
0.60921
0.5122
15.92
yy
1.27771
1.3302
4 . 1 1
0.3
0.3
zz
0.20055
0.2454
22.36
xy
0.94346
0.8567
9 . 1 9
0.3
5.19002 103
5.1784103
0.22
102
E

```
```

v
5.19002103
5 . 1 7 8 4 1 0 3
0 . 2 2
0 . 6
xX
0.33425
0 . 4 3 1 8
29.18
yy
0.33425
0 . 5 3 1 5
59.01
0 . 6
0.6
zz
0.20055
0 . 2 8 9 0
4 4 . 1 0
xy
1.33425
1.2686
4 . 9 2
0 . 6
U
4 . 6 7 1 6 1 0 3
4 . 6 6 4 1 1 0 3
0 . 1 6
102
B
v
0.
eps
xx
0.
1.3198102
0 . 0 5
0.05
yy

```
0.530.3
U
3.31039103
3.2974103
0.39
102
F
\(v\)
3.31039103
3.2974103
0.39
0.15
\(x x\)
0.33369
0.2976
10.81
0.15
yy
0.33369
0.3245
2.75
0.15
zz
0.20021
0.1866
6.80
0.15
xy
0.33369
0.3415
2.34

\subsection*{12.2 Remarks}
The grid is insufficient for linear elements.

\subsection*{12.3 Parameters of execution}
Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords

\title{
Time CPU To use: 5.87 seconds
}

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HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

\section*{Key:}

V2.07.100-C Page:
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\section*{13 Modeling F}

\subsection*{13.1 Characteristics of modeling}

Elements D_PLAN (QUAD8 + TRIA6)
y
B
D
Face blocked in dx
F
With
Normally blocked face
C
E
\(45^{\circ}\)
Face with imposed pressure
X
Limiting conditions:
side AB
DDL_IMPO:
(Group_no: GRNM11
Dy: 0. )
side EF
FACE_IMPO:
(Group_ma: GRMA12
Dnor: 0. )
pressure on AE
PRES_REP:
(Group_ma: GRMA13
Near: 1. )
Names of the nodes:
\(\mathrm{A}=\mathrm{N} 2\)
\(\mathrm{B}=\mathrm{N} 361\)
\(\mathrm{C}=\mathrm{N} 121\)
D=N584
E=N155
F=N503

\subsection*{13.2 Characteristics of the grid}

A number of nodes: 591
A number of meshs and types: 200 TRIA6 50 QUAD8
13.3 Functionalities tested

\section*{Orders}

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
[U4.61.01]
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:
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\section*{14 Results of modeling \(\mathbf{F}\)}
14.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
0.
0.

102
With
\(v\)
7.3398103
7.3326103
0.10

102
\(x x\)
1.6685
1.6669
0.09

102
yy
-1.
0.9959
0.41

102
\(z z\)
0.20055
0.
eps
102
B
\(v\)
4.6716103
4.6682103
0.07
102
\(x x\)
0.66738
0.66758
0.03
102
0.
0.00033

102
zz
0.20021
0.20037
0.08

102
xy
0.
5.1132104

102
U
1.79157103
1.7865103
0.28

102
D
\(v\)
4.32523103
4.3129103
0.28

102
\(x x\)
0.56964
0.56962
0.003

102
yy
0.09774
0.09805
0.32

102
\(z z\)
0.20021
0.200298
0.044

102
xy
0.23595

\title{
0.23623
}
0.12

102
U
3.31039103
3.3009103
0.29

102
F
\(v\)
3.31039103
3.3009103
0.29

102
\(x x\)
0.33369
0.33371
0.006

102
yy
0.33369
0.33366
0.009

102
zz
0.20021
0.20021
0.

102
xy
0.33369
0.33392
0.069

102
14.2 Parameters of execution

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 6.05 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
16/24
15 Modeling G
15.1 Characteristics of modeling

D_PLAN (QUAD9)

\section*{F}

Normally blocked face
y
D
E
C
\(45^{\circ}\)
X
With
B
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO:
(Group_no: GRNM11
Dy: 0.)
side EF
FACE_IMPO:
(Group_ma: GRMA12
Dnor: 0. )
pressure on AE
PRES_REP:
(Group_ma: GRMA13
Near: 1.)
Names of the nodes:
\(\mathrm{A}=\mathrm{N} 1\)
\(\mathrm{B}=\mathrm{N} 347\)
\(\mathrm{C}=\mathrm{N} 21\)
\(\mathrm{D}=\mathrm{N} 432\)
E=N39
\(\mathrm{F}=\mathrm{N} 229\)

\subsection*{15.2 Characteristics of the grid}

A number of nodes: 441
A number of meshs and types: 100 QUAD9
15.3 Functionalities tested

Orders

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version

Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
17/24
16 Results of modeling G
16.1 Values tested

\section*{Localization}

Sizes
Reference
Aster
\% difference

\section*{tolerance}
\(U\)
7.3398103
7.3329103
0.09

102
With
\(v\)
0.
eps
-
102
\(x x\)
-1.
0.9968
0.32

102
yy
1.6685
1.6655
0.18

102
zz
0.20055
0.20059
0.02
0 .
2.97104
102
\(U\)
6.78109103
6.7747103
0.09
102
C
\(v\)
2.80882103
2.8062103
0.09
102
\(x x\)
0.60921
0.60695
0.37
102
yy
1.27771
1.27563
0.16
102
\(z z\)
0.20055
0.20060
0.02
102
xy
0.94346
0.94128
0.23
102
U
5.19002103
5.1851103
0.09
102
E
```

v
5.19002103
5 . 1 8 5 1 1 0 3
0 . 0 9
102
xx
0.33425
0.33403
0 . 0 6
102
yy
0.33425
0 . 3 3 4 6 3
0 . 1 1
102
zz
0.20055
0 . 2 0 0 5 9
0 . 0 2
102
xy
1.33425
1.33117
0 . 2 3
102
U
4 . 6 7 1 6 1 0 3
4 . 6 6 8 2 1 0 3
0 . 0 7
102
B
v
0.
eps
1 0 2
xx
0.
2.394 103
102
yy
0.66738

```
3.31039103
3.3009103
0.29

102
F
\(v\)
3.31039103
3.3009103
0.29

102
\(x x\)
0.33369
0.33366
0.009

102
yy
0.33369
0.33371
0.006

102
zz
0.20021
0.20021

0 .
102
xy
0.33369
0.33392
0.07

102
16.2 Parameters of execution

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 5.38 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures HI-75/96/005 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
18/24
17 Modeling H
17.1 Characteristics of modeling

Elements axis (TRIA3 + QUAD4)
Center cylinder
Face blocked out of Dy
y
E
F
C
0.01 m

D
X
With
B
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO:
(Group_no: GRNM11
Dy: 0. )
side EF
FACE_IMPO:
(Group_ma: GRMA12
Dnor: 0. )
pressure on AE
PRES_REP:
(Group_ma: GRMA13
Near: 1. )
Names of the nodes:
\(\mathrm{A}=\mathrm{N} 111\)
\(\mathrm{B}=\mathrm{N} 1\)
\(\mathrm{C}=\mathrm{N} 112\)
D=N3
\(\mathrm{E}=\mathrm{N} 113\)
\(\mathrm{F}=\mathrm{N} 4\)
17.2 Characteristics of the grid

A number of nodes: 113
A number of meshs and types: 40 QUAD4 80 TRIA3
17.3 Functionalities tested

\section*{Orders}

Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"AXIS"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
[U4.61.01]
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3

\section*{Titrate:}

Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:
19/24
18 Results of modeling \(\mathbf{H}\)
18.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3390103
0.01

102
With
\(v\)
0 .
eps
0.2
\(x x\)
-1.
0.9430
5.72
yy
0.20055
0.2248
12.19
0.2
0.2
zz
1.6685
1.6923

0 .
eps
-
7.3398103
7.3390103
0.01
102
C
\(v\)
0 .
eps
0.2
xx
-1.
0.9430
5.72
yy
0.20055
0.2248
12.19
0.2
0.2
\(z z\)
1.6685
1.6923
1.46
xy
0 .
eps
0.2
\(U\)
7.3398103
7.3390103
0.01
102
```

E
v
0.
0.
0.2
xx
-1.
0.9430
5.72
yy
0.20055
0 . 2 2 4 8
12.19
0 . 2
0.2
zz
1.6685
1.6923
1 . 4 6
xy
0.
eps
0.2
4 . 6 7 1 6 1 0 3
4 . 6 7 1 3 1 0 3
0 . 0 1
102
B
v
0.
eps
xx
0.
0 . 0 1 1 0
0 . 0 5
0 . 0 5

```
```

yy

```
0.20021
0.1954
2.35
0.05
zz
0.66738
0.6625
0.72
xy
0 .
0.0011
0.05
\(U\)
4.6716103
4.6713103
0.01
102
D
\(v\)
0 .
eps
\(x x\)
0.
0.0110
0.05
0.05
yy
0.20021
0.1954
2.35
0.05
zz
0.66738
0.6625
0.72
xy
0 .
```

eps
0.05
U
4 . 6 7 1 6 1 0 3
4 . 6 7 1 3 1 0 3
0 . 0 1
102
F
v
0.
eps
0 . 0 5
xx
0.
0 . 0 1 1 0
0 . 0 5
yy
0.20021
0 . 1 9 5 4
2 . 3 5
0 . 0 5
Z,
0.66738
0 . 6 6 2 5
0 . 7 2
0.05
xy
0.
+0.0011

```

\subsection*{18.2 Parameters of execution}

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 5.60 seconds

\section*{Handbook of Validation}

V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:
20/24
19 Modeling I
19.1 Characteristics of modeling

Elements axis (TRIA6 + QUAD8)
Center cylinder
Face blocked out of Dy
y
E
F
0.01 m

C
D
X
With
B
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO:
(Group_no: GRNM11
Dy: 0. )
side EF
FACE_IMPO:
(Group_ma: GRMA12
Dnor: 0. )
pressure on AE
PRES_REP:
(Group_ma: GRMA13
Near: 1. )
Names of the nodes:
\(\mathrm{A}=\mathrm{N} 8\)
\(\mathrm{B}=\mathrm{N} 174\)
\(\mathrm{C}=\mathrm{N} 5\)
\(\mathrm{D}=\mathrm{N} 170\)
E=N3
\(\mathrm{F}=\mathrm{N} 159\)

\subsection*{19.2 Characteristics of the grid}

A number of nodes: 175
A number of meshs and types: 20 QUAD8 40 TRIA6
19.3 Functionalities tested

\section*{Orders}

\section*{Keys}

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"AXIS"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
[U4.61.01]
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
21/24
20 Results of modeling I
20.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3397103
0.00

102
With
\(v\)
0 .
eps
102
\(x x\)
-1.
0.9984
0.16

102
yy
0.20055
0.20055

102
\(z z\)
1.6685

0 .
eps
102
\(U\)
7.3398103
7.3397103
0.00

102
C
v
0 .
eps

102
xx
-1.
0.9984
0.16

102
yy
0.20055
0.20055

102
zz
1.6685
1.669
0.57

102
xy
0 .
eps

102
7.3398103
7.3397103
0.00

0 .
```

yy
0.20021
0.2002
102
zz
0.66738
0 . 6 6 7 1 6 1 0 5
0 . 0 3
102
xy
0.
102
U
4 . 6 7 1 6 1 0 3
4 . 6 7 1 6 1 0 3
0 . 0 0
102
D
v
0.
eps
102
xx
0.
3.8104
102
yy
0.20021
0 . 2 0 0 2
102
zz
0.66738
0 . 6 6 7 1 6
0 . 0 3
1 0 2
xy
0.
eps

```

\title{
20.2 Parameters of execution
}

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 5.06 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures HI-75/96/005 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES

Key:
V2.07.100-C Page:
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\section*{21 Modeling J}
21.1 Characteristics of modeling

Elements axis (QUAD9)
Center cylinder
Face blocked out of Dy

Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO:
(Group_no: GRNM11
Dy: 0. )
side EF
FACE_IMPO:
(Group_ma: GRMA12
Dnor: 0. )
pressure on AE
PRES_REP:
(Group_ma: GRMA13
Near: 1. )
Names of the nodes:
A=N196
\(\mathrm{B}=\mathrm{N} 1\)
\(\mathrm{C}=\mathrm{N} 200\)
D=N5
\(\mathrm{E}=\mathrm{N} 202\)
\(\mathrm{F}=\mathrm{N} 7\)
21.2 Characteristics of the grid

A number of nodes: 205
A number of meshs and types: 40 QUAD9

\subsection*{21.3 Functionalities tested}

\section*{Orders}

Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"AXIS"
ALL
[U4.22.01]
COMB_MATR_ASSE
[U4.53.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
[U4.61.01]
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3

\section*{Titrate:}

Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):

\section*{X. DESROCHES}

Key:
V2.07.100-C Page:
23/24
22 Results of modeling \(\mathbf{J}\)
22.1 Values tested

Localization
Sizes
Reference
Aster
\% difference
tolerance
\(U\)
7.3398103
7.3397103
0.00

102
With
\(v\)
0 .
eps
102
\(x x\)
-1.
0.9984
+0.16
102
yy
0.20055
0.2005

102
\(z z\)
1.6685
1.667
0.57

102
0 .
eps
102
\(U\)
7.3398103
7.3397103
0.00
102
C
\(v\)
0.
eps
102
xx
-1.
0.9984
+0.16
102
yy
0.20055
0.2005
102
\(z z\)
1.6685
    1.667
0.57
102
xy
0 .
eps
102
\(U\)
7.3398103
7.3397103
0.00
102
E

\section*{0.}
eps
102
\(x x\)
-1.
0.9984
+0.16
102
yy
0.20055
0.2005

102
zz
1.6685
1.667
0.57

102
xy
0 . eps

102
U
4.6716103
4.6716103
0.00

102
B
\(v\)
0.
eps
102
xx
0.
1.1104

102
yy
0.20021
0.20021
0.
0.
1.1104

102
yy
0.20021
0.20021
0.02

102
\(z z\)
0.66738
0.66727
0.04

102
xy
0 .
eps
-
102
22.2 Parameters of execution

Version: 3.02
Machine: CRAY C90
System: UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use: 4.84 seconds
Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
Harmonic SHLV100 Response of a hollow roll
Date:
20/09/99
Author (S):
X. DESROCHES
Key:
V2.07.100-C Page:
24/24
23 Summary of the results
Summary
3D
D_PLAN
Axis
errors
max in \%
MOD A
MOD B
MOD C
MOD D
MOD E
MOD F
MOD G
MOD H
MOD I
MOD J
Displacements
WITH, C, E
0.27
0.10
0.10
0.17
0.27
0.10
0.09
0.01
0.00
0.00
B, D, F
0.39
0.02

\section*{Constraints}
```

xx

```
WITH, C, E
29.23
0.10
16.92
9.20
29.18
0.11
0.37
5.72
0.27
0.27
B, D, F
24.39
0.02
15.69
3.64
24.36
0.03
0.18
Constraints
yy
WITH, C, E
59.04
0.41
47.19
    12.67
    59.01
    0.41
    0.18
    12.19

B, D, F
2.75
0.30
1.97
0.78
2.75
0.32
0.03
2.35
0.09
0.02

Constraints
\(z z\)
WITH, C, E
44.50
0.37
21.51
4.11
44.10
0.37
0.02
1.46
0.57
0.57

B, D, F
6.80
0.04
4.05
1.20
6.80
0.08
0.001
0.72
0.03
0.04

Constraints
xy
WITH, C, E
9.20
0.34
3.30
- The grids for the elements of order 1 are not fine enough.
- The results are more precise with elements of order 2.
- The problem is adapted more to an axisymmetric modeling (H, I, J) - > better results.
- Results of the elements 3D and the plane elements having spaces of interpolation in correspondence are identical.
- The results of axisymmetric elements QUAD8 and QUAD9 are identical.

Handbook of Validation
V2.07 booklet: Harmonic response of the voluminal structures
HI-75/96/005 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

\section*{Titrate:}

SSLL10 Gantry with side connections

\section*{Date:}

19/01/98
Author (S):

\section*{J.M. PROIX}

Key:
V3.01.010-A Page:
1/6
Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures

\section*{Document: V3.01.010 \\ SSLL10 - Gantry with side connections \\ Summary:}

Static test in linear elasticity, being used to validate the elements of right beam POU_D_T for a loading specific and a loading distributed (key word FORCE_POUTRE).
The relations and moments bending are tested.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL10 Gantry with side connections
Date:
19/01/98
Author (S):

\section*{J.M. PROIX}

Key:
V3.01.010-A Page:
2/6
1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Plane problem}

\section*{C}

G
With
B
D
Beam
Length
Moment of inertia
\(A B\)
\(L\)
=
\(=64\)
8
4
\(A B\)
4 m

Another characteristic of the beams not being used for calculations: the beams are of square section.
With

4
\(A B\)
1610
m

With
=
4
\(A D\)
110
m

With

AC
110
m

With
=
4
AE
410
m
1.2

Material properties
Isotropic linear elastic material:
\(\mathrm{E}=21011 \mathrm{~Pa}\)
1.3

Boundary conditions and loadings
1) Points
\(C\) : articulated \((U=v=\)
C
C
)0.
C
F
P
D
B
G
With

Specific force in: \(G: F=-105 N R\).
E
Force distributed on beam \(A D: p=103 N R / M r\).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL10 Gantry with side connections
Date:
19/01/98
Author (S):

\section*{J.M. PROIX}

Key:
V3.01.010-A Page:
3/6
2
Reference solution
2.1

Method of calculation used for the reference solution
One poses:
I.E.(internal excitation)

K
Year
=
Year
lAn
with or
\(N=B, C, D\)
E
3
\(K=K\)
\(+K\)
\(+K+K\)
\(A B\)
\(A D\)
AE
AC
4
K
```

R

```
year
=
Year
K
with or
\(N=B, C, D\)
\(E\)
Fl
pl2
C
\(A D\)
\(A B\)
= +
1
8
12
- Rotation in \(a\) :
\(=C 1\)
4K
- Moment in \(a\) :
pl2
M
\(A B\)
= +
+ R. \(C\)
\(A B\)
\(A B\)
1
12
Fl
M
\(A D\)
= -
+ R. \(C\)
\(A D\)
\(A D\)
1
8
M
\(=R . C\)
AE
AE

1

M

\(=R . C\)

AC

\(A C\)

2.2
Results of reference
Value of rotation and the moments in A.

\subsection*{2.3 References}

\section*{bibliographical}
[1]
Guide VPCS - Edition 1990.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL10 Gantry with side connections

\section*{Date:}

19/01/98
Author (S):

\section*{J.M. PROIX}

Key:
V3.01.010-A Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling
Elements POU_D_T
1 element for the section AG
1 element for the section GD
1 element for section AE
1 element for the section AC
1 element for section AB
Boundary conditions:
DDL_IMPO

\section*{ALL: "yes"}

D2: 0
DRX: 0 DRY: 0
NODE: (D, B, E) DX: 0
DY: 0
DRZ: 0
NODE C
DX: 0
DY: 0
)
FORCE_NODALE
NODE: G
Fy \(=-1.105\)
FORCE_AUTRE
NET: AB
Fy \(=-1.103\)
3.2

Characteristics of the grid
5 éléménts POU_D_T
6 nodes

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_MODELE
POU_D_T
[U4.22.01]
AFFE_CHAR_MECA
FORCE_POUTRE
[U4.25.01]
FORCE_NODALE
AFFE_CARA_ELEM
BEAM
SECTION:
"RIGHT-ANGLED"
[U4.24.01]
CALC_ELEM
OPTION:

\section*{"EFGE_ELNO_DEPL"}
[U4.61.02]
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL10 Gantry with side connections

\section*{Date:}

19/01/98
Author (S):
J.M. PROIX

\section*{Key:}

V3.01.010-A Page:
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4
Results of modeling A
4.1 Values
tested
Not
Size and unit

\section*{Reference}

Aster
\% difference
With
Z, rotation
0.227118
0.227441
0.14
(rad)
With
\(M A B\), moment
11023.72
11020.99
0.03
(Nm)
With
MAC, moment
113.559
113.718
0.14
(Nm)
With
MAD, moment
+12348.588
12347.477

\title{
4.2 Parameters
}
of execution

\author{
Version: NEW 3.3.31
}

Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
8 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
SSLL10 Gantry with side connections
Date:
19/01/98
Author (S):
J.M. PROIX

Key:
V3.01.010-A Page:
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5
Summary of the results
The results show the correct operation of elements POU_D_T in cross-bending under load specific and distributed (FORCE_POUTRE).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLL11 - Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):
Key J.M. PROIX
:
V3.01.011-E Page:
1/10
Organization (S): EDF/MTI/MMN

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures

\section*{Document: V3.01.011}

SSLL11-Lattice of pin jointed struts under load specific

\section*{Summary:}

This test makes it possible to check the elements of bar and beam for the structural analysis out of lattice. The lattice considered is plane. Calculation is static, elastic, linear. The reference solution is analytical.

Three modelings make it possible to test elements POU_D_T with and without rotulées connections, as well as

\author{
Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL11-Lattice of pin jointed struts under concentrated loading Date:
16/02/00
Author (S):
Key J.M. PROIX
:
V3.01.011-E Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(D\)
\(F\)
\(C\)
\(L\)
\(L / 2\)
With
\(B\)
\(y, v\)
\(L / 2\)
\(L / 2\)
\(L\)
\(X, U\)

Length \(L=1 \mathrm{~m}\)
elements \(A C\) and \(B C\) surface \(A=2.104 \mathrm{~m} 2\)
elements \(C D\) and data base surface \(A=1.104 \mathrm{~m} 2\)

\section*{Co-ordinates of the points (in m):}

WITH B
\(C D\)
X 0. 1 .
0.52.
y 0.0 .
0.51 .

Z0.0.0.0.

\section*{1.2 \\ Material properties}
\(E=1.9621011 \mathrm{~Pa}\)

\section*{1.3 \\ Boundary conditions and loadings}

The nodes \(A\) and \(B\) are articulated: \(U=v=0\)
Vertical specific force in \(D: F=9.81103 \mathrm{NR}\)
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLL11 - Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):
Key J.M. PROIX
:
V3.01.011-E Page:
3/10

2

\section*{Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card SSLL11/89 of guide VPCS.
It is obtained by the method of displacements in [bib1].

\section*{2.2 \\ Results of reference \\ Displacements of the points \(C\) and \(D\).}

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

RAO (J.S.): The finite element method in engineering, problem 5.1, p. 275.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL11-Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):
Key J.M. PROIX

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Taking into account the twinges, the taking into account of the articulations modifies the results little.
For
this modeling the articulations in \(A, B, C\) and \(D\) are rigidified (continuity of the 3 components generalized efforts).

4 beams of full circular section: 4 meshs SEG2
elements \(A C\) and \(B C\)
ray \(R=7.978845103 \mathrm{~m}\)
(surface \(A=2.104 \mathrm{~m} 2\) )
elements \(C D\) and data base
ray \(R=5.641895103 \mathrm{~m}\)
(surface \(A=1.104 \mathrm{~m} 2\) )
Poisson's ratio:
\(=0.3\)

\section*{Limiting conditions:}
in all the nodes:

DDL_IMPO:
(ALL: "YES"
DZ: 0. , DRX: 0. , DRY: 0. )
(NODE: (A, B) DX: 0. , DY: 0. )

Name of the nodes:

\title{
Not \(A=A\)
}

Not \(C=C\)
Not \(B=B\)
Not \(D=D\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 4
A number of meshs and types: 4 SEG2

\subsection*{3.3 Functionalities \\ tested}

\section*{Orders}

\author{
Keys \\ AFFE_CARA_ELEM BEAM \\ NET \\ "CIRCLE" \\ [U4.24.01] \\ AFFE_CHAR_MECA DDL_IMPO \\ ALL \\ [U4.25.01] \\ GROUP_NO \\ FORCE_NODALE \\ NODE
}
"MECHANICAL" AFFE_MODELE "POU_D_T"
[U4.22.01]
DEFI_MATERIAU ELAS
[U4.23.01]

\author{
Handbook of Validation
}

V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL11 - Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):
Key J.M. PROIX
:
V3.01.011-E Page:
5/10

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested
Not Reference Displacement
Aster \%
difference
\((\boldsymbol{m})\)
0.00

\section*{\(u D\)}
3.47902 10-3 3.4784

10-3
-0.02
D

VD
-5.60084 10-3 -5.5994
10-3
-0.03

\subsection*{4.2 Parameters \\ of execution}

Version: 5.2

Machine: SGI/ORIGIN 2000 R10000
Obstruction memory: 64 Mo
Time CPU To use:
2.1 seconds

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

SSLL11 - Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):

\section*{5 Modeling \\ B}

\section*{5.1}

\section*{Characteristics of modeling}

4 elements \(P O U \_D \_T\) of full circular section: 4 meshs SEG2
elements \(A C\) and \(B C\)
ray \(R=7.978845103 \mathrm{~m}\)
(surface \(A=2.104 \mathrm{~m} 2\) )
elements \(C D\) and data base
ray \(R=5.641895103 \mathrm{~m}\)
(surface \(A=1.104 \mathrm{~m} 2\) )
Poisson's ratio:
\(=0.3\)

\section*{Limiting conditions:}

\author{
DDL_IMPO: (All: "YES" \\ DZ: 0. , DRX: 0. , DRY: 0. )
}

To treat the rotary joints, one creates as many nodes as of ends of bar.

\section*{D3}
- with the nodes A1, B2 and D4

C3
C1
D4
DDL_IMPO: (DX: 0, D4: 0)
- with the nodes C1, C2, C3 and D3, D4 continuity

C2
translations, by LIAISON_DDL DX and DY,
- no rotation is imposed.

Al
B4
B2

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 4
A number of meshs and types: 4 SEG2

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\section*{Keys}

AFFE_CARA_ELEM BEAM
NET
"CIRCLE"
[U4.24.01]
AFFE_CHAR_MECA DDL_IMPO
NODE
[U4.25.01]
LIAISON_DDL

\section*{FORCE_NODALE}
"MECHANICAL" AFFE_MODELE "POU_D_T"
[U4.22.01]
DEFI_MATERIAU ELAS

V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL11 - Lattice of pin jointed struts under concentrated loading
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Key J.M. PROIX
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6
Results of modeling B

\subsection*{6.1 Values}
tested

\section*{Not Reference Displacement}

Aster \%
difference
(m)

\section*{CPU}
2.6517 10-4 2.6517

10-4
0.00

C

Vc
0.8839 10-4 0.8838

10-4
-0.01

\subsection*{6.2 Parameters \\ of execution}

Version: 5.2

Machine: SGI/ORIGIN 2000 R10000
Obstruction memory: 64 Mo
Time CPU To use: 3.7 seconds

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLL11 - Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):

\section*{7 Modeling}

C

\section*{7.1}

\section*{Characteristics of modeling}

4 elements BARS full circular section: 4 meshs SEG2
elements \(A C\) and \(B C\)
ray \(R=7.978845103 \mathrm{~m}\)
(surface \(A=2.104 \mathrm{~m} 2\) )
elements \(C D\) and data base
ray \(R=5.641895103 \mathrm{~m}\)
(surface \(A=1.104 \mathrm{~m} 2\) )
Poisson's ratio:
\(=0.3\)

\section*{Limiting conditions:}

DDL_IMPO:
(
ALL: "YES"
DZ:
0 .
)
(NODE: (A, B) DX: 0. , DY: 0. )

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 4
A number of meshs and types: 4 SEG2

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

\author{
Keys \\ AFFE_CARA_ELEM BARS \\ NET \\ "CIRCLE" \\ [U4.24.01] \\ AFFE_CHAR_MECA DDL_IMPO \\ NODE
}
[U4.25.01]
FORCE_NODALE

\author{
[U4.23.02] \\ "MECHANICAL" AFFE_MODELE "BAR"
}

\section*{[U4.22.01]}

DEFI_MATERIAU ELAS
[U4.23.01]

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL11-Lattice of pin jointed struts under concentrated loading

Date:
16/02/00
Author (S):
Key J.M. PROIX
:
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\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested

\author{
Not Reference Displacement \\ Aster \% \\ difference \\ (m) \\ CPU \\ 2.6517 10-4 2.6517 \\ 10-4 \\ -0.002 \\ C \\ Vc \\ 0.8839 10-4 0.8839 \\ 10-4 \\ -0.002
}
\(u D\)
3.47902 10-3 3.47902

10-3
0
D

VD
-5.60084 10-3 -5.60035
10-3
-0.009

\subsection*{8.2 Parameters \\ of execution}

Version: 5.2

Machine: SGI/ORIGIN 2000 R10000
Obstruction memory: 64 Mo
Time CPU To use: 2.4 seconds

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL11 - Lattice of pin jointed struts under concentrated loading
Date:
16/02/00
Author (S):
Key J.M. PROIX
V3.01.011-E Page:
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\section*{9 \\ Summary of the results}

Results in conformity with the reference solution for three modelings:
model of beams,
model of linear beams + relations,
model of bars.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL12 Lattice of bars under three requests
Date:
19/01/98
Author (S):
J.M. PROIX, L. VIVAN

Key:
V3.01.012-A Page:
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Organization (S): EDF/IMA/MMN, CISI

\section*{Handbook of Validation}

\section*{V3.01 booklet: Linear statics of the linear structures}

\section*{Document: V3.01.012}

SSLL12 - Lattice of bars under three requests

\section*{Summary:}

Static response in linear mechanics of the structures of a triangulated system of pin jointed struts (plane lattice)
under 3 requests:
- displacement of support,
- specific forces,
- effect of dilation.

This test makes it possible to validate the element BARS under various cases of loading. It validates also the option
LIAISON_OBLIQUE of order AFFE_CHAR_MECA.
Handbook of Validation
\(V 3.01\) booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL12 Lattice of bars under three requests
Date:
19/01/98
Author (S):
J.M. PROIX, L. VIVAN

Key:
V3.01.012-A Page:
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1
Problem of reference
1.1 Geometry

5 m
5 m
5 m
5 m
5 m
5 m
5 m
4 m
C
With
D
4 m
y, v
B
X, U
1.2

Material properties
Isotropic linear elastic material:
\(\mathrm{E}=2.1 \mathrm{E}+11 \mathrm{~Pa}\)
Linear dilation coefficient:
\(=1\). \(05^{\circ} \mathrm{C} 1\)

\section*{1.3}

Boundary conditions and loadings
Articulation of A \((\mathrm{uA}=\mathrm{vA}=0)\).
Support with roller out of \(B\) and \(C\left(v B=v^{\prime} C=0\right)\).
Handbook of Validation

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL12 Lattice of bars under three requests
Date:
19/01/98
Author (S):
J.M. PROIX, L. VIVAN

Key:
V3.01.012-A Page:
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1.4 Conditions
initial
Office plurality of 3 requests:
- displacement of support: \(\mathrm{vA}=0.02 \mathrm{~m}, \mathrm{vB}=0.03 \mathrm{~m}, \mathrm{v}^{\prime} \mathrm{C}=0.015 \mathrm{~m}\)
- specific forces: \(\mathrm{FE}=150 \mathrm{kN}, \mathrm{FF}=100 \mathrm{kN}\)
- effect of dilation of all the bars for a variation in temperature of \(30^{\circ} \mathrm{C}\) compared to
temperature of assembly (geometry of reference).
```

F
F
E
F
E
F
v'
u'
With
C
GOES
V'C
B
VB
2
Reference solution
2.1
Method of calculation used for the reference solution

```

Determination of the unknown the hyperstatic one by the method of cut to know the tractive effort.
2.2
```

Results of reference
Not
Size and unit
Value
Data base
Tractive effort (NR)
8.2112 E+03
2.3
Uncertainty on the solution
Analytical solution.

```

\subsection*{2.4 References}

\section*{bibliographical}
```

[1]
Mr. LAREDO, Resistance of materials, Paris, Dunod, 1970, p. 579.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL12 Lattice of bars under three requests

\section*{Date:}

19/01/98
Author (S):
J.M. PROIX, L. VIVAN

Key:
V3.01.012-A Page:
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3 Modeling
With
3.1

Characteristics of modeling
Type of modeling used: element BARS.
A2
A2
A2
A1
A1
A1
A1
A1
A1
A1
A1
C
With
A2
A2
A2
D
A2
A2
A2
B

\section*{3.2}

Characteristics of the grid
\(=30^{\circ}\).
\(\mathrm{A} 1=1.41 \mathrm{E} 03 \mathrm{~m} 2\).
\(\mathrm{A} 2=2.82 \mathrm{E} 03 \mathrm{~m} 2\).

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
LIAISON_OBLIQUE
FORCE_NODALE
TEMP_CALCULEE
MECA_STATIQUE
[U4.31.01]
CALC_ELEM
EFGE_ELNO_DEPL
[U4.61.02]
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL12 Lattice of bars under three requests

\section*{Date:}

19/01/98
Author (S):

\section*{J.M. PROIX, L. VIVAN}

Key:
V3.01.012-A Page:
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4
Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference
Charge: thermal dilation
Option: "EFGE_ELNO_DEPL"
Net M10, Noeud: B, Cmp: NR
12946.
1.29541 E+04
0.063

Net M16, Noeud: C, Cmp: NR
4285.2
4.28926 E+03
0.095

Net M17, Noeud: C, Cmp: NR
-10189.
1.02076 E+04
0.183

Charge: specific forces
Option: "DEPL"
Node: E, Cmp: DY
1.0566 E02
1.05800 E 02
0.133

Option: "EFGE_ELNO_DEPL"
Net M10, Noeud: B, Cmp: NR
-87137.
8.71128 E+04
0.028

Net M16, Noeud: C, Cmp: NR
24158.
2.41596 E+04

Net M17, Noeud: C, Cmp: NR
-57524.
5.74954 E+04
0.050

Charge: imposed displacements
Option: "EFGE_ELNO_DEPL"
Net M10, Noeud: B, Cmp: NR
65979.1
6.59757 E+04
0.005

Net M16, Noeud: C, Cmp: NR
21839.1
2.18453 E+04
0.029

Net M17, Noeud: C, Cmp: NR
51925.6
5.19877 E+04
0.120

Charge: office plurality of the 3 requests
Option: "EFGE_ELNO_DEPL"
Net M10, Noeud: B, Cmp: NR
8211.2
8.18302 E+03
0.343

Net M16, Noeud: C, Cmp: NR
50282
5.02942 E+04
0.024

Net M17, Noeud: C, Cmp: NR
\(1.1964 \mathrm{E}+05\)
1.19691 E+05
0.043

\subsection*{4.2 Remarks}

No deformation of inflection intervenes in the calculation of the solution.

\subsection*{4.3 Parameters}

\section*{of execution}

Version: 3.02.11
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
60 seconds

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster © \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL12 Lattice of bars under three requests
Date:
19/01/98
Author (S):

\section*{J.M. PROIX, L. VIVAN}

Key:
V3.01.012-A Page:
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5

\section*{Summary of the results}

The variations compared to the references are lower than \(0.18 \%\) for the requests (dilation thermics, specific face, imposed displacement) separate and lower than \(0.34 \%\) when these requests are cumulated.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL14 plane Gantry articulated in foot

\section*{Date}

21/03/02
Author (S):
J.M. PROIX, L. VIVAN Key

V3.01.014-A Page:
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Organization (S): EDF/AMA, CS IF

\title{
Handbook of Validation
}

V3.01 booklet: Linear statics of the linear structures
V3.01.014 document

SSLL14-Plane gantry articulated in foot

\section*{Summary}

This test relates to the study of a gantry made up of hurled beams, articulated in foot, in static analysis
linear.
The gantry is modelled with elements linear (SEG2) and subjected to four loadings (distributed or specific).

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL14 plane Gantry articulated in foot

\section*{Date}
```

:
21/03/02

```
Author (S):
J.M. PROIX, L. VIVAN Key
:
V3.01.014-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

F1

\(L=20 \mathrm{~m}\)

\(H=8 m\)

C

\(=4 \mathrm{~m}\) have

\(P\)

has

F2

D

E

M

H

\(y\)

With

B

\(X\)

\(L\)
Sections AD, EB
\(I 1=5 . E 4 m 4\)
Sections cd., EC
I2 \(=2.5 \mathrm{E} 4 \mathrm{~m} 4\)

The gantry consists of symmetrical beams of sections, so that IY=IZ.
One takes account only of the energy of inflection, because the beams are very slim. This is why them other characteristics of section of beam do not intervene.

\section*{1.2}

\section*{Material properties}

Isotropic linear elastic material:
\(E=2.1 E 11 \mathrm{~Pa}\)

\section*{1.3 \\ Boundary conditions and loadings}

Feet of articulated posts \(A\) and \(B\).

\section*{Loadings}

Nodal force out of \(C\) :
\(F y=2000 N R=F 1\)
Nodal force in D:
\(F x=10.000 N R=F 2\)
Moment in D:
\(M X=100.000 \mathrm{~N} . \mathrm{m}=\mathrm{M}\)
Force distributed on section cd.:
\(P z=3000 \mathrm{~N} / \mathrm{m}\)
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
Version
5.0

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SSLL14 plane Gantry articulated in foot
Date

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2

\section*{Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

The method of calculation and the solution are exposed in the document [V3.90.01].

\section*{2.2}

Results of reference
Horizontal and vertical reactions at point \(A\).
Bending moment out of \(C\).
Displacements horizontal and vertical of the point \(C\).

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
F.VOLDOIRE: Calculation of an elastic hyperstatic plane gantry [V3.90.01].

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

\section*{C \\ D \\ E \\ y \\ With \\ B \\ X}

\section*{Modeling POU_D_E}

10 elements section by section, is 40 elements \(\operatorname{SEG} 2\)
Displacement in the plan: \(D z=0\) on all the grid
Feet of articulated posts \(A\) and \(B: D x=D y=0\)

\section*{3.2 \\ Characteristics of the grid}

Sections AD and EB: Il inertia \(=5\) E4 m4
Sections cd. and EC: I2 inertia \(=2.5 E 4 m 4\)

\subsection*{3.3 Functionalities \\ tested}

AFFE_CHAR_MECA
FORCE_NODALE:
Fx, Fy, Mz
FORCE_POUTRE:
Fy
CALC_ELEM
"EFGE_ELNO_DEPL"
CALC_NO
"REAC_NODA"

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4
Results of modeling \(A\)

\subsection*{4.1 Values \\ tested}

\section*{Loading Value}
tested

\section*{Reference}

\author{
Aster
}
\% difference
Effort distributed p on CD
Displacement out of C Dx
0.0110476
0.01104720 .003

Displacement out of C Dy
0.012422374
-0.0124233 0.004
Moment out of C Dz
18672.994
18673.20 3.10-6

Reaction of \(A D x\)
5175.37
5175.36 2.10-4

Reaction of A Dy
24233.24
24233.2 2.10-4

Force concentrated F1 out of C
Displacement out of C Dx
0.00000
0.00000

Displacement out of C Dy
0.01497330
-0.0149734 0.005
Moment out of C Dz
41422.161
41422.40 3.10-6

\section*{Reaction of A Dx}
4881.487
4881.47 2.10-4

\section*{Reaction of A Dy}
10000.00
10000.0 2.10-4

Force concentrated \(F 2\) in \(D\)
Displacement out of C Dx
0.03000956
-0.0300098 0.001
Displacement out of C Dy
0.00299466
-0.00299450 0.005
Moment out of C Dz
8284.432
8284.34 3.10-6

Reaction of A Dx
5976.297
5976.31 1.10-4

Reaction of A Dy
4000.00
4000.00

\author{
Moment out of \(C\) \\ Displacement out of C Dx \\ 0.0273532 \\ 0.02735360 .001 \\ Displacement out of C Dy \\ 0.001215646 \\ -0.00121583 0.015 \\ Moment out of C Dz \\ 4916.724 \\ 4916.62 3.10-6
}

\title{
Reaction of A Dy
}
5000.00
5000.00

\author{
Handbook of Validation
}

V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
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Date

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J.M. PROIX, L. VIVAN Key

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\section*{5 \\ Summary of the results}

The results obtained with modeling \(P O U_{-} D_{-} E\) are in very good agreement with the solution analytical and thus validate the calculation of lattice of beams subjected to efforts specific or distributed.

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Symmetrical SSLL100 Structure of beams with an elbow
Date:
22/01/98
Author (S):

\section*{J.M. PROIX}

Key:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures

\section*{Document: V3.01.100}

\section*{SSLL100-Symmetrical structure of beams}

\section*{with an elbow}

\section*{Summary:}

This test in statics, linear elasticity makes it possible to validate the elements of rectangular beam and curves in inflection
plane, as well as the discrete elements. Four loadings are defined, of which some in local reference mark.
Two modelings make it possible to test on the one hand the right elements (the elbow is modelled using 20
right elements) and in addition right and curved elements.
The reference solution results from the file of validation of the code LICE. Results obtained with
Code_Aster are very close (lower deviation than 2.104 for modeling to the arc of circle with POU_C_T, a little higher variation (3\%) for that with \(P O U_{-} D_{-} T\), which is due to a too coarse discretization). Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
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1
Problem of reference

\subsection*{1.1 Geometry}
\(F\)
\(F\)
\(Z\)
\(D\)
\(C\)
\(E\)
\(B\)
\(F\)
\(y\)
With

\section*{G}

Symmetrical plane structure compared to the line \(y=4\).
Beams of section
circular
external diameter
of \(=0.04 \mathrm{~m}\)
internal diameter
\(d i=0.01 \mathrm{~m}\)
Bend
of center
( \(y=4 Z=0\) )
and of ray \(=22 \mathrm{~m}\)
Connection node-node
\(K x=K z=105 \mathrm{~N} / \mathrm{m}\)
in the local repére

\section*{Co-ordinates of the points (in m):}

\section*{With}

B
C
D
E
\(F\)
G
\(X\)
0.
0.
0.
0.
0.

0 .
0.
y
-2.
0 .
2.
4.
6.
8.
10.

Z
-2.
0 .
2.

\section*{Material properties}

\section*{\(E=2.11011 \mathrm{~Pa}\)}
\(=0.3\)
\(=7800 . \mathrm{kg} / \mathrm{m} 3\)
\(=106 \mathrm{~m} /{ }^{\circ} \mathrm{C}\)
1.3

Boundary conditions and loadings
Embedded points \(A\) and \(G\)
( \(v=W=0\) )
(except in the case of load 2)
Loading:
1) loading concentrated out of \(C\) and \(E\)
\(F=1000 \mathrm{NR}\)
2) displacement imposed of \(A\) and \(G\)
\(D x=2\) in local reference mark of meshs \(A B\) and \(G F\)
3) thermal expansion with \(T=100^{\circ} \mathrm{C}\)
4) actual weights

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\section*{Code_Aster \({ }^{\circledR}\)}

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2
Reference solution
2.1

\section*{Method of calculation used for the reference solution}

The reference solution is that given in the card of validation STA.MPACO/B of the code LICE of
the D.E.R [bib1].

\section*{2.2}

\section*{Results of reference}

Displacements of the points \(B, C\) and \(D\).
2.3

\section*{Uncertainty on the solution}
- modeling \(a\) : < 103 (finite element providing of the exact values to the nodes),
- B: a few \% (numerical solution function of the discretization).

\subsection*{2.4 References \\ bibliographical}
[1]
Computer code of structures of beam LICE. Card-index validation of module EFPOU
MPACO/B - Direction of the Studies and Research E.D.F (1988)
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
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Titrate:
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3 Modeling

\section*{With}
3.1

Characteristics of modeling
6 meshs SEG2:
2 meshs CD, EC
curved beam
POU_C_T
2 meshs BC, EF
right beam
POU_D_T
2 meshs \(A B, F G\)
connection element
DIS_T

\section*{Limiting conditions:}

DDL_IMPO:
(GROUP_NO: Beam
DX: 0. DRY: 0. DRZ: 0. )
(NODE: (A G)
DX: 0. DY: 0. DZ: 0. )
loading case 2
(NODE: With
DX: 0. DY: 1. DZ: 1. )
(only)
(NODE: G
DX: 0. DY: - 1. DZ: 1. )
loading case 1 FORCE_NODALE: (NODE: (C E) FZ = -1000. )
loading case 3 AFFE_CHAM_NO (SIZE: "TEMP_R"
AFFE: (ALL: "yes" NOM_CMP: "TEMP" VALE_R: 100.)
loading case 4 GRAVITY: (9.81 0. 0. -1. )
Name of the nodes: WITH, B, C, D, E, F
Name of the meshs: \(A B, B C, C D, O F, E F, F G\)
3.2

Characteristics of the grid
A number of nodes: 7
A number of meshs and types: 6 SEG2

\subsection*{3.3 Functionalities}
tested
Orders

\section*{Keys}

AFFE_CARA_ELEM
BEAM
SECTION
"GENERAL"
[U4.24.01]
DISCRETE
K_T_D_L
"LOCAL"
DEFI_ARC
ORIE_ARC
ORIENTATION

\section*{CARA}
"VECT_Y"
AFFE_CHAM_NO
"TEMP_R"
"TEMP"
[U4.26.01]

\section*{AFFE_CHAR_MECA}

DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
NODE
TEMP_CALCULEE
GRAVITY
AFFE_MATERIAU
GROUP_MA
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_C_T"
[U4.22.01]
"POU_D_T"
"DIS_T"
DEFI_MATERIAU
ELAS
[U4.23.01]
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Symmetrical SSLL100 Structure of beams with an elbow
Date:
22/01/98
Author (S):

\section*{J.M. PROIX}

Key:
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4
Results of modeling \(A\)
4.1 Values
tested
Case
Not
displacement

Aster
\%diff
tolerance
(m)

B
\(v B\)
8.120103
8.1201103
0.00
1. 103

1
\(w B\)
1.000102
1.0000102
0.00

Forces
C
vC
7.389103
7.3895103
0.00
nodal
D
\(w D\)
2.553102
2.5530102
0.00

B
\(v B\)
9.858101
9.8585101
0.00
1. 103

2
\(w B\)
1.000
1.0000
0.00

\author{
Displacement \\ C \\ vC \\ 1.738101 \\ 1.7382101 \\ 0.01 \\ imposed \\ D \\ \(w D\) \\ 1.812 \\ 1.8120 \\ 0.00 \\ B \\ \(v B\) \\ 5.660106 \\ 5.6597106 \\ 0.01 \\ 1. 103 \\ 3 \\ \(w B\)
}
```

Dilation
C
vC
1.305104
1.3047 104
0 . 0 2
D
wD
5.248104
5.2480 104
0 . 0 0
B
vB
3.111 103
3.1107 103

```

Gravity
C
\(\nu C\)
1.180103
1.1802103
0.02

D
\(w D\)
8.850103
8.8504103
0.00

\subsection*{4.2 Parameters}
of execution
Version: 3.5.2
Machine: CRAY C90
Obstruction memory:
8 megawords
Time CPU To use:
12 seconds
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Symmetrical SSLL100 Structure of beams with an elbow
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22/01/98
Author (S):

\section*{J.M. PROIX}

Key:
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\section*{5 Modeling}

\section*{B}

\section*{5.1}

\section*{Characteristics of modeling}

The arc of beam was modelled in a polygonal line of 2*20 SEG2

\section*{Limiting conditions:}

DDL_IMPO:
(GROUP_NO: Npoutre
DX: 0. DRY: 0. DRZ: 0.)
(NODE: (A G)
DX: 0. DY: 0. DZ: 0. ) except for
loading case 2
(NODE: With
DX: 0. DY: 1. DZ: 1. )
(NODE: G
DX: 0. DY: - 1. DZ: 1. )
loading case 1
FORCE_NODALE: (NODE: (C D) Fz =-1000. )
loading case
AFFE_CHAM_NO (SIZE: "TEMP_R"
AFFE: (ALL: 'OUI' Nom_cmp: 'TEMP' VALE_R: 100.)
loading case 4
GRAVITY: (9.81 0. 0. -1. )
Name of the nodes: WITH, B, C, D, E, F

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 45
A number of meshs and types: 44 SEG2

\subsection*{5.3 Functionalities}
tested
Orders

\section*{Keys}

AFFE_CARA_ELEM
BEAM
SECTION
"GENERAL"
[U4.24.01]
DISCRETE
\(K_{-} T \_D \_L\)
"LOCAL"
ORIENTATION
CARA
"VEC_Y"
```

AFFE_CHAM_NO
"TEMP_R"
"TEMP"
[U4.26.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
NODE
TEMP_CALCULEE
GRAVITY
AFFE_MATERIAU
GROUP_MA
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"POU_D_T"
GROUP_MA
[U4.22.01]
"DIS_T"
NET
DEFI_MATERIAU
ELAS
[U4.23.01]
Handbook of Validation
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```
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
Symmetrical SSLL100 Structure of beams with an elbow
Date:
22/01/98
Author (S):

\section*{J.M. PROIX}

Key:
V3.01.100-B Page:
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6
Results of modeling B

\author{
6.1 Values \\ tested \\ Case \\ Not \\ displacement \\ Reference \\ Aster \\ \%diff \\ tolerance \\ (m) \\ B \\ \(v B\) \\ 8.120103 \\ 8.1209103 \\ 0.01 \\ 1. 103 \\ 1 \\ \(w B\) \\ 1.000102 \\ 1.0000102 \\ 0.00 \\ Forces \\ C \\ vC \\ 7.389103 \\ 7.3863103 \\ 0.04 \\ nodal \\ D \\ \(w D\) \\ 2.553102 \\ 2.5528102 \\ 0.01 \\ B \\ \(v B\) \\ 9.858101 \\ 9.8585101 \\ 0.00 \\ 1. 103 \\ 2
}

\subsection*{6.2 Remarks}

The modeling of the elbow by right elements requires a very fine grid, for a precision sufficient (in particular for a loading distributed).

\subsection*{6.3 Parameters}
of execution
Version: 3.2.11
Machine: CRAY C90
Obstruction memory:
8 megawords
Time CPU To use:
7.6 seconds

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\section*{Code_Aster \({ }^{\circledR}\)}

Version

\section*{4.0}

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Date:
22/01/98
Author (S):

\section*{J.M. PROIX}

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\section*{Summary of the results}

The results obtained with Code_Aster coincide well with those of the code LICE (solution of reference) in particular for modeling A (POU_C_T).
For modeling B, they are very close also (<4104) except in the case of load to
gravity (3\% of variation to the maximum) because of the dependence of the solution to the smoothness of discretization.
This test thus validates element POU_C_T.
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):

\author{
J.M. PROIX, F. LEBOUVIER Key
}
:
V3.01.101-C Page:
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Organization (S): EDF/MTI/MMN, DeltaCAD

\author{
Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ Document: V3.01.101
}

SSLL101 - Piping: Problem of HOVGAARD

\section*{Summary:}

It is about a linear elastic test, in statics, of a noncoplanar three-dimensional piping comprising elbows. There is a test in dynamics of same structure (SDLX02) [V2.05.002].

One tests elements POU_D_T, POU_C_T, PIPE (SEG3 and SEG4) and TUYAU_6M (SEG3) by the intermediary
of five modelings, each elbow is represented by:

20 elements POU_D_T in modeling A,
2 elements POU_C_T in modeling B, 28 elements PIPE (SEG3) in modeling C,

28 elements TUYAU_6M (SEG3) in modeling D,
28 elements PIPE (SEG4) in modeling E.
The loadings are:
gravity,

V3.01 booklet: Linear statics of the linear structures HI-75/01/010/A

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Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):

\section*{J.M. PROIX, F. LEBOUVIER Key}
:
V3.01.101-C Page:
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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}

Geometry, as well as the points of modeling are represented on the following figure:
0.

With
-1.828
0.922
2.75

Z
0.922

B
=
-0.922
=
0.

45

With

8
9
10
B
3.69
11
2
12
y
13
=
14
=
\(=\)
15
=
1.96
=
X
1
lengths given in meters
diameter external of the pipe:
0.185 m
thickness of the pipe:
6.12 mm
radius of curvature of the elbows:
0.922 m
piping full of water
1.2
Material properties
\(E=1.658 E+11 \mathrm{~Pa}\)
\(=0.3\)

\section*{1.3 \\ Boundary conditions and loadings}

Items 1 and 15 embedded,
Loading:
1) Gravity according to Z,
2) Uniform rise in temperature of \(472.22 C^{\circ}\),
3) Forces
nodal.
Nodes
2
3
4-10
5-9
6-7-8
11
12-13
14
Fz (NR)
-
327.654
-102.5145-222.687
-176.580
624.897788 .724
214.839
117.720

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Code_Aster \({ }^{\circledR}\)
Version
5.0

\author{
Titrate: \\ SSLL101 - Piping: problem of HOVGAARD
}

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solutions adopted to check Code_Aster modelings are as follows:
- for modelings BEAM: comparison with the codes: LICE, ADL and TITUS-T [bib1], using a modeling of the beam type,
- for modelings PIPES: comparison with code ABAQUS, using one
modeling of the pipe type. The number of mode of Fourier (M) used during calculation of reference is identical to that used during calculations with Code_Aster.

\section*{2.2 \\ Results of reference}

\author{
Case of \\ Displacement \\ Modeling Beam \\ Modeling \\ Modeling \\ Loading \\ at item 3 \\ (LICE, ADL, TITUS) \\ Pipe: M=3 \\ Pipe: \(M=6\) \\ (ABAQUS) \\ (ABAQUS) \\ Actual weight
}

DY 0.2040E4 0.13870E4 0.13946E4

\section*{DZ 0.8010E5 0.80376E5}
0.80369E5

DX 0.1651E3
0.16445E3
0.16441E3

Nodal force
DY 0.2080E4 0.14245E4
0.14320E4

DZ 0.9516E5 0.10047E4
0.10047E4

\author{
DX 6.1418E3 \\ \(6.3277 E 3\) \\ \(6.3236 E 3\) \\ Dilation \\ DY 13.090E3 \\ 13.092E3 13.093E3 \\ DZ 16.799E3 \\ 16.798E3 16.798E3 \\ \section*{2.3 \\ \\ Uncertainty on the solution}
}
\(2 \%\)

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}
M.W. KELLOG Co. Design of Piping Systems. New York, 1956 - Problem n \({ }^{\circ} 5.9\)

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Author (S):
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The curved elements are modelled by right elements.
A curved half element is modelled by 20 right elements.
2.75
\(=\)
\(=\)
N400
=
N500 N600

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 93
A number of meshs and type: 92 POU_D_T

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

\title{
AFFE_CHAR_MECA GRAVITY
}

TEMP_CALCULEE
"MECHANICAL" AFFE_MODELE "POU_D_T"
ALL
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key
:
V3.01.101-C Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

Identification Displacement
Reference
beam
Aster \%

\section*{Actual weight}

N300 DX
\(0.1658 E 3\)
\(0.1658 E 3\)

\author{
N300 DX \\ 0.1651E3 \\ \(0.1651 E 3\) \\ 0.04 \\ Nodal force \\ DY 0.2080E4 \\ 0.2080E4 \\ -0.01 \\ DZ 0.9516E5 \\ 0.9516E5 \\ 0.004
}

\author{
N300 DX \\ \(6.1418 E 3\) \\ 6.1413E3 \\ 0.007 \\ \section*{Dilation} \\ DY 13.090E3 \\ 13.091E3 0.012 \\ DZ 16.799E3 \\ 16.799E3 0.003
}

\subsection*{4.2 Notice}

The differences between the Aster results and the reference solution beam are all lower than 0.04\%

\subsection*{4.3 Parameters \\ of execution}

Version: 5.6

Machine: Origin 2000

Obstruction memory:
16 Mo
Time CPU To use:
3 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):

\section*{J.M. PROIX, F. LEBOUVIER Key}
:
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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

The curved elements are modelled by elements "POU_C_T".
2.75
=
\(=\)
N400
=
N500 N600
=
N300
N700
=
With
N800
N900
N1000
B
3.69

N1100
N200
N1200
\(y\)
N1300
=
N1400
=
=
N1500
=
1.96
=
\(X\)
N100

Lengths given in meters

\section*{5.2}

Characteristics of the grid

A number of nodes: 15
```

A number of meshs and type:
10 POU_D_T
4
POU_C_T

```

\author{
5.3 Functionalities \\ tested \\ Orders
}

AFFE_CHAR_MECA GRAVITY

\section*{TEMP_CALCULEE}

\section*{"MECHANICAL" AFFE_MODELE}
"POU_D_T"
ALL
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key
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\title{
6 \\ Results of modeling B
}

\subsection*{6.1 Values}
tested

\section*{Identification Displacement \\ Reference \\ beam Aster \%}

Actual weight
N300 DX
0.1658E3
\(0.1658 E 4\)
0.017

DY 0.2040E4
0.2053E5
0.65

DZ 0.8010E5
0.8010E6
-0.006

\author{
N300 DX \\ 0.1651E3 \\ \(0.1652 E 4\) \\ 0.04 \\ Nodal force \\ DY 0.2080E4 \\ 0.2080E5 \\ 0.02 \\ DZ 0.9516E5 \\ 0.9516E6 \\ 0.002
}

N300 DX
6.1418E3
\(6.1404 E 3\)
-0.02

\section*{Dilation}

DY 13.090E3
13.090E2 0.005

DZ 16.799E3
16.799E2
0.003

\subsection*{6.2 Notice}

The differences between the Aster results and the results of reference beam are all lower than \(0.02 \%\) except for DY in actual weight where the variation is \(0.65 \%\)

\subsection*{6.3 Parameters \\ of execution}

Version: 5.6

Machine: Origin 2000

Obstruction memory:
16 Mo
Time CPU To use:
3 seconds

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key

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\section*{7 Modeling}

C

\section*{7.1 \\ Characteristics of modeling}

5
E
Modeling PIPE (SEG3)
D
4
Z
With
F
B
y
5
10
G
4
Boundary conditions: Points \(C\) and \(H\)
H
- DDL of Beam: \(D X=D Y=D Z=D R X=D R Y=D R Z=0\)
- DDL of Hull: UIm \(=\) VIm \(=\) Wim \(=0(m=2,3)\)

C
\(U O m=V O m=W O m=O(m=2,3)\)
X
\(W I I=W O 1=W O=0\)

\title{
7.2 \\ Characteristics of the grid
}

A number of nodes:
57
A number of meshs and type:
28 SEG3

\subsection*{7.3 Functionalities \\ tested}

Orders

AFFE_MODELE AFFE
MODELISATION =' TUYAU'
AFFE_CARA_ELEM
BEAM: (SECTION: "CIRCLE")
ORIENTATION: (CARA: "GENE_TUYAU"
VALE: (X Y Z)
AFFE_CHAR_MECA GRAVITY

TEMP_CALCULEE

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:

\subsection*{8.1 Values}
tested

\section*{Identification Displacement}

Reference
pipe Aster \%
( \(M=3\) )
Actual weight
Not D DX
0.16517E3
0.1636E3
-0.93
DY
0.13870E4
0.1251E4
-9.80

DZ
\(0.80376 E 5\)
\(0.8018 E 5\)
-0.24

\author{
Not D DX \\ \(0.16445 E 3\) \\ 0.1629E3 \\ -0.94
}

\author{
Nodal force \\ DY \\ \(0.14245 E 4\) \\ 0.1288E4 \\ -9.61 \\ DZ \\ 0.10047E4 \\ 0.1003E4 \\ -0.20
}

\author{
Not D DX
}
6.3277E3
6.4534E3
1.99

Dilation
DY
13.092E3
13.103E3
0.08

DZ
16.798E3
16.880 E3
0.49

\subsection*{8.2 Notice}

The results obtained with Code_Aster are similar to those of ABAQUS by elements pipes except for the displacement DY (actual weight and nodal force) where the variation is about \(10 \%\).

\subsection*{8.3 Parameters \\ of execution}

Version: 5.6

Obstruction memory: 16 Mo
Time CPU To use: 4,7 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key

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10/14

\section*{9 Modeling}

D

\section*{9.1}

Characteristics of modeling
5
E
Modeling TUYAU_6M (SEG3)
D
4
Z
With
F
B
y

Boundary conditions: Points \(C\) and \(H\)
\(H\)
- DDL of Beam: \(D X=D Y=D Z=D R X=D R Y=D R Z=0\)
- DDL of Hull: UIm \(=\) VIm \(=\) Wim \(=0(m=2,6)\)

C
\(U O m=V O m=W O m=0(m=2,6)\)
\(X\)
\(W I I=W O 1=W O=0\)

Lengths given in meters

\section*{9.2}

Characteristics of the grid
A number of nodes:
57
A number of meshs and type:
28 SEG3

\subsection*{9.3 Functionalities}
tested

\section*{Orders}
\(A F F E \_M O D E L E A F F E\)
MODELISATION=' TUYAU_6M'
AFFE_CARA_ELEM
BEAM: (SECTION:"CIRCLE")
ORIENTATION: (CARA: "GENE_TUYAU"
VALE: (X Y Z)
AFFE_CHAR_MECA GRAVITY

\author{
Handbook of Validation
}

V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key

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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested

\section*{Identification Displacement \\ Reference \\ pipe Aster \% \\ ( \(M=6\) ) \\ Actual weight \\ Not D DX \\ 0.16512E3 \\ 0.1636E3 \\ -0.93 \\ DY \\ 0.13946E4 \\ 0.1258E4 \\ -9.78}
```

DZ
0.80369E5
0.8018E5
-0.24

```
Not D D \(X\)
\(0.16441 E 3\)
\(0.1629 E 3\)
-0.94
Nodal force
DY
\(0.14320 E 4\)
\(\mathbf{0 . 1 2 9 5 E 4}\)
-9.58

DZ
\(0.10047 E 4\)
\(0.1003 E 4\)
-0.21
-0.21
Not D DX
6.3236E3
6.4495E3
1.99
Dilation
DY
13.093E3
13.104E3
0.08
DZ
16.798E3
16.880E3
0.49

\subsection*{10.2 Notice}

The results obtained with Code_Aster are similar to those of ABAQUS for elements pipes except for the displacement DY (actual weight and nodal force) where the variation is about \(10 \%\).

\subsection*{10.3 Parameters}
of execution
Version: 5.6

Machine: SGI-Origin2000 R12000

Obstruction memory: 16 Mo
Time CPU To use: 10,56 seconds
Handbook of Validation
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HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key
:
V3.01.101-C Page:
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\section*{11 Modeling}

E

\subsection*{11.1 Characteristics of modeling}
```

5
E
Modeling PIPE (SEG4)
D
4
Z
With
F
B
y
5
10
G
4
Boundary conditions: Points C and H
H

- DDL of Beam: DX = DY=DZ = DRX = DRY=DRZ =0
- DDL of Hull: UIm = VIm = Wim = O(m=2,3)
C
UOm=VOm=WOm=O(m=2,3)
X

```
\(W I I=W O 1=W O=0\)

\subsection*{11.2 Characteristics of the grid}

A number of nodes:
85
A number of meshs and type:
28 SEG4

\subsection*{11.3 Functionalities \\ tested}

Orders

CREA_MAILLAGE MODI_MAILLE
OPTION: "SEG3_4"
AFFE_MODELE AFFE
MODELISATION=' TUYAU'
AFFE_CARA_ELEM
BEAM: (SECTION: "CIRCLE")
ORIENTATION: (CARA: "GENE_TUYAU"
VALE: (X Y Z)
AFFE_CHAR_MECA GRAVITY

\section*{TEMP_CALCULEE}

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

Date:
29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key

V3.01.101-C Page:
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12 Results of modeling \(E\)
12.1 Values
tested

Identification Displacement
Reference
pipe Aster \%
( \(M=3\) )
Actual weight
Not D DX
0.16517E3
0.1648E3
-0.22
DY
0.13870E4
0.1321E4
-4.73
DZ
0.80376E5
\(0.8024 E 5\)
-0.18

\author{
Not D DX \\ \(0.16445 E 3\) \\ 0.1638E3 \\ -0.37
}

Nodal force
DY
\(0.14245 E 4\)
0.1400E4
-1.74
DZ
0.10047E4
0.9997E5
-0.50

\section*{Not D DX}
6.3277E3
6.329E3
0.02

\section*{Dilation}

DY
13.092E3
13.105E3
0.10

DZ
16.798E3
16.843E3
0.27

\subsection*{12.2 Remarks}

The grid in SEG4 is obtained starting from a grid SEG3 with order CREA_MAILLAGE, MODI_MAILLE with option "SEG3_4". It is important that the node medium of the SEG3 is well with
medium, Code_Aster checks this condition with a tolerance.
The results obtained with Code_Aster are similar to those of ABAQUS with elements pipes except for displacement DY (actual weight and nodal force) or the variation is about \(5 \%\) and \(2 \%\).

\subsection*{12.3 Parameters}
of execution
Version: 5.6

\section*{Machine: SGI-Origin2000 R12000}

Obstruction memory: 16 Mo
Time CPU To use: 7,62 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL101 - Piping: problem of HOVGAARD

\section*{Date:}

29/10/01
Author (S):
J.M. PROIX, F. LEBOUVIER Key
:
V3.01.101-C Page:
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\section*{13 Summary of the results}

\section*{Modeling beam:}

The results are similar to the reference solution (modeling beam: average of results of 3 codes) as well for modeling \(A\), where each elbow is discretized by 20 right elements, POU_D_T, that for modeling B, where one uses the elements curves POU_C_T. One notes simply, in this case, a variation a little more important in a value of displacement (0.65\%).

\section*{Modeling pipe:}

The Code_Aster results are similar to those of ABAQUS (for elements pipes), except for displacement DY and for the loadings actual weight and nodal forces where the variation with the solution of
reference is more important with meshs SEG3 (10\%) that with meshs SEG4 (5\%).
The loading of dilation gives similar results.
This case-test makes it possible to test a noncoplanar piping.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 1/18
Organization (S): EDF-R \& D /AMA, DeltaCAD

\section*{Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ Document: V3.01.102}

SSLL102-Fixed beam subjected to efforts
unit

\section*{Summary:}

This test allows a simple checking of calculations of right beams and hull 1D in mechanics of the structures
linear statics. The model is linear.
- 7 modelings make it possible to test the various types of elements of rectangular beams in Code_Aster.
For each modeling, one calculates simultaneously 3 beams of different sections: rectangle, circle, angle.

Modeling A makes it possible of more than test the change of reference mark: the beam is directed according to
trissectrice with the total reference mark.
Modeling \(E\) tests the loading distributed on voluminal edges of elements.
Modeling F corresponds to a loading distributed varying linearly with modeling \(P O U_{-} D_{-} E\).

Modeling G corresponds to a loading distributed varying linearly with modeling POU_D_TG.
\(\cdot\) Modeling \(H\) makes it possible to test the element of hull 1D (COQUE_C_PLAN) subjected to loads unit.
- Modeling I makes it possible to test a loading distributed varying linearly with modeling TUYAU_3M.

The values tested are the generalized displacements, efforts and the constraints.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)

\section*{Version}
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02

\section*{Author (S):}
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 2/18

\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

Right beam length L, direction \(X\).
\(y\)
L
O
\(X\)
O
B
\(L=2\)
Z

One calculates simultaneously 3 types of different cross sections:
\(B=0.1\)
1 rectangular section
\(=0.2\) have

G
0.008

Z
1 corner section with equal wings
0.12

Gy
0.008
\(H=0.12\)

\section*{1 circular section}

\section*{1.2}

Material properties
\(E=2.1011 \mathrm{~Pa}=0.3\)

\section*{1.3}

Boundary conditions and loadings
Embedding out of \(O\)
- 6 unit loadings in b:
\(F x=1\)
\(M X=1\)
\(F y=1\)
\(M y=1\)
\(F z=1\)
\(M z=1\)
- 1 loading combined inflection + traction: \(F x=1 M y=1 M z=1\)
- 1 loading combined sharp efforts + torsion: \(\mathrm{Fy}=1 \mathrm{Fz}=1 \mathrm{MX}=1\)
\(\cdot 1\) loading distributed linear: Circular Fy = 1000.x section (modelings F, G, I) (with support simple of A and B in this case)
1.4

Notation of the characteristics of cross sections
The geometrical characteristics of the cross sections are noted: `

\section*{A:}
surface of the section
I, I
y
Z:
geometrical moments of inertia compared to the principal axes
of inertia of the section
\(J X\) :
constant of torsion
ay, az:
coefficients of shearing in the directions Gy and Gz
With
With

\section*{\(A^{\prime}=\)}
and \(A^{\prime}\)
\(y\)
\(=\)
:
equivalent reduced surfaces
ay
Z
\(a z\)
E, \(E\)
eccentricity of the center of torsion
\(y\)
Z:
\(J G\) :
constant of warping

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V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 3/18

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}
- Analytical Solution [bib1] and [bib2]: displacements out of B
\(L\)
Simple traction
\(U=F\)
```

X
XES
F L3
2

```
\(y\)
\((4+y)\)
LFy
Pure bending
uy =
\(=\)
12th I
Z
2nd I
Z
Z
3
2
\(L\)
\(L\) F
Pure bending
\(U=\)
\(4+\)
= -
Z
F
12th I
Z
Z)
Z
\(y\)
2nd I
\(y\)
\(y\)
MR. L
Torsion
\(X\)
\(X\)
=
G Jx
Mr. L2

\section*{MR. \(L\)}
```

y

```
\(y\)
Pure inflection
\(u z=-\)
\(=\)
2nd I
\(y\)
EI
\(y\)
y
M2
M
Pure inflection
Z L
Z L
uy =
= +
2nd I
Z
Z
E Iz
12th I
12th I
with \(y\)
=
=
L2 GA'
Z
2
\(y\)
L Gas

\section*{Notice 1:}

For the corner section, as the center of shearing is not confused with the center of gravity (ey) 0 , it are necessary to add the torque:
\(M=F . E\)
\(X\)
Z
\(y\) with the Fz loading \(=1\)
This modifies displacement:
L3
\(U=\)
F
Z
\(Z(4+Z\)
) + .e
12th I
\(X\)
\(y\)
y

Mr. L
X
\(X\)
\(=\) G.Jx
In the same way, the loading \(M X=1\) involves a displacement \(U=+. e\) Z
\(X\)
\(y\).
Loading distributed linear:
4
2
2
4
max
00652
0
4
\(U\)
=
3
-10
\(+7\)
\(=\)
\(y\) ()
px
```

X
(X
LX
L)
pL
U
360LEI
y
I.E.(internal excitation)
in X=519
O
L
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

```
    Code_Aster \({ }^{\circledR}\)
    Version
6.2
Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER
Key: V3.01.102-D Page: 4/18

\section*{Notice 2:}
1
With regard to modeling \(A\), the beam is carried by the vector \(\mathbf{e 1}=\)
1.
31
other vectors of the local reference mark are:
-1
\(-1\)
1
1
\(e 2=\)

\author{
NR \\ \(N R(O)=F x\)
}
```

y
Z(O)=T.L
T=F
y
y
y
xx (y) =
=
xy
KS
Z
y

- M.z
y
T
M
Z
y(O)=-T.L
T
Z
Z()
O=Fzxx (y)=
I
xz
KS
y
Z
MR. R
M
X
T
X(O)=MX()
B
xy=xz=Jx
M.z
M
y
y(O)=My()
B
xx(Z) = Iy

```
```

My
M
y
Z(O)=Mz()
B
xx (y) = Iz

```

Loading distributed linear:
```

1000

```
1000 L \(21000 x 2\)
\(M\) max. \(R\)
M
2
3
Z
\(Z(X)=-\)
\((L X X) V y(X)=+\)
max
\(x X\)
\(=\)
6
6
2
Iz
\(L 3\)
in \(X=\)
3
2.2
Results of reference
- Déplacement of the point B,
- efforts generalized at the point \(O\),
- forced point \(O\).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.2

\section*{Titrate:}

SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 5/18

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
J.L. BATOZ, G. DHATT: "Modeling of the structures by finite elements" - Volume 2

ED. HERMES.
[2]
N.D. PIKLEY: "Formulated for Stress, Stain \& Structural Matrices" ED. John Wiley \& Sounds.

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

2 elements \(P O U_{-} D_{-} E K=K\)
y
\(Z=1=0\) per type of section
S1: Rectangular section modelled by SECTION: "GENERAL"
With \(=\)
\(I y=\)
-4
\(I z=\)
```

O2
0 1 6 6 6 1 0
0666610
4
X=045776
10
Ry=01
Rz=00
. }
RT=00892632
(Not of calculation of the constraints)

```

\section*{S2: Corner section}

With \(=\)
1856
```

103

```
\(I y=\)
4167339
\(104 \mathrm{Iz}=\)
1045547
104 J
-8
\(X=039595\)
10
\(E=\)
41012
103 rd
```

y
Z=0.

```
S3: Rectangular section modelled by SECTION: RECTANGLE
\(H y=02\)
\(H z=01\)
S4: Section RINGS R \(=01\)
R4
\(I=I\)
-4
10
\(y\)
\(Z=\)
=
4
4

\section*{3.2 \\ Characteristics of the grid}

4 X 2 elements \(P O U_{-} D \_E\). The beam is directed according to the vector \((1,1,1)\).

\subsection*{3.3 Functionalities}
tested
Orders

AFFE_CARA_ELEM
BEAM: SECTION
"GENERAL"
```

"CIRCLE"
CALC_ELEM "EFGE_ELNO_DEPL"

```
"SIGM_ELNO_DEPL"
"SIPO_ELNO_DEPL"
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.2

\section*{Titrate:}

SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 6/18

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Loading case}

\section*{Beam}

\section*{Identification}

\section*{Reference}

Aster \%
difference
\(F x=1\)
```

ux (B)

```
\(3.1110-93.11\)
10-9 0
S4
ux (B)
1.838 10-10 1.838
\(10-100\)
\(x x\)
31.8331 .831
0
\(F y=1\)
\(S 1=S 3\) uy (B)
+1.414 10-7 1.414
10-7 0
\(Z(B)\)
1.225 10-7 1.225
10-7 0
xx (0)
30003000
0
S2
uy (B)
9.017 10-8 9.017
10-8 0
S4
\(x x\) (0)
2546.4792546 .48
0
\(F z=1\)
\[
y(B)
\]
\[
-4.24310-7-4.243
\]
\[
10-70
\]
\[
x x(0)
\]
\[
60006000
\]
\[
0
\]
\[
x z_{(0)}
\]
\[
5050
\]
0
S2
\(u z\) (B)
9.279 10-7 9.279
10-7 0
\(y(B)\)
1.553 10-5
    1.553 10-5
0
\(X(B)\)
1.555 10-5 1.555
10-5 0
S4
\(u z(B)\)
    1.386 10-7 1.386
    10-7 0
\(y\) (B)
    -9 10-8 -9
    10-8 0
\(x x\) (0)

\subsection*{2546.4792546 .479}

0
```

xz(0)

```
31.83131 .831
0
\(M X=1\)
\(S 1=S 3 X(B)\)
3.279 10-7 3.279
10-7 0
\(x y=x z(0)\)
19501950
0
S2
\(X(B)\)
3.791 10-4 3.791
10-4
\(u z\) (B)
2.199 10-5 2.199
10-5 0
S4
\(X\) (B)
9.556 10-8 9.556
10-8 0
\(x y=x z(0)\)
636.62636 .62
0
\(M y=1\)
\(S 1=S 3 u z(B)\)
-4.899 10-7 -4.899
10-7 0

\section*{\(y\) (B)}
4.243 10-7 4.243

10-7 0
```

xx (0)
3000 3000
O
S2
uz (B)
-1.959 10-8 -1.959
10-80
y(B)
1.697 10-8 1.697
10-8 0
S4
uz (B)
-1.04 10-7 -1.04
10-7 0
y(B)
9 10-8
9 10-8
O
xx (0)
1273.2395 1273.2395
O
Mz=1
S1=S3
uy (B)
1.061 10-7 1.061
10-7 0
Z(B)
1.225 10-7
1.225 10-7
O
xx (0)
15001500
O
S2

```
0
Handbook of Validation
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HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}
6.2

\section*{Titrate:}

SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 7/18
\(M\)
\(S\)
\(x\)
4
0
0
F
,
\(X=1\)
xx 22
S4
xx max (0)
1832.46361832 .46
0
\(F y=1\)
S1, S3
xy (0)
20002000
0
\(F z=1\)
\(x z\) (0)
20002000
0
\(M X=1\)
xx max (0)
90009000
0
B has
S1, S3
-9000
-9000
0
\(x x\)
```

2

```
S4
xx max (0)
3601.273601 .27
0
\(x y\) (0)
668.451
0

\author{
Handbook of Validation
}

V3.01 booklet: Linear statics of the linear structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

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\section*{5 Modeling \\ B \\ 5.1 \\ Characteristics of modeling}

2 elements \(P O U_{\_} D_{\_} T\).
The coefficients of shearing are:
S1: Rectangular section
\(A Y=A Z=12\)
. =
ky
S2: Corner section

1
\(A Y=A Z=\)
0358

S4: Section RINGS
10
\(A Y=A Z=\)

9

\title{
5.2 \\ Characteristics of the grid
}

4 X 2 elements \(P O U_{-} D_{-} T\)

\author{
5.3 Functionalities \\ tested \\ Orders
}

AFFE_CARA_ELEM
BEAM: SECTION
"GENERAL"
"RIGHT-ANGLED"
"CIRCLE"
CALC_ELEM "EFGE_ELNO_DEPL"
"SIGM_ELNO_DEPL"

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V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 9/18

\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested
One gives only the values which differ from modeling A (because of the taking into account of transverse shearing).

\section*{Loading Section}

Identification Reference
Aster \%
difference
\(F y=1\)
\(S 1=S 3\)
uy (B)
2.0156 10-7 2.0156

10-7
0
\(x y\) (0)
60. 60.

0
S2
uy (B)
1.666552 10-7 1.666552

10-7
0
S4
uy (B)
1.70684 10-7 1.70684

10-7
0
\(x y\) (0)
0
\(F z=1\)
S1, S3
\(u z\) (B)
8.0156 10-7 8.0156
10-7
0
\(x z\) (0)
60. 60.
0
S2
\(u z\) (B)
1.17559754 10-6 1.17559754
10-6
0
S4
uz (B)
1.70684 10-7 1.70684
10-7
0
\(x z\) (0)
35.36776535 .367765
0
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.2

\section*{Titrate:}

SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

\section*{7 Modeling}

\section*{C}

\section*{7.1}

\section*{Characteristics of modeling}

2 elements POU_D_TG.
Warping is not constrained.
The coefficients of shearing are identical to those of modeling B.

\section*{7.2}

Characteristics of the grid
4 X 2 elements \(P O U \_D \_T G\)

\subsection*{7.3 Functionalities}
tested
Orders

AFFE_CARA_ELEM
BEAM: SECTION
"GENERAL"
"RIGHT-ANGLED"

\section*{"CIRCLE"}

CALC_ELEM "EFGE_ELNO_DEPL"
"SIGM_ELNO_DEPL"

\section*{8 \\ Results of modeling \(C\)}

\author{
8.1 Values
}

\section*{Loading Section}

Identification
Reference
Aster \%
difference
\(F y=1\)
\(S 1=S 3\)
uy (B)
2.0156 10-7 2.0156

10-7
0
\(x y\) (0)
60. 60.

0
S2
uy (B)
1.666552 10-7 1.666552

10-7
0
S4
uy (B)
1.70684 10-7 1.70684

10-7
0
\(x y(0)\)
35.36776535 .367765

0
\(F z=1\)
S1, S3
uz (B)
8.0156 10-7 8.0156

10-7
0
\(x z\) (0)
60.60.

0
1.17559754 10-6 1.17559754

0
S4
\(u z\) (B)
1.70684 10-7 1.70684

10-7
0
\(x z(0)\)
35.36776535 .367765

0

\subsection*{8.2 Notice}

Warping is not constrained. The results are thus identical to those of modeling \(\boldsymbol{B}\).

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts

\section*{Date:}

20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 11/18

\section*{9 Modeling}

D
9.1

Characteristics of modeling
- Eléments POU_D_TG,
- constrained torsion

\section*{- 8}

\subsection*{5.555610}
for S1
\(J G=\)
- 11
4.43982210
for \(S 2\)
- in 0 GRX \(=0\)
9.2

Characteristics of the grid
- 10 elements,
- refinement towards embedding.

10 Results of modeling \(D\)

\subsection*{10.1 Values}
tested
Same results as for modeling C, except those which relate to the effects of warping.
Loading Section
Identification Reference
Aster \%
difference
\(F z=1\)
S2
\(X=D R X\)
2.62034 10-5 2.62021

10-5 5.
10-5
\(u z=D Z\)
1.14578 10-6 1.14573

10-6 5.
10-5
GRX
1.34652 10-5 1.34652

10-5 1.
10-5
\(M X=1\)
S1
\(X=\operatorname{DRX}\)
5.52 10-7 5.52

10-7 5.
10-5
GRX
2.84 10-7 2.84

10-7
0
S2
uz
2.6203 10-5 2.6202

10-5 5.
10-5
\(X\)
6.3892 10-4 6.3889

10-4 5.
10-5
GRX
3.28324 10-4 3.28324

10-4
0

\subsection*{10.2 Remarks}

For \(X\) the solution is (cf [bib1]):
Mr. L
M
\(X\)
\(X\)
```

2L
L
GJ
2
X=
+
1-E
-2nd
=
3
2L
G Jx
E JG(
1+E
)(
)
E JG
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
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```
Code_Aster \({ }^{\circledR}\)
Version
6.2

\section*{Titrate:}

SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 12/18

11 Modeling
E

\subsection*{11.1 Characteristics of modeling}

The beam is with a grid in solid elements quadratic HEXA20.
\(f z\)
```

L2
B=0.1
surf1
Z
y
= 0.2 have
X
L=2

```

The beam is embedded on the level of the section surf1. It is subjected to a unit sharp effort who is modelled by a linear density of load fz applying to 4 constituent meshs SEG3 the higher edge L2.
11.2 Characteristics of the grid

The beam is with a grid with 640 solid elements quadratic HEXA20.
The model comprises 3665 nodes.

\subsection*{11.3 Functionalities}
tested
One tests functionality FORCE_ARETE of AFFE_CHAR_MECA.

12 Results of modeling \(E\)
12.1 Values
tested
One tests the value of the arrow according to \(Z\) of the node medium of the section where the loading is applied
(N62 node).
Identification Reference Aster \%
difference
dz of the N62 node
-8 10-7 -7.9523
10-7
-0.596

\subsection*{12.2 Remarks}

The value of reference corresponds to the value given by the R.D.M.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102-Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

Key: V3.01.102-D Page: 13/18

\section*{13 Modeling}

F

\subsection*{13.1 Characteristics of modeling}

The model is composed of 10 elements right beam of Euler. The section is circular full, of ray \(0.1 m\).
13.2 Characteristics of the grid

It consists of 10 elements POU_D_E. The length of the beam is \(L=6 \mathrm{~m}\)

\subsection*{13.3 Functionalities tested}

\section*{Orders}

\author{
AFFE_CARA_ELEM \\ BEAM: SECTION
}
"GENERAL"
AFFE_CHAR_MECA DDL_IMPO

CALC_ELEM EFGE_ELNO_DEPL

SIGM_ELNO_DEPL
14 Results of modeling \(F\)
14.1 Values
tested
14.1.1 Interior efforts
Results
analytical
Results
Aster Variation
(\%)
\(V y(0) 6.0000 E+036.0000 E+03\)
0.0000
Vy (6) 1.2000E+04 1.2000E+04
0.0000
MFZ (2 3)
\(1.3856 E+041.3856 E+04\)
0.0000
14.1.2 Constraint
Results
analytical
Results
Aster Variation
(\%)
SIXX (2 3)
\(1.7642 E+071.7642 E+07\)
0.0000
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.2
Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:

\title{
J.M. PROIX, J. PELLET, F. LEBOUVIER
}

Key: V3.01.102-D Page: 14/18

\section*{15 Modeling}

G

\subsection*{15.1 Characteristics of modeling}

The model is composed of 10 elements right beam of Timoshenko with warping. The section is circular full, of ray 0.1m.
15.2 Characteristics of the grid

It consists of 10 elements POU_D_TG. The length of the beam is \(L=6 \mathbf{m}\)
15.3 Functionalities tested

Orders

AFFE_CARA_ELEM
BEAM: SECTION
"GENERAL"
AFFE_CHAR_MECA DDL_IMPO

CALC_ELEM EFGE_ELNO_DEPL

SIGM_ELNO_DEPL

16 Results of modeling \(G\)

\subsection*{16.1 Values}
tested
16.1.1 Interior efforts

\section*{Results}
analytical
Results
Aster Variation
(\%)
\(V y(0) 6.0000 E+036.0000 E+03\)
0.0000
Vy (6) 1.2000E+04 1.2000E+04
0.0000
MFZ (2 3)
1.3856E+04 1.3856E+04
0.0000
16.1.2 Constraint
Results
analytical
Results
Aster Variation
(\%)
SIXX (2 3)
1.7642E+07 1.7642E+07
0.0000
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.2
Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER
Key: V3.01.102-D Page: 15/18
17 ModelingH
17.1 Characteristics of modeling
```

y
Modeling COQUE_C_PLAN

```
4
- Rectangular Section
O
B
- Limiting Conditions: Not \(O \boldsymbol{U}=v=Z=0\)
Z
- Unit Loading: Not B Fx, Fy and Mz
17.2 Characteristics of the grid

A number of nodes: 9 A number of meshs and types: 4 SEG3
17.3 Functionalities tested

Orders
AFFE_MODELE AFFE
"COQUE_C_PLAN"
AFFE_CARA_ELEM HULL
THICK
CALC_ELEM OPTION
"EFGE_ELNO_DEPL"
"SIGM_ELNO_DEPL"
AFFE_CHAR_MECA
FORCE_NODALE
FX FY MZ

18 Results of modeling \(H\)
18.1 Values
tested
Loading case
Beam Identification
Reference
Aster \%
difference
\(\boldsymbol{F} \boldsymbol{x}=1\)
S1
5. 10-10 5. 10-10 0.
```

xx (0)

```
5.
5.
0.
\(F y=1\)
S1
uy (B)
2. 10-7 2.007

10-7 0.333

Z (B)
\(1.510-71.5\)
10-7 0.
\(x x\) (0)
300. 289.27
-3.576
\(M z=1\)
S1
uy (B)
\(1.510-71.5\)
10-7 0.

Z (B)
1.5 10-7
\(1.510-7\)
0.
\(x x(0)\)
150. 150.
0.

\subsection*{18.2 Remarks}

The width for modeling COQUE_C_PLAN is imposed on 1 in Code_Aster. In consequence, we multiplied by 0.1 the Young modulus to take account of the real width of
the beam. This width of 1 modifies the inertia of the beam and consequently the value of the constraint \(x x\) which is 10 with times lower than the value of reference. Moreover, for displacements, the results differ from modeling A because of the change of reference mark.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.2

\section*{Titrate:}

SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

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\section*{19 Modeling}

I

\subsection*{19.1 Characteristics of modeling}

The model is composed of 21 elements TUYAU_3M.
19.2 Characteristics of the grid

It consists of 21 meshs SEG3. The length of the beam is \(L=6 \mathbf{m}\)
19.3 Functionalities tested

Orders

AFFE_CARA_ELEM
BEAM: SECTION
"GENERAL"
AFFE_CHAR_MECA DDL_IMPO

\section*{AFFE_CHAR_MECA_F FORCE_POUTRE}

\section*{MECA_STATIQUE}

\author{
STAT_NON_LINE COMP_INCR \\ RELATION \\ ELAS \\ CALC_ELEM \\ OPTION \\ SIEF_ELNO_ELGA \\ CALC_NO \\ OPTION \\ FORC_NODA \\ CALC_NO \\ OPTION \\ REAC NODA \\ CALC_NO \\ OPTION \\ EFGE_NOEU_DEPL
}

\section*{20 Results of modeling I}

\subsection*{20.1 Values}
tested

\subsection*{20.1.1 Displacements}

Results
analytical
Results
Aster Variation
(\%)
Maximum Dy
9.38888E-03
9.44033E-03
0.55
20.1.2 Interior efforts

Results
analytical
Results
Aster Variation
(\%)
\(V y(x=0) 6.0000 E+03\)
\(6.0076 E+03\)
0.127
\(V y(x=L=6) 1.2000 E+041.1995 E+04\)
0.042

MFZ (2 3)
\(1.3856 E+041.388 E+04\)
0.171

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HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102-Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

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\section*{21 Summary of the results}

This test makes it possible simultaneously to check the correct operation of elements POU_D_E, POU_D_T and
POU_D_TG on 3 types of different sections. The perfect coincidence of the results with the solutions analytical (RDM) is normal, and must always be observed, since the solution is contained in functions of form of the elements.

Moreover, modeling E makes it possible to test the loading distributed on edges of elements voluminal. The variation with the analytical solution (RDM) is lower than 0.6\%.

Modelings \(F, G\) and I make it possible to test the loading distributed (linear variation) for elements of beam POU_D_E, POU_D_TG and pipe sections. The variation with the analytical solution
(RDM) is lower than 0.6\%.

For modeling COQUE_C_PLAN the results are satisfactory (displacements and constraints)
for the unit loadings of extension type and inflection (imposed moment). For the loading of inflection (load imposed at an end) the error on displacement is weak 0.5\%. It is more important on the constraint: 3.6\%.
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HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.2

Titrate:
SSLL102 - Fixed beam subjected to unit efforts
Date:
20/12/02
Author (S):
J.M. PROIX, J. PELLET, F. LEBOUVIER

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V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98

\section*{Author (S):}

\section*{J. PELLET}

Key:
V3.01.103-A Page:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

\section*{V3.01 booklet: Linear statics of the linear structures}

Document: V3.01.103
SSLL103 - Elastic buckling of an angle

\section*{Summary:}

A right beam (corner with equal wings) biarticulée is subjected to a normal effort (excentré or not) or to one
bending moment.
One seeks the critical loads of elastic buckling.
- mechanical linear rubber band,
- buckling of a beam,
- eccentricity of the center of torsion,
- interest of the test: calculation of the geometrical matrix of rigidity of elements \(P O U_{-} D_{\_} T G\) and POU_D_T,

\section*{- 2 modelings.}

An uncertainty persists on the number of modes of buckling of the reference solution [§5].
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V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.01.103-A Page:
2/8
1
Problem of reference
1.1 Geometry

Z
M
```

y
P
With
With
P
l
2
X
L= 1200 mm
Characteristics of the section:

```
```

To = 1856 mm2
Z
I=4167339 mm4
y
Section A and A
I
I
2
Z = 1045547 mm4
With
J=39595 mm4
I = 44398819 mm6
G
I
= 84948392 mm5
C
yr2
y
yc}=41.012\textrm{mm
has
Z=0
C
= 120 mm have
E
E=8 mm
CG=41.012 mm
1.2
Material properties
E=2.1 105 MPa
= 0.3
1.3

```

\section*{Boundary conditions and loadings}
C.L. :
\(A 1: D X=D Y=D Z=D R X=0\)
A2: \(D Y=D Z=D R X=0\)
Loading
- case 1: axial load P in G
- case 2: axial load P out of C
- case 3: axial load P in A
- case 4: bending moment M

\subsection*{1.4 Remarks}

For cases 2 and 3, one applies in A2 an effort in G, then one superimposes in A1 and A2 one moment of inflection (according to OZ for cases 2 following OY for case 3) to offset the effort out of C (or in A).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.01.103-A Page:
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2
Reference solution
2.1

\section*{Method of calculation used for the reference solution}

With taking into account of warping, the calculations made by V. Of City De Goyet [bibl] give: that is to say:
```

;
2 =
+
;
2 =
+
y
Z dA Iz
With
ydA Iyr
With
(yy Z) dA
Izr
With
Z (y Z) dA
With
Pcry = 2
E I/L2; Pcrz = 2
E I/L2; Pcrx =
+2
2
Z
y
(GJ I.E.(INTERNAL EXCITATION)/L

```

\section*{) Macaw}

Arc \(=(I+I /+2+2+\)
/
- 2
\(+\)
\(2 /\)
\(y\)
Z) With \(y\)
Z
\(y\)
C
C
C (I
I
\(y\)
yrz

Z
c)
\(z c\) (I
I
\(2 z\)
Zr
\(y\)
c)

Macaw \(=(I+I /+2+2+\)
/
- 2
\(+\)
(
-
Zr
/Iy
Z
2
\(2 c\) )
y
Z) With \(y\)

Z
y
C
C
(I has
\(I\)
\(y\)
\(y r z\)
Z
c)

Z
I
has
with:
( \(\mathrm{y}, \mathrm{Z}\)
has
has): co-ordinates of the point of load application
( \(y, Z\)
C
c): co-ordinates of the center of torsion

Case 1, 2, 3 :
One obtains 3 critical loads by solving the equation of the \(3^{\circ}\) degree out of \(P\) :

\section*{Case 4:}

The moment criticizes Mcr (around the axis y) is worth:
1/2
\(M c r= \pm(G J+\) the \(2 n d I / L 2\)
) Pcry)
By neglecting warping: the analytical solution of reference is given in [bib2] [bib3].
2.2

\section*{Results of reference}

Values of the critical loads corresponding to the first modes of buckling for the various cases of load.

\section*{2.3}

\section*{Uncertainty on the solution}

Analytical solution. The values of reference are obtained using NAG (routine COSAGF, EPS = 10-8).

\subsection*{2.4 References}

\section*{bibliographical}
[1]
V. OF TOWN OF GOYET "Analyzes static nonlinear by the finite element method of formed space structures of beams with nonsymmetrical section " - Thesis of doctorate University of Liege, MSM, academic year (1988-1989).
[2]
P. PENSERINI "elastic Instability of the beams with open mean profile: theoretical aspects and numerical " EDF/DER/HM77/112 Notes.
[3]
J. CHERRY TREE "Propagation of two cases tests of modeling of the calculation of the beams in elastic buckling in Code_Aster" HM77/184
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

\section*{Titrate:}

Elastic SSLL103 Buckling of an angle
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.01.103-A Page:
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3 Modeling
With
3.1

Characteristics of modeling
8 elements \(P O U \_D \_T G\)
With
With
1
2
3.2

Characteristics of the grid
A number of nodes: 9
A number of meshs and types: 8 SEG2

\subsection*{3.3 Functionalities}
tested
Orders
Keys
CALC_MATR_ELEM
"RIGI_GEOM"
[U4.41.01]
MODE_ITER_SIMULT
"PLUS_PETITE"
[U4.52.02]
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98

\section*{Author (S):}

\section*{J. PELLET}

Key:
V3.01.103-A Page:
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```

4

```

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification}

\section*{Reference}

Aster
\% difference
Case 1
mode 1
\(6.92531 E+05\)
\(6.92533 E+05\)
0.000
mode 2
1.50487E+06
\(1.50492 E+06\)
0.003
mode 3
1.00589E+07
\(1.00593 E+07\)
0.003

Case 2
mode 1
1.50487E+06
\(1.50492 E+06\)
0.003
mode 2
\(5.99812 E+06\)
\(5.99831 E+06\)
0.003
mode 3
1.47904E+06
\(1.47904 E+06\)
0.000

Case 3
mode 1
\(5.72260 E+05\)
\(5.72265 E+05\)
0.001
mode 2
\(2.45950 E+06\)
\(2.45957 E+06\)
0.003
mode 3
\(1.85673 E+07\)
\(1.85679 E+07\)
0.003

Case 4
mode 1
\(7.00631 E+07\)
\(7.00642 E+07\)
0.002

\subsection*{4.2 Remarks}

The precision is excellent with 8 elements in the length.

\subsection*{4.3 Parameters}
of execution
Version: 3.02
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
11 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98
Author (S):
J. PELLET

Key:
V3.01.103-A Page:
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5 Modeling

\section*{B}
5.1

Characteristics of modeling
8 elements \(P O U_{-} D_{\_} T\)
With
With
1
2
5.2

Characteristics of the grid
A number of nodes: 9
A number of meshs and types: 8 SEG2

\subsection*{5.3 Functionalities}
tested
Orders
Keys
CALC_MATR_ELEM
"RIGI_GEOM"
[U4.41.01]
MODE_ITER_SIMULT
"PLUS_PETITE"
[U4.52.02]
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.01.103-A Page:
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6
Results of modeling B
6.1 Values
tested
Identification

\section*{Reference}

Aster
\% difference

\section*{Case 1}
mode 1
\(6.796 E+05\)
\(6.8 E+05\)
0.06
mode 2
\(1.505 E+06\)
\(1.50492 E+06\)
0.005
mode 3
\(1.0055 E+07\)
\(9.968 E+07\)
0.816

Case 2
mode 1
\(1.505 E+06\)
\(1.50492 E+06\)
0.005
mode 2
\(5.998 E+06\)
\(5.99831 E+06\)
+0.005
Case 3
mode 1
\(5.638 E+05\)
\(5.649 E+05\)
0.2
mode 2
\(2.453 E+06\)
\(2.443 E+06\)
0.4
mode 3
\(1.8525 E+07\)
\(1.7883 E+07\)
3.5

Case 4
mode 1
\(6.9376 E+07\)
\(6.982 E+07\)
0.064

\subsection*{6.2 Remarks}

The precision is rather good with 8 elements in the length. The solution differs a little that obtained with warping (modeling A).

\subsection*{6.3 Parameters}
of execution
Version: 3.6
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
50 megawords
Time CPU To use:
15 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}
4.0

Titrate:
Elastic SSLL103 Buckling of an angle
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.01.103-A Page:
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7

\section*{Summary of the results}

The analytical solution gives us 3 modes of buckling of which the critical loads are roots of an equation of the \(3^{\circ}\) degree.
\(Y\)-a it of other critical loads intercallées between the 3 found values?
Aster finds the good critical loads, but in the middle of much of others... for example for case 3, the 3 sought critical loads corresponds to the nume_mode: 1, 10 and 19!
This is true for two modelings.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/96/039 - Ind A
Code_Aster \({ }^{\circledR}\)
Version

\section*{Titrate:}

SSLL104 - Initial deformations in a right beam

Date:
14/09/00
Author (S):
J. Mr. PROIX, J. PELLET, L. VIVAN Key

V3.01.104-B Page:
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Organization (S): EDF/MTI/MMN, CS IF

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
Document: V3.01.104

SSLL104 - Initial deformations in a beam right-hand side

\section*{Summary:}

This test validates the taking into account of the initial deformations in the elastic design of a right beam. characteristics of calculation are:

\author{
Handbook of Validation
}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL104 - Initial deformations in a right beam
Date:
14/09/00
Author (S):
J. Mr. PROIX, J. PELLET, L. VIVAN Key

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1
Problem of reference

\subsection*{1.1 Geometry}

Z
\(X\)
Y
B
With
\(X\)

A beam AB length \(L=100 \mathrm{~mm}\) is located on the trissectrice of trihedron \((X, Y, Z)\) : coordinated bridge \(B\) are:

The local reference mark \((A, X, y, Z)\) results from the total reference mark \((A, X, Y, Z)\) by the nautical angles
\(={ }^{\circ}\)
45

The material is elastic linear.
Young modulus \(E=1.0 \mathrm{MPa}\) (without influence on the result).
Poisson's ratio: = 0
1.3

Boundary conditions and loadings
Embedding in a: \(D X=D Y=D Z=D R X=D R Y=D R Z=0\).
Loading: initial deformation in the local reference mark (A, \(X, y, Z\) )
elongation according to \(X: E P X=0.001\)
curve according to \(A y: K Y=0.002\)
curve according to \(A z: K Z=0.003\)
1.4

Characteristics of the section of beam
All the characteristics (surface, inertias,...) are taken equal to 1.

\section*{They are without influence on the result.}

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL104-Initial deformations in a right beam
Date:
14/09/00
Author (S):
J. Mr. PROIX, J. PELLET, L. VIVAN Key
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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The solution is analytical. It is calculated in the local reference mark.
\(\boldsymbol{U}\)
\(=(U, v, W, X, y, Z)\)
the displacement of the beam
That is to say: \(E=\)
( \(X, y, Z, x y, x z\) ) generalized deformation of the beam
\(x 2\)
\(x 2\)
\(U\)
\(=X ; v=\)
; \(W=\)
That is to say the solution:
2
\(=U, X\)
\(X\)
\(=\)
\(=, X\)
\(y\)
\(y\)
\(=\)
then:
\(=\boldsymbol{X}\)
\(Z\)
Z
=
\(=v, X\)
\(x y\)
\(-Z=0\)
\(=W, X\)
\(x z\)
\(+y=0\)
If one chooses \(=E P X(=0.001) ;=K Y(=0.002) ;=K Z(=0.003)\) then \(E-\) Einit \(=0\) efforts are null. Balance is checked. In addition, the solution checks the C.L. (embedding in A). Thus it is the solution of the problem arising.

\section*{2.2}

Results of reference
The results expressed in the local reference mark are:
In b: \(D x=0.1 ; D y=15 . ; D z=10 . ; D R x=0 . ; D R y=0.2 ; D R z=0.3\)

Out of \(C\) : \(D x=0.05 ; D y=3.75 . ; D z=2.5 ; D R x=0 . ; D R y=0.1 ; D R z=0.15\)
In the total reference mark, one finds at the points \(B\) and \(C\) :
```

3
3
3
3
DX(B)=
+5
(-36+2 2)
DX(C)=
+5
(-36+2 2)
30
6
60
24
3
3
3
3
DY(B)=
+5
(36+2 2)
DY(C)=
+5
(36+2 2)
30
6
6 0
24
3
3
3
3
DZ(B)=
+5
(-42)
DZ (C) =
+5
(-42)
30

```
```

6
60
24
1
1
DRX (B)=
(-6-2 2)
DRX (C) =
(-6-2 2)
20
4 0
I
1
DRY (B)=
(-6+22)
DRY (C) =
(-6+22)
20
4 0
1
1
DRZ (B) =
(2 6)
DRZ (C) =
(2 6)
2 0
4 0

```

\section*{2.3}
```

Uncertainty on the solution

```

The solution is exact for the theory of the beams of Euler (or Timoshenko because it there not of shearing). Torsion not intervening, the solution is also valid for elements POU_D_TG.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:

SSLL104-Initial deformations in a right beam
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14/09/00
Author (S):
J. Mr. PROIX, J. PELLET, L. VIVAN Key

V3.01.104-B Page:
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3 Modeling
With

\section*{3.1 \\ Characteristics of modeling}

\section*{Z \\ \(X\) \\ Y \\ B \\ With \\ \(X\)}
\(A B\) is cut out in 10 of the same elements length (10.). (Only one element would be sufficient).
3 identical calculations are successively made on this grid with 3 modelings different:
with 10 elements POU_D_E
with 10 elements POU_D_T
with 10 elements POU_D_TG

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 11

\section*{A number of meshs and types: 10 SEG2}

\author{
3.3 Functionalities \\ tested \\ Orders \\ Key word factor \\ Key word argument \\ AFFE_CHAR_MECA \\ EPSI_INIT \\ EPX, KY, KZ \\ AFFE_MODELE MODELING \\ \(P O U_{-} D \_E\) \\ AFFE_MODELE MODELING \\ POU_D_T \\ AFFE_MODELE MODELING \\ POU_D_TG \\ Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ HT-66/02/001/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
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Titrate:
SSLL104- Initial deformations in a right beam
Date:
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Author (S):
J. Mr. PROIX, J. PELLET, L. VIVAN Key
:
V3.01.104-B Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
```

Modeling Identification Reference
Aster %
difference
POU_D_E
B DX
6.4664E+00
6.4664E+00
< 10-9
DY
1.4747E+01
1.4747E+01
< 10-9
DZ
8.1072E+00
8.1072E+00
< 10-9
DRX
2.6390E01
2.6390E01
< 10-9
DRY
1.8947E02
1.8947E02
< 10-9
DRZ
2.4495E01
2.4495E01
< 10-9
C DX
1.6022E+00
1.6022E+00
< 10-9
DY
3.7011E+00
3.7011E+00
< 10-9
DZ
2.0124E+00
2.0124E+00
< 10-9
DRX

```

\section*{B DX}
\(6.4664 E+00\)
\(6.4664 E+00\)
< 10-9
DY
\(1.4747 E+01\)
1.4747E+01
< 10-9
DZ
8.1072E+00
8.1072E+00
< 10-9
DRX
2.6390E01
2.6390E01
< 10-9
DRY
1.8947E02
1.8947E02
< 10-9
DRZ
2.4495E01
2.4495E01
< 10-9
C DX
\(1.6022 E+00\)
\(1.6022 E+00\)

DY 3.7011E+00
\(3.7011 E+00\)
< 10-9
DZ 2.0124E+00
\(2.0124 E+00\)
< 10-9
DRX
1.3195E01
1.3195E01
< 10-9
DRY 9.4734E03
9.4734E03
< 10-9
DRZ 1.2247E01
1.2247E01
< 10-9
\(P O U \_D \_T G\)
B DX
\(6.4664 E+00\)
\(6.4664 E+00\)
< 10-9
DY
1.4747E+01
1.4747E+01
< 10-9
DZ
8.1072E+00
8.1072E+00
< 10-9
DRX
2.6390E01
2.6390E01
< 10-9
DRY
1.8947E02
1.8947E02
< 10-9
DRZ
2.4495E01
C DX
\(1.6022 E+00\)
\(1.6022 E+00\)< 10-9DY
\(3.7011 E+00\)
\(3.7011 E+00\)
< 10-9
DZ
\(2.0124 E+00\)
\(2.0124 E+00\)
< 10-9
DRX
1.3195E01
1.3195E01
< 10-9
DRY
9.4734E03
9.4734E03
< 10-9
DRZ
1.2247E01
1.2247E01< 10-9
4.2 Parameters
of execution
Version: 5.3
Machine: SGI Origin2000-R12000
System:
IRIX 64
Obstruction memory:
64 Mo
Time CPU To use: 3 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL104-Initial deformations in a right beam
Date:
14/09/00
Author (S):
J. Mr. PROIX, J. PELLET, L. VIVAN Key

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\section*{5}

Summary of the results
As one could expect it, the results are very precise. They validate the good taking into account initial strains and stresses in the elements of beam.

The test does not test the curved beams ( \(\mathrm{POU} \_C_{-} T\) ) because one does not have reference solution.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLL105-Elastic buckling of a structure in L
Date:
28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key
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V3.01.105-B Page:
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Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
Document: V3.01.105

SSLL105-Elastic buckling of a structure
in \(L\)

\section*{Summary:}

An L-shaped structure made up of two slim beams of mean rectangular section is subjected with a force at an end, and is embedded at the other end. One seeks the critical loads elastic buckling associated the positive and negative values of the force. The field of the test is:
linear elastic mechanics,
buckling of beams,
-
The first 3 modelings relate to (POU_D_E, POU_D_T, POU_D_TG).
The fourth modeling tests the criterion of buckling in the nonlinear operator of statics.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLL105-Elastic buckling of a structure in L

\section*{Date:}

28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key
:
V3.01.105-B Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Y}
\(L\)
With
Z
B

\section*{Section:}

X
B
\(L\)
\(T\)
C
\(P\)
Geometrical characteristics:
\(L=240 \mathrm{~mm}\)
\(B=30 \mathrm{~mm}\)
\(T=0.6 \mathrm{~mm}\)
1.2Material properties
\(E=71240 \mathrm{MPa}\)
\(=0.3\)
1.3
Boundary conditions and loadings
C.L. : embedding in \(A\)
Loading: \(F=X\)

\(\boldsymbol{P}\)
case 1: \(P=1 \mathrm{NR}\)
case 2: \(P=+1\) NR
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
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SSLL105-Elastic buckling of a structure in L
Date:
28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key

:
V3.01.105-B Page:
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\]

\section*{Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
Average of codes (8 results in [bib1]).

\section*{2.2}

Results of reference
Values of the critical load for the two loading cases.

\section*{2.3}

Uncertainty on the solution
\(2 \%\) (maximum variation compared to the average of results used).

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}
G. DEVESA: Treatment of great displacements in the element of angle with 7 ddl established in Code_Aster validation by a traditional case test (HM-77/94/079).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLL105-Elastic buckling of a structure in L
Date:
28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key

\section*{V3.01.105-B Page:}

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

20 elements POU_D_E

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 21 \\ A number of meshs and types: 20 SEG2
}

\subsection*{3.3 Functionalities}
tested

\author{
Orders
}

\section*{CALC_MATR_ELEM "RIGI_GEOM"}

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Loading case
Reference
Aster \%
difference
11.088
1.0900.19
\(2-0.680\)-0.6813
0.19
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2
Titrate:
SSLL105-Elastic buckling of a structure in L
Date:
28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key

:
V3.01.105-B Page:
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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

20 elements POU_D_T
5.2

\title{
Characteristics of the grid
}

A number of nodes: 21
A number of meshs and types: 20 SEG2

\subsection*{5.3 Functionalities}
tested
Orders

\section*{CALC_MATR_ELEM "RIGI_GEOM"}

6
Results of modeling B

\subsection*{6.1 Values}
tested

\section*{Loading case}

Reference
Aster \%
difference
11.088
1.090
0.19

2 -0.680
-0.6813
0.19

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLL105-Elastic buckling of a structure in L
Date:
28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key

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\section*{7 Modeling}

C

\section*{7.1 \\ Characteristics of modeling}

20 elements POU_D_TG
7.2

Characteristics of the grid
A number of nodes: 21
A number of meshs and types: 20 SEG2

\subsection*{7.3 Functionalities \\ tested}

\section*{Orders}

CALC_MATR_ELEM "RIGI_GEOM"

\section*{8 \\ Results of modeling C}

\author{
8.1 Values \\ tested
}

\section*{Loading case}

Reference
Aster \%
difference
11.088
1.0918
0.35
\(2-0.680\)
-0.688
1.2

\subsection*{8.2 Remarks}

The results of this modeling differ slightly from the others and are identical to those obtained by calculation Aster of the reference [bibl].

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
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\section*{Titrate:}

SSLL105-Elastic buckling of a structure in L
Date:
28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key
:
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\section*{9 Modeling}

\section*{D}

\section*{9.1 \\ Characteristics of modeling}

20 elements POU_D_E
9.2

Characteristics of the grid
A number of nodes: 21
A number of meshs and types: 20 SEG2

\subsection*{9.3 Functionalities}
tested

\section*{Orders}

STAT_NON_LINE CRIT_FLAMB

10 Results of modeling \(D\)
10.1 Values
tested

\section*{Loading case}

Reference
Aster \%
difference
11.088
1.0866
0.12

Titrate:
SSLL105-Elastic buckling of a structure in L

\section*{Date:}

28/10/03
Author (S):
J.M. PROIX, G. DEVESA Key
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11 Summary of the results
The results of 4 modelings are very close to the reference solution which is an average results of 8 codes. One notes a small effect due to warping since the results of
modeling \(\mathbf{C}\left(\mathbf{P O U} \_D_{\_} T G\right)\) are slightly different from the others, while remaining with less than \(\mathbf{2 \%}\) of reference.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLL106-Right pipe

Date:
05/01/04

\title{
Author (S):
}
J. Mr. PROIX

Key: V3.01.106-B Page: 1/22
Organization (S): EDF-R \& D /AMA

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
Document: V3.01.106

SSLL106 - Right pipe

\section*{Summary:}

This test allows a simple checking of the right pipe sections in static mechanics of the structures linear.

The model is linear.
For each modeling, 6 types of loading are applied at the end: a traction, 2 efforts edges, 2 moments bending and a torsion. One applies moreover one internal pressure, a linear force distributed and a thermal expansion.

The values tested are displacements, the efforts with the nodes, and the constraints and deformations at the points
of Gauss. The reference solution is analytical (RDM).

Two modelings (A and B) make it possible to test the element PIPE with 3 modes of Fourier (modeling
TUYAU_3M)
: modeling A uses MECA_STATIQUE, modeling B uses STAT_NON_LINE
(elastic behavior).
Two modelings (C and D) make it possible to test the element PIPE with 6 modes of Fourier (modeling TUYAU_6M).

Two modelings ( \(E\) and \(F\) ) make it possible to test the element PIPE with 3 modes of Fourier and 4 nodes (modeling TUYAU_3M).
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Titrate:
SSLL106-Right pipe

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Author (S):
J. Mr. PROIX

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

Right beam length L, directing vector (4, 3, 0).

L
Z
0.04
\(\boldsymbol{O}\)
X
\(O\)
4
\(L=5\)
Z

Section of the pipe
Tubular section of external ray has \(=0.04 \mathrm{~m}\), of ray interns \(B=0.032 \mathrm{~m}\), thickness \(E=0.008 \mathrm{~m}\)
1.2

Material properties
\(E=2.1011 P a=0.3\)
density \(=7800 \mathrm{~kg} / \mathrm{m3}\), thermal dilation coefficient
5
10-
=

\section*{1.3}

Boundary conditions and loadings
- Encastrement out of \(O\)
- 6 elementary Loadings at the end \(B\)
in reference mark \((X, y, Z)\) related to the beam:
\[
\begin{aligned}
& F x=5.102 \mathrm{NR} M X=5.102 \mathrm{Nm} \\
& F y=5.102 \mathrm{NR} M y=5.102 \mathrm{Nm} \\
& F z=5.102 \mathrm{NR} \mathrm{Mz}=5.102 \mathrm{Nm}
\end{aligned}
\]
maybe, in the total reference mark \((X, Y, Z)\) :
1 loading of traction: \(F X=4.102\) NR and \(F Y=3.102 N R\)

2 sharp efforts: in the plan (oxy) \(F X=3.102\) NR and \(F Y=4.102\) NR and in the plan (oyz) \(F Z=5.102 \mathrm{NR}\)

1 torque: \(M X=4.102 \mathrm{Nm}\) and \(M Y=3.102 \mathrm{Nm}\)
2 sharp efforts: in the plan (oxy) \(M X=3.102 \mathrm{Nm}\) and \(M Y=4.102 \mathrm{Nm}\) and in the plan (oyz) \(M Z=5.102 \mathrm{Nm}\)
- Internal Pression: P=107 Pa
- Pesanteur, with \(g=10 \mathrm{~m} / \mathrm{s}^{2}\), in the direction \(-Z\)
- Linear Chargement, \(F z=-141.146 \mathrm{~N} / \mathrm{m}\) (what corresponds to the load due to gravity:

Fz=mg)
- Thermal Dilatation: \(\operatorname{Temp}=100^{\circ} \mathrm{C}\)

\section*{1.4}

Notation of the characteristics of cross sections
The geometrical characteristics of the cross sections are noted:

\section*{\(S:\)}
surface of the section
I, I
\(y\)
Z:
geometrical moments of inertia compared to the principal axes of inertia of section
\(J x\) :
constant of torsion
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SSLL106-Right pipe

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J. Mr. PROIX

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}
- Analytical Solution [bib1]: displacements out of B in the reference mark (Oxyz) related to the beam.

L
Traction simple \(\boldsymbol{U}=\boldsymbol{F}\)
\(X\)
X E S
(F L3
2
\(y\)
)
L Fy
Pure bending
\(u y=\)
\(=\)
3rd I
Z
2nd I
Z
\(Z\)
F 3
L2 F
Pure bending
Z L
Z
\(U=\)
= -
Z
\(y\)
3rd I
2nd I
\(y\)
\(y\)
MR. L
Torsion
X
\[
X=G J x
\]
Mr. L2
\[
M R . L
\]
\[
y
\]

Pure inflection
\[
u z=-
\]
\[
=
\]
2nd I
\[
y
\]
\[
E I
\]
\[
y
\]
\[
y
\]
\[
M
\]
\[
2
\]
\[
M
\]
Pure inflection
\[
Z L
\]
\[
Z L
\]
\[
U y=
\]
\[
=+
\]
2nd I
\[
Z
\]
\[
Z
\]
E I Z
\[
P
\]
\[
\text { has } 2
\]
b2
+ B has

Pressure
\[
U=
\]
\[
\boldsymbol{R} \text { ( }
\]
\[
1-)+(1+)
\]
\[
=
\]
\(\boldsymbol{R}\)
calculated in \(R\)
E b2-a2
R 2
2
in fact \(\boldsymbol{U} v\) arie enters
, 2106 in \(R=B\)

Here, the values are obtained with:
```

S=
-3 m2 Iy = Iz=
-6 m4 J =
1809557 10
118707 10
237414
106 m4
X
L=5m

```

For the generalized deformations of beam, one obtains, by the law of behavior:
\(\boldsymbol{F x}\) simple
```

Traction
Ex=ES
Fy
Fy(L - X)
simple
Inflection
xy=
Z=
GS
I.E.(internal excitation) Z
Z

```
```

F
Fz(L-X)
simple
Inflection
=
xz=GS
y
I.E.(internal excitation) y
MX
Torsion
X=GJx
My
pure
Inflection
y = EIy
M Z
pure
Inflection
Z = +EIz
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```
Code_Aster \({ }^{\circledR}\)
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Loading of gravity and linear loading:
2

\section*{\(p L\)}

If \(p\) indicates the distributed load, the moment in the beginning is worth: \(M(O)=\) and of following displacement
2
pLA
\(Z\) at the end \(B\) is worth: \(U(B)=\)

Z
8EI

The thermal loading of dilation led to an axial displacement (in the local direction X ):
\(U(B)=L\)
\(X\)
(T)

The deformations of free dilation of the surface of the pipe are simply, in local reference mark:
```

= =

```
\(x x\)
(T
\(y y\)
)
Finally to validate the calculation of the matrix of mass, a modal analysis of the first 12 modes clean (with embedding out of \(O\) ) must give, for the modes of inflection:

\section*{2}
I.E.(internal excitation)

F
I
=
I
L
\(S\)

Lambdai mode
Frequency
1 1,87510407
2,9030234

\title{
12 36,1283155
}

1077,69337

\section*{2.2}

Results of reference
- Déplacement at the point \(B\), efforts, constraints and deformations in the vicinity of the point \(O\).
- Déformation generalized.
- Eigen frequencies

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
Handbook of validation, test SSLL102 fixed Beam subjected to unit efforts
[V3.01.102]

\section*{Handbook of Validation}

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Author (S):
J. Mr. PROIX

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling \\ 10 elements PIPE.}

\section*{3.2}

Characteristics of the grid
10 meshs SEG3. The beam is directed according to the vector \((4,3,0)\).

\subsection*{3.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE MODELING \\ PIPE \\ AFFE_CARA_ELEM BEAM \\ SECTION RINGS \\ MACRO_ELAS_MULT \\ OPTION \\ SIEF_ELGA_DEPL \\ \section*{OPTION} \\ EPSI_ELGA_DEPL
}

\section*{OPTION}

EFGE_ELNO_DEPL

Notice on the contents of the fields:
Fields at the points of Gauss for the element PIPE
, EPSI_ELGA_DEPL and
SIEF_ELGA_DEPL, which provide the strains and the stresses to the points of integration in the local reference mark of the element, are organized in the following way:

The values are stored:
- for each point of Gauss in the length, \((n=1,3)\)
- for each point of integration in the thickness, ( \(n=1,2 N C O U+1=7\) )
- for each point of integration on the circumference, \((n=1,2 N S E C T+1=33)\)
- 6 components of strain or stresses:

EPXX EPYY EPZZ EPXY EPXZ EPYZ or SIXX SIYY SIZZ SIXY
SIXZ SIYZ
where \(X\) indicates the direction given by the two nodes tops of the element, Y represents the angle describing the circumference and \(Z\) represent the ray. EPZZ and EPYZ corresponding to,
in
\(r r\)
R
case of the deformations and SIZZ and SIYZ corresponding to,
in
rr R
the case of the constraints is taken equal to zero.
(for MECA_STATIQUE or MACRO_ELAS_MULT, the number of layers is fixed, and equal to 3, and
the number of sectors is equal to 16).
EFGE_ELNO_DEPL represents the efforts generalized with the 3 nodes in the traditional way: NR, VY, VZ, MT, MFY, MFZ.
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V3.01 booklet: Linear statics of the linear structures
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}

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}

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{Loading case}

Size
Reference
Aster \%
difference
\(F X=4.102\)
DX
5.53E06
5.52E06
-0.04
\(F Y=3.102 \mathrm{DY}\)
4.14E06
4.14E06
-0.04
\(F X=3.102\) DRZ
2.63E02
2.63E02
-0.04
\(F Y=4.102 \mathrm{DX}\)
5.27E02
5.26E02
-0.056
DY
7.02E02
7.02E02
-0.056
\begin{tabular}{l}
\(F Z=5.102 \quad D R X\) \\
\(1.58 E 02\) \\
\(1.58 E 02\) \\
-0.04 \\
\(D R Y\) \\
\(2.11 E 02\) \\
\(2.11 E 02\) \\
-0.039 \\
\(D Z\) \\
\(8.78 E 02\) \\
\(8.77 E 02\) \\
-0.056 \\
\(M X=4.102 \quad D R X\) \\
\(1.10 E 02\) \\
\(1.10 E 02\) \\
0 \\
\(M Y=3.102 \quad D R Y\) \\
\(8.21 E 03\) \\
\(8.21 E 03\) \\
0 \\
\(M X=3.102 \quad D R X\) \\
\(6.32 E 03\) \\
\(6.32 E 03\) \\
-0.04 \\
\(M Y=4.102 \quad D R Y\) \\
\(8.42 E 03\) \\
\(8.42 E 03\) \\
-0.04 \\
\(D Z\) \\
\(2.63 E 02\) \\
\(2.63 E 02\) \\
-0.04 \\
\(M Z=5.102 \quad D R Z\) \\
\(1.05 E 02\) \\
\(1.05 E 02\) \\
-0.039 \\
\(D X\) \\
\(1.58 E 02\) \\
\(1.58 E 02\) \\
-0.04 \\
\(D Y\) \\
\(2.11 E 02\) \\
\(2.11 E 02\) \\
\\
\hline
\end{tabular}

\author{
2.11E02
}
9: charge distributedDZ
4.646 E-02 4.642
E-02
0.09
Loading case
FieldNet Point Component Reference
Aster \%
difference
1
EFGE_ELNO_DEPL
M18 1 NR
\(5.00 E+02\)
\(5.01 E+020.136\)
1
EPSI_ELGA_DEPL
M18 1 EPXX 1.38E06
1.38E06-0.031
I
SIEF_ELGA_DEPL
M18 1 SIXX 2.76E+05
\(2.73 E+05\)-1.159

4
EFGE_ELNO_DEPL
M18 1 MT
\(5.00 E+02\)\(5.00 E+020\)
4
EPSI_ELGA_DEPL
M18 1 EPXY 8.77E05 8.76E05
-0.102
4
EPSI_ELGA_DEPL
M18 693 EPXY 1.09E04 1.10E04 0.049
4
SIEF_ELGA_DEPL
M18 1 SIXY 6.75E+06 6.74E+06
-0.159
4
SIEF_ELGA_DEPL
M18 693 SIXY
\(8.42 E+068.42 E+060.049\)
5
EFGE_ELNO_DEPL
M18 1 MFY 5.00E+02
\(5.01 E+020.123\)
5
EPSI_ELGA_DEPL
M18 479 EPXX
6.74E05
6.74E05 0.046
5
SIEF_ELGA_DEPL
M18 479 SIXX
\(1.35 E+07\)
\(1.33 E+071.288\)
6
EFGE_ELNO_DEPL
M18 1 MFZ 5.00E+02
\(5.01 E+020.123\)
6
EPSI_ELGA_DEPL
M18 471 EPXX
6.74E05
6.74E05 0.046
6
SIEF_ELGA_DEPL
M18 471 SIXX
\(1.35 E+07\)
\(1.33 E+071.288\)
```

7
EPSI_ELGA_DEPL
M18 1 EPYY 2.28E04
2.24E04
-1.716
7
EPSI_ELGA_DEPL
M18 693 EPYY
1.78E04
1.79E04 0.741
7
SIEF_ELGA_DEPL
M18 1 SIYY 4.56E+07
4.53E+07
-0.641
7
SIEF_ELGA_DEPL
M18 693 SIYY
3.56E+07
3.54E+07 0.371
8
EFGE_ELNO_DEPL
M1 1 MFY
1764.3
1 7 2 8
2
9
EFGE_ELNO_DEPL
M1 1 MFY
1764.3
1728
2
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```
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:

\author{
Date: \\ 05/01/04 \\ Author (S): \\ J. Mr. PROIX \\ Key: V3.01.106-B Page: 7/22
}

Generalized deformations DEGE_ELNO_DEPL:

\author{
Loading case \\ Loadings \\ Size \\ Reference \\ Aster \% \\ difference \\ 1 FX = 4.102 \\ EPXX \\ 1.38155E-06 \\ 1.38155E-06 \\ -0.04 \\ \(F Y=3.102\) \\ \(2 F X=3.102\) GAXY 3.5920E-06 4.7415E06 32 \\ \(F Y=4.102 \mathrm{KZ}\) \\ 1.0530E02 1.04E02 1.2 \\ 3 FZ = 5.102 GAXZ 3.5920E-06 4.7415E06 \\ 32 \\ KY \\ 1.0530E02 \\ 1.04E02 \\ -1.2 \\ 4 MX = 4.102 GAT 2.73783E-03 2.73783E-03 0 \\ \(M Y=3.102\)
}

\section*{\(5 \mathrm{MX}=3.102 \mathrm{KY}\)}

\subsection*{2.1060E-03 2.1052E-03 0.04}
\(M Y=4.102\)
\(6 M Z=5.102 \mathrm{KZ}\)
2.1060E-03 2.1052E-03
-0.04

\section*{Fréquenc}

Reference

\author{
Aster \%
}
difference
E clean
12.90229
2.90378
0.05
22.90229
2.

90378
0.05
318.18967
18.2047
0.08
418.18967
18.2047
0.08
550.99367
51.0060 .02
650.99367
51.0060 .02
799.81783
100.0478
0.2
899.81783
100.0478
0.2
9157.0190
157.01850 .001
10164.9922

\subsection*{4.2 Remarks}

The values of shearings corresponding to the shearing action are not precise for this modeling. This is due to the functions of interpolation of order 2 of this element, for displacements of beam and rotations of beams. As transverse shearings of beam are obtained by:
duy
= -
, and that for the pure bending, rotations vary like polynomials of order 2,
\(x y\)
Z
\(d x\)
but displacements, like polynomials of order 3, which is badly approached by the functions of interpolation. The derivative of displacements is thus not precise.

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Titrate:
SSLL106-Right pipe

Date:
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Author (S):

\section*{J. Mr. PROIX}

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\section*{5 Modeling}

B
5.1

\section*{Characteristics of modeling}

10 elements PIPE, calculation with STAT_NON_LINE.

\section*{5.2 \\ Characteristics of the grid}

10 meshs SEG3. The beam is directed according to the vector \((4,3,0)\).

\subsection*{5.3 Functionalities}
tested

\author{
Orders
}

\author{
AFFE_MODELE \\ MODELING \\ PIPE \\ AFFE_CARA_ELEM BEAM \\ SECTION \\ RING \\ STAT_NON_LINE COMP_INCR \\ RELATION ELAS
}

COMP_INCR
TUYAU_NCOU

\section*{Notice on the contents of the fields:}

Stress fields at the points of Gauss for the element PIPE, SIEF_ELGA, in the local reference mark of the element, are organized in the following way:

The values are stored:
- for each point of Gauss in the length, \((n=1,3)\)
- for each point of integration in the thickness, ( \(n=1,2 \mathrm{NCOU}+1\) )
- for each point of integration on the circumference, ( \(n=1,2 N S E C T+1\) )
- 6 components of strain or stresses:

\section*{EPXX EPYY EPZZ EPXY EPXZ EPYZ or SIXX SIYY SIZZ SIXY}

SIXZ SIYZ
where \(X\) indicates the direction given by the two nodes tops of the element, \(Y\) represents the angle describing the circumference and \(Z\) represent the ray. \(\mathbf{E P Z Z}\) and \(\boldsymbol{E P Y Z}\) corresponding to, in
\(r r\)
\(R\)
case of the deformations and SIZZ and SIYZ corresponding to,
in
rr R
the case of the constraints is taken equal to zero.
(in STAT_NON_LINE, the number of layers is variable, as well as the number of sectors.
One uses here 3 layers and 16 sectors by analogy with modeling A).
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

\author{
Titrate: \\ SSLL106-Right pipe
}

Date:
05/01/04
Author (S):
J. Mr. PROIX

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\section*{6 \\ Results of modeling B}

\author{
6.1 Values
}
tested
Loading case
Size
Reference
Aster \%
difference
1 DX
5.53E06
5.52E06
-0.04
1 DY
4.14E06
4.14E06
-0.04
2 DRZ
2.63E02
2.63E02
-0.04
2 DX
5.27E02
5.26E02
-0.056
2 DY
7.02E02
7.02E02
-0.056
3 DRX
1.58E02
1.05E02
1.05E02
-0.039
\[
6 D X
\]
\[
1.58 E 02
\]
\[
1.58 E 02
\]
\[
-0.04
\]
\[
6 D Y
\]

\section*{Loading case}

\section*{Field}

Net

\section*{Not Component Reference}

\author{
Aster \%
}
difference

\author{
1 \\ SIEF_ELGA \\ M18 Z SIXX 2.76E+05
}
2.73E+05
-1.159
1
SIEF_ELNO_ELGA
M18 1 NR 5.00E+02
\(5.01 E+020.136\)
4
SIEF_ELGA
M18 1 SIXY 6.75E+06
\(6.74 E+06\)
-0.159
4
SIEF_ELGA
M18 693
SIXY 8.42E+06
\(8.42 E+060.049\)
4
SIEF_ELNO_ELGA
M18 1 MT 5.00E+02
\(5.00 E+020\)
5
SIEF_ELGA
M18 479
SIXX 1.35E+07
1.33E+07 1.288

5
SIEF_ELNO_ELGA
M18 1 MFY 5.00E +02
\(5.01 E+020.123\)
```

6
SIEF_ELGA
M18471
SIXX 1.35E+07
1.33E+07 1.288
6
SIEF_ELNO_ELGA
M18 1 MFZ 5.00E+02
5.01E+02 0.123
7
SIEF_ELGA
M18 1 SIYY 4.56E+07
4.53E+07
-0.641
7
SIEF_ELGA
M18 }69
SIYY 3.56E+07
3.54E+07 0.371

```

Generalized deformations DEGE_ELNO_DEPL:

\section*{Loading case}

Loadings
Size
Reference
Aster \%
difference
\(1 F X=4.102\)
EPXX
1.38155E-06 1.38155E-06
-0.04
\(F Y=3.102\)
\(2 F X=3.102\) GAXY 3.5920E-06
4.7415E06

32
\(F Y=4.102 K Z ~ 1.0530 E 02\)
```

5 MX = 3.102 KY
2.1060E-03
2.1052E-03
-0.04
MY=4.102
6 MZ = 5.102 KZ 2.1060E-03
2.1052E-03
-0.04
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures

```
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLL106 - Right pipe

Date:

\subsection*{6.2 Remarks}

The values of shearings corresponding to the shearing action are not precise for this modeling. This is due to the functions of interpolation of order 2 of this element, for displacements of beam and rotations of beams. As transverse shearings of beam are obtained by:
duy
= -
, and that for the pure bending, rotations vary like polynomials of order 2,
\(x y\)
\(Z\)
\(d x\)
but displacements, like polynomials of order 3, which is badly approached by the functions of interpolation. The derivative of displacements is thus not precise.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
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SSLL106-Right pipe

Date:
05/01/04
Author (S):
J. Mr. PROIX

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\section*{7 Modeling}

C
7.1

Characteristics of modeling
10 elements TUYAU_6M.
7.2

Characteristics of the grid
10 meshs SEG3. The beam is directed according to the vector \((4,3,0)\).
7.3 Functionalities
tested
Orders

AFFE_MODELE MODELING
TUYAU_6M
AFFE_CARA_ELEM BEAM
SECTION RINGS

\section*{MACRO_ELAS_MULT OPTION \\ SIEF_ELGA_DEPL}

\section*{OPTION}

EPSI_ELGA_DEPL

\section*{OPTION \\ EFGE_ELNO_DEPL}

Notice on the contents of the fields:
Fields at the points of Gauss for the element PIPE
, EPSI_ELGA_DEPL and
SIEF_ELGA_DEPL, which provide the strains and the stresses to the points of integration in the local reference mark of the element, are organized in the following way:

The values are stored:
- for each point of Gauss in the length, \((n=1,3)\)
- for each point of integration in the thickness, \((n=1,2 N C O U+1=7)\)
- for each point of integration on the circumference, \((n=1,2 N S E C T+1=33)\)

\section*{- 6 components of strain or stresses:}

EPXX EPYY EPZZ EPXY EPXZ EPYZ or SIXX SIYY SIZZ SIXY
SIXZ SIYZ
where \(X\) indicates the direction given by the two nodes tops of the element, \(Y\) represents the angle describing the circumference and \(Z\) represent the ray. EPZZ and EPYZ corresponding to, in
\(r r\)
R
case of the deformations and SIZZ and SIYZ corresponding to,
in
rr R
the case of the constraints is taken equal to zero.
(for MECA_STATIQUE or MACRO_ELAS_MULT, the number of layers is fixed, and equal to 3, and
the number of sectors is equal to 16).
EFGE_ELNO_DEPL represents the efforts generalize with the 3 nodes in the traditional way: NR, VY, VZ, MT, MFY, MFZ.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures

\section*{HT-66/03/008/A}

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLL106 - Right pipe

Date:
05/01/04
Author (S):
J. Mr. PROIX

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\section*{8}

Results of modeling \(C\)
8.1 Values
tested

\section*{Loading case}

Size
Reference
Aster \%
difference
\(1 F X=4.102\)
DX
5.53E06
5.52E06
-0.04
\(1 F Y=3.102 D Y\)
4.14E06
4.14E06
-0.04
\(2 F X=3.102 D R Z\)
2.63E02
2.63E02
-0.04
\(2 F Y=4.102 D X\)
5.27E02
5.26E02
-0.056
2
DY
7.02E02
7.02E02
-0.056
3 FZ \(=5.102\) DR \(X\)
1.58 E 02
1.58 E 02
-0.04
3
DRY
2.11E02
2.11E02
-0.039
3
DZ
8.78E02
8.77E02
-0.056
\(4 M X=4.102 D R X\)
```

1.10E02
1.10E02
0
4MY=3.102 DRY
8.21E03
8.21E03
0
5 MX = 3.102 DRX
6.32E03
6.32E03
-0.04
5 MY = 4.102 DRY
8.42E03
8.42E03
-0.04
5
DZ
2.63E02
2.63E02
-0.04
6 MZ = 5.102 DRZ
1.05E02
1.05E02
-0.039
6
DX
1.58E02
1.58E02
-0.04
6
DY
2.11E02
2.11E02
-0.039
7: pressure
WO
7.38E06
7.16E06
-2.946
8: gravity
DZ
4.646 E-02 4.642
E-02

```
```

0.09
9: charge distributed
DZ
4.646 E-02 4.642
E-02
0.09

```
Loading case
Field
Net Point Component Reference
Aster \%
difference

1
EFGE_ELNO_DEPL
M18 1 NR
\(5.00 E+02\)
\(5.01 E+020.136\)

1
EPSI_ELGA_DEPL
M18 1 EPXX 1.38E06
1.38E06
-0.0311
SIEF_ELGA_DEPL
M18 1 SIXX 2.76E+05
\(2.73 E+05\)
-1.159

4
EFGE_ELNO_DEPL
M18 1 MT
\(5.00 E+02\)
\(5.00 E+020\)

4
EPSI_ELGA_DEPL
M18 1 EPXY 8.77E05 8.76E05
-0.102
4
EPSI_ELGA_DEPL
M18 693
EPXY 1.09E04 1.10E04 0.049

4
\(S I E F_{-} E L G A \_D E P L\)
M18 1 SIXY 6.75E+06 6.74E+06-0.159
4
SIEF_ELGA_DEPL
M18 693
SIXY 8.42E+06 8.42E+06 0.049
5
EFGE_ELNO_DEPL
M18 1 MFY 5.00E+02
\(5.01 E+020.123\)
5
EPSI_ELGA_DEPL
M18 479
EPXX
6.74E05
6.74E05 0.046
5
SIEF_ELGA_DEPL
M18 479
SIXX
\(1.35 E+07\)
1.33E+07 1.288
6
EFGE_ELNO_DEPL
M18 1 MFZ 5.00E+02
\(5.01 E+020.123\)
6
EPSI_ELGA_DEPL
M18 471
EPXX
6.74E05
6.74E05 0.046
6
SIEF_ELGA_DEPL
M18 471
SIXX
\(1.35 E+07\)
\(1.33 E+071.288\)
7
EPSI_ELGA_DEPL
M18 1 EPYY 2.28E04
2.24E04
-1.716
```

7
EPSI_ELGA_DEPL
M18 693
EPYY
1.78E04
1.79E04 0.741
7
SIEF_ELGA_DEPL
M18 1 SIYY 4.56E+07
4.53E+07
-0.641
7
SIEF_ELGA_DEPL
M18 693
SIYY
3.56E+07
3.54E+07 0.371
8
EFGE_ELNO_DEPL
M1 1 MFY
1764.3
1728
2
9
EFGE_ELNO_DEPL
M1 1 MFY
1764.3
1728
2
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HT-66/03/008/A
Code_Aster (®)
Version
7.2
Titrate:
SSLL106 - Right pipe
Date:

## Author (S):

J. Mr. PROIX

Key: V3.01.106-B Page: 13/22

## Generalized deformations DEGE_ELNO_DEPL:

## Loading case

Loadings
Size
Reference
Aster \%
difference
1 FX = 4.102
EPXX
1.38155E-06 1.38155E-06
-0.04
$F Y=3.102$
$2 F X=3.102$ GAXY 3.5920E-06
4.7415E06

32
$F Y=4.102 \mathrm{KZ}$ 1.0530E02
1.04E02
-1.2
$3 F Z=5.102 G A X Z$
3.5920E-06
4.7415E06

32

## KY

1.0530E02
1.04E02
-1.2
4 MX = 4.102 GAT 2.73783E-03
2.73783E-03

0
$M Y=3.102$
5 MX = 3.102 KY
2.1060E-03
2.1052E-03
-0.04
$M Y=4.102$
$6 M Z=5.102 \mathrm{KZ} 2.1060 \mathrm{E}-03$
2.1052E-03
-0.04

## Fréquenc

Reference
Aster \%
difference
E clean
12.902292 .90378
0.05
22.902292.

90378
0.05
318.1896718 .20470 .08
418.1896718 .20470 .08
550.9936751 .0060 .02
650.9936751 .0060 .02
799.81783100 .04780 .2
899.81783100 .04780 .2
9157.0190157 .01850 .001
10164.9922
165.606
0.3
11164.9922
165.606
0.3
12253.185
247.82

2

### 8.2 Remarks

The values of shearings corresponding to the shearing action are not precise for this modeling. This is due to the functions of interpolation of order 2 of this element, for displacements of beam and rotations of beams. As transverse shearings of beam are obtained by:
duy
= -
, and that for the pure bending, rotations vary like polynomials of order 2,
$x y$
Z
$d x$
but displacements, like polynomials of order 3, which is badly approached by the functions of interpolation. The derivative of displacements is thus not precise.

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## Date:

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Author (S):
J. Mr. PROIX

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9 Modeling
D

## 9.1

Characteristics of modeling
10 elements TUYAU_6M, calculation with STAT_NON_LINE.

## 9.2

Characteristics of the grid

10 meshs SEG3. The beam is directed according to the vector $(4,3,0)$.
9.3 Functionalities
tested

Orders

AFFE_MODELE
MODELING
TUYAU_6M

AFFE_CARA_ELEM BEAM

SECTION
RING

# STAT_NON_LINE COMP_INCR <br> RELATION ELAS 

## COMP_INCR

TUYAU_NCOU
3
COMP_INCR
TUYAU_NSEC
16

## OPTION

SIEF_ELNO_ELGA

Notice on the contents of the fields:
Stress fields at the points of Gauss for the element PIPE, SIEF_ELGA, in the local reference mark of the element, are organized in the following way:

The values are stored:

- for each point of Gauss in the length, (n=1, 3)
- for each point of integration in the thickness, $(n=1,2 N C O U+1)$
- for each point of integration on the circumference, ( $n=1,2 N S E C T+1$ )
- 6 components of strain or stresses:


## EPXX EPYY EPZZ EPXY EPXZ EPYZ or SIXX SIYY SIZZ SIXY

SIXZ SIYZ
where $X$ indicates the direction given by the two nodes tops of the element, Y represents the angle describing the circumference and $Z$ represent the ray. EPZZ and EPYZ corresponding to,
in
$r r$
$\boldsymbol{R}$
case of the deformations and SIZZ and SIYZ corresponding to,
in
rr R
the case of the constraints is taken equal to zero.
(in STAT_NON_LINE, the number of layers is variable, as well as the number of sectors.
One uses here 3 layers and 16 sectors by analogy with modeling A).
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## HT-66/03/008/A

## Code_Aster ${ }^{\circledR}$

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Author (S):
J. Mr. PROIX

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10 Results of modeling $D$

### 10.1 Values

tested
Loading case
Size
Reference
Aster \%
difference
1 DX

### 5.53E06 <br> 5.52E06

-0.04
1 DY
4.14E06
4.14E06
-0.04
2 DRZ
2.63 E 02
2.63E02
-0.04
2 DX
5.27E02
5.26E02
-0.056
2 DY
7.02E02
7.02E02
-0.056
3 DRX
1.58E02
1.58E02
-0.04
3 DRY
2.11E02
2.11E02
-0.039
3 DZ
8.78E02
8.77E02
-0.056
4 DRX
1.10E02
1.10E02

0
4 DRY
8.21E03
8.21E03

0
5 DRX
6.32E03
6.32E03
-0.04

```
5 DRY
8.42E03
8.42E03
-0.04
5 DZ
2.63E02
2.63E02
-0.04
6 DRZ
1.05E02
1.05E02
-0.039
6 DX
1.58E02
1.58E02
-0.04
6 DY
2.11E02
2.11E02
-0.039
7 WO
7.38E06
7.16E06
-2.946
Loading case
Field
Net
Not Component Reference
Aster %
difference
I
SIEF_ELGA
M18 Z SIXX 2.76E+05
2.73E+05
-1.159
1
SIEF_ELNO_ELGA
M18 1 NR 5.00E+02
5.01E+02 0.136
4
SIEF_ELGA
```

```
M18 1 SIXY 6.75E+06
6.74E+06
-0.159
4
SIEF_ELGA
M18 }69
SIXY 8.42E+06
8.42E+06 0.049
4
SIEF_ELNO_ELGA
M18 1 MT 5.00E+02
5.00E+02 0
5
SIEF_ELGA
M18 479
SIXX 1.35E+07
1.33E+07 1.288
5
SIEF_ELNO_ELGA
M18 1 MFY 5.00E+02
5.01E+020.123
6
SIEF_ELGA
M18 471
SIXX 1.35E+07
1.33E+07 1.288
6
SIEF_ELNO_ELGA
M18 1 MFZ 5.00E+02
5.01E+020.123
7
SIEF_ELGA
M18 1 SIYY 4.56E+07
4.53E+07
-0.641
7
SIEF_ELGA
M18 }69
SIYY 3.56E+07
3.54E+07 0.371
```

Generalized deformations DEGE_ELNO_DEPL:

Loading case<br>Loadings<br>Size<br>Reference<br>Aster \%<br>difference<br>$1 F X=4.102$<br>EPXX<br>1.38155E-06 1.38155E-06<br>-0.04<br>$F Y=3.102$<br>$2 F X=3.102$ GAXY 3.5920E-06<br>4.7415E06<br>32<br>$F Y=4.102 \mathrm{KZ}$ 1.0530E02<br>1.04E02<br>-1.2<br>$3 F Z=5.102$ GAXZ<br>3.5920E-06<br>4.7415E06<br>32<br>KY<br>-1.0530E02<br>-1.04E02<br>-1.2<br>$4 M X=4.102$ GAT 2.73783E-03<br>2.73783E-03<br>0<br>$M Y=3.102$

$5 M X=3.102 \mathrm{KY}$
2.1060E-03
2.1052E-03
$M Y=4.102$
$6 M Z=5.102 \mathrm{KZ} 2.1060 \mathrm{E}-03$
2.1052E-03
-0.04
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## Date:

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Author (S):
J. Mr. PROIX

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### 10.2 Remarks

The values of shearings corresponding to the shearing action are not precise for this modeling. This is due to the functions of interpolation of order 2 of this element, for displacements of beam and rotations of beams. As transverse shearings of beam are obtained by:
duy
= -
, and that for the pure bending, rotations vary like polynomials of order 2,
$x y$
Z
$d x$
but displacements, like polynomials of order 3, which is badly approached by the functions of interpolation. The derivative of displacements is thus not precise.

## Handbook of Validation

V3.01 booklet: Linear statics of the linear structures
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Code_Aster ${ }^{\circledR}$
Version
7.2

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## Date:

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Author (S):
J. Mr. PROIX

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## 11 Modeling

E

### 11.1 Characteristics of modeling

8 elements PIPE with 3 modes of Fourier and 4 nodes
11.2 Characteristics of the grid

8 meshs SEG4. The beam is directed according to the vector (4, 3, 0).
11.3 Functionalities
tested
Orders

AFFE_MODELE MODELING
PIPE
AFFE_CARA_ELEM BEAM
SECTION
RING
MACRO_ELAS_MULT
OPTION
SIEF_ELGA_DEPL

## OPTION

EPSI_ELGA_DEPL

## OPTION <br> EFGE_ELNO_DEPL <br> CREA_MAILLAGE OPTION

SEG3_4

Notice on the contents of the fields:
Fields at the points of Gauss for the element PIPE
, EPSI_ELGA_DEPL and
SIEF_ELGA_DEPL, which provide the strains and the stresses to the points of integration in the local reference mark of the element, are organized in the following way:

The values are stored:

- for each point of Gauss in the length, $(n=1,3)$
- for each point of integration in the thickness, ( $n=1,2 \mathrm{NCOU}+1=7$ )
- for each point of integration on the circumference, $(n=1,2 N S E C T+1=33)$
- 6 components of strain or stresses:

EPXX EPYY EPZZ EPXY EPXZ EPYZ or SIXX SIYY SIZZ SIXY
SIXZ SIYZ
where $X$ indicates the direction given by the two nodes tops of
the element, Y represents the angle describing the circumference and $Z$
represent the ray. EPZZ and EPYZ corresponding to,
in
$r r$
R
case of the deformations and SIZZ and SIYZ corresponding to,
rr R
in the case of the constraints are taken equal to zero.
(for MECA_STATIQUE or MACRO_ELAS_MULT, the number of layers is fixed, and equal to 3, and
the number of sectors is equal to 16).
EFGE_ELNO_DEPL represents the efforts generalize with the 3 nodes in the traditional way: NR,

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J. Mr. PROIX

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## 12 Results of modeling $E$

### 12.1 Values

tested

## Loading case

Size
Reference
Aster \%
difference
$F X=4.102$
DX
5.53E06
5.52E06
-0.04
$F Y=3.102$ DY 4.14E06
4.14E06
-0.04
$F X=3.102$ DRZ 2.63E02 2.63E02
-0.04
$F Y=4.102 D X$
5.27E02
5.264E02
-0.02
DY
7.02E02
7.019E02
-0.02
$F Z=5.102$ DR $X$

| $1.58 E 02$ |
| :--- |
| $1.58 E 02$ |
| -0.04 |
| $D R Y$ |
| $2.11 E 02$ |
| $2.11 E 02$ |
| -0.04 |
| $D Z$ |
| $8.78 E 02$ |
| $8.77 E 02$ |
| -0.02 |
| $M X=4.102$ DRX |
| $1.10 E 02$ |
| $1.10 E 02$ |
| 0 |
| $M Y=3.102 \quad D R Y$ |
| $8.21 E 03$ |
| $8.21 E 03$ |
| 0 |
| $M X=3.102 ~ D R X$ |
| $6.32 E 03$ |
| $6.32 E 03$ |
| -0.04 |
| $M Y=4.102 \quad D R Y$ |
| $8.42 E 03$ |
| $8.42 E 03$ |
| -0.04 |
| $D Z$ |
| $2.63 E 02$ |
| $2.63 E 02$ |
| -0.04 |
| $M Z=5.102 ~ D R Z$ |
| $1.05 E 02$ |
| $1.05 E 02$ |
| -0.039 |
| $D X$ |
| $1.58 E 02$ |
| $1.58 E 02$ |
| -0.04 |
| $D Y$ |
| $2.11 E 02$ |
| $2.11 E 02$ |
| -0.039 |
|  |

7: pressure
WO
7.38E06
7.16E06
-2.946
8: gravity
DZ
4.646 E-02 4.644

E-02 0.04
9: charge distributed
DZ
4.646 E-02 4.644

E-02 0.04

## Loading case

Field
Net
Not Component Reference
Aster \%
difference
1
EFGE_ELNO_DEPL
M18 1 NR
$5.00 E+02$
$5.01 E+02$
0.136

1
EPSI_ELGA_DEPL
M18 1 EPXX 1.38E06
1.38E06
-0.031
1
SIEF_ELGA_DEPL
M18 1 SIXX 2.76E+05
$2.73 E+05$
-1.159
4
EFGE_ELNO_DEPL
M18 1 MT 5.00E+02
$5.00 E+02$
0

```
4
EPSI_ELGA_DEPL
M18 1 EPXY
8.77E05 8.76E05
-0.102
4
EPSI_ELGA_DEPL
M18 }69
EPXY 1.09E04 1.10E04 0.049
4
SIEF_ELGA_DEPL
M18 1 SIXY 6.75E+06 6.74E+06
-0.159
4
SIEF_ELGA_DEPL
M18 }69
SIXY 8.42E+06 8.42E+06 0.049
5
EFGE_ELNO_DEPL
M18 1 MFY 5.00E+02
5.01E+02
0 . 1 2 3
5
EPSI_ELGA_DEPL
M18479
EPXX 6.74E05
6.74E05 0.046
5
SIEF_ELGA_DEPL
M18479
SIXX
1.35E+07
1.33E+07 1.288
6
EFGE_ELNO_DEPL
M18 1 MFZ 5.00E+02
5.01E+02
0.123
6
EPSI_ELGA_DEPL
M18 471
EPXX 6.74E05
6.74E05 0.046
```

```
6
SIEF_ELGA_DEPL
M18471
SIXX
1.35E+07
1.33E+07 1.288
7
EPSI_ELGA_DEPL
M18 1 EPYY 2.28E04
2.24E04
-1.716
7
EPSI_ELGA_DEPL
M18 }69
EPYY 1.78E04
1.79E04 0.741
7
SIEF_ELGA_DEPL
M18 1 SIYY 4.56E+07
4.53E+07
-0.641
7
SIEF_ELGA_DEPL
M18693
SIYY
3.56E+07
3.54E+07 0.371
8
EFGE_ELNO_DEPL
M1 1 MFY
1764.3
1 7 6 0
0 . 2
9
EFGE_ELNO_DEPL
M1 1 MFY
1 7 6 4 . 3
1760
0 . 2
```

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## Code_Aster ${ }^{\circledR}$

Version
7.2

Titrate:
SSLL106-Right pipe

Date:<br>05/01/04<br>Author (S):<br>J. Mr. PROIX<br>Key: V3.01.106-B Page: 19/22

## Generalized deformations DEGE_ELNO_DEPL:

Loading case
Loadings
Size
Reference
Aster \%
difference
$1 F X=4.102$
EPXX
1.38155E-06
1.38155E-06
-0.04
$F Y=3.102$
$2 F X=3.102$ GAXY 3.5920E-06 4.7415E06 1.1
$F Y=4.102 \mathrm{KZ}$
1.0530E02
1.04E02 0.05

3 FZ $=5.102$ GAXZ 3.5920E-06
4.7415E06 1.1

KY
1.0530E02

```
1.04E02
-0.05
4 MX = 4.102 GAT 2.73783E-03
2.73783E-03
0
MY = 3.102
```

$5 M X=3.102 \mathrm{KY}$
2.1060E-03 2.1052E-03 0.04
$M Y=4.102$
$6 \mathrm{MZ}=5.102 \mathrm{KZ}$
2.1060E-03 2.1052E-03
-0.04

## Fréquenc

 ReferenceAster \%
difference
E clean
12.90229
2.90303
0.02
22.90229
2.90303
0.02
318.18967
18.1710 .1
418.18967
18.1710 .1
550.99367
50.7810 .4
650.99367
50.7810 .4
799.81783

### 12.2 Remarks

The values of shearings corresponding to the shearing action are precise for this modeling.
This is due to the functions of interpolation of order 3 of this element, for displacements of beam and rotations of beams.

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## 13 Modeling

F

### 13.1 Characteristics of modeling

1 elements TUYAU_3M with 4 nodes, calculation with STAT_NON_LINE.
13.2 Characteristics of the grid

1 meshs SEG4. The beam is directed according to the vector (4, 3, 0).

### 13.3 Functionalities

tested

AFFE_MODELE<br>MODELING<br>PIPE<br>AFFE_CARA_ELEM BEAM<br>SECTION<br>RING

STAT_NON_LINE COMP_INCR<br>RELATION ELAS<br>COMP_INCR<br>TUYAU_NCOU<br>3<br>COMP_INCR<br>TUYAU_NSEC<br>16<br>OPTION<br>SIEF_ELNO_ELGA<br>CREA_MAILLAGE OPTION<br>SEG3_4

Notice on the contents of the fields:
Stress fields at the points of Gauss for the element PIPE, SIEF_ELGA, in the local reference mark of the element, are organized in the following way:

The values are stored:

- for each point of Gauss in the length, $(n=1,3)$
- for each point of integration in the thickness, ( $n=1,2 N C O U+1)$
- for each point of integration on the circumference, $(n=1,2 N S E C T+1)$
- 6 components of strain or stresses:

EPXX EPYY EPZZ EPXY EPXZ EPYZ or SIXX SIYY SIZZ SIXY

## SIXZ SIYZ

where $X$ indicates the direction given by the two nodes tops of the element, Y represents the angle describing the circumference and $Z$
represent the ray. EPZZ and EPYZ corresponding to,
in
$r r$
R
case of the deformations and SIZZ and SIYZ corresponding to,
in
rr R
the case of the constraints is taken equal to zero.
(in STAT_NON_LINE, the number of layers is variable, as well as the number of sectors.
One uses here 3 layers and 16 sectors by analogy with modeling A).
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## 14 Results of modeling $F$

### 14.1 Values

tested

## Loading case

Size
Reference
Aster \%
difference
1 DX
5.53E06
5.52E06 0.04

1 DY
4.14E06

### 4.14E06 0.04

2 DRZ
2.63E02
2.63E02 0.04

2 DX
5.27E02
5.26E02 0.02

2 DY
7.02E02
7.02E02 0.02

3 DRX
1.58E02
1.58E02 0.04

3 DRY
2.11E02
2.11E02 0.02

3 DZ
8.78E02
8.77E02 0.04

4 DRX
1.10E02
1.10E02 0

4 DRY
8.21E03
8.21E03 0

5 DRX
6.32E03
$6.32 E 030.04$
5 DRY
8.42E03
8.42E03 0.04

5 DZ
2.63E02
2.63E02 0.04

6 DRZ
1.05E02
1.05E02 0.04

6 DX
1.58E02
1.58E02 0.04

6 DY
2.11E02
2.11E02 0.04

Loading case
Field
Net
Not Component Reference
Aster \%
difference
1
SIEF_ELGA
M18 Z SIXX 2.76E+05
$2.73 E+05$
-1.159
1
SIEF_ELNO_ELGA
M18 1 NR 5.00E+02
$5.01 E+020.136$
4
SIEF_ELGA
M18 1 SIXY 6.75E+06
$6.74 E+06$
-0.159
4
SIEF_ELGA
M18 693
SIXY 8.42E+06
$8.42 E+060.049$
4
SIEF_ELNO_ELGA
M18 1 MT 5.00E+02
$5.00 E+020$
5
SIEF_ELGA
M18 479
SIXX 1.35E+07
$1.33 E+071.288$
5
SIEF_ELNO_ELGA
M18 1 MFY 5.00E+02
$5.01 E+020.123$

```
6
SIEF_ELGA
M18471
SIXX 1.35E+07
1.33E+07 1.288
6
SIEF_ELNO_ELGA
M18 1 MFZ 5.00E+02
5.01E+02 0.123
7
SIEF_ELGA
M18 1 SIYY 4.56E+07
4.53E+07
-0.641
7
SIEF_ELGA
M18 }69
SIYY 3.56E+07
3.54E+07 0.371
```

Generalized deformations DEGE_ELNO_DEPL:

Loading case
Loadings
Size
Reference
Aster \%
difference
$1 F X=4.102$
EPXX
1.38155E-06 1.38155E-06
-0.04
$F Y=3.102$

2 FX = 3.102 GAXY 3.5920E-06
4.7415E06

21
$F Y=4.102 K Z$ 1.0530E02

```
1.04E02
-0.04
3 FZ = 5.102 GAXZ
3.5920E-06
4.7415E06
2 1
KY
1.0530E02
1.04E02
-0.04
4 MX = 4.102 GAT 2.73783E-03
2.73783E-03
0
MY = 3.102
```

$5 M X=3.102 K Y$
2.1060E-03
2.1052E-03
-0.04
$M Y=4.102$
$6 \mathrm{MZ}=5.102 \mathrm{KZ} 2.1060 \mathrm{E}-03$
2.1052E-03
-0.04
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### 14.2 Remarks

The values of shearings corresponding to the shearing action are not precise for this modeling. This is due to the weak discretization for this modeling (only one element).

## 15 Summary of the results

This test makes it possible to check the correct operation of the element PIPE (3 modes and 6 modes of Fourier)
in linear elasticity, with operators MECA_STATIQUE and STAT_NON_LINE, for the whole of loadings applicable to this element.

The variations compared to the analytical reference solution (solution in assumption of beam) are very
weak for displacements ( $0,04 \%$ to $0,06 \%$ ), except for the loading of pressure where the variation of 3\%
is due to the fact that Wo represents an average radial displacement. Actually this radial displacement varies
in the thickness. The variation on the strains and the stresses (~
$-1 \%$ ) is more important than that
on displacements but remains acceptable taking into account the fact that these values are calculated in
points of integration located in the thickness of the pipe.
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## Handbook of Validation

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SSLL107-Validation of MACRO_CARA_POUTRE

## Summary:

The whole of modelings of this test make it possible to validate the macro_commande calculation of characteristics of section of beam, MACRO_CARA_POUTRE, for all the options suggested. Sections studied are different according to modelings.

- Modeling A validates the calculation of the characteristics of section of a corner type.
- Modeling $B$ validates the calculation of the characteristics of a circular section.
- Modeling C validates the calculation of the characteristics of a rectangular section.
- Modeling D validates the calculation of the characteristics of an alveolate rectangular section.
- Modeling E validates the calculation of the characteristics of an octagonal section.
- Modeling $F$ validates the calculation of the characteristics of a circular section with a sequence on a calculation of beam.
- Modeling $\boldsymbol{G}$ validates the calculation of a network of 2 rectangular beams of section.
- Modeling $\boldsymbol{H}$ validates the calculation of the characteristics of a mean section out of $\boldsymbol{U}$.
- Modeling I validates the calculation of the constant of torsion for a perforated section.

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## 1

Problem of reference

### 1.1 Geometry

The geometry of the various sections is provided via a plane grid. It are different for each modeling, and will thus be described in the corresponding paragraphs.

Modeling $G$ also implements the chained calculation of the characteristics of a section circular, and the use of these characteristics in a calculation of a right beam, $L=1 m$ length, in pure traction.

## 1.2 <br> Material properties

Without object, except for modeling $G$, where the treated beam has a Young modulus of 2.E11Pa and one
Poisson's ratio of 0.3.
1.3

Boundary conditions and loadings
Without object, except for modeling G: the right beam is embedded at an end, and is subjected to
the other end with a tractive effort $F=1000 \mathrm{~N}$.

## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

Since the solutions are specific to each modeling, they are described in corresponding paragraphs. They are drawn mainly from [bib1] and [bib2];
2.2

Results of reference
One describes here the characteristics calculated by MACRO_CARA_POUTRE [R3.08.03]:

- Caractéristiques geometrical of the sections

1) In reference mark $O X Y$ of description of the grid 2 D for the grid provided by the user - surface: AIRE_M

- position of the centre of gravity: $C D G_{-} X_{-} M, C D G_{-} Y_{-} M$
- moments and product of inertia of surface, in the centre of gravity $G$ in reference mark $G X Y$ :
$I X_{-} G_{-} M$,
$I Y_{-} G_{-} M, I X Y_{-} G_{-} M$

2) In the same total reference mark, for the grid obtained by symmetrization if SYME_X or SYME_Y: - surface: SURFACE

- position of the centre of gravity: $C D G_{-} X, C D G_{-} Y$
- moments and product of inertia of surface, in the centre of gravity $G$ in reference mark $G X Y: I X \_G$, IY_G,
IXY_G

3) In the principal reference mark of inertia GYZ. cross-section, whose denomination corresponds to that used with the description of the elements of neutral fibre beam $\boldsymbol{G X}$ [U4.24.01].

- principal moments of inertia of surface in reference mark GYZ, usable for the calculation of rigidity of inflection of the beam: $I Y_{-} P R I N \_G$ and IZ_PRIN_G
- angle of flow of reference mark GXY to the principal reference mark of inertia GYZ: ALPHA - characteristic distances, compared to the centre of gravity $G$ of the section for calculations of maximum constraints: $Y_{-} M A X, Y_{-} M I N, Z_{-} M A X, Z_{-} M I N$ and $R_{-} M A X$.

4) In the total reference mark, in a point $P$ provided by the user:

- $X_{-} P Y_{-} P$ : not calculation of the moments of inertia
- IX_P IY_P IXY_P: moments of inertia in reference mark PXY
- IY_PRIN_P IZ_PRIN_P: moments of inertia in reference mark PYZ.
- Characteristic mechanics:

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## Identification

## Significance

CT
Constant of torsion
EY
Position of the center of torsion/shearing
EZ
Position of the center of torsion/shearing
PCTX
PCTY
AX
Coefficient of shearing
AY
Coefficient of shearing
JG
Constant of warping

## 2.3 <br> Uncertainty on the solution

Analytical solution.

### 2.4 References bibliographical

PILKEY W.D.: "Formulated for stress, Strain and Structural Matrices". Wiley \& Idiots, New York, 1994.
[2]
D. BLEVINS: Formulated for natural frequency and shape mode.

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Corner section:
The co-ordinates of the points are:
P1
0.0
0.0

P2
1.3E02
0.0

P3
4.75E02
0.0

P4
5.0E02

P5
5.0E02
5.5E03

P6
4.750E02
8.0E03

P7
1.30E02
8.0E03

P8
$9.4645 E 03$
9.4645E03

P9
4.750E02
5.500E03

P10
8.0E03
1.300E02

P11
8.0E03
4.75E02

P12
5.5E03
5.0E02

P13
0 .
5.0E02

P14
5.5E03
4.75E02

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## 3.2 <br> Characteristics of the grid

182 meshs TRIA6.

### 3.3 Functionalities

tested

## Orders

## MACRO_CARA_POUTRE GROUP_MA_BORD

## 3.4 <br> Reference solution

No the exact analytical solution. The values are values of nonregression.
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## 4

Results of modeling $A$

### 4.1 Values

tested

Identification Reference Aster \% difference
SURFACE 7.39E04
7.39E04
-0.043
ALPHA 1.35E+02
$1.35 E+02$
0
CDG_X 1.53E02
1.53E02
0.131

CDG_Y 1.53E02
1.53E02
0.131

CT 1.43E08
1.60E08
11.0

EY 0.00E+00
7.13E08

0
EZ 1.60E02
1.51E02

IX_G 1.64E07
1.64E07
0.138

IXR2_P 1.41E08
1.39E08
-1.295
IXY_G 9.50E08
9.49E08
-0.066
IY_G 1.64E07
1.64E07
0.138

IY_PRIN_G 6.95E08 6.93E08
-0.303
IYR2_P 1.41E08
1.39E08
-1.295
IZ_PRIN_G 2.60E07 2.59E07
-0.321
PCTX 4.00E03
4.67E03
6.66E04

PCTY 4.00E03
4.67E03
6.66E04

R_MAX 3.79E02
3.79E02
-0.018
Y_MAX 3.54E02
3.54E02
-0.013
Y_MIN 3.54E02
$3.54 E 02$
-0.013
$2.17 E 02$
$2.17 E 02$
0.026

Z_MAX

## Z_MIN 1.83E02

1.83E02

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## 5 Modeling <br> B

## 5.1 <br> Characteristics of modeling

Section external circle of radius $R=0.025 \mathrm{~m}$ and thickness 0.005 m . One represents only one quarter of section. This modeling makes it possible to test key word CARA_GEOM of POST_ELEM, employee also by MACRO_CARA_POUTRE to calculate the geometrical characteristics of a surface plane.

## 5.2 <br> Characteristics of the grid

30 meshs QUAD8.

### 5.3 Functionalities

tested

## Orders

POST_ELEM CARA_GEOM
SYME_X
SYME_Y
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## 5.4 <br> Reference solution

For the complete section:
With $=[2$
$R$-(
$R$
-ep) 2]
-4
2
$=76714$
1
10 m

```
I
=
y
[4
R(
Rep) 4]
7
4
81132
l
10 m
4
p
[4
R(
Rep) 4]
2
```


## 6 <br> Results of modeling B

### 6.1 Values

## Identification Reference

Aster \%
difference
AIRE_M 1.76714E04
1.76714E04
7.76E05

CDG_X_M 1.438288E02 1.43829E02
1.25E04

CDG_Y_M 1.438288E02 1.43829E02
1.25E04

IX_G_M 8.7265757E09
8.7266E09
2.78E04

IY_G_M 8.7265757E09
8.7266E09
2.78E04

IXY_G_M 7.72837E09 7.7284E09
3.83E04

SURFACE 7.0685745E04
7.06858E04
7.76E05

CDG_X 0.00000E+00
$0.00000 E+00$
0
CDG_Y 0.00000E +00
$0.00000 E+00$
0
IX_G 1.81132E07
1.81132E07
4.19E06

IY_G 1.81132E07
1.81132E07
4.19E06

IXY_G 0.00000E+00
$0.00000 E+00$
IY_PRIN_G 1.81132E07 1.81132E07
4.19E06

IZ_PRIN_G 1.81132E07 1.81132E07
4.19E06

Y_MIN 2.50000E02
$2.50000 E 02$
$0.00 E+00$
Y_MAX 2.50000E02
$2.50000 E 02$
$0.00 E+00$
Z_MIN 2.50000E02
$2.50000 E 02$
$0.00 E+00$
Z_MAX 2.50000E02
$2.50000 E 02$
$0.00 E+00$
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## 7 Modeling

C

## 7.1

## Characteristics of modeling

Rectangular section digs which one represents a quarter. This modeling makes it possible to test the word
key CARA_GEOM of POST_ELEM, employee also by MACRO_CARA_POUTRE to calculate them geometrical characteristics of a level surface.
$H x=0.02 m, H y=0.05 m$, epx=0.002m, epy=0.005m.

## 7.2

## Characteristics of the grid

The co-ordinates of the nodes are:

N1 8.00E-03 0.00E +00
N2 8.00E-03 2.00E-02
N3 $0.00 E+002.00 E-02$
N4 0.00E +00 2.50E-02
N5 1.00E-02 2.50E-02
N6 1.00E-02 $0.00 E+00$
N7 8.00E-03 6.6667E-03
N8 8.00E-03 1.3333E-02
N9 4.00E-03 2.00E-02
N10 5.00E-03 2.50E-02
N11 1.00E-02 1.66667E-02
N12 1.00E-02 8.3333E-03

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7.3 Functionalities tested<br>\section*{Orders}<br>POST_ELEM CARA_GEOM<br>SYME_X<br>SYME_Y<br>\section*{7.4<br><br>Reference solution}

With $=H H$
=
y Z
(H2 ep H2 ep
6

310 m
$y$
y) $(Z$
Z)

4
2
1
I

```
=
[3
H H
```

H2 ep H2 ep
10
23
1
$m$
$y$
$Z$
Z
(y
y) (Z
) 3
Z
1
7
4
12
1
$I$
$=$
$y$
[3
H H

- H-ep
2
$H-e p$
2
10
968

```
l
=
Z
y
(Z
Z) (y
)3
y
]
8
12
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```

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## 8 <br> Results of modeling C

### 8.1 Values

tested

## Identification

Reference
Aster
\% difference
SURFACE 3.60000E04
3.60000E04

```
2.26E13
ALPHA 9.00000E+01
9.00000E+01
0.00E+00
CDG_X 0.00000E+00
0.00000E+00
O
CDG_Y 0.00000E+00
0.00000E+00
0.00E+00
IX_G 1.23000E07
1.23000E07
6.46E14
IXY_G_M 1.11111E09 1.11111E09 1.00E04
IY_G 1.96800E08
1.96800E08
2.52E13
IY_PRIN_G 1.96800E08 1.96800E08
2.52E13
IZ_PRIN_G 1.23000E07 1.23000E07
6.46E14
R_MAX 2.69258E02
2.69260E02
6.54E04
Y_MAX 2.50000E02
2.50000E02
0.00E+00
Y_MIN 2.50000E02
2.50000E02
0.00E+00
Z_MAX 1.00000E02
1.00000E02
0.00E+00
Z_MIN 1.00000E02
1.00000E02
0.00E+00
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```


## Code_Aster ${ }^{\circledR}$

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## 9 Modeling

D

## 9.1 <br> Characteristics of modeling

Hollow rectangular section. This modeling makes it possible to test MACRO_CARA_POUTRE to calculate
geometrical and mechanical characteristics of a level surface.
Two calculations are carried out:

- first is carried out with the key word SYME_Y= "YES", i.e. that the section considered is obtained by symmetry around the axis $Y$ (alveolate section). Moreover inertias are calculated compared to the point of co-ordinates ( $0,0.025$ ) (key word ORIG_INER), - second is carried out without symmetry, on the section with a grid, with a calculation of inertias with center grid, C of co-ordinates ( $0.005,0$ ), and 2 different groups of meshs, which correspond each one to the vertical half of the grid (on both sides of the axis Cy).


## 9.2

## Characteristics of the grid

40 meshs QUAD4.
Co-ordinates of the nodes tops of
rectangle are:
N1 0.00E+00 2.50E02
N2 0.00E+00 2.50E02
N3 1.00E02 2.50E02
N4 1.00E02 2.50E02
N5 2.00E03 2.00E02
N6 2.00E03 2.00E02
N7 8.01E03 2.00E02
N8 8.01E03 2.00E02

### 9.3 Functionalities

tested
Orders

MACRO_CARA_POUTRE SYME_Y<br>YES<br>MACRO_CARA_POUTRE ORIG_INER

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## Code_Aster ${ }^{\circledR}$

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Author (S):

## J.M. PROIX

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## 10 Results of modeling $D$

### 10.1 Values

tested

## For the section symmetrized according to OY, the geometrical characteristics are: <br> Identification <br> Reference

Aster<br>\% difference<br>AIRE_M 2.600E04<br>2.6000E04<br>1.25E13<br>SURFACE 5.200E04<br>5.2000E04<br>1.25E13<br>ALPHA 9.000E +01<br>$9.0000 E+01$<br>$0.00 E+00$<br>CDG_X_M 5.000E03<br>5.0000E03<br>5.20E14<br>$C D G \_X 0.000 E+00$<br>$0.0000 E+00$<br>$0.00 E+00$<br>$C D G_{-} Y_{-} M$ 0.000E+00<br>$1.4008 E 18$<br>1.40E18<br>CDG_Y 0.000E +00<br>1.4002E18<br>1.40E18<br>IX_G_M 7.21667E08<br>7.21667E08<br>4.62E05<br>IX_G 1.44333E07<br>1.44333E07<br>2.31E04<br>IX_P 4.69333E07<br>4.69333E07<br>7.10E05<br>IXY_G_M 0.000E+00<br>$4.332 E 26$<br>4.33E26<br>IXY_G 0.000E+00<br>$4.332 E 26$<br>4.33E26<br>IY_G_M 3.44667E09<br>3.446667E09<br>9.67E05<br>IY_G 1.98933E08<br>1.989333E08

IY_PRIN_G 1.98933E08 1.989333E08
1.68E04

IY_P 1.98933E08
1.989333E08
1.68E04

IZ_PRIN_G 1.44333E07
1.443333E07
2.31E04

R_MAX 2.69260E02
2.692582E02
$6.54 E 04$
$Y_{-}$MAX 2.500E02
2.5000E02
$0.00 E+00$
Y_MIN 2.500E02
2.5000E02
0.00E+00

Z_MAX 1.000E02
1.0000E02
1.73E14

Z_MIN 1.000E02
1.0000E02
1.73E14

For the not symmetrized section, the geometrical characteristics are:

Place<br>Identification<br>Reference<br>Aster<br>\% difference<br>ALL<br>$I X \_P$<br>3.60833E08<br>3.60833E08<br>9.24E05<br>GR1<br>$I X \_P$<br>3.60833E08<br>3.60833E08<br>9.24E05

GR2<br>IX_P<br>7.21667E08<br>7.21667E08<br>4.62E05<br>ALL<br>$I Y_{-} P$<br>1.72333E09<br>1.72333E09<br>1.93E04<br>GR1<br>$I Y \_P$<br>1.72333E09<br>1.72333E09<br>1.93E04<br>GR2<br>$I Y \_P$<br>3.44667E09<br>3.44667E09<br>9.67E05

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## 11 Modeling

E

### 11.1 Characteristics of modeling

Hollow octagonal section, which one nets a quarter.
11.2 Characteristics of the grid

N1 2.30969E-02 0.00000E+00
N2 2.30969E-02 9.56708E-03
N3 9.56708E-03 2.30969E-02
N4 0.00000E+00 2.30969E-02
N5 2.11835E-02 0.00000E+00
N6 2.11835E-02 8.77452E-03
N7 8.77452E-03 2.11835E-02
N8 0.00000E+00 2.11835E-02

### 11.3 Functionalities

tested
Orders

## MACRO_CARA_POUTRE SYME_Y <br> YES

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12 Results of modeling $E$

### 12.1 Values

tested
For the section symmetrized according to OY, the geometrical characteristics are:

## Identification

Reference
Aster
\% difference
ALPHA 9.00000E+01
$9.000000 \mathrm{E}+01$
$0.00 E+00$
IXY_G 0.00000E +00
$0.000000 E+00$
0.00E+00

IY_G 7.28824E08
7.288478E08
0.003

IY_PRIN_G 7.28824E08
7.288478E08
0.003

IZ_PRIN_G 7.28824E08
7.288478E08
0.003

R_MAX 2.50000E02
2.500000E02
4.58 E13

Y_MAX 2.30967E02
2.309698E02
0.001

Y_MIN 2.30967E02
2.309698E02
0.001

Z_MAX 2.30967E02
2.309698E02
0.001

```
Z_MIN 2.30967E02
2.309698E02
0.001
```


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## 13 Modeling

F

### 13.1 Characteristics of modeling

Full circular section, of ray 0.025 m . The characteristics calculated are then used directly in a calculation of right beam ( $L=1 m$ length), in pure traction ( $F=1000 \mathrm{~N}$ ).
Young modulus is worth 2.E11 Pa.
The characteristics of the section are introduced into AFFE_CARA_ELEM via variables python.
13.2 Characteristics of the grid

A number of meshs: 52 TRIA6, 299 QUAD8

### 13.3 Functionalities

tested
Orders

## MACRO_CARA_POUTRE <br> GROUP_MA_BORD

### 13.4 Reference solution

With $=[2$
R]
3

2
$=9635$

1
10 m
10 m
4
10
With
With

```
With = [R2
]
F
U(X)=
X
EA
F
U(L)=
L
-6
= 54648
2
10m
EA
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```


## Code_Aster ${ }^{\circledR}$

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## 14 Results of modeling $F$

### 14.1 Values

tested
For the section symmetrized according to OY, the geometrical characteristics are:
Identification
Reference
Aster
\% difference
SURFACE
1.963495E03
1.963495E03
1.46E06

AY
$1.166666 E+00$
$1.166664 E+00$
1.45E04

AZ
$1.166666 E+00$
$1.166664 E+00$
1.45E04

CDG_X
$0.000000 E+00$
1.033285E19
1.03E19

CDG_Y
$0.000000 E+00$
3.715818E19
3.72 E19

CT
6.135900E07
6.135909E07
1.54E04

EY

## $0.000000 E+00$

### 1.263839E18

### 1.26E18

EZ
$0.000000 E+00$
9.610445E20
9.61E20
$I X \_G$
3.067961E07
3.067960E07
1.99E05

IXY_G
$0.000000 E+00$
2.774478E22
$2.77 E 22$
IY_G
3.067961E07
3.067960E07
1.99E05

IY_PRIN_G 3.067961E07
3.067960E07
1.99E05

IZ_PRIN_G
3.067961E07
3.067960E07
1.99E05

JG
$0.000000 E+00$

### 9.814751E41

9.81E41
$Y_{-} M A X$
2.500000E02
2.500000E02
$0.00 E+00$
Y_MIN
2.500000E02
2.500000E02
0.00E+00
$Z_{-} M A X$
2.500000E02
2.500000E02
0.00E+00

For the calculation of traction of beam, the result is:

## Identification

Reference
Aster
\% difference
DEPL
2.546479E06
2.546479E06
1.66E14

FORC_NOD
1.000000E+03
1.000000E+03
$0.00 E+00$
With
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## 15 Modeling

G

### 15.1 Characteristics of modeling

Full rectangular section, of width 0.02 m and height 0.05 m . It is divided into two rectangles respective heights $0.025 m$, in order to test the calculation of the characteristics on groups of meshs for a network here made up of two parallel beams, ranging between two floors distant of $L=0.0002$ (what makes it possible to obtain characteristics (coefficient of shearing) very close to that of the complete section).
$y$
B
B
$B=0.01$
GR2
H
$H=0.025$
0
$X$
GR1
H
15.2 Characteristics of the grid

A number of meshs: 32 QUAD8

### 15.3 Functionalities

tested
Orders

## MACRO_CARA_POUTRE GROUP_MA

## MACRO_CARA_POUTRE LENGTH

0.0002

## MACRO_CARA_POUTRE CONNECTION <br> EMBEDDING

MACRO_CARA_POUTRE MATERIAL

Geometrical characteristics for the complete section and each half-section:

```
PLACE
SURFACE
CDG_X
CDG_Y
IX_G
IY_G
IXY_G
All
1.00E-03
O
O
2.08E-07
3.33E-08
O
GR1
5.00E-04
```

0
-1.25E-02
2.60E-08
1.67E-08
0
GR2
5.00E-04
0
1.25E-02
2.60E-08
1.67E-08
0
PLACE
$X_{-} P$
$Y_{-} P$
$I X_{-} P$
$I Y_{-} P$
$I X Y_{-} P$

IY_PRIN_P<br>IZ_PRIN_P<br>All<br>$0.00 E+00$<br>$0.00 E+00$<br>2.08E-07 3.33E-08 0<br>3.33E-08<br>2.08E-07<br>GR1<br>$0.00 E+00$<br>$0.00 E+00$<br>1.04E-07 1.67E-08 0<br>1.67E-08<br>1.04E-07<br>GR2<br>$0.00 E+00$<br>0.00E+00<br>1.04E-07 1.67E-08 0<br>1.67E-08<br>1.04E-07

Coefficients of shearing: for each rectangular section, it is worth
$A y=A z=2$
1.

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## 16 Results of modeling $G$

### 16.1 Values

tested
For the complete section, the geometrical and mechanical characteristics are:

Identification<br>Reference<br>Aster<br>\% difference<br>SURFACE<br>1.0000000E03<br>1.000000E03<br>$0.00 E+00$<br>ALPHA<br>9.0000000E+01<br>$9.000000 \mathrm{E}+01$<br>$0.00 E+00$<br>AY<br>1.2000000E +00<br>1.199951E+00<br>-0.004<br>AZ<br>1.2000000E+00<br>1.199218E+00<br>-0.065<br>CDG_X<br>$0.0000000 E+00$<br>1.032683E19<br>1.03E19<br>CDG_Y<br>$0.0000000 \mathrm{E}+00$<br>2.669725E19<br>2.67E19

## CT

9.9805000E08
9.968135E08
-0.124
EY
0.0000000E +00
1.550963E18
$1.55 E 18$
EZ
$0.0000000 E+00$
4.792477E18
4.79E18

IX_G
2.0833333E07
2.083333E07
1.60E06

IXY_G
$0.0000000 E+00$
1.395261E24
1.40E24

IY_G
3.3333330E08
3.333333E08
1.00E05

PCTX
$0.0000000 E+00$
4.895746E18
4.90E18

PCTY
$0.0000000 E+00$
1.817936E18
1.82E18

Y_MAX
2.5000000E02
2.500000E02
$0.00 E+00$
Y_MIN
2.5000000E02
2.500000E02
$0.00 E+00$
Z_MAX
1.0000000E02
1.000000E02

1.73E14

Z_MIN
1.0000000E02
1.000000E02
1.73E14

For the two disjoined groups, one obtains:

## Identification place

Reference
Aster
\% difference
GR2
SURFACE
5.00000E04
5.00000E04
2.17E14

GR1
SURFACE
5.00000E04
5.00000E04
4.34E14

ALL
AY
$1.20000 E+00$
$1.19924 E+00$
-0.064
GR1
AY
1.20000E +00
$1.19922 E+00$
-0.065
GR2
AY
$1.20000 E+00$
$1.19922 E+00$
-0.065
GR1
AZ
$1.20000 E+00$
$1.19922 E+00$
-0.065
GR2

AZ<br>1.20000E +00<br>1.19922E+00<br>-0.065<br>GR1<br>CDG_X<br>0.00000E+00<br>1.59374E19<br>1.59E19<br>GR2<br>CDG_X<br>$0.00000 E+00$<br>2.11345E19<br>2.11E19<br>GR1<br>CDG_Y<br>1.25000E02<br>1s. 25000 E 02<br>1.39E14<br>GR2<br>CDG_Y<br>1.25000E02<br>1.25000E02<br>$4.16 E 14$<br>GR1<br>IX_G<br>2.60417E08<br>2.60417E08<br>1.28E04<br>GR2<br>IX_G<br>2.60417E08<br>2.60417E08<br>1.28E04<br>GR1<br>$I X Y_{-} G$<br>$0.00000 E+00$<br>1.57868E24<br>$1.58 E 24$<br>GR2<br>IXY_G<br>0.00000E+00<br>1.98030E24

# 1.98E24 

GR1
IY_G
1.66667E08
1.66667E08
2.00E04

GR2
IY_G
1.66667E08
1.66667E08
2.00E04

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## 17 Modeling

H

### 17.1 Characteristics of modeling

Section out of $U$, of dimension $l=20 \mathrm{~mm}$, and thickness $e=0.5 \mathrm{~mm}$
17.2 Characteristics of the grid

A number of meshs: 236 QUAD8

### 17.3 Functionalities

tested

## Orders

## MACRO_CARA_POUTRE GROUP_MA_BORD

### 17.4 Reference solution

The approximate analytical values result from [bib1].

## Identificatio

Reference
Numerical value
$N$
SURFACE
With $=i t$
$2+(L-2 n d) E=E(L$
3-2nd)
29.5

AY
$4.25300 E+00$
$A Z$
1.61800E +00

CDG_X
$X=20+25$
0

- 2
/
6.8602

G
(it
(LE) E) A
CDG_Y
L/2
10
CT
12

### 2.4984 [bib2]

```
3
```

3
$C=$
$L E=12$
1
the $=2.8[\mathrm{bib1}]$
3
II
$i=$,
13
1
33
33
$C=L E$
I
I
$=L E$
I
I
= 2.4984 [bib2]
3
2
2
2
2
$i=$,
13
$L i+E L L$
I
I
$+E L$
I
EY
0
0
EZ
E
$=-15.43$
15.09 [bib4]
$Z=-x g-(3 / 7) L+$
25

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## 18 Results of modeling $\boldsymbol{H}$

### 18.1 Values

tested

For the complete section, the geometrical and mechanical characteristics are:

## Identification <br> Reference

Aster \%
difference
SURFACE 29.5
$2.9500000 E+01$

$0.00 E+00$<br>AY 4.25300E +00<br>4.4862058E+00<br>5.483<br>AZ 1.61800E+00<br>$1.916878 E+00$<br>18.472<br>$C D G \_X 6.86026 .860169 E+00$<br>-0.015<br>CDG_Y 10<br>$1.00000 E+01$<br>\subsection*{2.31E13}<br>CT 2.4984<br>$2.449398 E+00$<br>-1.9<br>EY 0 4.E-11<br>$4.18 E 11$<br>EZ $15.431 .507743 E+01$

-0.089
JG
$8.69 E+04[b i b 4]$
8.711924E+04
0.253

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19 Modeling<br>I<br>19.1 Characteristics of modeling

Hollow circular section, of external ray 10mm, and thickness 1 mm .
19.2 Characteristics of the grid

A number of meshs: 300 QUAD8

19.3 Functionalities<br>tested<br>Orders<br>\title{ MACRO_CARA_POUTRE GROUP_MA_BORD }<br>MACRO_CARA_POUTRE GROUP_MA_INTE

### 19.4 Reference solution

4
R
( $\boldsymbol{R}$ - $E$ ) 4
4
C
=

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## 20 Results of modeling I

20.1 Values
tested
The constant of torsion is worth:

Identification
Reference
Aster \%
difference
CT 5401.97
5391.48
-0.194

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## 21 Summary of the results

This test makes it possible simultaneously to check the correct operation of the order MACRO_CARA_POUTRE for various types of sections.

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## Code_Aster ${ }^{\circledR}$

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## Titrate:

SSLL108 - Discrete elements 2D

## Date:

19/08/02
Author (S):
J.M. PROIX, G. BERTRAND Clé
:
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Organization (S): EDF/AMA, CS IF

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SSLL108 - Discrete elements 2D

## Summary:

The problem is quasi-static linear in mechanics of the structures.
One analyzes the response of a bar, modelled by 10 discrete elements, with a loading of traction, for to validate the two-dimensional discrete elements.

Only one modeling uses at the same time operators MECA_STATIQUE, and STAT_NON_LINE, to validate
the use of these elements (of which the behavior remains linear) with other finite elements with behavior
unspecified.

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SSLL108 - Discrete elements $2 D$

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## 1 <br> Problem of reference

### 1.1 Geometry

A bar $L=10 \mathrm{~m}$ length, along axis $X$, modelled by 10 discrete elements with 2 nodes.

## 1.2 <br> Material properties

Each discrete element has a stiffness: $K=1.000 \mathrm{NR} / \mathrm{m}$

## 1.3 <br> Boundary conditions and loadings

$$
\begin{aligned}
& \text { In } x=0 \\
& d x=D y=0 \\
& \text { In } x=L \\
& F x=10 N
\end{aligned}
$$

## 2 <br> Reference solution

## 2.1 <br> Method of calculation used for the reference solution

# Analytical solution: displacement for an element is given by: 

$U x=F / K x$
Thus for $N$ springs: $U x=N F / K x$

## 2.2

Results of reference
Values of displacement for $x=L / 2$ and $X=L$, as well as effort in the elements (constant):
$U(L / 2)=0.05 \mathrm{~m}, U(L)=0.1 \mathrm{~m}, N=10 \mathrm{~N}$

## 2.3 <br> Uncertainty on the solution

Exact analytical solution.

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## 3 Modeling

With

## 3.1

# Characteristics of modeling 

Modeling 2D_DISCRET

## 3.2 <br> Characteristics of the grid

10 meshs SEG2.

### 3.3 Functionalities

tested

Order

Key word factor
Simple key word
Argument
AFFE_CARA_ELEM
DISCRETE
$K_{-} T_{-} D_{-} L$
MECA_STATIQUE
STAT_NON_LINE
COMP_INCR
RELATION
ELAS
CALC_ELEM
OPTION
SIEF_ELGA_DEPL
OPTION
SIEF_ELGA_DEPL

## 4

Results of modeling $A$

### 4.1 Values

tested

## Identification Reference

Aster \%
difference

```
DX (L/2)
0.05 0.05 0s
DX (L)
0.10.10
NR (SIEF_ELGA)
10100
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HT-66/02/001/A
```

Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLL108-Discrete elements $2 D$

Date:
19/08/02
Author (S):

## J.M. PROIX, G. BERTRAND Clé

:
V3.01.108-A Page:
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```
5
Summary of the results
```

This very simple test voluntarily makes it possible to check the correct operation of the discrete elements $2 D$
with STAT_NON_LINE, which makes it possible to use them with other modelings.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLL110 - System of 3 bars out of $U$ under actual weight
Date:
23/09/02
Author (S):

## J.M. PROIX

Key: V3.01.110-A Page: 1/4
Organization (S): EDF/AMA

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures

## Document: V3.01.110

## SSLL110-System of 3 bars out of $U$ under weight

 clean
## Summary:

This test allows a simple checking of calculations of gravity for the elements of bar in mechanics of structures linear statics. The model is linear.

Only one modeling is used: it makes it possible to test the application of gravity on elements of bar, located in a reference mark different from the direction of gravity.

The values tested are the generalized displacements, efforts and the constraints.
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Date:
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## J.M. PROIX

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1
Problem of reference

### 1.1 Geometry

## A system of 3 bars out of $\boldsymbol{U}$ :

## B

## With

## $y$

The surface of the cross sections is worth $A=1 m^{2}$. The length of each of the 3 bars is worth $L=10 m$.
1.2

Material properties
$E=2.1011$ Pa for the 3 bars.
Rho $=8000 \mathrm{~kg} / \mathrm{m} 3$ only for the bar CD. For the 2 other bars, Rho=0.
1.3

Boundary conditions and loadings
Embedding in A and B.
In order to avoid the movements of rigid body, $D Z=0$ for all the nodes, and $D X=0$ out of $C$ and $D$.
1 only loading is applied: gravity, with $g=2 \mathrm{~m} / \mathrm{s}^{2}$, in the direction (0.866, -0.5, 0), which
to $G=10 \mathrm{~m} / \mathrm{s}^{2}$ is equivalent, in direction $Y$.

## 2 <br> Reference solution

2.1

Method of calculation used for the reference solution

- Analytical Solution:

Normal effort in each bar $A C$ and data base: $N=R h o * L * A * g / 2$
Uy displacement out of C and D: $U y=N L / E S$

## 2.2

Results of reference

- Normal Effort in bars AC and data base: $N=4.105$ NR
- displacements out of C and D: Uy=2 10-5m
2.3

Uncertainty on the solution

Analytical solution.<br>Handbook of Validation<br>V3.01 booklet: Linear statics of the linear structures<br>HT-66/02/001/A

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Each bar is modelled by only one element.

## 3.2 <br> Characteristics of the grid <br> Three meshs SEG2.

3.3 Functionalities<br>tested<br>Orders

## AFFE_CARA_ELEM <br> BAR

CALC_ELEM<br>"EFGE_ELNO_DEPL"

```
4
Results of modeling A
```


### 4.1 Values

```
tested
Identification Reference Aster \%
difference
ux (C)
2 10-5 2
10-5 0
ux (C)
2 10-5 2
10-5 0
NR (AC) 4.1054 .1050
NR (DATA BASE) 4.1054 .1050
```


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## 5

Summary of the results
This test, very simple, makes it possible simultaneously to check the correct operation of gravity in elements of bar, which is checked by the perfect coincidence of the results with the solution analytical. It was introduced following the discovery $D$ `an anomaly on gravity into the bars, and allows to validate the correction.

## Handbook of Validation

V3.01 booklet: Linear statics of the linear structures

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6.2

## Titrate:

SSLL111-Elements of multifibre beam (right-hand sides)

Date:
05/11/02
Author (S):
S. MILL, L. DAVENNE, F.GATUINGT Key

V3.01.111-A Page:
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Organization (S): EDF-R \& D /AMA, LMT Cachan

V3.01 booklet: Linear statics of the linear structures
Document: V3.01.111

SSLL111 - Static response of a beam concrete armed (section in T) with linear behavior

## Summary:

The problem consists in analyzing the response of a concrete beam reinforced via a modeling multifibre beam. This test corresponds to a static analysis of a beam having a linear behavior. Three successive loading cases are tested: a specific force, the actual weight and a rise in temperature. For the first loading case, two grids of the section, one coarse and the other finer are tested.

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## Date:

05/11/02
Author (S):
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1 Characteristics
general

### 1.1 Geometry

Beam in inflection three points, defined by:

```
y
X
With
B
m
```

With a section in double T:
30 cm
10 C m
10 Cm
5 cm
$y$
12, 5 cm
O
$20 C m$
Z
10 cm
$12,5 \mathrm{~cm}$
8 cm
8 cm
5 cm
20 cm
On this diagram, $O$ is located at middle height of the section.
The total section of higher steels is 3.104 m 2 and that of lower steels is 4.104 m 2 .
1.2

Material properties
$\cdot$ concrete: $E=2.1010 \mathrm{~Pa} ;=0.2 ;=2400 \mathrm{~kg} . \mathrm{m3} ;=10-5 \mathrm{~K} 1$
$\cdot$ steel: E = 2,1. 1011 Pa ; 0.33; = $7800 \mathrm{~kg} . \mathrm{m3}$; = 10-5 K1
1.3

Boundary conditions
Simple support in b: $D y=0$
Support "doubles" in a: $d x=D y=d z=0$ just as $X-r a y=r y=0$.

### 1.4 Loadings

Three loading cases are tested successively:

Loading 1: effort concentrated in the mediums of the beam, $F=10000$ NR
Loading 2: actual weight of the beam, $G=9,8 \mathrm{~m} . \mathrm{s} 2$
Loading 3: homogeneous heating of the beam $T=100 \mathrm{~K}$
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2
Reference solution
Calculations of reference are carried out starting from a simple elastic design in RdM.

### 2.1 Center

rubber band
In pure bending, for an elastic behavior, the neutral axis passes by the elastic center (barycentre of the sections balanced by the modules of materials):
C such as $E C M d S=0$
$S$
One determines initially the position of the centres of gravity of the concrete only $G$ and steel only $G$ by
B
has
report/ratio at the point $O$.
$y=0,125 \times 0,3 \times 0,05-0,125 \times 0,2 \times 0,05=1,38888.10-2 m$
$\boldsymbol{G} \boldsymbol{B}$
$0,2 \times 0,05+0,1 \times 0,2+0,3 \times 0,05$

$$
y=0,125 \times 3-0,125 \times 4=-1,78571 \cdot 10-2 m
$$

G has
3+4
$Z=Z=0 m$
Ga
GB
One can then determine the position compared to $O$ of the elastic center $C$.
$E a S a O G a+E b S b$
B
$O G$
$O C=$
$E a S a+E b S b$
The concrete $S$ section is $0,045 \mathrm{~m} 2$ and the section of steel S is 7.104 m . The Young modulus of B
has
concrete is 2.1010 MPa and that of steel 21.1010 MPa. One thus has
$y=2 \times 0,045 \times 1,38888-21 \times 7.10-4 \times 1,78571=0,94317.10-2 \mathrm{~m}$
C
$2 \times 0,045+21 \times 7.10-4$
$Z=0 m$
C
2.2 Moments
quadratic
The quadratic moments of the rectangular concrete sections are calculated by the formula following:
3
bh
2
$+B \times H \times D$
12
Where, B represents the width, $H$ the height and $D$ the distance from the centre of gravity of the section by report/ratio with the axis for which one calculates the moment.
One then obtains the quadratic moment of the concrete section compared to axis $Z$ passing by center elastic:

```
3
3
0\times05
,
x
concrete =
+(3
0\times05
,
)(125
O
10
94317
0
)
3
2
2
1
O
,
0
+
+(1,
0\times,
0 2)(
94317
0
```


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Inertias of steels are calculated by the following formula:
$4+S \times d 2 S \times d 2$
64
Where, represents the diameter of steel, S the steel section and D the distance from the centre of gravity from
section compared to the axis for which one calculates the moment. The diameter of steels being small, one
neglect the first term.
One then obtains the quadratic moment of the steel sections compared to axis $Z$ passing by the center rubber band:

$$
3 \cdot 10-4 \times(0,125-0,94317 \cdot 10-2) 2+4 \cdot 10-4 \times(0,125+0,94317 \cdot 10-2) 2=0,1124 \cdot 10-4 \mathrm{~m} 4
$$

For the complete section of the beam, the quadratic moment balanced by the Young moduli of materials is:
I.E. $($ internal excitation $)=2.1010 \times 0,4547 \cdot 10-3+21 \cdot 1010 \times 0,1124 \cdot 10-4=11,4544.106$ Pa.m4

## 2.3 <br> Loading case 1

In the case of load 1 (loading concentrated in the middle of the beam), the arrow is calculated by formulate following RDM:

FL 3
$\times$
$F=$
$48 E I$

What gives the arrow:
$F=$
$10000 \times 53$
$=2,2735 \cdot 10-3 \mathrm{~m}$
$48 \times 11,4544.106$
One can also calculate the following generalized efforts:
F

- the shearing action at the beginning of the beam (left left) is worth
$=5000 \mathrm{NR}$,
2
$F \times L$
- the bending moment in the middle of the beam is worth:
= 1,25.104 N.m.
4
2.4

Loading case 2
In the case of load 2 (actual weight of the beam), the arrow is calculated by the formula of RDM following:
$F=5 \times p \times l 4$
384 I.E.(internal excitation)
where $p$ is the linear load due to the weight of materials:
$p=G(S+S)=9,8 \times(2800 \times 7.10-4+2400 \times 0,045)=1111,9 \mathrm{NR} . \mathrm{m}-1$
has has
B B
What gives the arrow:
$F=5 \times 1111,9 \times 54=7,9 \cdot 10-4 \mathrm{~m}$
$384 \times 11,4544.106$
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Author (S):

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## 2.5 <br> Loading case 3

In the case of load 3 (homogeneous rise in temperature), the beam being isostatic and them dilation coefficients of the concrete and steel being identical, the solution is simple:
The generalized constraints and efforts are null.
The lengthening of the beam is: $L=\times L \times T$
What gives with the values of our case:
$L=10-5 \times 5 \times 100=5.10-3 \mathrm{~m}$

## 3 Modeling

## 3.1

Characteristics of modeling

## Longitudinal grid of the beam:

We have 3 nodes and two elements (POU_D_EM).
With
C
B

The concrete part of the cross section of the beam is with a grid (AFFE_SECT) while steels are given directly in the form of 4 specific fibres in AFFE_CARA_ELEM (AFFE_PONCT).

Two grids of the concrete part are tested in the case of load 1. The fine grid consists of 120 fibres and the coarse grid consists of 16 fibres:

## Note:

The problem being 2D, only one fibre in the width could seem sufficient (multi-layer), but that would result in having null terms in the matrix of rigidity
(the own inertia of fibres not being taken into account) and with an error at the time of the resolution
of
system of equations.

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### 3.2 Functionalities

tested

## Orders

CREA_MAILLAGE
CREA_GROUP_MA

AFFE_MODELE<br>GRID<br>"MECHANICAL"<br>"POU_D_EM"

AFFE_MATERIAU
GROUP_MA
MATER
AFFE_CARA_ELEM
BEAM
GROUP_MA
SECTION
ORIENTATION
GROUP_MA
CARA
"ANGL_VRIL"
AFFE_SECT
GROUP_MA
MAILLAGE_SECT
TOUT_SECT
"YES"
COOR_AXE_POUTRE
NAME
AFFE_FIBRE
GROUP_MA
"SURFACE"
CARA
VALE
COOR_AXE_POUTRE
NAME
AFFE_CHAR_MECA
MODEL

# CHAM_MATER 

CARA_ELEM

EXCIT

CHARGE
CALC_ELEM
REUSE
RESULT
MODEL
CHAM_MATER
CARA_ELEM
OPTION
$E F G E \_E L N O \_D E P L$
EXCIT
CALC_NO
REUSE

## RESULT

OPTION
EFGE_NOEU_DEPL

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4 Results
4.1

Loading case 1

## Reference

Modeling Ar
Relative error \%
Arrow
(fine grid)
2,2735 10-3 2,2740
10-3 0,02
Arrow

## (coarse grid)

2,2735 10-3 2,2956
10-3 1,0
(1)

Sharp effort
(supports A)

## Bending moment

(Medium) 1,25
104 6,25
103 0,0

1) Calculations are carried out without taking into account the own inertia of each fibre. Results show that it is not nevertheless very useful to hold account of it because the difference between one coarse grid and a fine grid is not obvious.
The grid of the section does not need to be very fine to have precise results (in elasticity).
2) Option EFGE_NOEU_DEPL used to calculate the efforts generalized with the nodes does one average of the generalized efforts of all the elements connected to the node. In our case, we have 2 superimposed elements of beam (for the concrete, for steel), the efforts calculated are thus divided by 2.
If one adds the values with efforts by element (EFGE_ELNO_DEPL) of the element concrete and with
the element steel, one finds the theoretical values well.

## Note:

If one makes a calculation of arrow by taking $O$ (middle height) like reference axis to the place elastic center (COOR_AXE_POUTRE), the relative error on the arrow is $0,2 \%$ here (bus it center elastic is practically with middle height (see 1.2.1).

## 4.2

Loading case 2

Reference<br>Modeling Ar<br>Relative error \%<br>Arrow<br>(fine grid)<br>7,900 10-4 7,902<br>10-4 0,02

## 4.3 <br> Loading case 3

Reference<br>Modeling Ar<br>Relative error \%<br>Lengthening 5,00<br>10-3 5,00<br>10-3 0,0<br>Efforts 0,00<br>0,00<br>0,0

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## 5

Summary of the results
The results obtained are in concord with the results of reference.
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Version
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Titrate:
SSLL112 - Arch circular under uniform pressure

Date:
20/08/02
Author (S):
Key J.M. PROIX
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Organization (S): EDF/AMA

Handbook of Validation
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SSLL112 - Arch circular under uniform pressure

## Summary:

This test makes it possible to check the internal efforts on the curved model of beam POU_C_T.

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## 1

Problem of reference

### 1.1 Geometry

### 1.1.1 Arch circular

## R

$p$
$y$
B
With
$X$
Appear 1.1.1-a
Ray: $R=1 m$

## 1.2

Properties of materials
Young modulus:

E = 2. 1011 Pa
Poisson's ratio:
$=0.3$

# 1.3 <br> Boundary conditions and loading 

Boundary condition:
$D X=D Y=D Z=D R X=0$ on point $A$
$D Y=D Z=0$ on the point $B$
Loading: Force distributed
$p=100 \mathrm{~N} / \mathrm{m}$ on AB

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## .

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## 2 <br> Reference solutions

## 2.1

Method of calculation used for the reference solutions
The beam of the figure [Fgure 1.1.1-a] checks the equilibrium equations (plane problem).

```
dN
FD
DM
V=
,N+
y=-pR,
+RV=
0
y
D
D
y
D
```

( $p:$ normal constant loading divided into any point of the beam).
NR (), Vy (), Mz () indicate the efforts (normal, edge and moment bending) in a point of the vault expressed in the local reference mark.

Their integration with the limiting conditions:

V()
$0=0, M()$
$0=0$
$y$
Z
give:
$V=$
$y$ ()
$0, M()=0, N R()=p$.
R
2.2

Results of reference

Interior efforts for $=0^{\circ}, 6^{\circ}, 42^{\circ}$ and $60^{\circ}$.

## 2.3

Uncertainty on the solution
Analytical solution.

### 2.4 References <br> bibliographical

## [1]

Report/ratio n ${ }^{\circ}$ 2314/A of the Institute Aerotechnics "Proposal and realization for new cases tests missing with the validation beams ASTER"

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3 Modeling
With

## 3.1 <br> Characteristics of modeling

The model is composed of 30 elements curved beam of Timoshenko.

## 3.2

Characteristics of the grid

It consists of 30 elements POU_C_T.

### 3.3 Functionalities

tested

Orders

AFFE_CARA_ELEM<br>BEAM<br>SECTION<br>RING<br>CALC_ELEM<br>OPTION<br>EFGE_ELNO_DEPL<br>SIGM_ELNO_DEPL<br>AFFE_CHAR_MECA<br>FORCE_POUTRE<br>VY

## 4

Results of modeling $A$

### 4.1 Values

tested
Type of effort
Reference
Aster Variation
(\%)
Vy ( $0^{\circ}$ ) 0.0000
5.E-5
5.E-5

Vy ( $6^{\circ}$ ) 0.0000
5.E-5
5.E-5
$N R\left(60^{\circ}\right) 1.000 E+02$
$9.99 E+01$
0.1

MFZ (42 ${ }^{\circ} 0.0000$
3.93E05
3.93E05

## 5 <br> Summary of the results

The normal effort in the vault (only effort not no one) is calculated with a good precision (0,1\%) for adopted modeling.

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Titrate:
FORMA06-TP Poutre formation post treatment

Date:
14/10/02
Author (S):
NR. TARDIEU Key
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Organization (S): EDF-R \& D /AMA

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Document: V3.01.114

FORMA06 - TP beam of the formation post treatment

## Summary:

The objective of this T.P. is to use the possibilities of POSTPROCESSING available in Code_Aster in the case of a modeling of the BEAMS type.

It is about a gantry with side connections made up of a linear elastic material. In this T.P, one is interested with the following operators of postprocessing:

- operators allowing to define groups of entities of the grid type: DEFI_GROUP (definition and handling of groups of nodes and meshs),
- total operators allowing to enrich the concepts results: CALC_ELEM for the calculation of fields with the elements and CALC_NO for the calculation of the fields to the nodes, - impressions: IMPR_RESU (impression of the results to the formats RESULT, IDEAS, GIBI), $\cdot$ localised examinations: POST_RELEVE_T to raise of values on lines and calculation of derived quantity,
- layout of curve: IMPR_COURBE to visualize the evolution of a size according to space.

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Titrate:
FORMA06-TP Poutre formation post treatment

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Author (S):
NR. TARDIEU Key

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## 1 <br> Problem of reference

The study relates to a side gantry with connection (see [Figure 1-a]).

## DC <br> $D D G G$

AA
$B B$
y
EE
X

## Appear 1-a

[^0]```
1104m
AB=4m
    IAB=1 10-8m4
    12
    AB
    l
    With
=
-
1104m
AC
L
AD
AC=1m
IAC =
10-8m4
12
-4
I
With
= 110m
AC
AD
stable-lad = lm
I AD=
10-8m4
12
With
=
1104 m
AE
l
AE
LAE=2m
IAE =
10-8m4
12
```

Finally to note that $G$ is in the middle of $D A$.

1.1<br>Material properties<br>Isotropic linear elastic material:<br>$E=21011 \mathrm{~Pa}$<br>Handbook of Validation<br>V3.01 booklet: Linear statics of the linear structures<br>HT-66/02/001/A

## Code_Aster ${ }^{\circledR}$

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## 1.2 <br> Boundary conditions and loadings

1) Points
$C$ : articulated $(U=v$
C
$C=$ )
0
2) Specific force in $G: F=10666.66 \mathrm{NR}$
3) Force set out again on AD : $p=1000 . \mathrm{N} / \mathrm{m}$
4) Points $B B, D D$ and $E E$ embedded

F
$P$

Appear 1.2-a

## 1.3 <br> Reference solution

```
One poses:
I.E.(internal excitation) Year
K Year=
lAn
with N=BB,DC,DD or EE
3
K=K
+K
+K
+K
AB
AD
AE
AC
4
K
R
year
Year = K
with N = BB, DC, DD or EE
2
FlAD
plAB
l
C=+
```

8
12

- Rotation in $A A$ :
C1
=
$4 K$
- Moment in AA:
2
$p l$
$M$
$A B$

```
= +
+R.
AB
AB
l
C
12
Fl
M
AD
= -
+R.
AD
AD
l
C
8
M
= R.
AE
AE
l
C
M
= R.
AC
AC
l
C
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```

Code_Aster ${ }^{\circledR}$
Version
6.3

Titrate:
FORMA06 - TP Poutre formation post treatment

## Date:

14/10/02
Author (S):

# NR. TARDIEU Key 

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1.4 References<br>bibliographical<br>[1]<br>Guide VPCS - Edition 1990.

## 2 Modeling <br> With

Modeling A corresponds to the statement of the TP Poutre.

## 2.1 <br> Characteristics of modeling

5 elements for the section $A G$
5 elements for the section $G D$
20 elements for section $A E$
10 elements for the section $A C$
20 elements for section $A B$

## 2.2 <br> Characteristics of the grid

80 elements $P O U_{-} D_{-} T$
81 nodes
4
54
55
56
57
58

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### 2.3 Functionalities

tested
Orders

## AFFE_MODELE

POU_D_T
AFFE_CHAR_MECA
FORCE_POUTRE
FORCE_NODALE
$A F F E \_C A R A \_E L E M$
BEAM
SECTION:
"RIGHT-ANGLED"

## 3

Results of modeling $A$

3.1 Values

tested

## Not

Size and unit
Reference
Aster \%
difference
GG
$Z$, rotation
3.32217E04 3.32217E04
0.
(rad)

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Author (S):
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## 4 Modeling <br> B

Modeling B corresponds to corrected TP Poutre.

## 4.1

## Characteristics of modeling

5 elements for the section $A G$
5 elements for the section $G D$
20 elements for section $A E$
10 elements for the section $A C$
20 elements for section $A B$

## 4.2

Characteristics of the grid
80 elements $P O U_{-} D_{-} T$

## Handbook of Validation

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### 4.3 Functionalities

tested
Orders

AFFE_MODELE<br>POU_D_T<br>AFFE_CHAR_MECA<br>FORCE_POUTRE<br>FORCE_NODALE<br>AFFE_CARA_ELEM<br>BEAM<br>SECTION:<br>"RIGHT-ANGLED"<br>DEFI_GROUP<br>UNION<br>NOEUD_ORDO<br>CALC_ELEM<br>EFGE_ELNO_DEPL<br>SIPO_ELNO_DEPL<br>EFGE_ELNO_CART<br>CALC_NO<br>EFGE_ELNO_DEPL<br>SIPO_ELNO_DEPL<br>EFGE_ELNO_CART<br>IMPR_RESU<br>FORMAT<br>RESULT

# CASTEM 

# POST_RELEVE_T 

OPERATION
EXTRACTION
EXTRACTION
IMPR_COURBE
FORMAT
AGRAF

## 5 <br> Results of modeling B

### 5.1 Values

tested
Not
Size and unit
Reference
Aster \%
difference
GG
Z, rotation
3.32217E04 3.32217E04
0.
(rad)

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Code_Aster ${ }^{\circledR}$
Version
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## Titrate:

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## 6 <br> Summary of the results

The results obtained with Code_Aster are in concord with the reference solution.
The recovery in this case test of the command file of the TP on the post treatment of the beams allows to make sure that this file remains in agreement with the evolutions of syntax of the code.
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Titrate:
SSLL116 - Lattice articulated 3D

Date

23/09/03
Author (S):
Mr. Key ABBAS
:
V3.01.116-A Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation<br>V3.01 booklet: Linear statics of the linear structures<br>V3.01.116 document

SSLL116-Lattice reinforced 3D

## Summary:

This test relates to the study of a lattice made up of hurled beams, in linear static analysis.
The lattice is modelled with elements linear (SEG2) and subjected to a specific loading and the effect of
gravity.
There is a modeling with a first geometry, then a modeling with bars of reinforcement.
This test is an example with didactic aiming since it shows the construction of the solution by finite elements rather
to use MECA_STATIQUE directly.

## Handbook of Validation

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Titrate:
SSLL116 - Lattice articulated 3D
Date

## 23/09/03

Author (S):
Mr. Key ABBAS
:
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## 1

Problem of reference

### 1.1 Geometry

| 2,8 |
| :--- |
| $\boldsymbol{Y}$ |
|  |
|  |
| $Z$ |
| 21 |
| 7 |
| $\boldsymbol{X}$ |

## With <br> 7 <br> 0,7 <br> 1,05

The lattice consists of annular beams of sections everywhere ( $R=0.05$, ep=0.02), except on the part "crane", made up of circular sections of two sizes: $\mathbf{R = 0 . 0 5}$ for the full rounds and $R=0.07$ for the hatched round.
Point $A$ is in the middle of the final stem.

## 1.2

Material properties
Isotropic linear elastic material:
$E=1.962 \mathrm{E} 11 \mathrm{~Pa} ;=0.3$

## 1.3

Boundary conditions and loadings
The base of the lattice is embedded.
Loadings
Vertical nodal force in a:
Fy = 20E6 NR
Field of gravity (according to $X$ )
$\boldsymbol{G}=9.81 \mathrm{~m} / \mathrm{s}^{2}$
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## 2 <br> Reference solution

## 2.1 <br> Results of reference

Displacements and rotations of node $A$ (DEPL).
The results calculated in this case test result from a former execution of Aster. It is a case test of nonregression.

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Date

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

2,8
Y

## Z

21

7
X

With
7
0,7

1,05

## Modeling POU_D_T

No reinforcements

## 3.2 <br> Characteristics of the grid

The grid is obtained by GMSH.
A number of nodes:
247
A number of meshs: 267

### 3.3 Functionalities

tested

## Orders Options

PRE_GMSH
LIRE_MAILLAGE
DEFI_MATERIAU ELAS
AFFE_MATERIAU ALL
AFFE_MODELE
"MECHANICAL" "POU_D_T"
DEFI_VALEUR
AFFE_CARA_ELEM
BEAM "RINGS"
AFFE_CHAR_MECA DDL_IMPO
FORCE_NODALECALC_MATR_ELEM "RIGI_MECA"CALC_VECT_ELEM "CHAR_MECA"NUME_DDL MATR_RIGI
ASSE_MATRICE
ASSE VECTEUR
FACT_LDLT
RESO_LDLT
DEFUFI
IMPR_RESU "GMSH"
TEST_RESU "NON_REGRESSION"
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## Results of modeling $A$

### 4.1 Values

tested

## Loading Value

tested
Aster
Force concentrated vertical in A
Displacement of A Dx
7,20564x101

Displacement of A Dy
2,02277x100

Displacement of $A D z$
1,12417x100
Rotation of A Drx
9,88004x101
Rotation of A Dry
1,83637x101
Rotation of A Drz
1,12592x101

### 4.2 Remarks

It is seen that the not-symmetry of the arrow of the lattice involves displacements according to $Z$, although
force applied is it according to $Y$ and $X$ only (force of gravity)
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Date

## 23/09/03

## Author (S):

Mr. Key ABBAS

```
:
```

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## 5 Modeling <br> B

## 5.1

Characteristics of modeling

2,8

Y

Z

## Modeling POU_D_T

## Bars of reinforcement

## 5.2

Characteristics of the grid
The grid is obtained by GMSH.
A number of nodes:
265
A number of meshs: 287

5.3 Functionalities<br>tested<br>Orders Options<br>PRE_GMSH<br>LIRE_MAILLAGE<br>DEFI_MATERIAU ELAS<br>AFFE_MATERIAU ALL<br>AFFE_MODELE<br>"MECHANICAL" "POU_D_T"

DEFI_VALEUR
AFFE_CARA_ELEM
BEAM "RINGS"
AFFE_CHAR_MECA DDL_IMPO
FORCE_NODALE
CALC_MATR_ELEM "RIGI_MECA"
CALC_VECT_ELEM "CHAR_MECA"
NUME_DDL MATR_RIGI

ASSE_MATRICE

# ASSE_VECTEUR 

FACT_LDLT
RESO_LDLT
DEFUFI
IMPR_RESU "GMSH"
TEST_RESU "NON_REGRESSION"
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Mr. Key ABBAS
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6
Results of modeling B

### 6.1 Values

tested
Loading Value
tested
Aster
Force concentrated vertical in A
Displacement of A Dx6,61627x101
Displacement of A Dy
1,82145x100
Displacement of A Dz
2,6628x101
Rotation of A Drx8,48048x101
Rotation of A Dry
1,68397x101
Rotation of A Drz
9,43511x102
6.2 Remarks
The reinforcements made it possible to decrease displacements of the arrow of the lattice.
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Author (S):
Mr. Key ABBAS
$:$
$V$
$8 /$
7
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Summary of the results

This example shows a way of carrying out the "didactic" calculation of manner while building explicitly vectors and matrices necessary for a standard calculation by finite elements.
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLL400 - Non-prismatic beam, subjected to specific efforts
Date:
03/05/02
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.400-A Page: 1/18
Organization (S): EDF/AMA, IAT St CYR, CNAM

## Handbook of Validation

V3.01 booklet: Linear statics of the linear structures
V3.01.400 document

SSLL400 - Non-prismatic beam, subjected with efforts specific or distributed

## Summary:

This test be resulting from the validation independent of version 4 of the models of doors.
This test allows the checking of calculations of beam right-hand sides in the linear static field. (a modeling
with elements of beams POU_D_E, right beam of EULER).
One calculates simultaneously 3 beams of the different sections: section rings, right-angled, and general. These
beams are subjected to efforts specific or distributed.
The values tested are displacements and rotations, the efforts generalized, and the constraints.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
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Titrate:
SSLL400 - Non-prismatic beam, subjected to specific efforts
Date:
03/05/02
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

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## 1

Problem of reference

### 1.1 Geometry

### 1.1.1 Right beam of variable circular section

## $Z$

$y$
$X$
Appear 1.1.1-A

## Length

: 1 m

Ray with embedding: 0,1 m
Ray at the end
: 0,05 m
free
1.1.2 Right beam of variable rectangular section

```
y
X
Z
S1
S
I
Appear 1.1.2-A
Length
: 1m
with embedding: Hy=0,05 m
Hz=0,10 m
at the loose lead: Hy=0,05 m
Hz=0,05 m
```


### 1.1.3 Right beam of variable general section

## $Z$

```
y
S1
S2
X
```

```
Appear 1.1.3-A
```

Appear 1.1.3-A
Length
: 1 m
with embedding: $T o=10-2 \mathrm{~m}^{2}$
Iy $=8,3333$ 10-6 m4
at the loose lead: $T o=2,510-3 \mathrm{~m}^{2}$
$\boldsymbol{I} \boldsymbol{y}=5,20833 \mathrm{10}-7 \mathrm{m4}$
Handbook of Validation

```

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\section*{1.2 \\ Properties of materials}

Young modulus:
\(E=2.1011 \mathrm{~Pa}\)
Poisson's ratio: \(=0,3\)
Density:
\(=7800 \mathrm{Kg} \cdot \mathrm{m}-3\)

\section*{1.3 \\ Boundary conditions and loading}

\section*{Boundary condition:}

Embedded end: \(D X=D Y=D Z=D R X=D R Y=D R Z=0\)

\section*{Loading:}

On the right beam of variable circular section and on the right beam of rectangular section variable, one applies successively:

\section*{Loading case}

\section*{Nature}

1
a specific effort following \(X\) at the loose lead, \(F x=100\) NR
a specific effort following Y at the loose lead, Fy = 100 NR
3
one specific moment around axis \(X\) at the loose lead, \(M X=100 \mathrm{~m} \cdot \mathrm{~N}\)
4
one specific moment around axis \(Z\) at the loose lead, \(M z=100 \mathrm{~m} . \mathrm{N}\)

To the right beam of variable general section, one applies:

\section*{Loading case}

\section*{Nature}

7
an effort of gravity according to \(Z\) with \(G=9,81 \mathrm{~m} . \mathrm{s} 2\)

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\section*{2 \\ Reference solutions}

\section*{2.1 \\ Method of calculation used for the reference solutions}

\subsection*{2.1.1 Section \\ circular}

\subsection*{2.1.1.1 Beam subjected to a specific tractive effort Fx}

The equilibrium equation is:
( \(\boldsymbol{U}\)
```

X
EA X)
0 with
(
With X)

```
```

To 1 C
X
X=
=
+
I
L
With

```
and \(C=\)
\(2-1\),
A1
\(N R(L)=F x\)

While integrating twice [R3.08.01], we obtain displacements according to the force applied, that is to say:

L F
X
\(U(X)=\)
\(X\)
E WITH L \(+C X\),
1
and thus at the end L of the beam:
L
\(U(L)=\)
F
E WITH
With X
1
2

The efforts intern are given by:
\(\boldsymbol{U}\)
\(N R(X)=E A(X)\)
\((X)=F\)
\(X\)
\(X\)
and constraints by:
NR (X)
\(x \boldsymbol{x}=\)
With (X)

\subsection*{2.1.1.2 Beam subjected to a specific bending stress Fy}

The equilibrium equation, under the assumption of Euler, is given by the equation:
```

I 4
2
Z
and C=
1,
I -
I
Z
V(L)=F
y
y.
We solve the equation by integration by taking account of the law of behavior modified
2v
MF
MF = I.E.(INTERNAL EXCITATION)
Z
Z
Z
+V=0
x2 and the equilibrium equation X
y
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```

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Four successive integrations, by holding account for the calculation of the constants of integration that:
\(2 v\)

\section*{I.E.(internal excitation)}

\section*{(X)}
\((L)=V\)
\(-(L)=-\)
```

v()=
O
O
X
v()
0=0

```
lead to the expression of \(v(X)\) :
FL2
2
y
X \((L\)
3
\(-X+2 c x)\)
\(v(X)=+\)
E I
2
```

6
Z
(L+cx)
l
and with the expression of Z (X)
2
2
2
2
2
+
+
y
FL
X(6 L Lx
3
6Lcx cx 2 C X)
Z(X) = +
E I
3
6
Z
(L
+cx)
l
The efforts intern are given by:
$V(X)=F$
y
y
MF (X) = F (L-
X)
Z
y
and constraints by:

```

\subsection*{2.1.1.3 Beam subjected to one specific torque MX}

\section*{The movement is given by the equation:}
```

X4
G I (X
X
)
0 with I (X)

```
I
1 C
\(X\)
p
\(X\)
\(p\)
p
=
\(+\)
1
L
1
Ip 4
and \(C=\)
2

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After integration, and by taking account of the fact that:

\section*{GI(L}

X
)
\((L)=M\)
p
,
\(X\)
\(X\)
and ()
\(X 0=0\)
we obtain the expression of \(X(X)\) :
2
2
2
L M
X \(3 L+3 L c x+C X\)
X
(
)
\(X(X)=3 G I\)
3
pl
\((L+c x)\)

We must also have for the internal efforts and the constraints:
\(M(X)=M\)
\(X\)
\(X\)
\(M(X)\)
\((X)\)
\(X\)
\(=\)
\(R X\)
\(x y\)
\(I(X)\)
()
\(T\)
\(p\)
\(M(X)\)
\((X)\)
\(X\)
\(=\)
\(R X\)
\(x z\)
\(I(X)\)
()
\(T\)
\(p\)

\subsection*{2.1.1.4 Beam subjected to one specific bending moment My}

The reasoning to find the solution analytical is the same one as previously. We use \(2 w\)
MFy
law of behavior \(M(X)=-I . E .(\) internal excitation \()(X\)
\(y\)
\(y\)
)
- V = 0. Calculation
\(x 2\) and the equilibrium equation \(X\)
Z
constants of integration differs: there are \(V\)
\(L\)
\(Z()=0\) and \(M\)
\((L)=M\)
F
\(y\).
\(y\)
The expression of \(W(X)\) is obtained:
```

$L$ M
2
$y$
$X(3 L+2 c x)$
$W(X)=$

```
6th I
2
\(y\)
\((L+c x)\)
1
and the expression of \(y(X)\) :
\(L \boldsymbol{M}\)
X
2
22
\(y\)
\((3 L+3 L c x+C X)\)
\(y(X)=\)
3rd I
3
y1
\((L+c x)\)

One must also have for the internal efforts and the constraints:
```

V (X
Z
)=0
MF (X) = My
y
R(X)
(X)=MF(X)
xx

```
```

y
I (X)
y
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```

Code_Aster \({ }^{\circledR}\)
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2.1.1.5 Beam subjected to a tractive effort regularly distributed \(f x\)

Balance is described by the equation
\(U\)
\(X 2\)
\(E A(X)\)
\(=\)
\(F\)
with
\((\)
With \(X)\)

To 1 C
\(X\)
\(X\)
\(X\)
\(=\)
+
1
```

L
I
To }
and C=2

- 1. 

A1
By integrating first once this equation, we obtain:
$U$
E With (X)
$=-\boldsymbol{F} \boldsymbol{X}+\boldsymbol{C}$
$X$
$X$
1.
The condition limits $N R(L)=0$ implies $C$
F L
$1=$
X
. We thus have:

```
```

U

```
U
(L-x)
```



```
\(\boldsymbol{F}\)
\(\boldsymbol{X}\)
X E With (X)
that is to say:
(L X)
\(\boldsymbol{U}(\boldsymbol{X})=\boldsymbol{F}\)
\(d x+C\)
X E With (X)
2
c2 is given so that \(U\) ()
\(0=\).
0
```

Taking everything into account, we have:

```
L
2
2
CX+CX+(L+CX)Log
LF
L+CX
U X
X
()=
.
E With c2
L+CX
l
U
The efforts intern are deduced from the law of behavior NR (X)=E A(X):
X
NR(X)=F(L-X
X
),
and the constraints are given by:
()
F (L-X)
NR X
X
xx (X) =
=
With (X)
X2
With +
I
(A - WITH
2
1)
```

```
L

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\subsection*{2.1.1.6 Beam subjected to a bending stress regularly distributed fy}

On the basis of the equilibrium equation:
\[
\begin{aligned}
& 2 \\
& 2 \\
& 4 \\
& v \\
& X \\
& E I(X) \\
& =-F \\
& \text { with } I(X)=I \\
& 1+C \\
& 2 \\
& X \\
& Z \\
& 2 \\
& y \\
& Z \\
& 1 \\
& X \\
& Z \\
& L
\end{aligned}
\]
```

1
I 4
2
Z
and c=
1,
I -
1
Z

```
we carry out four successive integrations. The determination of the constants of integration is made starting from the following limiting conditions:

V (L
\(y\)
) \(=0\)
M (L
Z
) = 0
\(v()\)
\(0=0\)
\(X\)
\(v\) ()
\(0=0\)
The analytical expression for \(v(X)\) and \((Z)\) in the presence of a loading distributed is, taking everything into account:
- F L3
\(v(X)=\)
\(y\)
\([-6 L 2 c x+x 2\)
2
\((-L c-L c)+X(-C+C-c)+\) 12 I.E.(internal excitation) \(C(L+c x)\)
z1

\section*{\(X\)}
\(\log 1+C\)
(6L3 12L2cx 6Lc2x2)
\(+3\)
(
\(L \boldsymbol{F} \boldsymbol{X}\)
\(\boldsymbol{X})\)
\(y\)
2
2
2
Z
\(=\)
3
3
3
1
\(3[L-L x+L c x+X(-C+c)]\)
6EI \((L+c x)\)
z1
The efforts intern are given by:
\(V(X)=F(L-X\)
\(y\)
\(y\)
```

)
I
and
MF (X)=
F(L-X)2
Z
y
2
constraints by:
V (X)
y
xy(X)=A(X)
R(X)
(X)=MF(X)
xx
Z
Iz(X)
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```

\subsection*{2.1.2 Section rectangular}
2.1.2.1 Beam subjected to a specific tractive effort Fx

The equilibrium equation is:
```

U
EA (X)
=0 with
(
With X)
A1 (A
With
2
1)
X
X
=
+
L
NR(L)=F

```
\(X\)

While integrating twice, and by taking account of the fact that:
```

U
E WITH (L)
(L)=F,
X
X
(
U)
0 = ,
0

```
for the determination of the constants of integration, we obtain the analytical expression of \(\boldsymbol{U}(\boldsymbol{X})\), that is to say
```

:
F L
X
UX
X
() =
Log 1+C

```
\(L\)
1
For the internal and forced efforts, we have:
\(N R(X)=F x\)
\(N R(X)\)
\(x x=\)
(
With X)

\subsection*{2.1.2.2 Beam subjected to a specific bending stress Fy}

The movement is given by the equation:
2
2
3
\(v\)
\(X\)
EI(X)
\(=0\) with \(I(X)=I 1+C\)
2
X
\(Z\)
and \(C=\)
1 ,
I-
1
Z
\(V(L)=F\)
\(y\)
\(y\).

The same reasoning that for the circular section leads to the following result:
```

L
2
2
3
2
2Lcx+CX-CX+2L(L+cx)Log

```
F L
\(L+c x\)
\(y\)
\(v(X)=-\) the \(2 n d\) I c3
Z
\((L+c x)\)
1
F L2
\(y\)
\(X(2 L-X+c x)\)
(X)
Z
2EI
```

2
Z
(L+cx)
I
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```

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We must have for the internal efforts and the constraints:
```

V(X)=F
y
y
M(X)=F(L-X)
F
y
Z
H(X)M(X)
(X)
y
Fz
xx
=
2I (X)
Z
V (X)
(X)
y
xy
=(Ax)

```

\subsection*{2.1.2.3 Beam subjected to one specific torque MX}

\section*{The movement is given by the equation:}
```

X 3
G I (X
X
0 with I (X)
I
1C
X
p
X
p
p
=
=
l
L
1
I
p
3
and c=
2
1
I -
p1
M(L)=M
X
X.

```

By the same reasoning as the beam with circular section, we obtain the analytical expression of ()
X X:
\(L M X X(2 L+c x)\)
(X)
\(X\)
\(=2 I\)
2
\(p G(L+c x)\)
1
I
\(p\) and I
are calculated according to formulas' given in the reference material [R3.08.01].
1
p2
The internal efforts and the constraints are given by:
\(M(X)=M\)
X
X
\(M(X)\)
(X)

X
\(=\)
RX
xy
\(=\)
\(I(X)\)
()
\(T\)
\(x z\)
p
2.1.2.4 Beam subjected to one specific bending moment My

The same reasoning is taken again that previously, the analytical expressions are obtained following for \(W(X)\) and ( \(X\) )
```

LM x2
y
W(X)=-2nd Iy (L +cx)
1
LMX(2L+cx)
(
2
2nd I y (L+cx)
1
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for the efforts:
\(V(X\)
Z
) = 0
\(M F(X)=M\)
\(y\)
\(y\)
and for the constraints:
\(\boldsymbol{H}(X) \boldsymbol{M F}(X)\)
```

Z
y
xx}(X)
2 I y (X)
2.1.2.5 Beam subjected to a tractive effort regularly distributed $f x$

```

The equilibrium equation is:
```

U
X
EA (X)
F
with A (X)

```
To 1 C
X
\(X\)
\(X\)
= -
\(+\)
1
L
With
and \(c=2-\)
1
A1

After two integrations and by taking account of the fact that:
\(N R(L)=0\) to determine the first constant of integration,
and \(U\) ()
\(0=0\) to determine the second, we obtain the analytical expression of \(U(X)\) :
```

L
UX
X
() =
C X +
2
(L+Lc) Log

```
E With C
\(L+C X\)
1

The efforts intern are known by the following expression:
\(N R(X)=F(L-X\)
\(X\)
)
and constraints by:
\(\boldsymbol{F}(\boldsymbol{L}-\boldsymbol{X})\)
\(X\)
\(x x(X)=\)
With (X)

\subsection*{2.1.2.6 Beam subjected to a bending stress regularly distributed fy}

The equilibrium equation is:
```

2
2
3
v
X
I.E.(internal excitation) (X)
= -F
with I (X) = I
1 +C
2

```
```

X
Z
2
y
Z
z1
X
L
1
I 3
Z 2
and c=
1.
I -
z1
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```

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We integrate successively four times this equation. The constants of integration are calculated by taking account of the fact that:
\(V(L\)
```

y
)=0
MF (L
Z
)=0
()
0=0
X
v()
0=0

```

The analytical result for the arrow and rotation in \(L\) is as follows:
```

L3 F
$v$
$y$
$(X)=$
$X 6 L C+4 L c 2+x 25 c 2+2 c 3-c 4$
4th I c4
Z
$(L+c x)[($
)
,
1
$+($
L
$6 L 2+4 L 2 c+8 L c x+4 L c 2 X+2 c 2 x 2) L o g L+c x$

```
3
L F
\(X\) )
\(y\)
3
2
2
3
4
Z
\(=\)
```

2
+2
+
3
+2-
3
2 [X(Lc
Lc) X(C
C
c)]
4EI C (L+cx)
z1
+(
L
2L2+4Lcx + 2c2 x2) Log
L+cx

```

The efforts intern are given by the following expressions:
```

V(X)=F(L-X

```
\(y\)
\(y\)
)
1
\(M F(X)=\)
\(F(L-X) 2\)
Z
\(y\)
2
constraints by:
\(V(X)\)
\(y\)
\(x y(X)=A(X)\)
MF Z (X) H
\(y\)
\(x x(X)=\)
Iz (X) 2

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\subsection*{2.1.3 Section \\ general}
2.1.3.1 Beam subjected to the forces of gravity

The efforts of gravity are applied along axis \(Z\). The movement of the beam induced by these efforts is thus a movement of inflection in the plan ( \(X O\) Z).

The equilibrium equation is given by the expression:
2
\(2 w\)
E I (X)
\(=A(X) G\)
2
\(X\)
\(y\)
2
X
12
43
4
linear weight
2
with
```

X
With X)=1

```
With \(1+C\)
```

L
l
2
With
2
c=
-1
I
With
4
X
and I (X)=I
y
y1 1 +D

```
\(L\)
\(I\)
\(I\)
y2 4
D =
1.
I-
y1

By integrating first once, we obtain the shearing action intern:
\(\boldsymbol{V} \boldsymbol{X}=-\)
With \(X G d x+C\)
Z ()
()

1

\section*{\(C 1\) is given so that \(V(L)\)}

Z
\(=0\).

We obtain:
L With G
\(X\)
3
V
\(X=\)
\(-1+C\)
(1 C
Z ()
).
C
3

L +

By integrating second once, we obtain the internal bending moment:
\(M(X)=V(X) d x+C\)
\(y\)
Z
2
\(C 2\) is calculated so that \(M(L)\)
\(y\)
\(=0\).

We obtain:
With \(G\)

2
\(2 L C X\)
M
X
1
2
2
22
6
8
3
4
2
\(y()=\)
\((L X) L+L C+L C+L c x+L c x+\)

2

2
2
12 L
\(+C X\)
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\(y\)
We calculate then rotation starting from the law of behavior \(=E I(X\)
\(y\)
).
X
\(M(X\)
\(y\)
)
We thus have \((X)=\)
\(d x+C\)
E I (X
y
)
3
with such as \((0)=0\).
W
The arrow \(W(X)\) is given starting from the relation of Euler: \(y=-\)
X
We calculate \(W(X)\) by integration of \(y(X)\) :
\(W(X)=-\)
\((X)+C\)
\(y\)
4
with C4 such as W()
\(0=0\).
The analytical expressions of \(y(X)\) and \(W(X)\) are not retranscribed here because they are much too much heavy. They were calculated, like the preceding ones, by the formal computation software MATHEMATICA.

\section*{2.2 \\ Results of reference}
- Déplacements and rotations at the loose lead
- Interior Efforts at the two ends
- Contraintes at the two ends

\section*{Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References bibliographical}

\section*{[1]}

Report/ratio n \({ }^{\circ}\) 2314/A of the Institute Aerotechnics "Proposal and realization for new cases tests missing with the validation beams ASTER"

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The model is composed of 10 elements right beam of Euler.
S1 section: variable circular section
with embedding,
\(R 1=0.1 \mathrm{~m}\) (full section)
in the loose lead, \(R 2=0.05 m\) (full section)
S2 section: variable rectangular section
with embedding,
Hyl \(=0.05 \mathrm{~m}\)
\(\mathrm{Hzl}=0.10 \mathrm{~m}\)
in the loose lead, Hy2 \(=0.05 \mathrm{~m}\)
\(H z 2=0.05 \mathrm{~m}\)
S3 section: variable general section
with embedding,
\(A 1=102 \mathrm{~m}^{2}\)
Iyl \(=8.3333106 \mathrm{~m} 4\)
at the loose lead,
\(A 2=2.5 .103 \mathrm{~m}^{2}\)
\(I y 2=5.20833107 \mathrm{~m} 4\)

\section*{3.2 \\ Characteristics of the grid}

3 sections X 10 elements \(P O U \_D \_E\)

\section*{3.3 \\ Functionalities tested}

Orders

\section*{AFFE_CARA_ELEM}

BEAM
SECTION
RING

\section*{RECTANGLE}

\section*{GENERAL}

MECA_STATIQUE
OPTION
EFGE_ELNO_DEPL

\title{
AFFE_CHAR_MECA FORCE_NODALE
}

\author{
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Loading case}

\section*{Section}

\section*{Identification}

\author{
Reference
}

Aster Variation
\%
1 S1
\(U(L)\)
3.1831E08
3.1831E08
\(0.00 E+00\)
\(N(0)\)
1.0000E+02
1.0000E+02
\(0.00 E+00\)
\(N(L)\)
1.0000E+02
1.0000E+02
\(0.00 E+00\)
```

xx (0)
$3.1831 E+033.1831 E+03$
$0.00 E+00$

```

\author{
xx (L) \\ 1.2732E+04 1.2732E+04 \\ \(0.00 E+00\) \\ 2 S1 \\ \(v(L)\) \\ 4.2441E06 \\ 4.2441E06 \\ \(0.00 E+00\)
}
\(Z(L)\)
8.4882E06 8.4882E06
\(0.00 E+00\)
vy (0) 1.0000E+02
1.0000E+02 \(0.00 E+00\)
vy (L) 1.0000E+02
1.0000E+02 0.00E+00
\(m f z(0) 1.0000 E+02\)
\(1.0000 E+020.00 E+00\)
\(m f z(L) ~ 0.0000 E+00\)
2.0008E11 0.00E+00
xx (0)
1.2732E+05 1.2732E+05
\(0.00 E+00\)
xx (L)
0.0000E+00 2.0380E07
\(0.00 E+00\)
\(x y\) (0)
\(3.1831 E+033.1831 E+03\)
\(0.00 E+00\)
\(x y(L)\)
\(1.2732 E+041.2732 E+04\)
\(0.00 E+00\)
3 S1
\(X(L)\)
3.8621E05 3.8621E05
\(0.00 E+00\)

MX (0) 1.0000E+02
\(1.0000 E+020.00 E+00\)
\(M X(L) 1.0000 E+02\)
1.0000E \(+020.00 E+00\)
\(x y(0)\)
\(6.3661 E+046.3661 E+04\)
\(0.00 E+00\)
\(x y(L)\)
\(5.0929 E+055.0929 E+05\)
\(0.00 E+00\)
\(x z\) (0)
\(6.3661 E+046.3661 E+04\)
\(0.00 E+00\)
\(x z(L)\)
\(5.0929 E+055.0929 E+05\)
\(0.00 E+00\)
4 S1
\(W(L)\)
8.4882E06
8.4882E06
\(0.00 E+00\)
\(y(L)\)
2.9708E05 2.9708E05
\(0.00 E+00\)
\(x x\) (0)
\(1.2732 E+051.2732 E+05\)
\(0.00 E+00\)
\(x x(L)\)
\(1.0185 E+061.0185 E+06\)
0.00E+00

5 S1
\(U(L)\)
1.2296E08
1.2335E08
0.323
\(N(0)\)
\(1.0000 E+02\)
\(1.0000 E+02\)
\(0.00 E+00\)
\(N(L)\)
\(0.0000 E+00\)
9.6633E13
\(0.00 E+00\)
\(x x\) (0)
\(3.1831 E+033.1831 E+03\)
\(0.00 E+00\)
\(0.0000 E+00\) 1.2303E10
\(0.00 E+00\)
6 S1
\(v(L)\)
1.3486E06
1.3486E06
0.001

Z (L)
2.1220E06 2.1220E06 0.003
vy (0) \(1.0000 E+02\)
1.0000E \(+020.00 E+00\)
vy (L) \(0.0000 E+00\)
1.8195E10 0.00E+00
\(m f z(0) 5.0000 E+01\)
\(5.0000 E+010.00 E+00\)
\(m f z(L) 0.0000 E+00\)
2.1245E12 0.00E+00
\(x x\) (0)
\(6.3662 E+046.3662 E+04\)
\(0.00 E+00\)
\(x y(0)\)
\(3.1831 E+033.1830 E+03\)
0 .

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\section*{Loading case}

\section*{Section}

\section*{Identification}

\section*{Reference}

\author{
Aster Variation
}
\%
1 S2
\(U(L)\)
1.3862E07
1.3865E07
0.022
\(N(0)\)
1.0000E+02
1.0000E+02
0.00E +00
\(N(L)\)
1.0000E+02
1.0000E+02
\(0.00 E+00\)
xx (0)
\(2.0000 E+042.0000 E+04\)
\(0.00 E+00\)
\(x x(L)\)
4.0000E+04 4.0000E+04
\(0.00 E+00\)
2 S2
\(v(L)\)
1.8969E04
1.8546E04
-2.232
vy (0) \(1.0000 E+02\)
\(1.0000 E+020.00 E+00\)
vy \((L) 1.0000 E+02\)
\(1.0000 E+020.00 E+00\)
\(m f z(0) 1.0000 E+02\)
\(1.0000 E+020.00 E+00\)
\(m f z\) (L) \(0.0000 E+00\)
8.0035E11 0.00E+00
\(x x\) (0)
\(2.4000 E+062.4000 E+06\)
\(0.00 E+00\)

\author{
\(x x(L)\) \\ 0.0000E+00 3.8417E06 \\ \(0.00 E+00\)
}
xy (0)
\(2.0000 E+042.0000 E+04\)
\(0.00 E+00\)
\(x y(L)\)
\(4.0000 E+044.0000 E+04\)
\(0.00 E+00\)
\(3 S 2\)
\(X(L)\)
\(8.3506 E 047.8827 E 045.603\)

MX (0) \(1.0000 E+02\)
\(1.0000 E+020.00 E+00\)

MX (L) 1.0000E+02
1.0000E+02 \(0.00 E+00\)
xy (0)
1.5600E+06 1.5600E+06
0.00E+00
\(x y\) (L)
4.0371E+06 3.8400E+06 4.882
\(x z\) (0)
\(1.5600 E+061.5600 E+06\)
\(0.00 E+00\)

\section*{\(x z(L)\)}
4.0371E+06 3.8400E+06 4.882

4 S2
\(W(L)\)
1.2000E04
1.2001E04
0.014
\(y(L)\)
3.600E04 3.6012E04 0.034
\(v z\) (0) \(0.0000 E+00\)
3.2014E10 0.00E+00
\(v z(L) 0.0000 E+00\)
\(0.0000 E+000.00 E+00\)
mfy (0) 1.0000E+02
1.0000E+02 0.00E+00
mfy (L) 1.0000E+02
1.0000E+02 \(0.00 E+00\)
\(x x(L)\)
\(4.8000 E+064.8000 E+06\)
\(0.00 E+00\)
5 S2
\(U(L)\)
6.1370E08
6.1463E08
0.151
\(N(0)\)
\(1.0000 E+02\)
\(1.0000 E+02\)
\(0.00 E+00\)
\(N(L)\)
\(0.0000 E+00\)
\(1.8758 E 12\)
\(0.00 E+00\)
\(x x(0)\)
\(2.0000 E+042.0000 E+04\)
\(0.00 E+00\)

\section*{\(x x(l / 2)\)}
\(1.3333 E+041.3333 E+04\)
\(0.00 E+00\)
\(x x(L)\)
\(0.0000 E+007.5033 E 10\)
\(0.00 E+00\)
6 S2
\(v(L)\)
\(6.8626 E 05\)
\(6.7302 E 05\)
-1.929
vy (0) \(1.0000 E+02\)
1.0000E+02 \(0.00 E+00\)
vy (L) \(0.0000 E+00\)
4.3661E10 0.00E+00
\(m f z(0) 5.0000 E+01\)
\(5.0000 E+010.00 E+00\)
\(m f z(L) ~ 0.0000 E+00\)
2.3042E11 0.00E+00
xx (0)
1.2000E+06 1.2000E+06
\(0.00 E+00\)
\(x x(L)\)
0.0000E+00 1.1060E06
\(0.00 E+00\)
xy (0)
\(2.0000 E+042.0000 E+04\)
\(0.00 E+00\)
\(x y(L)\)
0.0000E +00 1.7464E07
\(0.00 E+00\)
7 S3
\(W\) (L)
3.8259E05
3.8259E05
\(0.00 E+00\)

\author{
\(y(L)\)
}
\(v z(0) 4.4633 E+02\)
\(4.4635 E+020.004\)
mfy (0) \(1.7535 E+02\)
\(1.7535 E+020.00 E+00\)

\subsection*{4.2 Remarks}

Modeling being made in beams of Euler, coefficients of shearing \(k y=k z=1\).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL400 - Non-prismatic beam, subjected to specific efforts
Date:
03/05/02
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.400-A Page: 18/18

\section*{5}

Summary of the results
The results obtained confirm that elements \(P O U_{\_} D_{\_} E\) with variable section present a good degree of reliability.

For the circular section, the results all are exact with the nodes (one finds the properties of the element with constant section) except for the efforts distributed where the effect of the smoothness of discretization fact of feeling.

For a rectangular section and a general section, it is necessary to discretize finely to have one correct solution.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)

\section*{Version}
4.0

Titrate:
Dynamometric SSLL402 Rings
Date: 01/12/98
Author (S)
:
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
V3.01.402-A Page:
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Organization (S): EDF/IMA/MMN, IAT St CYR
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
Document: V3.01.402
SSLL402 - Dynamometric ring

\section*{Summary:}

This test makes it possible to check in linear elasticity the calculation of the interior efforts and the constraints on a beam
curve.
A modeling makes it possible to test the curved elements of Timoshenko (POU_C_T).
The reference solution is analytical and the results obtained are of very good quality.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Dynamometric SSLL402 Rings
Date: 01/12/98
Author (S)
:
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
V3.01.402-A Page:
2/6
1
Problem of reference
1.1 Geometry

Circular ring
F
B
\(X\)
\(y\)
G
Y
R
C
With
0
\(X\)
D
F
Appear 1.1-a
\(R=2 m\)
The section (full) is a circle of radius \(0,01 \mathrm{Mr}\).
1.2
Properties of materials
Young modulus:
E = 2. 1011 Pa
Poisson's ratio: \(=0.3\)
1.3
Boundary conditions and loading
Boundary condition:
\(D X=D Y=D Z=D R X=0\) on point \(A\)
\(D Y=D Z=0\) on the point \(C\)
Loading:
on \(B, F=1 N R\),
on \(D, F=-1 N R\).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
Dynamometric SSLL402 Rings
Date: 01/12/98
Author (S)
:
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK
Key:
```

V3.01.402-A Page:
3/6
2
Reference solutions
2.1 Method of calculation used for the reference solutions
:
analytical

```

On the section \((A, B) 0 \ll\)
, we have:

F
F
FR
\(N R=\)
\(\cos , V y=-\)
\(\sin , M=-\)
1 -
+
\(Z\)
(cos)
2
2
2

On the section (B, C) \ll
F
FR
\(N R=-\)
\(\cos , V y=\)
\(\sin , M=-\)
\(1+\)

\section*{\(+\) \\ Z \\ (cos)}

2
2
2
By the use of the law of behavior connecting \(M\) to the rotation of the normal and given that this last is null in \(A\) and out of \(B\), we have:

\section*{2}
\(M D=0\),
O
- 2
from where:
\(=\boldsymbol{M}=\)
M
=
F R
With
C
2
2.2

Results of reference
Interior efforts for \(=0^{\circ}\) et \(90^{\circ}\).
2.3

Uncertainty on the solution
Analytical solution.
2.4 References
bibliographical
[1]
Report/ratio n \({ }^{\circ}\) 2314/A of the Institute Aerotechnics "Proposal and realization for new cases tests missing with the validation beams Aster"
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Dynamometric SSLL402 Rings
Date: 01/12/98
Author (S)
:
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
V3.01.402-A Page:
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3 Modeling
With
3.1

Characteristics of modeling
The model is composed of 4 elements curved beam of Timoshenko.
3.2

Characteristics of the grid
It consists of 4 elements POU_C_T.
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
BEAM
SECTION
RING
[U4.24.01]
CALC_ELEM
OPTION
EFGE_ELNO_DEPL
[U4.31.01]
SIGM_ELNO_DEPL
AFFE_CHAR_MECA
FORCE_NODALE
FY
[U4.25.01]
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
Dynamometric SSLL402 Rings
Date: 01/12/98
Author (S)
:
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK
Key:
V3.01.402-A Page:
5/6
4
Results of modeling A
4.1 Values
tested
4.1.1 Interior effort with \(=0^{\circ}\)
Reference
Aster
Variation \%
NR
5.000E01
5.000E01
0.0000
Vy
0.0000
0.0000
0.0000 *MFz
3.6338E01
3.6338E01
0.0000
* Absolute deviation
4.1.2 Interior effort with \(=90^{\circ}\)
Reference
Aster
Variation \%
NR0.0000
0.0000
0.0000 *
Vy
5.0000E01
5.000E01

\subsection*{4.1.3 Constraint}
with
\(=0^{\circ}\)
Reference
Aster
Variation \%
SIXX
\(4.6426 E+05\)
4.6426E+05
0.0000

SIXY
0.0000
0.0000 *
0.0000
* Absolute deviation
4.1.4 Constraint
with
\(=90^{\circ}\)
Reference
Aster
Variation \%
SIXX
8.1056E +05
8.1056E+05
0.0000

SIXY
\(1.7683 E+03\)
\(1.7683 E+03\)
0.0000
4.2 Remarks

Symmetry compared to axis (A,C) implies the nullity of the shearing action \(T\) in \(A\) and \(C\). balance F according to \(O Y\) of the half-ring \((A, B, C)\) imposes in \(A\) and \(C\) a normal effort equal to . Symmetry by
2
report/ratio with the axis \((B, D)\) implies that the moments in \(A\) and \(C\) are equal in absolute value and of direction
opposites.
4.3 Parameters
of execution
Version: 4.02.13
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU to use:
4 seconds
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Dynamometric SSLL402 Rings
Date: 01/12/98
Author (S)
:
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key:
V3.01.402-A Page:
6/6
5
Summary of the results
The results agree with the analytical solution and make it possible to validate the calculation of the efforts
interns (EFGE_ELNO_DEPL) and of constraints (SIGM_ELNO_DEPL) by the elements of beams curves (POU_C_T).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL403 Buckling of a beam under the effect of its actual weight
Date:
22/01/98

\author{
Author (S): \\ J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK \\ Key: V3.01.403-A Page: 1/6 \\ Organization (S): EDF/IMA/MMN, IAT St CYR
}

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
Document: V3.01.403

SSLL403-Buckling of a beam under the effect of its actual weight

\section*{Summary:}

This test makes it possible to validate in linear elasticity the loading due to the forces of gravity for a modeling
of right beam type of Euler (POU_D_E). It also allows the implementation and the validation of the calculation of stamp geometrical rigidity.

The reference solution is analytical and the results considered to be satisfactory.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures

\section*{HI-75/01/010/A}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLL403 Buckling of a beam under the effect of its actual weight
Date:
22/01/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.403-A Page: 2/6

\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

With
Z
0
\(X\)
total reference mark
Appear 1.1-A

Rectangular section: \(\mathrm{Hy}=0.01 \mathrm{~m}, \mathrm{~Hz}=0.01 \mathrm{~m}\)
Length: L= 1 m

\section*{1.2}

Properties of materials
Young modulus:
\(E=2.1011 \mathrm{~Pa}\)
Poisson's ratio: \(=0,3\)
Density:
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loading}

\section*{Boundary condition:}

Embedded end (0): \(D X=D Y=D Z=D R X=D R Y=D R Z=0\).

\section*{Loading:}

Force gravity: \(p\) weight per unit of length with \(G=(00-9,81)\) (given in total reference mark).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL403 Buckling of a beam under the effect of its actual weight

\section*{Date:}

22/01/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.403-A Page: 3/6

\section*{2 \\ Reference solutions}

\section*{2.1}

Method of calculation used for the reference solutions
In local reference mark, \(X\) following axis \(O A\) of the beam, the bending moment, with \(X\)-coordinate \(X\), has for expression:
\(L\)
M
\((X)=p\)
\([v()-v(X)] D\)
Fy
\(X\)
Arrow \(v(X)\) satisfied thus the equation:
```

L
L
D 2v

```
```

E I

```
E I
=p
=p
() - () = -
() - () = -
()+(-)()
```

()+(-)()

```
2
[v
\(v X] D\)
p
\(\boldsymbol{v}\)
D
\(L X v X\)
\(Z d x\)
\(X\)
\(X\)

By deriving the two members, one obtains the differential equation:
D 3v
p
FD
+
( \(L\) - \(\boldsymbol{X}\) )
\(=0\).
dx 3
E I
\(d x\)
Z
FD
The function \(v^{\prime}(X)=\)
satisfied the linear and homogeneous differential equation with the second order:
\(d x\)
```

D 2v'
p
+
(L-X) v}=0
dx2
EIz

```
who can be solved using the functions of Bessel.

One finds the value of the linear weight then criticizes equalizes with:
```

E I
p
Z
C=7837
L3

```

The analytical solution gives numerically:

The value criticizes multiplier:

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Report/ratio \(n^{\circ}\) 2314/A of the Institute Aerotechnics "Proposal and realization for new cases tests missing with the validation beams Aster"
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
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4.0

Titrate:
SSLL403 Buckling of a beam under the effect of its actual weight
Date:
22/01/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.403-A Page: 4/6

\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
The model is composed of 10 elements right beam of Euler.

\section*{3.2}

Characteristics of the grid
It consists of 10 elements POU_D_E.

\subsection*{3.3 Functionalities}
tested
Orders

\author{
Keys \\ AFFE_CARA_ELEM \\ BEAM \\ SECTION \\ RECTANGLE \\ [U4.24.01] \\ AFFE_CHAR_MECA GRAVITY
}

\author{
[U4.25.01] \\ CALC_CHAM_ELEM SIEF_ELGA_DEPL
}

\author{
[U4.61.01] \\ MODE_ITER_SIMULT METHOD \\ JACOBI
}
[U4.52.02]

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested
Eigenvalue of the system \((K+K G) X=0\) :
Reference
Aster Variation
\%
-170.701-170.0005
-0.408

\subsection*{4.2 Notice}

\section*{Since \(p\)}
\(\boldsymbol{S} \boldsymbol{G}\)
\(C=\)
, (S G represents linear prestressing), we have like critical loading:
```

p
NR m
C=

```
130084
1
4.3 Parameters
of execution
Version: 4.02.13

Machine: CRAY C90

Obstruction memory:
8 MW
Time CPU to use:
5.8 seconds

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL403 Buckling of a beam under the effect of its actual weight
Date:
22/01/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.403-A Page: 5/6

5
Summary of the results

The results are very close to the analytical solution (variation: 0,4\% per 10 elements). This variation is
function of the smoothness of discretization being given assumptions used for rigidity geometrical (cf [R3.08.01]). This thus validates this type of loading for the buckling of Euler.

\author{
Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLL403 Buckling of a beam under the effect of its actual weight
Date:
22/01/98
Author (S):
J.M. PROIX, m.t. BOURDEIX, P. HEMON, O. WILK

Key: V3.01.403-A Page: 6/6

\title{
Intentionally white left page. \\ Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLL404-Buckling of an arch

Date:
23/09/02
Author (S):
J.M. PROIX, F. SOULIE Key
:
V3.01.404-A Page:
1/6
Organization (S): EDF/AMA, SAMTECH

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
V3.01.404 document

SSLL404-Buckling of an arch

\section*{Summary}

The applicability of this test is the analysis of stability of the structures. The studied structure is an arch
bent by moments applied at the two ends; it is modelled by elements of beams right-hand sides. The goal is to calculate the breaking values of the moments.

The interest of this test lies in the following aspects:
\(\cdot\) calculation of a geometrical matrix of rigidity for elements POU_D_E.
- test of modal methods MODE_ITER_SIMULT and MODE_ITER_INV of stability - presence of close eigenvalues

The calculated clean loads are compared with values obtained analytically for a model of beam of Euler-Bernoulli.

In this test, one also validates the RAYLEIGH option of order MODE_ITER_INV.

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
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5.0

Titrate:
SSLL404-Buckling of an arch

\section*{Date:}

23/09/02
Author (S):
J.M. PROIX, F. SOULIE Key
:
V3.01.404-A Page:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}
```

B
y
Y
HX
M
y
B
R
With
X
M

```

Radius of curvature
\(R=0.3 \mathrm{~m}\)
Height of the profile
\(H=0.015 \mathrm{~m}\)
Width of the profile
\(B=0.002 \mathrm{~m}\)
Section
\(S=b \boldsymbol{h}\)
the 1st inertia of inflection
I \(X=b h 3 / 12\)
the 2nd inertia of inflection
\(I Y=h b 3 / 12\)
Inertia of torsion
\(J=h b 3 / 3\)
1.2
Properties of materials
Young modulus
\(E=7 . E 10 \mathrm{~N} / \mathrm{m}^{2}\)
Poisson's ratio
\(=0.3\)
Modulus of rigidity
\(G=E / 2(1+)\)
1.3
Boundary conditions and loading

The beam Bi-is supported. One prevents the torsion of the section at ends \(A\) and B. to respect the assumptions of the ideal model taken as reference, it is important that the moment is constant and that the normal effort is null along the beam. This is why free it is left displacement \(\boldsymbol{U}\) according to \(X\) at the point B. the boundary conditions are:

At point a: \(U=v=W=0 ; Y=0\)
At point b: \(v=W=0 ; X=0\)
The initial state of stress which makes it possible to carry out the analysis of stability is obtained by imposing one
bending moment around axis Z:
At points \(A\) and \(b: M=1 \mathrm{Nm}\)

\subsection*{1.4 Conditions initial}

Without object in static analysis of stability.
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL404-Buckling of an arch

Date:
23/09/02
Author (S):
J.M. PROIX, F. SOULIE Key
:
V3.01.404-A Page:
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2
Reference solution
2.1

\section*{Method of calculation used for the reference solution}

The reference solution is obtained analytically for a beam of Euler-Bernoulli. Aspects theoretical are developed in the reference [bib1].

By using the notations of the paragraph [\$1], the values criticize are given by the expression:
I.E.(INTERNAL EXCITATION) + GJ
I.E.(INTERNAL EXCITATION) - GJ 2
I.E.(INTERNAL EXCITATION) GJ

M
\(X\)
\(X\)
\(\pm\)
\(4 n 2\)
\(X\)
= -
\(N\)
CR
12
, 3
,,\(\ldots\)
\(\boldsymbol{R}\)
\(+\)
=
2
2
R2

The plus sign corresponds to positive moments such as they are indicated on the figure of [\$1.1].

\section*{2.2}

Results of reference
The first 5 critical loads are classified by order of increasing module.

Mode Moment criticizes (Nm)

With Code_Aster, one finds the opposites of these critical loads (what is logical compared to formulation of the problem to be solved).

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution

\subsection*{2.4 References \\ bibliographical}
[1]
TIMOSHENKO Stephen P., MANAGES James Mr., Theory of Elastic Stability, McGraw-Hill, International Edition, 1963, pp. 313-318.

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL404-Buckling of an arch

Date:
23/09/02
Author (S):
J.M. PROIX, F. SOULIE Key
:
V3.01.404-A Page:
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\section*{3 Modeling}

\section*{With}

\section*{3.1 \\ Characteristics of modeling}

\author{
\(\boldsymbol{Y}, \mathbf{D Y}\) \\ 19 \\ 1 \\ \(\boldsymbol{X}, \mathrm{DX}\)
}

The arch is with a grid by means of elements of right beam of type \(P O U_{-} D_{-} E\).
Boundary conditions:
At point \(A\) such as \(X=R, Y=0\) :
\(D X=D Y=D Z=0\) and \(R Y=0\)
At the point B such as \(X=0, Y=R\) :
\(D Y=D Z=0\) and \(X-r a y=0\)
For the static analysis, unit moments around \(Z\) are defined in nodes 1 and 19.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 19
A number of meshs: 18 POU_D_E

\author{
3.3 Functionalities \\ tested \\ Orders \\ AFFE_MODELE \\ AFFE \\ MODELING \\ "POU_D_E" \\ AFFE_CHAR_MECA
}

\title{
DDL_IMPO
}

\author{
AFFE_CARA_ELEM
}

BEAM

\author{
CALC_MATR_ELEM OPTION \\ "RIGI_GEOM"
}

\author{
"RIGI_MECA" \\ MODE_ITER_SIMULT \\ METHOD \\ "SORENSEN" \\ \section*{CALC_FREQ} \\ OPTION \\ PLUS_PETITE' \\ NMAX_FREQ \\ MODE_ITER_INV \\ CALC_FREQ \\ OPTION \\ "NEAR" \\ CHAR_CRIT \\ CALC_MODE \\ OPTION \\ \section*{RAYLEIGH}
}

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL404-Buckling of an arch

\section*{Date:}

\section*{23/09/02}

Author (S):
J.M. PROIX, F. SOULIE Key
:
V3.01.404-A Page:
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```

4
Results of modeling A
Critical load
4 . 1
MODE_ITER_SIMULT with METHOD = "SORENSEN"

```

\section*{Identification}

Reference
Code_Aster \%
difference
\(N^{\circ}\) critical load
(multiplied by -1)
1 -2.86074
-2.75137
3.823

2 -8.63207
-8.30613
3.776

3
8.783828 .395544 .420

4-14.4147
-13.93216
3.348

5
14.555114 .011043 .738

\section*{4.2 \\ MODE_ITER_INV with OPTION = "NEAR"}

Identification
Reference
Code_Aster \%

\section*{difference}
\(N^{\circ}\) critical load
(multiplied by -1)
1 -2.86074
-2.75137
3.823

2 -8.63207
-8.30613
3.776
38.78382
8.39554
4.420

4-14.4147
-13.93216
3.348
514.5551
14.01104
3.738

\section*{Handbook of Validation}

V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

\section*{J.M. PROIX, F. SOULIE Key}

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\section*{5 \\ Summary of the results}

The methods of Sorensen and the iterations opposite give identical and satisfactory results since the maximum change with the analytical solution is lower than \(4.5 \%\).On recalls than the solution analytical takes into account the curve of the structure.

Elements MEPOUCT could not be used in this test because the calculation of the matrix of rigidity geometrical is not available for this type of element.

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

SSLL501- Piping VVP. Comparison Aster-SYSPIPE
Date:
16/02/02
Author (S):
J.M. PROIX, P.LE DELLIOU, F.CURTIT Key

\author{
Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ Document: V3.01.501
}

SSLL501-Piping VVP.
Comparison Aster-SYSPIPE

\section*{Summary:}

This test makes it possible to check the elements of beam for the calculation of line of piping real, in particular them
elements of curved beam POU_C_T. Calculation is static, elastic, linear.
The reference solution is numerical (code SYSPIPE).
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLL501- Piping VVP. Comparison Aster-SYSPIPE

\section*{Date:}

16/02/02
Author (S):
J.M. PROIX, P.LE DELLIOU, F.CURTIT Key
:
V3.01.501-A Page:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

This test is extracted from a study, carried out by the Department MTC, which related to modeling of an industrial piping (line VVP of stage CPY). It gave place to comparisons between Code_Aster and code SYSPIPE of FRAMATOME [bib2]

N50
N1

Overall length \(L=57.766 \mathrm{~m}\)
Co-ordinates of the points (in m):
N1-2.2621E+03 2.0129E+04 1.6700E+04
N26
\(1.6260 E+04-1.7089 E+031.8320 E+04\)
N2
-2.2348E+03 1.9961E+04 1.6700E+04
N27
\(1.6121 E+04-3.3058 E+031.8320 E+04\)
N3
\(-2.0514 E+031.8836 E+041.6700 E+04\)
N28
\(1.6120 E+04-3.3077 E+031.8320 E+04\)
N4
\(-1.8550 E+031.7631 E+041.6700 E+04\)
N29
\(1.6053 E+04-4.0826 E+031.8320 E+04\)
N5
\(-6.1086 E+021.6598 E+041.7300 E+04\)
N30
\(1.5943 E+04-4.4919 E+031.8320 E+04\)

\section*{N6}
\(7.9481 E+021.6827 E+041.8107 E+04\)
N31
\(1.5664 E+04-5.0899 E+031.8320 E+04\) N7
\(1.5780 E+031.6757 E+041.8320 E+04\)
N32
\(1.5331 E+04-5.8032 E+031.8320 E+04\) N8
\(3.0228 E+031.6302 E+041.8320 E+04\)
N33
\(1.4816 E+04-6.9089 E+031.9540 E+04\) N9
\(6.0461 E+031.5349 E+041.8320 E+04\)
N34
\(1.4816 E+04-6.9089 E+032.0525 E+04\)
N10
\(6.0480 E+031.5348 E+041.8320 E+04\)
N35
\(1.4816 E+04-6.9089 E+032.2030 E+04\)
N11
\(6.8360 E+031.5100 E+041.8320 E+04\)
N36
\(1.4816 E+04-6.9089 E+032.2830 E+04\)
N12
7.1690E \(+031.4935 E+041.8320 E+04\)

N37
\(1.4816 E+04-6.9089 E+032.4710 E+04\)
N13
\(9.6094 E+031.3227 E+041.8320 E+04\)
N38
\(1.4816 E+04-6.9089 E+032.6590 E+04\)
N14
\(1.0878 E+041.2338 E+041.8320 E+04\)
N39
\(1.4816 E+04-6.9089 E+032.8005 E+04\)
N15
\(1.1561 E+041.1860 E+041.8320 E+04\)
N40
\(1.4816 E+04-6.9089 E+032.8010 E+04\)
N16
1.1861E+04 1.1560E+04 1.8320E+04

N41
\(1.4816 E+04-6.9089 E+032.8015 E+04\)
N17
1.2899E+04 1.0078E+04 1.8320E+04

N42
1.4816E+04-6.9089E+03 2.9530E+04

N18
1.2900E \(+041.0076 E+041.8320 E+04\)

N43
\(1.4816 E+04-6.9089 E+033.0530 E+04\)
N19
1.4376E+04 7.9679E+03 1.8320E+04

N44
\(1.4816 E+04-6.9089 E+033.0780 E+04\)
N20
1.4921E+04 7.1906E+03 1.8320E+04

N45
1.3710E+04-6.3933E+03 3.2000E+04

N21
1.5100E \(+046.8067 E+031.8320 E+04\)

N46
\(1.3594 E+04-6.3392 E+033.2000 E+04\)
N22
1.5345E+04 5.8901E+03 1.8320E+04

N47
\(1.0757 E+04-5.0164 E+033.2000 E+04\)
N23
1.6235E+04 2.5702E+03 1.8320E+04

N48
\(1.0621 E+04-4.9531 E+033.2000 E+04\)
N24
1.6481E+04 1.6537E+03 1.8320E+04

N49
\(9.5162 E+03-4.4374 E+033.0780 E+04\)
N24
1.6517E+04 1.2316E+03 1.8320E+04

N50
\(9.5162 E+03-4.4374 E+032.9938 E+04\)
Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version

\author{
J.M. PROIX, P.LE DELLIOU, F.CURTIT Key
}

\section*{Characteristics of the sections:}

Right parts:
- \(R=406.4 \mathrm{~mm}, E P=32 . \mathrm{mm}\)
- Mesh 1 (pipe Steam Generator), \(R=410 . \mathrm{mm}, E P=38 . \mathrm{mm}\);
- Mesh 49 (exit Br), \(R=444.4 \mathrm{~mm}, E P=70 . \mathrm{mm}\);

\section*{Elbows:}
- meshs: (M2 M6 M18 M21 M26 M30 M35 M39 M44 M46): \(R=406.4 m m ; E P=34 . \mathrm{mm}\);
- Coefficient of flexibility for all the elbows, cflex = 6.032;
- Rays of bending of the elbows: 1220 mm

\section*{1.2}

Material properties
\[
E=1.8604 E+5 \mathrm{MPa}\left(\text { with } 287^{\circ} \mathrm{C}\right)
\]

Alpha \(=12.81 e-6{ }^{\circ} \mathrm{Cl} ;=0.3\)

\section*{1.3 \\ Boundary conditions and loadings}

Displacement imposed on node 50 (mesh, side Steam Generator)
\(d x=46.466 \mathrm{~mm}\);
\(D y=-30.494 \mathrm{~mm}\);
\(d z=76 . \mathrm{mm}\);
null rotations: \(d r x=d r y=d r z=0\)
The N1 node is embedded:
\[
d x=d y=d z=d r x=d r y=d r z=0
\]

\author{
Handbook of Validation
}

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLL501- Piping VVP. Comparison Aster-SYSPIPE

\section*{Date:}

16/02/02
Author (S):
J.M. PROIX, P.LE DELLIOU, F.CURTIT Key

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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

This test is extracted from a study, carried out by the Department MTC, which related to modeling of an industrial piping (line VVP of stage CPY). It gave place to comparisons between Code_Aster and code SYSPIPE of FRAMATOME [bib1]. The reference solution is numerical (code SYSPIPE).

\section*{2.2 \\ Results of reference}

Inflection and torques to the nodes N50, N48 N44, N16 and N2
Net Noeud
\(|M T| M F=R A C I N E(M Y * M Y+M Z * M Z)\)
M1 N50
41.084
95.294

M2 N48
91.024
83.743

M3 N48
91.024
83.738

M6 N44
11.992
102.100

M7 N44
11.992
102.104

M34 N16
0.356
58.870

M35 N16
0.355
58.868

M48 N2
6.114
97.174

M49 N2
6.114
97.174

\section*{2.3 \\ Uncertainty on the solution}

Numerical solution, obtained with identical data and comparable elements. One can thus to estimate the precision at \(1 \%\).

\subsection*{2.4 References \\ bibliographical}
[1]
P.LE DELLIOU, P.HORNET.

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HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
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SSLL501- Piping VVP. Comparison Aster-SYSPIPE

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J.M. PROIX, P.LE DELLIOU, F.CURTIT Key
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

10 POU_C_T
39 POU_D_T

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 50
A number of meshs and type: 49 SEG2

\author{
3.3 Functionalities \\ tested \\ Orders
}

AFFE_CARA_ELEM BEAM
NET
RING
AFFE_CARA_ELEM DEFI_ARC
NET
CENTER
AFFE_CARA_ELEM DEFI_ARC
NET

\section*{TEMP_CALCULEE}

\section*{FORCE_NODALE \\ NODE}

\author{
"MECHANICAL" AFFE_MODELE "POU_D_T" \\ "MECHANICAL" AFFE_MODELE "POU_C_T" \\ DEFI_MATERIAU ELAS \\ Handbook of Validation \\ V3.01 booklet: Linear statics of the linear structures \\ HT-66/02/001/A \\ \section*{Code_Aster \({ }^{\circledR}\)} \\ Version \\ 5.0 \\ Titrate: \\ SSLL501- Piping VVP. Comparison Aster-SYSPIPE
}

Date:
16/02/02
Author (S):
J.M. PROIX, P.LE DELLIOU, F.CURTIT Key
:
V3.01.501-A Page:
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{\(|M T|\)}

\author{
Net Noeud Reference Aster \% difference \\ M1 N50 41.084 \\ 41.0830 .002 \\ M2 N48 91.024 \\ 91.0230 .001 \\ M3 N48 91.024 \\ 91.0230 .001 \\ M6 N44 11.992 \\ 11.992 -0.003 \\ M7 N44 11.992 \\ 11.992 -0.003 \\ M34 N16 \\ 0.3560 .355 \\ 0.3 \\ M35 N16 \\ 0.3550 .355 \\ 0.3 \\ M48 N2 \\ 6.1146 .114 \\ 0.001 \\ M49 N2 \\ 6.1146 .114 \\ 0.001 \\ \(M F=R A C I N E(M Y * M Y+M Z * M Z)\)
}

\section*{Net Noeud Reference}

Aster \%
difference
M1 N50 95.294
95.2930 .001

M2 N48 83.743
83.7410 .002

M3 N48 83.738
83.741-0.003

M6 N44 102.100
```

102.099 0.001
M7 N44 102.104
102.099 0.005
M34 N16
58.870 58.870
0 . 0 0 1
M35 N16
58.868 58.870
0.003
M48 N2
97.174 97.172
0.003
M49 N2
97.174 97.172
0.003

```

\subsection*{4.2 Parameters \\ of execution}
```

Version: 5.5
Machine: SGI/ORIGIN 2000 R10000
Obstruction memory:
16 Mo
Time CPU To use: 3 s

```

\section*{5 \\ Summary of the results}

The results are in very good agreement with reference SYSPIPE. They validate the use of the elements POU_D_T and POU_C_T for calculations of lines of industrial pipings subjected to thermal and mechanical loadings.

Handbook of Validation
V3.01 booklet: Linear statics of the linear structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:

SSLP100 Stick in static substructure
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.02.100-B Page:
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Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V3.02 booklet: Linear statics of the plane systems
Document: V3.02.100

\section*{SSLP100-Stick in static substructure}

\section*{Summary:}
static Under-structuring: condensation of the matrices of rigidity and the loadings.
Linear behavior.
Plane model.
2 Modelings:
Model •a: "ordinary" plan: it is the reference solution.
- B: models with substructures.

\section*{Interest:}
- under-structuring on two levels,
- rotation of the macronutrients and the loadings (following or not),
- calculation of the fields inside the macronutrients.

The results of \(B\) are identical to those of \(A\) with a margin of 105.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLP100 Stick in static substructure
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.02.100-B Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(y\)
E
N27
N19
G
N25
N17
C
P2
4
N11
N8
F
P3
\(D\)
H
\(45^{\circ} 45^{\circ}\)
P4
\(45^{\circ}\)
O
X
With
B
4
I
P1
N33
J
1
3
1.2

Material properties
\(E=15 . P a\)
\(=0.3\)
1.3

Boundary conditions and loadings
\(\cdot[G H]: U+v=0 ; N 8, N 17\) and N25: \(U=v=0 ; J: U=2.0\)
- loading case 1: pressure distributed on \(\mathrm{ADFH} p=10.0\)
- loading case 2: N11, N19, N27, N33, P1: Fy = -20.0

\subsection*{1.4 Conditions}
initial
Without object.

\author{
Handbook of Validation
}

V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLP100 Stick in static substructure
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.02.100-B Page:
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2
Reference solution
2.1

Method of calculation used for the reference solution
This problem does not have a reference solution.
Modeling \(A\) is used as reference for modeling \(B\).
2.2

Results of reference
Displacements \(U\) and \(v\) at the points P1, P2, P3, P4.
2.3

Uncertainty on the solution
The solution of "reference" depends on the space discretization of the model; this is why grid is drawn in [§1.1].
Modeling B must respect this grid to lead to the same results as \(A\).
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLP100 Stick in static substructure
Date:
19/01/98
Author (S):
J. PELLET

Key:
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3 Modeling
With
3.1

Characteristics of modeling
24 elements QUAD4, modeling: "D_PLAN"
E
20
19
27
18
P2
17
C
G
12
28
25
9

\section*{F P3}

H 22
2
D
1
4
7
10 P4
O
With
B
30
32
34
36
29
31
33
35

P1
J
3.2

Characteristics of the grid
A number of nodes: 36 .
A number of meshs and types: 24 QUAD4
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLP100 Stick in static substructure
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.02.100-B Page:
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4

\author{
Results of modeling A \\ 4.1 Values \\ tested \\ Identification \\ Reference \\ Aster \\ \% difference \\ P1 U \\ 1.88327 \\ P1 v \\ 2.59224102 \\ P2 U \\ 8.27372102 \\ case of \\ P2 v \\ 8.27372102 \\ charge \\ P3 U \\ 2.70375101 \\ \(\mathrm{n}^{\circ} 1\) \\ P3 v \\ 5.69552101
}

P4 U
5.17703101

P4 v
5.43387101

P1 U
1.71883

P1 v
6.04367

P2 U
4.60196102
case of
P2 v
4.60196102
charge
P3 U
2.26903101
\(\mathrm{n}^{\circ} 2\)
P3 v
6.14296101

P4 U
9.57110101

\section*{4.2}

\section*{Contents of the file results}

Displacements
4.3 Parameters
of execution
Version:
3.02

Machine:
CRAY C90
System:
8.0 UNICOS

Obstruction memory:
8 megawords
Time CPU To use:
4 seconds
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLP100 Stick in static substructure
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Author (S):
J. PELLET

Key:
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5 Modeling
B
5.1

Characteristics of modeling
E

G
C
P2
F
H

\section*{P3 D \\ B \\ O \\ With \\ P4 \\ I \\ J \\ P1}

The initial grid my (level -2 of the under-structuring) contains only the 12 QUAD4 of IJBA and ABCD.
The macr_elem_stat (S_1) is defined starting from the elements of ABCD. This macr_elem_stat is digest on the nodes of AB and CD (level -2).
C
D
C
With
B
D
I
With
B
my
J
S_1
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
SSLP100 Stick in static substructure

\section*{Date:}

19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.02.100-B Page:
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The grid ma_123 of level -1 is defined while making turn 2 S_1 times to represent the crown ABCEGHFDA.
E
C
G
G
F
D
H
H
With
B
With
B
ma_123

S_123
The macr_elem_stat S_123 is defined starting from substructures ABCD, DCEF and FEGH. It macr_elem_stat is condensed on the nodes of AB and GH.
The grid mag0 is defined by the macr_elem_stat S_123.
The final grid mag (level 0 ) is defined by the grid mag0 which one assembles (ASSE_MAILLAGE) with the initial grid my to recover the meshs of IJBA.
The resolution is then made on this final grid, then one calculates displacements inside macr_elem_stat using operator DEPL_INTERNE.
G
\(\mathbf{H}\)
With
B
G
mag
H
With

\section*{B}
\(\operatorname{mag} 0\)
I
J

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 20.
A number of meshs and types: 12 QUAD4

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\section*{Keys}

MARCR_ELEM_STAT

\section*{DEFINITION}

OUTSIDE

\section*{RIGI_MECA}
[U4.44.01]
DEFI_MAILLAGE
DEFI_MAILLE
RECO_GLOBAL
DEFI_NOEUD
[U4.12.04]
AFFE_MODELE
AFFE_SOUS_STRUC
[U4.22.01]
ASSE_MAILLAGE
[U4.12.02]
DEPL_INTERNE
[U4.65.01]
CALC_VECT_ELEM
SOUS_STRUC
[U4.41.02]
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
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Titrate:
SSLP100 Stick in static substructure
Date:
19/01/98
Author (S):

\section*{J. PELLET}

Key:
V3.02.100-B Page:
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6
Results of modeling B
6.1 Values
tested
Identification

\section*{Reference}

Aster
\% difference
P1 U
1.88327
\({ }^{2} 105\)
P3 U
2.26903101
2.26903101
\({ }^{2} 105\)
P3 v
6.14296101
6.14296101
\({ }^{2} 105\)
P4 U
9.57110101
9.57110101
\({ }^{2} 105\)
P4 v
2.53878
2.53878
\({ }^{2} 105\)
6.2

\section*{Contents of the file results}

Displacements.

\subsection*{6.3 Parameters}

\section*{of execution}

Version:
3.02

Machine:
CRAY C90
System:
8.0 UNICOS

Obstruction memory:
8 megawords
Time CPU To use:
9 seconds
7

\section*{Summary of the results}

The precision of the results obtained (error \({ }^{2} 105\) ) is natural because the static under-structuring is one "exact" method (in infinite numerical precision).
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)

\author{
Version
}
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints
Date:
11/05/04
Author (S):
X. DESROCHES Key

V3.02.101-D Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
Document: V3.02.101

SSLP101 - Rate of refund of energy
in plane constraints

\section*{Summary:}

It is about a test of breaking process in statics for a two-dimensional problem. One is considered plate fissured in plane constraints, the functionalities tested are:
the rate of refund of energy \(G\),
the rate of refund of energy calculated starting from the calculation of the coefficients of constraints K1 and K2.

The interest of the test is to compare the traditional value of \(G\) and the value of \(G\) (IRWIN) obtained starting from K1 and
K2. It also makes it possible to test the invariance of calculation compared to the crowns of integration.

This test contains 3 different modelings: the modeling A which treated calculation of the Integral of Rice is not
more supported since Version 3.

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints
Date:
11/05/04
Author (S):
X. DESROCHES Key
:
V3.02.101-D Page:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

Rectangular plate with emerging crack OC.
For reasons of symmetry, the model is tiny room to the half-structure y 0.

Height plates: \(H=250 \mathrm{~mm}\)
Width plates: \(I=100 \mathrm{~mm}\)
Depth fissures: have \(=37.5 \mathrm{~mm}(O C)\)

\section*{1.2}

Material properties
\[
E=200000 M P a=0.3
\]

Assumption of the plane constraints.

\section*{1.3}

Boundary conditions and loadings

Constraint imposed in \(Y=H\) :
\(=1 \mathrm{MPa}\)

Displacement for the edge CA defined by: has \(X L\) and \(y=0\)
\(v=0\).
-
Not fixes \(a\) :
\(U=v=0\).
For modeling \(C\) one replaces the constraint imposed by a pressure on the lips of fissure.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
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Code_Aster \({ }^{\circledR}\)
Version

\section*{Titrate:}

SSLP101 - Rate of refund of energy in plane constraints
Date:
11/05/04
Author (S):
X. DESROCHES Key
:
V3.02.101-D Page:
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2
2.1

Method of calculation used for the reference solution
Reference solution of BROWN \& STRA WLEY [bib1]:
\[
J=F 2 \text { has } 2 / E \text { with } F=198
\]
.
has
mm
in

2
and \(E\)
in
N/mm

\section*{2.2}

Results of reference for \(G\)
Results of reference \(G=\)
\(2 \times \times\)
\(\times\)
\(-5=\)

I
The formula \(G(\) IRWIN \()=\)
(2)

K1 + K2) led, like K
0, with K
,
21491
E
\(2=\)
\(1=\)

\section*{2.3}

Results of reference for the derivative of \(G\)
While varying the Young modulus and the Fy loading, one notes that:

G
\(\boldsymbol{G}=\boldsymbol{F}\) with
3
310
2
=
that is to say
=
F
2
\(Y\)
\(Y\)
```

F
Y
G
G
G=
with =
4 6 0 ~ i s
= -
E
E
E
2.4 Reference
bibliographical

```
[1]
Special BROWN-STA WLEY ASTM Technical Publication \(n^{\circ} 410\) (1966)
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLP101-Rate of refund of energy in plane constraints
Date:
11/05/04
Author (S):
X. DESROCHES Key
:
V3.02.101-D Page:
\(4 / 10\)

\section*{3 Modeling B}

\section*{3.1 \\ Characteristics of modeling}

Ray
Center C

Ymax
Ymin
Xmin
Xmax

One calculates the field then the rate of refund of energy G, the coefficients of K1 constraints and K2, the rate of refund of energy obtained by the formula of IRWIN, the direction of propagation of the crack.

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 673 \\ A number of meshs and types: 112 meshs QUAD8 and 142 meshs TRIA6
}

\subsection*{3.3 Functionalities}
tested
Orders

MECHANICAL AFFE_MODELE
C_PLAN
ALL
AFFE_CHAR_MECA

\section*{MECA_STATIQUE}

CALC_THETA THETA_2D

\title{
Handbook of Validation \\ V3.02 booklet: Linear statics of the plane systems \\ HT-66/04/005/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints

\section*{Date:}

\section*{11/05/04}

Author (S):
X. DESROCHES Key
:
V3.02.101-D Page:
5/10

\section*{4 \\ Results of modeling \(B\)}

\subsection*{4.1 Values}
tested

> The values tested are the rate of refund of energy calculated by the method théta and the rate of restitution of energy calculated by the formula of IRWIN starting from the coefficients of intensity of constraints
> K1 and K2.

\section*{Identification Reference}

Aster \%
difference
Crown 1 G
2.3093 10-3 2.2994

10-3 4.
10-3

\section*{Crown 2 G}
2.3093 10-3 2.2993

10-3 4.
10-3
Crown 3 G
2.3093 10-3 2.2993

10-3 4.
10-3
Crown 4 G
2.3093 10-3 2.2991

10-3 4.
10-3
Crown 5 G
2.3093 10-3 2.2981

10-3 5.
10-3
Crown 6 G
2.3093 10-3 2.2906

10-3 8.
10-3
Crown 1 G (IRWIN)
2.3093 10-3 2.2990

10-3 4.
10-3
Crown 2 G (IRWIN)
2.3093 10-3 2.2989

10-3 4.
10-3
Crown 3 G (IRWIN)
2.3093 10-3 2.2988

10-3 5.
10-3
Crown 4 G (IRWIN)
2.3093 10-3 2.2986

10-3 5.
10-3
Crown 5 G (IRWIN)
2.3093 10-3 2.2976

10-3 5.
10-3
Crown 6 G (IRWIN)
2.3093 10-3 2.2891

10-3 8.
4.2 Notice

\section*{1}
The calculation of G, K1, K2, G (IRWIN) = (2 2
K + K2) was carried out starting from 6 different fields, E correspondents each one with a circular ring centered out of \(C\).

\section*{Handbook of Validation}

V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A

\title{
X. DESROCHES Key
}

\section*{5 Modeling}

C

\section*{5.1 \\ Characteristics of modeling}

Ray
Center C

\author{
Ymax
}

Ymin
Xmin
Xmax

The loading differs:
one relieves the stress imposed in \(Y=H\),
one imposes a pressure \(p=1\) on the lips of the crack.

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 673
A number of meshs and types: 112 meshs QUAD8 and 142 meshs TRIA6

\author{
5.3 Functionalities \\ tested \\ Orders \\ MECHANICAL AFFE_MODELE \\ C_PLAN ALL \\ AFFE_CHAR_MECA \\ \section*{MECA_STATIQUE}
}

CALC_THETA THETA_2D

CALC_G_THETA

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints
Date:
11/05/04
Author (S):
X. DESROCHES Key
:
V3.02.101-D Page:
7/10

\section*{Results of modeling \(C\)}

\subsection*{6.1 Values}
tested
Values of \(G\)

\section*{Identification Reference}

Aster \%
difference
Crown 1 G
2.3093 10-3 2.2994

10-3 4.
10-3
Crown 2 G
2.3093 10-3 2.2993

10-3 4.
10-3
Crown 3 G
2.3093 10-3 2.2993

10-3 4.
10-3
Crown 4 G
2.3093 10-3 2.2991

10-3 4.
10-3
Crown 5 G
2.3093 10-3 2.2981

10-3 5.
10-3
Crown 6 G
2.3093 10-3 2.2906

10-3 8.
10-3
Crown 1 G (IRWIN)
2.3093 10-3 2.2990

10-3 4.
10-3
Crown 2 G (IRWIN)
2.3093 10-3 2.2990

10-3 4.
10-3
Crown 3 G (IRWIN)

\subsection*{6.2 Notice}
E
that for preceding modeling. The results are identical.

\author{
Handbook of Validation
}

V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints

Date:
11/05/04
Author (S):
X. DESROCHES Key
:
V3.02.101-D Page:
8/10
7 Modeling
F
7.1

\section*{Characteristics of modeling}

Ray
Center C

Ymax
Ymin
Xmin
Хтах
One applies successively:
a surface force \(F y=1\) in \(Y=H\),
a pressure \(p=1\) on the lips of the crack.
and one calculates \(d G / d F y\) and \(d G / d p\) on 2 different crowns.
In the 2 cases one calculates \(d G / d E\) on the same crowns.

\section*{7.2}

Characteristics of the grid
A number of nodes: 673
A number of meshs and types: 112 meshs QUAD8 and 142 meshs TRIA6

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

\author{
MECHANICAL AFFE_MODELE \\ C_PLAN ALL \\ AFFE_CHAR_MECA
}

\section*{MECA_STATIQUE}

CALC_THETA THETA_2D

CALC_G_THETA_T SENSITIVITY

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints
Date:
11/05/04
Author (S):

\section*{X. DESROCHES Key}
:
V3.02.101-D Page:
9/10

\section*{8}

\section*{Results of modeling \(F\)}

\subsection*{8.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
dg/dE, crown \(n^{\circ} 1\)
1.15E-8
1.149E-8
0.03
\(d g / d E\), crown \(n^{\circ} 2\)
1.15E-8
1.149E-8
0.03
dg/dFy, crown \(n^{\circ} 1\)
4.6E-3
4.599E-3
0.03
dg/dFy, crown \(n^{\circ} 2\)
4.6E-3
4.599E-3
0.03
\(d g / d p\), crown \(n^{\circ} 1\)
4.6E-3
4.599E-3
0.03
\(d g / d p\), crown \(n^{\circ} 2\)
4.6E-3
4.599E-3
0.03

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLP101 - Rate of refund of energy in plane constraints
Date:
11/05/04

\section*{Author (S):}

\section*{X. DESROCHES Key}

\section*{:}

V3.02.101-D Page:

\section*{9 \\ Summary of the results}

The calculation of \(G\) and its derivative is not sensitive to the choice of the field of integration.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HT-66/04/005/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLP102 Rate of refund of energy with initial deformations
Date:
19/01/98
Author (S):

\section*{G. DEBRUYNE}

Key:
V3.02.102-A Page:
1/6
Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

V3.02 booklet: Linear statics of the plane systems

\section*{Document: V3.02.102}

SSLP102-Rate of refund of energy with
initial deformations (Lagrangian propagation)
Summary
This test makes it possible to calculate the rate of refund of energy \(G\) for a static problem of mechanics in plane deformations with initial deformations by a Lagrangian method of propagation of crack and of
to check the stability of the calculation of \(G\) with initial deformations on 4 different crowns of integration.
This test contains a modeling in plane deformations.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)

\section*{Version}
4.0

Titrate:
SSLP102 Rate of refund of energy with initial deformations

\section*{Date:}

19/01/98
Author (S):

\section*{G. DEBRUYNE}

Key:
V3.02.102-A Page:
2/6
1

\section*{Problem of reference}

\subsection*{1.1 Geometry}

It is about a rectangular plate with side crack (one represents only half of the structure).

\section*{\(X\)}
has
Height plates: \(H=200\).
Width plates: \(I=100\).
Length fissures: has \(=50\).

\section*{1.2}

Material properties
Young modulus:
\(E=2.104 \mathrm{MPa}\)
Poisson's ratio:
\(=0.3\)
We place ourselves on the assumption of the plane deformations.

\section*{1.3}

\section*{Boundary conditions and loadings}
- Déplacements for \(A<X<I: y=0 .: v=0\).
\(0<X<I: y=H .: v=0\).
- Point fixes b: \(U=v=0\).
- Initial Déformations: (=

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLP102 Rate of refund of energy with initial deformations
Date:
19/01/98
Author (S):

\section*{G. DEBRUYNE}

Key:
V3.02.102-A Page:
3/6
2
Reference solution

\section*{2.1}

\section*{Method of calculation used for the reference solution}

The results of reference result from a test WILSON and YU [bibl] or from a calculation of \(J\) (integral from
Rice) in thermoelasticity in the cards of the ECA [bib2] whose characteristics are as follows:
\[
\begin{aligned}
& = \\
& - \\
& 5106{ }^{\circ} \mathrm{C} \\
& T(X)=2 \mathrm{To} \mathrm{X} / \mathrm{L} \\
& \mathrm{To}= \\
& { }_{\circ} \\
& 100 \mathrm{C} \\
& E=20.000 \mathrm{MPa},=03
\end{aligned}
\]

One thus imposes
\[
\begin{aligned}
& = \\
& = \\
& =1=
\end{aligned}
\]
\(x x\)
yy zz \(F\)
2 To X/L
2.2

Results of references

The result for \(J\) in card ECA (Code CASTEM 2000) is \(J=0.390 \mathrm{kgf} / \mathrm{mm}\) (not indication on contours).
The results resulting from WILSON and YU vary for five different contours:
1
0.392

2
0.362

3
0.369

4
0.361

5
0.371

\subsection*{2.3 References}
bibliographical
[1]
The Uses of J-Integrals in thermal stress ace problems - International Newspaper of Fracture (1979) WILSON and YU.
[2]
Breaking process, methods numerical for engineer IPSI 18 November 20, 1986, card-index validation CASTEM pp 47-48
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLP102 Rate of refund of energy with initial deformations
Date:
19/01/98
Author (S):
G. DEBRUYNE

Key:
V3.02.102-A Page:
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\section*{3 Modeling}

\section*{With}
3.1

\section*{Characteristics of modeling}

Y
0

\section*{With}
\(X\)
\(F(X)\)
5.10-4

X
50.

The initial deformations are such as:

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes: 853
A number of meshs and types: 359 TRIA6 and 27 QUAD8

\subsection*{3.3 Functionalities}
tested
Orders

\section*{Keys}

AFFE_CHAR_MECA
EPSI_INIT
[U4.25.01]
CALC_MATR_ELEM
RIGI_MECA_LAGR
[U4.41.01]
CALC_VECT_ELEM
[U4.41.02]
CALC_G_THETA
OPTION
CALC_G_LAGR
[U4.63.03]
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLP102 Rate of refund of energy with initial deformations
Date:
19/01/98
Author (S):

\section*{G. DEBRUYNE}

Key:
V3.02.102-A Page:
5/6

\section*{4 \\ Results of modeling A}

\subsection*{4.1 Values}

\section*{tested}
\(J\) Reference
G

\section*{Identification}

\section*{J: CASTEM}

WILSON and YU

\section*{Aster}
\% difference
contour 1
0.195*
0.196
0.180

9 \%
contour 2
0.181
0.180
\(0.5 \%\)
contour 3
0.184
0.180

2 \%
contour 4
0.180
0.180

0 \%
* not of indication on contour corresponding to this value.

\subsection*{4.2 Remarks}

It is necessary to multiply by 2 the rough results since one accounted for 1 half-structure (in one half-crown).
The method of calculation used for this result (ECA) is the integral J. the results of J resulting from WILSON and YU are close to the Aster result (less than 2\%) except for crown 1 (or contour 1 in the case of J).
The classification of contours or the crowns corresponds to an order ascending of the ray of these the last

\subsection*{4.3 Parameters}
of execution
Version: 3.06
Machine: CRAY C90
System: UNICOS
8.0

\section*{Obstruction memory:}

8 megawords
Time CPU To use:
22 seconds
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLP102 Rate of refund of energy with initial deformations
Date:
19/01/98
Author (S):

\section*{G. DEBRUYNE}

Key:
V3.02.102-A Page:
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5

\section*{Summary of the results}

The variation of the results of G with the values of 5 data in WILSON and YU is less than \(2 \%\) except for smallest contour where the variation reaches \(9 \%\). No indication of contour is mentioned for the single value indicated in card CASTEM, this value is very close to that of WILSON and YU for smallest contour. The invariance of G following the crown is excellent for Aster calculation. Let us note the difference in modeling between this test where deformations directly are imposed initial and results of reference where these deformations are induced by an imposed temperature. The grid corresponds to this case test is also appreciably finer than that used for results of reference.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster ®}

Version
3
Titrate:
SSLP103 Calculation of \(K I\) and \(K I I\) for a fissured circular plate
Date:
24/08/99
Author (S):
E. SCREWS

Key:
V3.02.103-A Page:
1/6
Organization (S): EDF/IMA/MMN

\section*{Handbook of Validation}

\section*{V3.02 booklet: Linear statics of the plane systems}

\section*{Document: V3.02.103}

SSLP103 - Calculation of the coefficients of intensity of

\section*{constraints \(K I\) and \(K I I\) for a circular plate \\ fissured in linear elasticity \\ Summary}

It is about a test of breaking process in static linear elasticity for a two-dimensional problem. One consider a circular plate fissured (with a tilted crack of 30 degrees compared to the axis of
X-coordinates) for which one calculates:
- coefficients of intensity of constraints KI and KII,
- the rate of refund of energy G starting from the formula of IRWIN.

The interest of the test is to know the analytical solution which gives the coefficients of intensity of constraints and
to have a tilted crack.
This test includes/understands a modeling which treats successively the plane strains and the plane stresses
(elements of continuous mediums).
The numerical results do not deviate more than 1 to \(2 \%\) from the values of reference.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SSLP103 Calculation of \(K I\) and \(K I I\) for a fissured circular plate
Date:
24/08/99
Author (S):
E. SCREWS

Key:
V3.02.103-A Page:
2/6
1
Problem of reference

\subsection*{1.1 Geometry}

It is about a circular plate of ray \(0 \mathrm{~A}=100 \mathrm{~mm}\), with a tilted crack of 30 degrees by report/ratio with the x -axis.

\section*{Y}
\(30^{\circ}\)
0
X
With
1.2

Material properties

The characteristics of material are as follows:
\(\mathrm{E}=200.000 \mathrm{MPa}\)
\(=0.3\)
1.3

\section*{Boundary conditions and loadings}

Displacements are imposed on the contour of the plate. They result from the analytical solution singular in mixed mode (with \(K I=2\). and \(K I I=1\).).
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SSLP103 Calculation of \(K I\) and \(K I I\) for a fissured circular plate
Date:
24/08/99
Author (S):

\section*{E. SCREWS}

Key:
V3.02.103-A Page:
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2

\section*{Reference solution}
2.1

Method of calculation used for the reference solution
Y
x2
M
R
x1

O
X
In plane strains or plane stresses, the distribution of displacements is given in it locate ( \(0, \mathrm{x} 1, \mathrm{x} 2\) ) by:
\(1+\)
R
```

$=$
$K \cos (K-\cos )+$

```
K
\(\sin\)
( \(K \cos 2\) )
\(E\)
\(I\)
II
2
2
2
\(U=\)
\(K \sin\)
( \(K-\cos\) ) -
K
cos
\((K \cos 2)\)
2
\(E\)
\(I\)
2
2
2
with \(K=3-4\) in plane deformations
3 -
\(K=1+\) in plane constraints
\(U\)
\(=\cos u l-\sin U\)
\(X\)
2
or in the reference mark ( \(\mathrm{O}, \mathrm{X}, \mathrm{Y}\) ) by: \(U\)
\(=\sin u l+\cos U\)
Y
2
On the contour of the plate, one a: \(\mathrm{R}=0 \mathrm{~A}=100\) Misters.
One chooses to take \(K I=2\). and \(K I I=1\). and to impose displacements on the contour of the plate circular.

\section*{2.2}

Results of reference
\(K I=2\).
\(K I I=1\).
\(\mathrm{G}=2.275105\)
in plane deformations
\(\mathrm{G}=2.5105\)
in plane constraints

\subsection*{2.3 References \\ bibliographical}
[1]
Breaking process H.D. BUI Fragile - ED. Masson 1978
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
3
Titrate:
SSLP103 Calculation of \(K I\) and \(K I I\) for a fissured circular plate
Date:

\section*{E. SCREWS}

Key:
V3.02.103-A Page:
4/6
3 Modeling
With

\section*{3.1}

\section*{Characteristics of modeling}

Calculation is carried out in plane constraints (C_PLAN) then in plane deformations (D_PLAN).
0
With
3.2

Characteristics of the grid
A number of nodes: 737
A number of meshs and types: 204 meshs QUAD8, 30 meshs TRIA6

\subsection*{3.3 Functionalities}

\section*{tested}

Orders
Keys
CALC_G_THETA
CALC_K_G
U4.63.03
CALC_G_THETA
CALC_G
U4.63.03
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster ©}

Version
3
Titrate:
SSLP103 Calculation of \(K I\) and \(K I I\) for a fissured circular plate
Date:
24/08/99
Author (S):
E. SCREWS

Key:
V3.02.103-A Page:
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\section*{4}

\section*{Results of modeling A}

\subsection*{4.1 Values}

\section*{tested}

The values tested are the coefficients of intensity of constraints \(K I\) and \(K I I\) and the rate of refund of energy G calculated by the formula of IRWIN:

\section*{Identification}

\section*{Reference}

\section*{Aster}

\author{
\% difference
}

Plane constraints
K
2.0
2.0067
0.33

I
K
1.0
0.9877
1.23

II
G
2.5 10-5
2.5213 10-5
0.85

Plane deformations
K
2.0
2.0030
0.15

I
K
1.0
0.9960
0.39

II
G
\[
2.275 \text { 10-5 }
\]
2.2968 10-5
0.96
4.2 Remarks
(1-2) 22
The formula of IRWIN gives:
```

G=
(K +K
I
II)

```
in plane deformations
E
1
2
2
and
\(G=\)
( \(K+\) K
I
II)
in plane constraints
E

Calculations are carried out with a crown of lower integration of ray 10.0 and ray superior 20.0.
4.3 Parameters
of execution
Version: 3.06
Machine: CRAY C98
System: UNICOS
8.0

Obstruction memory:
8 MW
Time CPU To use:
22 seconds
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
3
Titrate:
SSLP103 Calculation of \(K I\) and \(K I I\) for a fissured circular plate
Date:
24/08/99
Author (S):
E. SCREWS

Key:
V3.02.103-A Page:

\section*{Summaries of the results}

Numerical values of the coefficients of intensity of constraints and the rate of refund of energy do not deviate more than 1 to \(2 \%\) from the values of reference, which is satisfactory.
The grid could be improved, in particular in the vicinity of the bottom of crack.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/96/016 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
6.5

\section*{Titrate:}

SSLP105 - Excavation of a circular tunnel in an elastic solid mass
Date:
09/09/03
Author (S):
A. COURTEOUS Key
:
V3.02.105-A Page:
1/8
Organization (S): EDF-R \& D /MMC

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
Document: V3.02.105

SSLP105-Excavation of a circular tunnel in
a linear elastic solid mass

\section*{Summary:}

This test constitutes an example of implementation of a total methodology for simulation two-dimensional of the digging and the supporting of a circular gallery in an underground solid mass with
Code_Aster.
To validate the step on the basis of simple analytical solution, one is brought to make assumptions restrictive on the geometry of the problem, the behavior of materials (elastic linear) and the field of constraint initial (isotropic). The reference solution is given by the method known as "convergence containment", traditional for this type of modeling 2D.

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Code_Aster \({ }^{\circledR}\)
Version
6.5

Titrate:
SSLP105-Excavation of a circular tunnel in an elastic solid mass
Date:
09/09/03
Author (S):
A. COURTEOUS Key

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1
Problem of reference

\subsection*{1.1 Geometry}

\title{
It is about a circular tunnel of section, covered by a concrete ring, which one excavates in one
} solid mass of ground. The two materials are supposed to be elastic linear.

\author{
Z \\ 18,20 m \\ \(y\) \\ B \\ 0,30 m \\ \(1,50 \mathrm{~m}\) \\ X \\ 20 m \\ With \\ 20 m
}

\section*{1.2}

Properties of material
The materials are elastic linear.

\subsection*{1.2.1 Ground}
\(E s=4 G P a\)
\(S=0,3\)

\subsection*{1.2.2 Concrete}
\(E b=20 G P a\)
B = 0,2
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\section*{A. COURTEOUS Key}

\section*{1.3}

Initial conditions, boundary conditions and loadings
The constraints in the solid mass are supposed initially isotropic ( \(x x=y y=z z=0\) ). method used to simulate the excavation and the installation of supporting is the method known as "convergence-containment" presented for example in [bib1] and [bib2].

The guiding principle rests on a reduction in the nodal reactions generated at the edge of the future one
gallery by the initial state of stresses. This operation is indicated by name "déconfinement". When déconfinement the value reached which corresponds to the conditions of building site which one wishes to model, one carries out the simulation of the installation of concrete supporting
at the edge of the gallery.
Solid mass of ground
Solid mass of ground
Excavation of
Pose
Initialization of
gallery
coating concrete
constraints
Calculation of the reactions
nodal
1

2

The boundary conditions and the loading are summarized in the following table. Phases correspond to those of the diagram above, the edges are composed with the nodes identified on the diagram of the paragraph [ \(\$ 3.1]\) and between brackets the name of the groups of mesh or node of file .com m).

\section*{Edges}

Phase 1
Phase 2
Phase 3

\title{
Phase 4 \\ N0N1 (in \\ \(D Y=0\) \\ \(D Y=0\)
}

\section*{- \\ no_bas1)}

NIN2
\(D Y=0\)
\(D Y=0\)
\[
D Y=0
\]
(bas_bet)
N2N3
\[
D Y=0
\]
\[
D Y=0
\]
\[
D Y=0
\]
\[
D Y=0
\]
(no_bas2)
N3N4
\[
D X=0
\]
\[
D X=0
\]
\[
D X=0
\]
\[
D X=0
\]
(no_droit)
N4N5
```

yy
5
-
N5N6
DX=0
DX=0
DX=0
DX=0
(no_left2)
N6N7
DX=0
DX=0
-
DX=0
(no_left_bet)
N7N0 (in
DX=0
DX=0
-
-
no_left1)
N6N2 (edge)

```
-
Nodal reactions
-
corresponding to
déconfinement
N7N1 -
-
-
Free

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\section*{A. COURTEOUS Key}

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

\subsection*{2.1.1 Behavior of the ground}

That is to say the rate of déconfinement, which represents the relative position of the section of tunnel considered
compared to the coal face. In the method "convergence - containment", one replaces the future ground excavated by a tensor of the constraint are equivalent, which one cause a drop in the intensity via for
to simulate the digging and the distance of the coal face.

Coal face
Tunnel
\(=0\)
0
= 1
(-).
0
=

The solution of the problem is thus similar to that of the infinitely thick tube charged by a pressure intern of intensity (1) . 0 and by an external pressure of intensity 0 (see [bib3] for the detail of calculations).

Constraints radial, orthoradiale as well as radial displacement with the wall of the tunnel in springy medium subjected toa rate of déconfinement are as follows

\section*{R}
. 2
0
\(R=1\)

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\subsection*{2.1.2 Behavior of supporting}

Supporting will be opposed to the movement convergence natural of the tunnel and thus will apply one artificial containment with the rock.

Either Ks the stiffness of supporting, it is given by the following relation if it is considered that it supporting is comparable to a thin tube ( \(B\) is the Poisson's ratio of the concrete):
\(=(E\)
K
B
\(S\)
- 2

1B) ER

K
If K
\(S\)
\(S=\)
represent the relative rigidity of the concrete compared to the solid mass and
2 G
D
the rate of
déconfinement with the installation of supporting, then radial constraints and orthoradiales thus

\section*{that radial displacement in wall are given by [bib1]:}
\[
\begin{aligned}
& =k s \\
& 0 \\
& R \\
& (1-D) \\
& 1+k s \\
& \\
& K \\
& = \\
& S \\
& (1+ \\
& 0 \\
& D) \\
& 1+k s \\
& 0 \\
& 1+D \\
& R= \\
& K \\
& U \\
& S \\
& R \\
& 1+k s \\
& 2 G
\end{aligned}
\]

\section*{2.2 \\ Sizes and results of reference}

One tests the following sizes on the level of the wall at points A and B of the figure of the 1.1, at the moment
where déconfinement is total:
- radial constraint: yy of \(A\) or \(z z\) out of \(B\);
- constraint orthoradiale: zz of \(A\) or yy out of \(B\);
- radial displacement: uy of \(A\) or uz out of \(B\).

\title{
2.3 \\ Uncertainties on the solution
}

None. Exact analytical result.

\subsection*{2.4 References \\ bibliographical}
[1]
The calculation of the tunnels by the method convergence-containment, Mr. Panet, Presses of the ENPC 1995
[2]
How to simulate the digging of a tunnel with Code_Aster? Principle of the method, implementation and validation, A. Courtois, R. Saidani, P. Sémété, note EDF HT-25/02/045/A 2002
[3]
Mechanics of the continuous mediums, volume 2, J. Salençon, ED. Ellipses - 1988
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\section*{A. COURTEOUS Key}

\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Modeling 2D in plane deformations.

\section*{3.2 \\ Characteristics of the grid}

A number of nodes:
8477
A number of meshs:
3304 of type QUAD 8

\subsection*{3.3 Functionalities \\ tested}

The objective of this case test is to test a method more than one quite precise functionality of Code_Aster, also the following table presents the principal orders which structure the file of orders.

\section*{Orders}

\section*{Comments}

CREA_CHAMP
Initialization of the constraints geostatics (here isotropic 5 MPa in compression)
STAT_NON_LINE Blocking of the nodes of the gallery for calculation of the reactions nodal to inject to simulate déconfinement

\section*{CREA_CHAMP}

Recovery of the nodal reactions
STAT_NON_LINE Re-injection of the nodal reactions
Intermediate STAT_NON_LINE Calculation to pass from a model without mesh representing the voussoirs concrete with a model with meshs them

\section*{A. COURTEOUS Key}

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\section*{3.4 \\ Sizes tested and results}

After the installation of the coating (urgent final), one tests the components and with the nodes \(N 2\) and N6
\(x x\)
\(y y\)
as well as radial displacement in these points (DX for N2, DY for N6).

\section*{Reference}

Aster Difference
(\%)
Node N 2
```

xx
-1,52821.106 -1,53154.106 0,218
yy
-8,47179.106 -8.52772.106 0,660
DX -1,6925.10-3 -1,6684.10-3 -1,422
N6 node

```
```

xx
-8,47179.106 -8,41147.106 -0,712
yy
-1,52821.106 -1,52943.106 0,080
DY -1,6925.10-3 -1.7184.10-3 1,529
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```
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\section*{4 \\ Summary of the results}

The values obtained with Code_Aster are in agreement with the values of the analytical solution of reference.

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLP303 - Plate cantilever at its end

Date:
23/09/02
Author (S):
J. Key Mr. PROIX
:
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Organization (S): EDF/AMA

\section*{Handbook of Validation}

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\section*{Document: V3.02.303}

SSLP303 - Plate cantilever charged with sound end

\section*{Summary:}

The goal of the test is to validate key word FORCE_CONTOUR, starting from a load applied at the end of one
plate.
The problem is dealt with in plane constraints.

\author{
Handbook of Validation
}

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Version
5.0

Titrate:
SSLP303-Plate cantilever at its end

Date:
23/09/02
Author (S):
J. Key Mr. PROIX
:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{\(P\) \\ Z}

H
\(C\)
\(\boldsymbol{F}\)
\(D\)
\(X\)

\section*{With}

Not \(E=\) medium of \(A B\)
not \(F=\) medium of \(\boldsymbol{c d}\).
Length: \(L=1 m\)
Width: \(L=0.1 \mathrm{~m}\)
Thickness: \(\boldsymbol{H}=0.005 \mathrm{~m}\)

Moment of inertia of section: \(l Y=\) \(=1.042 \times 10-9 \mathrm{m4}\)

Material properties
Young modulus: \(E=2.1\) X 1011 Pa
Poisson's ratio: \(v=0.3\)
1.3

Boundary conditions and loadings
- Encastrement of edge \(A D(U=v=0)\).
- Charge of resultant \(P=85\) NR, applied to edge BC (constant linear load).
1.4

Boundary conditions and loadings
Without object for the static analysis.
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\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

Titrate:
SSLP303 - Plate cantilever at its end

\section*{Date:}

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The value of the field of displacement \(v\), at the loose lead of the plate (edge BC) is given by:
PL3
\(v L=\)
(neglected shearing)
3EIY
from where \(v L=0.129 \mathrm{~m}\)

The stress field \(x x\) of inflection is given by:

PH
\(x x=\)
( \(L-X\) ) on edge \(A B\)
\(2 l Y\)
that is to say \(x x=2.04 X 108(L-X)(P a)\)

\section*{2.2}

Results of reference
- Déplacement vL of the nodes B and C
- Contraintes \(x x\) with nodes \(A\) and \(B\) and \(E\)

\section*{2.3}

Uncertainty on the solution
Analytical solution.
2.4 References
bibliographical
[1]
S. TIMOSHENKO, Resistance of Materials, 1st part. Polytechnic bookshop CH. Béranger, Paris, 1947. p 169 to 168

\section*{Handbook of Validation}

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\section*{3 Modeling \\ With}

\title{
3.1 \\ Characteristics of modeling \\ C-PLAN, meshs TRI6 and QUAD8
}
\(y, v\)
Meshs QUAD8
Meshs TRI6

F
C
D
\(X, U\)
With

B

\section*{E}

Not \(E=\) medium of \(A B\) not \(F=\) medium of \(C D\)
Cutting: 100 elements according to the length
2 elements according to the thickness
Boundary conditions:
on AD DDL_IMPO:
(GROUP_NO: encast
DX: 0 .
DY:
0.)

\section*{Loading:}
on BC FORCE_CONTOUR:
(GROUP_MA: bord_ch
FY: 170000.)

Name of the nodes:

Not \(A=N 1\)

Not \(D=N 403\)

Not \(B=N 455\)

Not \(E=N 201\)

Not \(C=N 756\)

Not \(F=\) N352

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: 905 \\ A number of meshs and types: 100 QUAD 8, 200 SORTED 6, 208 SEG 3}

\subsection*{3.3 Functionalities}
tested
Orders

AFFE-MODELE
"MECHANICAL"
"C_PLAN"
ALL
AFFE_CHAR_MECA
DDL-IMPO
GROUP_NO
FORCE_CONTOUR
GROUP_MA
CALC_CHAM_ELEM
OPTION
"SIGM_ELNO_DEPL"
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
5.0

\title{
Titrate: \\ SSLP303 - Plate cantilever at its end
}

\section*{Date:}

23/09/02
Author (S):
J. Key Mr. PROIX
:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Localization
Type of value
Reference
Aster
\% difference
Points B, C
\(v L\) (m)
0.1290 .12950 .4

Not A
xx (Pa)
2.041082 .08

108
2.1

Not E
\(x x\) (Pa)
1.02108
1.015108
0.5

\subsection*{4.2 Remarks}

The variation with the analytical solution of beam type or hurled plate, is due to the size of the grid could be tiny room with a finer grid.

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SSLP303-Plate cantilever at its end

Date:
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Author (S):

\author{
J. Key Mr. PROIX
}

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\section*{5 \\ Summary of the results}

This test, based on a solution of hurled plate, is treated in \(2 D\) (forced plane) in order to validate the loading of edge (key word FORCE_CONTOUR). The solution obtained is close to the solution analytical ( \(0.4 \%\) of difference on displacements) and thus this type of modeling validates.

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\section*{Titrate:}

SSLP304 - Orthotropic square plate in traction

Date:
12/12/02
Author (S):
J.M. PROIX, J.F. Key BILLAUD

\section*{Handbook of Validation}

V3.02 booklet: Linear statics of the plane systems
V3.02.304 document

SSLP304-Orthotropic square plate in traction uniaxial out of the axes of orthotropism

\section*{Summary:}

This test represents the static calculation of a square plate, out of orthotropic elastic material, of which axes othotropie are tilted 30 degrees compared to the basic edge, subjected to a uniaxial traction. It allows to validate the good taking into account of orthotropic elastic materials and the change of associated reference mark.
4 modelings are used: C_PLAN with meshs QUAD8 and TRIA6, in a first reference mark, C_PLAN in a second reference mark, COQUE_3D with meshs QUAD9 and TRIA7, in small displacements and COQUE_3D in
great displacements. Displacements and the constraints obtained are compared with a reference solution analytical.

\title{
The first two modelings of this test result from the validation independent of version 3 of
} Code_Aster (linear static batch).

\section*{Handbook of Validation}

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Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

SSLP304-Orthotropic square plate in traction

\section*{Date:}

12/12/02
Author (S):
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

A square plate, made up of a tilted orthotropic material of 30 degrees compared to edge AB.

With

\section*{B}

With \(B=1\) m, unspecified thickness (plane constraints), angle of orthotropism: \(=30\) degrees.

\section*{1.2}

Properties of materials
The properties of materials constituting the plate are:
orthotropic rubber band:
\(E_{-} L=4 . E 10\) Pa
\(E_{-} T=1 . E 10 \mathrm{~Pa}\)
\(G_{-} L T=0.45 E 10 \mathrm{~Pa}\)
\(G_{-} T N=0.35 E 10 \mathrm{~Pa}\)
\(N U_{-} L T=0.075\)
The axis Lis tilted 30 degrees compared to AB.

\section*{1.3}

Boundary conditions and loadings
- At point a: \(D X=0, D Y=0\)
- At point b: \(D X=0\),
- Linear Chargement distributed: \(\mathbf{F x}=104\) Pa on BC
- Linear Chargement distributed: \(\mathbf{F x}=-104\) Pa on DA

\subsection*{1.4 Conditions \\ initial}

Without object.
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

Analytical solution, obtained with the assumption of uniaxiality of the constraints:
\((X, y)\)
\(x x\)
\(=F\)
( \(X, y\) )
\(X\)
\(x y\)
\(=\)
( \(\boldsymbol{X}, \boldsymbol{y}\) )
\(y y\)
\(=\)
(X, y)
\(z z\)
\(=0\)
maybe in the reference mark \((A, L, T)\) :
\((X, y)=c 2 F,(X, y)=s 2 F\)
\((X, y)=-c s F\)
\(L L\)
\(X\)
\(T T\)
\(X\)
\(L T\)
\(X\)

By the orthotropic law of behavior elastic, by using conventions of Code_Aster in what relate to NU_LT, (cf document of use of order DEFI_MATERIAU [§3.5.2]), one obtains directly (see for example [bib1]):
\((X, y) F x\)
\(=\)
\(x y\)
, \((X, y)=-\)
F, 2
\(X\)
\((X, y)=\)
F
\(x x\)
E
\({ }^{y} \boldsymbol{y}\)
\(X\)
\(E\)
\(x y\)
\(X\)
\(E\)
\(X\)
\(X\)
\(X\)
with:
1
4
4
C
\begin{tabular}{|c|}
\hline \multirow[t]{2}{*}{\(S\)
1} \\
\hline \\
\hline 1 \\
\hline 1 \\
\hline 1 \\
\hline \(x y\) \\
\hline 4 \\
\hline 4 \\
\hline 22 \\
\hline = \\
\hline + \\
\hline + C S \\
\hline - 2 LT \\
\hline LT \\
\hline \(=(C+S)\) \\
\hline - c2s2 \\
\hline + \\
\hline - \\
\hline \(E\) () \\
\hline E \\
\hline \(E\) \\
\hline \(\boldsymbol{G}\) \\
\hline E \\
\hline \(E\) () \\
\hline E \\
\hline \(E\) \\
\hline E \\
\hline \(\boldsymbol{G}\) \\
\hline \(X\) \\
\hline \(X\) \\
\hline \(L\) \\
\hline T \\
\hline \(L T\) \\
\hline T \\
\hline \(T\) \\
\hline \(L\) \\
\hline \(T\) \\
\hline \(L T\) \\
\hline
\end{tabular}

As the deformations are uniform in the plate one obtains, by integration, displacements in the reference mark \((A, X, y)\) :
```

U(X,y)=. X
X
xx
U(X,y)=.y+2.X
y
yy
xy

```

\section*{2.2}
```

Results of reference
Displacements in the reference mark $(A, X, y)$ (in m):
Not B
$C$
$D$
$U$
0.
$5.91710-75.917$
$10-7$
$X$
$U$
-2.292 10-7 -5.028
$10-7-7.319$
$10-7$
$X$
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SSLP304-Orthotropic square plate in traction

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Constraints in the reference mark related to the orthotropism:
\[
(X, y)=7500 \mathrm{~Pa},(X, y)=2500 \mathrm{~Pa},(X, y)=4330.127 \mathrm{~Pa}
\]

LL
TT
LT

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution

\subsection*{2.4 References \\ bibliographical \\ [1] \\ GAY D: "Composite Materials"; 3rd edition, Hermès}

\author{
3 Modeling \\ With
}

\section*{3.1 \\ Characteristics of modeling}

Modeling C_PLAN. The plate is turned from -30 degrees around Z, i.e. axis X total is colinéaire with the axis of orthotropism L. the boundary conditions and loadings, to apply in locate \((A, X, y)\) related to the plate, are thus projected on the total reference mark \((A, X, Y)\) (use of LIAISON_DDL in B).

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\section*{3.2}

Characteristics of the grid
A number of nodes: 391
A number of meshs and types: 50 QUAD8, 100 TRIA6

\author{
3.3 Functionalities \\ tested \\ Orders \\ AFFE-MODELE \\ "AFFE" \\ MODELING = "C_PLAN" \\ AFFE_CARA_ELEM \\ SOLID MASS \\ \(A N G L \_R E P=0\) \\ AFFE_CHAR_MECA_F FORCE_CONTOUR FX, FY \\ AFFE_CHAR_MECA_F LIAISON_DDL \\ DEFI_MATERIAU ELAS_ORTH \\ MODI_REPERE \\ DEFI_REPERE \\ LOCATE = "USER"
}

\subsection*{3.4 Values}
tested
Value Identification Reference
Aster \%
difference
\(\boldsymbol{U x}(c)=\boldsymbol{U x}(\mathrm{D})\)
DX (C)
5.917 10-7 5.9167
10-7
0.007
Uy (B) DY(B)
-2.292 10-7-2.2916
10-7
0.01
Uy (C) DY
(C)
-5.028 10-7-5.0279
10-7
0.001
Uy (D)
DY(D)
-7.319 10-7 -7.3196
10-7
0.008
Sigma LL
SIXX (any point)
7500
7500.4
0.006
Sigma TT
SIYY (any point)
2500
2500.3
0.01
Sigma LL
SIXY (any point)
4300.127
433.06
0.01
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J.M. PROIX, J.F. Key BILLAUD
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\section*{4 Modeling}

B

\section*{4.1}

Characteristics of modeling
Modeling C_PLAN. The plate is parallel to the total axes, i.e. total axis \(X\) is colinéaire with axis \(X\). It is thus the axis of orthotropism \(L\) which is to be directed (using the MASSIVE key word of AFFE_CARA_ELEM).

\section*{4.2}

Characteristics of the grid
A number of nodes: 391
A number of meshs and types: 50 QUAD8, 100 TRIA6

\subsection*{4.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE

\author{
AFFE \\ MODELING = "C_PLAN" \\ AFFE_CARA_ELEM \\ SOLID MASS \\ ANGL_REP = 30 \\ AFFE_CHAR_MECA_F FORCE_CONTOUR \\ \(F X\), \\ FY \\ DEFI_MATERIAU ELAS_ORTH \\ MODI_REPERE \\ DEFI_REPERE \\ LOCATE = "USER" \\ Handbook of Validation \\ V3.02 booklet: Linear statics of the plane systems \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLP304-Orthotropic square plate in traction

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\subsection*{4.4 Values}
tested
Value Identification
Reference
Aster \%
difference
\(U x(c)=\boldsymbol{U x}(D)\)
DX (C)
5.917 10-7 5.9167
0.
Sigma TT
SIYY (any point)
2500
2500
0.

Sigma LL
SIXY (any point)
4300.127
4330.127
0.

\subsection*{4.5 Remarks}

Pace of the deformation: nonsymmetrical because of the orthotropism.

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\section*{:}

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\section*{5 Modeling}

C
5.1

Characteristics of modeling
Modeling COQUE_3D. The plate is parallel to the total axes, i.e. total axis \(X\) is colinéaire with axis \(X\). It is thus the axis of orthotropism \(L\) which is to be directed (using the MASSIVE key word of
AFFE_CARA_ELEM). The grid is identical to that of modeling B.

\section*{5.2}

Characteristics of the grid
A number of nodes: 541
A number of meshs and types: 50 QUAD9, 100 TRIA7

\subsection*{5.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE
AFFE
```

MODELING = "COQUE_3D"
AFFE_CARA_ELEM
HULL
ANGL_REP = 30
AFFE_CARA_ELEM
HULL
THICK = 1
AFFE_CHAR_MECA_F FORCE_ARETE
FX,
FY
DEFI_MATERIAU ELAS_ORTH

```

\subsection*{5.4 Values}
tested
Value Identification
Reference Aster \%
difference
\(U x(c)=\boldsymbol{U x}(D)\)
DX (C)
5.917 10-7 5.9167

10-7
0.006

Uy (B) DY
(B)
-2.292 10-7-2.29166
10-7
0.015

Uy (C) DY
(C)
-5.028 10-7 -5.0277
10-7
0.005

Uy (D)
DY (D)
-7.319 10-7 -7.3194
10-7
0.006

Sigma LL
SIXX (any point)
7500
7500
0.

Sigma TT
SIYY (any point)
2500
2500
0.

Sigma LL
SIXY (any point)
4300.127
4330.127
0.

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\section*{6 Modeling}

D
6.1

Characteristics of modeling
Modeling COQUE_3D in great displacements. The plate is parallel to the total axes, i.e. total axis \(X\) is colinéaire with axis \(X\). It is thus the axis of orthotropism \(L\) which is to be directed (using the MASSIVE key word of AFFE_CARA_ELEM). The grid is identical to that of modeling B.

\author{
6.2 \\ Characteristics of the grid \\ A number of nodes: 541 \\ A number of meshs and types: 50 QUAD9, 100 TRIA7 \\ \subsection*{6.3 Functionalities} \\ tested \\ Orders Key word factor \\ Key word \\ AFFE_MODELE \\ AFFE \\ MODELING = "COQUE_3D" \\ AFFE_CARA_ELEM \\ HULL \\ ANGL_REP \(=30\) \\ AFFE_CARA_ELEM \\ HULL \\ THICK = 1 \\ AFFE_CHAR_MECA_F FORCE_ARETE \\ \(F X\), \\ FY \\ DEFI_MATERIAU ELAS_ORTH \\ STAT_NON_LINE COMP_ELAS \\ DEFORMATION=' GRENN_GR'
}

\subsection*{6.4 Values}
tested
Value Identification
Reference Aster \%
difference
\(\boldsymbol{U x}(c)=\boldsymbol{U x}(\mathrm{D})\)
DX (C)
5.917 10-7 5.9167

10-7
0.006
\(U y(B) D Y\)
(B)

\section*{Handbook of Validation}

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\section*{7 \\ Summary of the results}

The results of four modelings are very close to the analytical solution: to the maximum 0.015 \% of variation for 4 modelings.

This test thus validates the taking into account of orthotropic elasticity.

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Titrate:
SSLP305-Thin disc in support under concentrated loading
Date:
23/09/02
Author (S):

\section*{J. Key Mr. PROIX}

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Organization (S): EDF/AMA

\title{
Handbook of Validation \\ V3.02 booklet: Linear statics of the plane systems \\ Document: V3.02.305
}

SSLP305 - Thin disc in support under load concentrated

\section*{Summary:}

The purpose of the test is to validate the calculation of the potential energy in linear elasticity.
Only one axisymmetric modeling is presented.
The reference solution is analytical.

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Titrate:

\section*{SSLP305-Thin disc in support under concentrated loading}

Date:
23/09/02
Author (S):
J. Key Mr. PROIX

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1
Problem of reference
1.1 Geometry

Thickness: \(\boldsymbol{H}=0.005 \mathrm{~m}\)

\section*{1.2 \\ Material properties}

Young modulus: \(E=2.1\) X 1011 Pa
Poisson's ratio: \(v=0.3\)

\section*{1.3}

Boundary conditions and loadings
- Appui on the edge \((W=0)\)
- Charge concentrated at point \(a: P=350 N R\)

\subsection*{1.4 Conditions}
initial
Without object for the static analysis.

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\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
- The value of axial displacement in the center of disc (not \(A\) ) is given by:
```

P23+v
WA=
X
64 D 1 +
v
Eh3
where D =
v2
121
(
)

- The value of the potential energy (with balance) is given by:
1
Ep=PW
2
has

```
- The absolute value of the potential energy by radian is:

1 PW
E
has
\(p=\)

2

2
2.2

Results of reference
- Déplacement at point a:
\(W A=0.4596 X 103 \mathrm{~m}\)
- Potential Energie by radian:
\(e p=0.012799 \mathrm{Nm} / r d\)

\begin{abstract}
2.3

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}
R.J. ROARK and W.C. YOUNG Formulated for stress and strain, 5th edition, New York, Mc Graw-Hill, 1975
\end{abstract}

\section*{Handbook of Validation}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ It is an axisymmetric modeling.}

\section*{With}

D

\section*{Limiting conditions:}
out of B
DDL_IMPO: (GROUP_NO: B
DY: 0.)
on \(A G\)
DDL_IMPO: (GROUP_NO: lAG
DX: 0.)
Loading:
in \(A\)
FORCE_NODALE: (GROUP_NO: With
FY:
-55.704)
Name of the nodes:
\(A=N 1\)
\(B=N 755\)
\(D=N 858\)
\(G=N 201\)
Cutting:
100 elements according to the ray
2 elements according to the thickness

\section*{3.2}

Characteristics of the grid
A number of nodes: 905
A number of meshs and types: 100 QUAD 8, 200 SORTED 6, 208 SEG 3
3.3 Functionalities
tested
Orders
AFFE_MODELE"MECHANICAL"
"AXIS"
ALL
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
FORCE_NODALE
GROUP_NO
POST_ELEM
ENER_POT
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\section*{4}
Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\author{
Localization
}

Type of value
Reference
Aster
\% difference
Not A
WA (m)
-0.4596 10-3
-0.4617 10-3
0.46
\(e p(N m / r d)\)
-1.2799 10-2
-1.2859 10-2
0.47

\subsection*{4.2 Remarks}
- The value of the load required is brought back to a sector of 1 radian. Consequently, the value potential energy given on the file result corresponds to the deformation of this sector (with the sign near).
- Option ENERPOT calculates in fact a deformation energy:

1
\(E=U T \boldsymbol{K} U\)
D
who is identical to the potential energy with the sign near:
2
1
1
1
\(E=U T K U-U T F=-U T F=-U T K U\)
p
(bus \(K U=F\) )
2
2
2
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\section*{5}

Summary of the results
These good results on the displacement and the deformation energy (variation similar of 0,5\% with analytical reference solution) show that the calculation of this energy is correct. To approach still better the value of reference, it would be necessary to discretize the grid more.

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SSLP310-Biblio_18. Crack pressurized in an unlimited plane field
Date:
05/11/02
Author (S):
S. GRANET, I. CORMEAU, B. KURTH

Key: V3.02.310-A Page: 1/10
Organization (S): EDF-R \& D /AMA, CS IF

\title{
Handbook of Validation
}

V3.02 booklet: Linear statics of the plane systems
V3.02.310 document

SSLP310-Biblio_18. Crack pressurized in one unlimited plane field

\section*{Summary:}

This test results from the validation independent of version 3 in breaking process.
It is about a two-dimensional test in statics (plane strains or stresses) which aims at checking of \(G\) and KI under loading by pressure distributed not uniform on the lips, in unlimited medium. One
also check the nullity of KII with option \(\operatorname{CALC} C_{-} K_{-} G\).
The behavior of the structure is elastic linear isotropic.
The case test includes/understands only one plane modeling 2D in which one studies the influence of the parameter C
intervening in the loading.

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Author (S):

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1
Problem of reference

\subsection*{1.1 Geometry}
\(y\)
\(p(X, c)\)
\(X\)
-1
1

One considers the rectilinear crack -1 X 1 in the unlimited plane field.

\section*{1.2}

Properties of material
The material is elastic linear homogeneous of Young E and Poisson's ratio modulus.
\(E=1000 \mathrm{Mpa}\), \(=0,3\)

\section*{1.3}

Boundary conditions and loadings
The loading car-being balanced, the model is limited to the half space y 0.
Boundary conditions
Linear relation \(U X(-1,0)+U X(1,0)=0\)
Condition of symmetry \(U Y=0\) for \(X-1, X 1\) and \(y=0\).
Loading \(\boldsymbol{n}^{\circ} \boldsymbol{1}\)
\(p(X)=1\)

\section*{Loading \(\boldsymbol{n}^{\circ} 2\)}
\(p(X, c)=\exp (c x)\) where \(C\) is a parameter
Loading \(\boldsymbol{n}^{\circ} 3\)
\(p(X, c)=H S(c x)\) where \(C\) is a parameter
Loading \(n^{\circ} 4\)
\(p(X, c)=C H(c x)\) where \(C\) is a parameter
Loading \(\boldsymbol{n}^{\circ} 5\)
\(p(X, c)=\cos (c x)\) where \(C\) is a parameter
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2
Reference solution
2.1

Method of calculation used for the reference solution
Exact calculation symbolic system using software MAPLE V [bib1].

\section*{2.2}

Results of reference
Loading \(\boldsymbol{n}^{\circ} 1\)
```

K(X =)
1=
I
Loading n }\mp@subsup{}{}{\circ

```
\(K(X=1, c)=(I\)
where I and I are related to Bessel modified of first
\(0(c)+I 1(C\)
I
))
0
1
species of indices 0 and 1 [bib2].
Loading \(n^{\circ} 3\)
\(K(X=1, c)=I\)
1 (C
I
)
Loading \(n^{\circ} 4\)
\(K(X=1, c)=I\)
0 (C
I
)
Loading \(\boldsymbol{n}^{\circ} 5\)
\(K(X=1, c)=J\)
where J is related to Bessel of first species of index 0 [bib2].
0 (C
I
)
0
In all the cases of loading
K 2
G
```

I
=
in plane constraints
E
(1-2) K2
G
I
=
in plane deformations
E

```

\subsection*{2.3 References} bibliographical
[1]
There the evaluation of stress intensity factors for is simple ace under parametric loading. Technical notes. N.I. IOKADIMIS and G.T. ANASTASSELOS. Computers and Structures, 51, \(n^{\circ} 6,791-794,1994\).
[2]
Handbook of mathematical functions, Chapter 9. Mr. ABRAMOWITZ and I.A. STEGUN (Editors). United States Dept. of Commerce, National Office of Standards.
Handbook of Validation
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The model is limited to the half space y 0 and the finished area \(-X\)
X X

\section*{, \\ max}
max
\(0 y\) ymax with \(X=y=15\).
max
max
It consists of 578 quadrangles with 8 nodes and 1699 triangles with 6 nodes. It comprises 5230 nodes.

One uses the hyphotèse plane constraints.

\section*{3.2 \\ Characteristics of the grid}

Use of procedure FISS2D_V1.
The topological parameters concerning refinement around the bottom of crack are:
\(\cdot n c=4\) ( \(a\) number of crowns)
- NS \(=8\) (a number of sectors)
\(\cdot N T=1\) (a number of crowns of déraffinement)

The radiant fine grid is limited at the right end of the crack.
The partly current density of the grid of the crack is selected in order to be able to discretize suitably the loading \(p(X, c)\).

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\section*{Code_Aster \({ }^{\circledR}\)}

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\subsection*{3.3 Functionalities}
tested
Calculation of the rate of refund of energy \(G\) by the method THETA for various crowns.

\section*{Orders}

\section*{DEFI_FOND_FISS}

MELTS
GROUP_NO

\section*{NORMAL}

CALC_THETA
THETA_2D
GROUP_NO

\title{
\(C A L C \_K \_G\)
}

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}

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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{Crown 0 (triangles)}
\(\operatorname{Rinf}=0 \mathrm{~mm}\), Rsup \(=0,02 \mathrm{~mm}\)

\section*{Identification Reference}

Aster \%
difference
G, loading \(n^{\circ} 1\)
3,14158E-3
3,04077E-3
-3,209
KII, loading \(n^{\circ} 1\)
0
0
0

KI, loading \(n^{\circ} 1\)
1,77245
1,6559
-6,574

G, loading \(n^{\circ} 2, c=1\)
1,05349E-2
1,01411E-2
-3,738
KII, loading \(n^{\circ} 2, c=1\)
0

0

0
KI, loading \(n^{\circ} 2, c=1\)
3,24576
3,03108
-6,614
G, loading \(n^{\circ} 2, c=5\)
8,356742
7,9065
-5,387
KII, loading \(n^{\circ} 2, c=5\)
0
0
0
KI, loading \(n^{\circ} 2, c=5\)
91,41522
85,4189
-6,559

G, loading \(n^{\circ} 3, c=1\)
1,00344E-3
9,6006E-4
-4,323
KII, loading \(n^{\circ} 3, c=1\)
0
0
0

KI, loading \(n^{\circ} 3, c=1\)
1,00172
0,93505
-6,655
G, loading \(n^{\circ} 3, c=5\)
1,86052
1,760148
-5,395
KII, loading \(n^{\circ} 3, c=5\)
0
0
0
KI, loading \(n^{\circ} 3, c=5\)
43,13380
40,33829
-6,481

G, loading \(n^{\circ} 4, c=1\)
5,03571E-3
4,86064E-3
-3,477
KII, loading \(n^{\circ} 4, c=1\)
0
0
0
KI, loading \(n^{\circ} 4, c=1\)
2,24404
2,09602
-6,596
G, loading \(n^{\circ} 4, c=5\)
2,331095
2,20566
-5,381
KII, loading \(n^{\circ} 4, c=5\)
0
0
0
KI, loading \(n^{\circ} 4, c=5\)
48,28142
45,08068
-6,629
```

G, loading n}\mp@subsup{n}{}{\circ}5,c=
1,839487E-3
1,78707E-3
-2,849
KII, loading n }\mp@subsup{}{}{\circ}5,c=
O
O
O
KI, loading n}\mp@subsup{}{}{\circ}5,c=
1,356277
1,267569
-6,541
G, loading n}\mp@subsup{}{}{\circ}5,c=2,4048255577
O
4,1738E-8
KII, loading n}\mp@subsup{}{}{\circ}5,c=2,404825557
O
O
O
KI, loading n}\mp@subsup{n}{}{\circ}5,c=2,4048255577
O
2,0383E-3
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\section*{Crown 1 (quadrangles)}

\author{
\(\operatorname{Rinf}=0,02 \mathrm{~mm}, \operatorname{Rsup}=0,04 \mathrm{~mm}\)
}

\section*{Identification Reference}

Aster
\% difference
G, loading \(n^{\circ} 1\)
3,14158E-3
3,1669E-3
0,807
KII, loading \(n^{\circ} 1\)
0
0
0
KI, loading \(n^{\circ} 1\)
1,77245
1,78079
0,471

G, loading \(n^{\circ} 2, c=1\) 1,05349E-2
1,056655E-2
0,30
KII, loading \(n^{\circ} 2, c=1\)
0
0
0
KI, loading \(n^{\circ} 2, c=1\)
3,24576
3,256597
0,334
G, loading \(n^{\circ} 2, c=5\)
8,356742
8,25545
-1,212
KII, loading \(n^{\circ} 2, c=5\)
0
0
0
KI, loading \(n^{\circ} 2, c=5\)

0
0
0
KI, loading \(n^{\circ} 3, c=5\)
43,13380
43,2091
0,175

G, loading \(n^{\circ} 4, c=15,03571 E-3\)
5,06348E-3
0,552
KII, loading \(n^{\circ} 4, c=1\)
0
0
0
KI, loading \(n^{\circ} 4, c=1\)
2,24404

\section*{2,25312}

0,405
G, loading \(n^{\circ} 4, c=5\)
2,331095
2,302636
-1,221
KII, loading \(n^{\circ} 4, c=5\)
0
0
0
KI, loading \(n^{\circ} 4, c=5\)
48,28142
48,3188
0,078

G, loading \(n^{\circ} 5, c=1\) 1,839487E-3
1,86066E-3
1,152
KII, loading \(n^{\circ} 5, c=1\)
0
0
0
KI, loading \(n^{\circ} 5, c=1\)
1,356277
1,363914
0,563
G, loading \(n^{\circ} 5, c=2,4048255577\)
0
3,98377E-8

KII, loading \(n^{\circ} 5, c=2,4048255577\)
0
0
0
KI, loading \(n^{\circ} 5, c=2,4048255577\)
0
4,721938E-3

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\section*{Crown 2 (quadrangles)}
\(\operatorname{Rinf}=0,04 \mathrm{~mm}\), Rsup \(=0,06 \mathrm{~mm}\)

\section*{Identification Reference}

Aster
\% difference
G, loading \(n^{\circ} 1\)
3,14158E-3
3,1678E-3
0,835
KII, loading \(n^{\circ} 1\)
0
0
0
KI, loading \(n^{\circ} 1\)
1,77245
1,78075
0,468

G, loading \(n^{\circ} 2, c=11,05349 E-2\)
1,056949E-2
0,328
KII, loading \(n^{\circ} 2, c=1\)
0
0
0
KI, loading \(n^{\circ} 2, c=1\)

3,256529
0,332
G, loading \(n^{\circ} 2, c=5\)
8,356742
8,257967
-1,182
KII, loading \(n^{\circ} 2, c=5\)
0
0
0
KI, loading \(n^{\circ} 2, c=5\)
91,41522
9,1527E1
0,123

G, loading \(n^{\circ} 3, c=1\)
1,00344E-3
1,001087E-3
-0,234
KII, loading \(n^{\circ} 3, c=1\)
0
0
0
KI, loading \(n^{\circ} 3, c=1\)
1,00172
1,0034589
0,174
G, loading \(n^{\circ} 3, c=5\)
1,86052
1,838717
-1,172
KII, loading \(n^{\circ} 3, c=5\)
0
0
0
KI, loading \(n^{\circ} 3, c=5\)
43,13380
43,2088
0,174
```

G, loading n}\mp@subsup{n}{}{\circ}4,c=1 5,03571E-
5,064887E-3
0,579
KII, loading n 4, c=1
O
O
O
KI, loading n }\mp@subsup{}{}{\circ}4,c=
2,24404
2,25307
0,402
G, loading n 4, c=5
2,331095
2,30333
-1,191
KII, loading n }\mp@subsup{n}{}{\circ
O
O
O
KI, loading n'4, c=5
48,28142
48,31838
0,077

```
G, loading \(n^{\circ} 5, c=11,839487 E-3\)
1,86117E-3
1,179
KII, loading \(n^{\circ} 5, c=1\)
0
0
0
KI, loading \(n^{\circ} 5, c=1\)
1,356277
1,363877
0,560
G, loading \(n^{\circ} 5, c=2,4048255577\)
0
4,0008E-8

KII, loading \(n^{\circ} 5, c=2,4048255577\)
0
0
0
KI, loading \(n^{\circ} 5, c=2,4048255577\)
0
4,711869E-3

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\section*{Crown 3 (quadrangles)}
\(\operatorname{Rinf}=0,06 \mathrm{~mm}\), Rsup \(=0,08 \mathrm{~mm}\)
Identification Reference
Aster
\% difference
G, loading \(n^{\circ} 1\)
3,14158E-3
3,16794E-3
0,839
KII, loading \(n^{\circ} 1\)
0
0
0
KI, loading \(n^{\circ} 1\)
1,77245
G, loading \(n^{\circ} 2, c=1\) 1,05349E-2
\(1,05699 E-2\)
0,333
KII, loading \(n^{\circ} 2, c=1\)
0
0
0
KI, loading \(n^{\circ} 2, c=1\)
3,24576
3,2566
0,334
\(G\), loading \(n^{\circ} 2, c=5\)
8,356742
8,25837
1,177
KII, loading \(n^{\circ} 2, c=5\)
0
0
0
KI, loading \(n^{\circ} 2, c=5\)
91,41522
91,5293
0,125

G, loading \(n^{\circ} 3, c=1\)
1,00344E-3
1,001132E-3
-0,230
KII, loading \(n^{\circ} 3, c=1\)
0
0
0
KI, loading \(n^{\circ} 3, c=1\)
1,00172
1,003481

0,176
G, loading \(n^{\circ} 3, c=5\)
1,86052
1,838809
-1,167
KII, loading \(n^{\circ} 3, c=5\)
0

0

0
KI, loading \(n^{\circ} 3, c=5\)
43,13380
43,20984
0,176

G, loading \(n^{\circ} 4, c=15,03571 E-3\)
5,065103E-3
0,584
KII, loading \(n^{\circ} 4, c=1\)
0
0
0
KI, loading \(n^{\circ} 4, c=1\)
2,24404
2,25312
0,405
G, loading \(n^{\circ} 4, c=5\)
2,331095
2,303447
-1,186
KII, loading \(n^{\circ} 4, c=5\)
0
0

0
KI, loading \(n^{\circ} 4, c=5\)
48,28142
48,31948
0,079

G, loading \(n^{\circ} 5, c=1\) 1,839487E-3
1,86124E-3
1,183
KII, loading \(n^{\circ} 5, c=1\)
0
0
0
KI, loading \(n^{\circ} 5, c=1\)
1,356277
1,363907
0,563
G, loading \(n^{\circ} 5, c=2,4048255577\)
0
4,00631E-8
KII, loading \(n^{\circ} 5, c=2,4048255577\)
0
0
0
KI, loading \(n^{\circ} 5, c=2,4048255577\)
0
4,71155E-3

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\subsection*{4.2 Remarks}

Constant 2,4048... is first zero of the function of Bessel J.

Crown 0 (surrounding the bottom of crack consists of triangles) gives poor results compared to the other crowns.

The relative variations maxima between crowns 1,2 and 3 for \(G\) and \(K I\) are given below for various loadings.

Loading 1 Loading 2 Loading 3 Loading 4 Loading 5
Variation on \(\boldsymbol{G}\)
0,03\%
0,03\% 0,03\% 0,03\% 0,03\%
Variation on KI 0,002\% 0,002\% 0,002\% 0,002\% 0,002\%
The variations on G and KI are negligible.

Comment on the results of the loading \(n^{\circ} 5\) with \(C=2,4048 \ldots\)
The loading \(n^{\circ} 5\) can be compared in order of magnitude with the loading \(n^{\circ} 1\). Indeed, the amplitude loading \(n^{\circ} 5\) is worth 1 and its resultant is worth 1,14 to compare with the unit constant value loading \(n^{\circ} 1\).

The absolute deviation for the loading \(n^{\circ} 1\) and crowns it \(n^{\circ} 1\) is of:
-2,53E-5 for \(G\)
- 8,34E-3 for KI

For the loading \(n^{\circ} 5\), the Aster values which are compared with a null value of reference are:
\(\cdot 3,98 E-8\) for \(G\)
- 4,72E-3 for KI

The order of magnitude on the absolute deviations is similar for the two loadings.
One can thus regard as correct the Aster results.

\section*{5}

Summary of the results
Except for the results obtained on crown 0, calculations of \(K\) and \(G\) are very close to exact theoretical solution. Indeed, the variations are always lower than 1,2\% for the calculation of \(G\) and
lower than 0,6\% for the calculation of KI.

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SSLP311 - Biblio_65. Fissure central oblique in a plate
Date:
05/11/02
Author (S):
S. GRANET, I. CORMEAU, E. LECLERE

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Organization (S): EDF-R \& D /AMA, CS IF

\section*{Handbook of Validation}

V3.02 booklet: Linear statics of the plane systems
V3.02.311 document

SSLP311-Biblio_65. Fissure central oblique in a finished rectangular plate, with two materials, subjected to uniform traction

\section*{Summary:}

This test results from the validation independent of version 3 in breaking process.
It is about a two-dimensional test in statics with Bi-material in the presence of an internal crack of interface
oblique.
The behavior of the structure (Bi-material) is elastic linear isotropic.
The case test includes/understands four modelings in plane constraints in which the influence of the slope of
the crack is studied (4 cases).

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1
Problem of reference

\subsection*{1.1 Geometry}

One considers 4 values of the angle \(=15^{\circ}, 30^{\circ}, 45^{\circ}\) and \(60^{\circ}\).
Other dimensions are selected such as \(H=2 W=4 a\).
The value of A is worth 1.E-3 Mr.
1.2

Properties of materials

\section*{Material \(n^{\circ} 1\)}

Rubber band, linear, isotropic, Young modulus E1 =2E+12 Pa and Poisson's ratio \(1=0,3\).
Material \(n^{\circ} 2\)
Rubber band, linear, isotropic, Young modulus E2 \(=2\) E+11 Pa and Poisson's ratio 2 = 0,3.
1.3

Boundary conditions and loading
- The loading being autoéquilibré, one is satisfied to block the 3 rigid modes:
\(U X=U Y=0\) with the left lower corner of the complete model.
\(U Y=0\) with the corner lower right of the complete model.
- On the lower edge, we impose \(U Y=0\)
- Chargement: uniform tension \(y \mathbf{y}=0\) on the higher edge:

The value of 0 is worth 100MPa.
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\section*{2}

Reference solution
2.1

Method of calculation used for the reference solution

Method of the elements of border, with quadratic elements [bib1].
The calculation of KI and KII is carried out by an integral of contour (integral M [bib2]) in which the constraints and displacements calculated in the part intervene, as well as the constraints and displacements deduced from analytically definite asymptotic solutions, in which KI and KII are alternatively null. The calculation of \(K\) is also carried out by the method of virtual extension, as comparison.

\section*{2.2}

Results of reference

\section*{Method}

On the left-hand side

On the right-hand side
\(=15^{\circ}\)
\(=30^{\circ}\)
\(=45^{\circ}\)
\(=60^{\circ}\)
\(=15^{\circ}\)
\(=30^{\circ}\)
\(=45^{\circ}\)
\(=60^{\circ}\)
integral FI
1,0115 0,7868 0,5211 0,2770 1,1266 0,9910 0,7646 0,4919
MR. FII -0,4434
-0,6244
\(-0,6723\)
-0,5804
0,0862 0,2961 0,4056 0,4057
extension FI
1,0110 0,7864 0,5210 0,2769 1,1260 0,9904 0,7643 0,4919
virtual FII -0,4429
-0,6240
-0,6720
-0,5801
0,0865 0,2960 0,4055 0,4056
K J

The relation between the total rate of restitution of energy \(G\) and \(K j\) is written as follows [bib3]:
1
```

\mu1
\mu
=
2
CH
16
2()
G=(K2+K2
I
II)
2.3
Uncertainty on the solution

```

Estimated at less than 0,1\%.
2.4 References
bibliographical
[1]
Stress intensity Factor analysis of interface ace using boundary element method. Application of contour-integral method. NR. MIYAZAKI, T. IKEDA, T.SODA and T. MUNAKATA.
Engng.Fract.Mechs., 45, \(n^{\circ} 5\), 599-610, 1993.
[2]
Year analysis of interface aces between dissimilar isotropic materials using conservation integrals in elasticity. J.F. YAU and T.C. CHANG. Engng.Fract.Mechs., 20, 423-432, 1984. [3]
Adhesive The strength of joints using the theory of aces. B. Mr. MALYSHEV and R.L. SALGANIK. Int.J.Fract.Mech., 1, 114-128, 1965.

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\section*{3}

Modelings A, B, C, D

\section*{3.1 \\ Characteristics of modeling}

Various modelings are identical to share the slope of the crack.

\section*{Complete grid for an angle of \(60^{\circ}\)}

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ray
center

\section*{Zoom on the point of crack}

The ray is worth 7.5E-5 Mr.
There are four crowns defined by order CALC_THETA:
crown 1: \(\operatorname{Rinf}=0\).
Rsup \(=1.875 E-5 m\)
crown 2: Rinf \(=1.875 E-5 m\)
Rsup \(=3.750 E-5 m\)
crown 3: Rinf \(=3.750 E-5 m\)
Rsup \(=5.625 E-5 m\)
crown 4: \(\operatorname{Rinf}=5.625 E-5 m\)
Rsup \(=7.500 E-5 m\)
The direction of propagation is defined by: cos, sin

\section*{3.2 \\ Characteristics of the grid}

The grid consists of 10676 nodes and 4584 elements, including 1392 elements QUA8 and 3168 elements TRI6.

\subsection*{3.3 Functionalities \\ tested}

\section*{Orders}

\author{
AFFE_MODELE \\ MECHANICS \\ C_PLAN \\ ALL \\ MECA_STATIQUE \\ AFFE_CHAR_MECA \\ FORCE_CONTOUR \\ CALC_THETA \\ THETA_2D
}

CALC_G_THETA
OPTION
CALC_G

The calculation of KI and KII is not valid for a bimatériau.
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
Left end, \(=15^{\circ}\)

G, crown 1
9,67362E+1
\(9,2428 E+1\)
-4,45
G, crown 2
9,67362E+1
9,6392E+1
-0,356
G, crown 3
```

9,67362E+1
9,6417E+1
-0,330
G, crown 4
9,67362E+1
9,6421E+1
-0,326
KI 5,6694E+6

```
KII
\(-2,4852 E+6\)

Right end, \(=15^{\circ}\)
```

G, crown 1
1,0125E+2
9,6763E+1
-4,33
G, crown 2
1,0125E+2
1,0093E+2
-0,315
G, crown 3
1,0125E+2
1,0095E+2
-0,295
G, crown 4
1,0125E+2
1,0095E+2
-0,291
KI 6,3145E+6
KII 4,8309E+5

```

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\section*{5 \\ Results of modeling \(B\)}

\subsection*{5.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
Left end, \(=30^{\circ}\)

G, crown 1
8,0017E+1
7,6431E+1
-4,48
G, crown 2
8,0017E+1
7,9707E+1
-0,387
G, crown 3
8,0017E+1
7,9730E+1
-0,358
G, crown 4
8,0017E+1
7,9734E+1
-0,353
KI 4,4100E+6

Right end, \(=30^{\circ}\)
\[
\begin{aligned}
& \text { G, crown 1 } \\
& 8,48417 E+1 \\
& 8,1080 E+1 \\
& -4,433 \\
& G, \text { crown } 2 \\
& 8,48417 E+1 \\
& 8,4583 E+1 \\
& -0,305 \\
& G, \text { crown } 3 \\
& 8,48417 E+1 \\
& 8,4602 E+1 \\
& -0,282 \\
& G, \text { crown } 4 \\
& 8,48417 E+1 \\
& 8,4602 E+1 \\
& -0,282 \\
& K I 5,5545 E+6
\end{aligned}
\]
KII 1,6596E+6
6

Results of modeling C

\subsection*{6.1 Values}
tested

\author{
Identification Reference \\ Aster \% \\ difference
}

Left end, \(=45^{\circ}\)
\[
\begin{aligned}
& G, \text { crown 1 } \\
& 5,73826 E+1 \\
& 5,48161 E+1 \\
& -4,473 \\
& G, \text { crown } 2 \\
& 5,73826 E+1 \\
& 5,71687 E+1 \\
& -0,373 \\
& G, \text { crown 3 } \\
& 5,73826 E+1 \\
& 5,71865 E+1 \\
& -0,342 \\
& G, \text { crown } 4 \\
& 5,73826 E+1 \\
& 5,7189 E+1 \\
& -0,337 \\
& \text { KI 2,92076E+6 } \\
& - \\
& - \\
& \text { KII } \\
& -3,7682 E+6 \\
& -- \\
& \text { Right end, }=45^{\circ}
\end{aligned}
\]

G, crown 1
\(5,94122 E+1\)
5,7039E+1
-3,994
G, crown 2
5,94122E+1
5,9505E+1
0,157
G, crown 3
5,94122E+1
5,9516E+1
0,175
G, crown 4
5,94122E+1
\(5,9518 E+1\)
0,179
KI 4,28557E+6

\author{
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\section*{7 \\ Results of modeling \(D\)}

\subsection*{7.1 Values \\ tested}

\section*{Identification Reference}

Aster \%
difference
Left end, \(=60^{\circ}\)

G, crown 1
3,28015E+1
3,10680E+1
\(-5,285\)
G, crown 2
3,28015E+1
3,24037E+1
-1,213
G, crown 3
\(3,28015 E+1\)
```

3,24140E+1
-1,181
G, crown 4
3,28015E+1
3,24156E+1
-1,177
KI 1,55258E+6
KII
-3,2531E+6
Right end, = 60
G, crown 1
3,22436E+1
3,11825E+1
-3,291
G, crown 2
3,22436E+1
3,25321E+1
0,895
G, crown 3
3,22436E+1
3,25383E+1
0,914
G, crown }
3,22436E+1
3,25398E+1
0,919
KI 2,75709E+6

```
KII 2,27394E+6

\subsection*{7.2 Remarks}

To obtain G on the bottom of crack, one calculates the rate of refund of energy using the relation between G and Kj [bib3]:
\[
=2076923
\]
,
\[
1
\]
\[
2
\]
\[
\mu=76923
\]
,
\[
E+11
\]
\[
1
\]
\[
\mu=76923
\]
\[
E+10
\]
\[
2
\]
\[
=-937742
\]
\[
E-2
\]
\[
=2524488
\]
\[
E-12
\]
\[
G=(2
\]
\[
2
\]
\[
K+K
\]
\[
I
\]
\[
I I)
\]

\section*{8}

Summary of the results
The calculation of \(G\) is not precise on the first crown in all the cases of slope of the crack. With regard to the other crowns, the variations are about 0,4\%. In the case of slope \(=60^{\circ}\) the variation exceeds \(1 \%\). As a whole the results are satisfactory for \(G\).

The calculation of KI and KII is not available for a crack located at the interface of a bimatériau.

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\section*{Titrate:}

SSLP313 - Crack inclined in an unlimited plate

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V3.02.313 document

SSLP313 - Crack inclined in a plate unlimited, subjected to a uniform traction ad infinitum

\section*{Summary:}

This test results from the validation independent of version 3 in breaking process.
One calculates KI, KII and the rate of refund of energy for a right crack, tilted of an angle, in one
large-sized plate subjected to a uniform traction. The model is two-dimensional in constraints plane. The material is elastic linear isotropic. This test of reference in 2D makes it possible to check separability
KI and KII in a mixed mode.
The reference solution, given for a theoretically unlimited field, is analytical.
In addition to the energy method (CALC_G_THETA), one tests the method of calculation of the factors of intensity
constraints by extrapolation of displacements (POST_K1_K2_K3). Modeling B makes it possible to test
this last method with a type of grid recommended (nodes mediums with the quarter) to obtain a solution
precise.
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SSLP313 - Crack inclined in an unlimited plate

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Author (S):
J.M.PROIX, I. CORMEAU, E. LECLERE Key
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{\(y\) has \\ \(X\) \\ has}

One allots an unspecified value to the slope, \(=37\) degrees.
One chooses has =1.E-3 Mr.

\section*{1.2}

Properties of material
The material is elastic linear isotropic, of Young E and Poisson's ratio modulus. \(E=2 . E 11 P a,=0.3\).
Assumption of the plane constraints
1.3

Boundary conditions and loadings

Arbitrary limits of the field with a grid:
- xmax X xmax with xmax = 10a
- ymax y ymax with ymax = 20a

Boundary conditions:
In order to block the 3 plane rigid modes exclusively.
\(U X=U Y=0\) with the left lower corner of the complete model.
\(U Y=0\) with the corner lower right of the complete model.
On the lower edge, we impose \(U Y=0\)

Loading: uniform tension \(y y=0\) on the higher edge:
The value of 0 is worth 100MPa, in plane constraints.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.3

Titrate:
SSLP313-Crack inclined in an unlimited plate

\section*{Date:}

15/10/01
Author (S):

\title{
J.M.PROIX, I. CORMEAU, E. LECLERE Key
}

\section*{:}

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\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
Function of constraint of Airy.

\section*{2.2}

Results of reference
\(K=\)
has
2
I
Ocos
\(K=\)
has
II
\(O \sin \cos\)
1
G
\(=\)
(K2 + K 2
ref.
) in plane constraints
E
I
II

\section*{2.3}

Uncertainty on the solution
Exact analytical solution (Irwin) in unlimited medium.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}
Y. MURAKAMI Stress intensity factors handbook, box 4.2, page 188. The Society of Materials Science, Japan, Pergamon Press, 1987.

Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
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15/10/01
Author (S):
J.M.PROIX, I. CORMEAU, E. LECLERE Key
:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\author{
Complete model \\ Handbook of Validation \\ V3.02 booklet: Linear statics of the plane systems \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.3

Titrate:

SSLP313-Crack inclined in an unlimited plate

\author{
Date: \\ 15/10/01 \\ Author (S): \\ J.M.PROIX, I. CORMEAU, E. LECLERE Key \\ : \\ V3.02.313-A Page: \\ 5/10
}

\author{
After symmetrization and orientation \\ Handbook of Validation \\ V3.02 booklet: Linear statics of the plane systems \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.3

Titrate:
SSLP313 - Crack inclined in an unlimited plate

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Initial block 2D
ray
center

The ray is worth \(7,5 E-5 \mathrm{~m}\)

\section*{3.2 \\ Characteristics of the grid}

The grid consists of 14888 nodes and 6674 elements, including 1392 elements QUA8 and 5282 elements TRI6.

\section*{3.3 \\ Functionalities tested}

\author{
Orders
}

\section*{MECHANICAL AFFE_MODELE}

C_PLAN
ALL
AFFE_CHAR_MECA FORCE_CONTOUR

\section*{MECA_STATIQUE}

CALC_THETA THETA_2D
CALC_G_THETA_T OPTION
CALC_G
CALC_G_THETA_T OPTION
\(C A L C \_K \_G\)
POST_K1_K2_K3

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Version
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Titrate:
SSLP313 - Crack inclined in an unlimited plate

Date:
15/10/01
Author (S):
J.M.PROIX, I. CORMEAU, E. LECLERE Key
:

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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\subsection*{4.1.1 Results obtained with CALC_G_THETA_T}

Identification Reference
Aster

\author{
\% difference \\ G 1.0019E+02 \\ 1.0126E+02 \\ 1.07 \\ KI 3,5750E+6 \\ 3,6038E+6 \\ 0,81 \\ KII 2,6939E+6 \\ \(2,7003 E+6\) \\ 0,24
}

\subsection*{4.1.2 Results obtained with POST_K1_K2_K3}

\section*{Identification Method Reference}

\section*{Aster \%}
difference
K1_MAX 1
\(3.57 E+06\)
\(3.54 E+06\)
-1.04
K1_MIN 1
\(3.57 E+06\)
3.19E+06
-10.72
K2_MAX 1
\(2.69 E+06\)
\(2.62 E+06\)
-2.82
K2_MIN 1
\(2.69 E+06\)
1.92E+06
-28.62
G_MAX 1
\(1.00 E+02\)
9.69E+01
-3.33
G_MIN 1
1.00E+02
\(6.94 E+01\)
-30.70
K1_MAX 2
\(3.57 E+06\)
\(3.51 E+06\)

K1_MIN 2
\(3.57 E+06\)
3.33E+06
-6.79
K2_MAX 2
\(2.69 E+06\)
\(2.61 E+06\)
-3.12
K2_MIN 2
\(2.69 E+06\)
\(2.25 E+06\)
-16.49
G_MAX 2
\(1.00 E+02\)
\(9.57 E+01\)
-4.50
G_MIN 2
\(1.00 E+02\)
\(8.08 E+01\)
-19.32

\section*{4.2}

Remarks on the 2 methods of POST_K1_K2_K3:
Two methods are programmed in POST_K1_K2_K3:

Method 1: one calculates the jump of the field of displacements squared and one divides it by \(R\).
Various values of K1 (resp. K2, K3) are obtained (except for a factor) by extrapolation in \(R=0\) of the segments of right-hand sides thus obtained. If the solution were perfect (field asymptotic analytical everywhere), one should obtain a line. Actually, one is obtained curve, therefore values different of K1, K2, K3. In order to give an indication of quality of the result, one lists the values maximum and minimum obtained on the unit of discussed items (that one names here K1_MAX, K1_MIN, etc...)

Method 2: one traces the jump of the field of displacements squared according to \(R\). Them approximations of K1 are (always except for a factor) equal to the slope of the segments connecting the origin at the various points of the curve. There still, one obtains various values of K1, K2, K3.et one lists the values maximum and minimum obtained on the unit of the points treaties (named K1_MAX, K1_MIN, etc...)

To provide to compare the solution obtained with that provided by CALC_G_THETA_T, one calculate \(G\) starting from K1 and K2 by the formula of Irwin, which gives \(G_{-} M A X\) and \(G_{-} M I N\).

\subsection*{4.3 Parameters \\ of execution}

Version: 5.3

\section*{Machine: SGI/ORIGIN 2000}

Obstruction memory:
128 Mo
Time CPU To use: 22 seconds
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
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\section*{5 Modeling \\ B}

\section*{5.1}

Characteristics of modeling
Even form of grid that previously, but modification of the co-ordinates of the nodes mediums edges touching the bottom of crack, to move them with the quarter of these edges (method of Barsoum).

This modification of the co-ordinates of the nodes is carried out by an accessible procedure GIBI in
to card-index data of grid (SSLP313B.datg).

\section*{5.2}

Characteristics of the grid
The grid consists of 14888 nodes and 6674 elements, including 1392 elements QUA8 and 5282 elements TRI6.

\author{
5.3 \\ Functionalities tested \\ Orders \\ MECHANICAL AFFE_MODELE \\ C_PLAN \\ ALL \\ AFFE_CHAR_MECA FORCE_CONTOUR
}

\section*{MECA_STATIQUE}

CALC_THETA THETA_2D

CALC_G_THETA OPTION
CALC_G
CALC_G_THETA OPTION
CALC_K_G
POST_K1_K2_K3

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Date:
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Author (S):
J.M.PROIX, I. CORMEAU, E. LECLERE Key:V3.02.313-A Page:9/10
6
Results of modeling B
6.1 Values
tested
6.1.1 Results obtained with CALC_G_THETA_T
Identification Reference
Aster
\% difference
G 1.0019E+02
\(1.0135 E+02\)1.16
KI 3,5750E+6
3.6033E+06
0.79
KII 2,6939E+6
\(2.6996 E+06\)
0.21
6.1.2 Results obtained with POST_K1_K2_K3
Identification Method Reference
Aster \%
difference
K1_MAX 1
\(3.57 E+06\)
3.6089E+060.95
K1_MIN 1
\(3.57 E+06\)

\author{
\(3.5995 E+06\) \\ 0.69 \\ K2_MAX 1 \\ \(2.69 E+06\) \\ \(2.7035 E+06\) \\ 0.36 \\ K2_MIN 1 \\ \(2.69 E+06\) \\ \(2.6944 E+06\) \\ 0.02 \\ G_MAX 1 \\ \(1.00 E+02\) \\ \(1.0142 E+02\) \\ 1.23 \\ G_MIN 1 \\ \(1.00 E+02\) \\ \(1.0120 E+02\) \\ 1.01 \\ K1_MAX 2 \\ \(3.57 E+06\) \\ \(3.6027 E+06\) \\ 0.78 \\ K1_MIN 2 \\ \(3.57 E+06\) \\ \(3.5344 E+06\) \\ -1.14 \\ K2_MAX 2 \\ \(2.69 E+06\) \\ \(2.6927 E+06\) \\ -0.05 \\ K2_MIN 2 \\ \(2.69 E+06\) \\ \(2.6478 E+06\) \\ -1.71 \\ G_MAX 2 \\ \(1.00 E+02\) \\ \(1.0115 E+02\) \\ 0.96 \\ G_MIN 2 \\ 1.00E+02 \\ \(9.7512 E+01\) \\ -2.67
}

\section*{6.2}

Remarks on the 2 methods of POST_K1_K2_K3:
Two methods are programmed in POST_K1_K2_K3:

Method 1: one calculates the jump of the field of displacements squared and one divides it by \(R\). Various values of K1 (resp. K2, K3) are obtained (except for a factor) by extrapolation in \(R=0\) of the segments of right-hand sides thus obtained. If the solution were perfect (field asymptotic analytical everywhere), one should obtain a line. Actually, one is obtained curve, therefore values different of K1, K2, K3. In order to give an indication of quality of the result, one lists the values maximum and minimum obtained on the unit of discussed items (that one names here K1_MAX, K1_MIN, etc...)

Method 2: one traces the jump of the field of displacements squared according to R. Them approximations of K1 are (always except for a factor) equal to the slope of the segments connecting the origin at the various points of the curve. There still, one obtains various values of K1, K2, K3.et one lists the values maximum and minimum obtained on the unit of the points treaties (named K1_MAX, K1_MIN, etc...)

To provide to compare the solution obtained with that provided by CALC_G_THETA_T, one calculate G starting from K1 and K2 by the formula of Irwin, which gives G_MAX and G_MIN.

\subsection*{6.3 Parameters}
of execution
Version: 5.3

Machine: SGI/ORIGIN 2000

Obstruction memory:
128 Mo
Time CPU To use: 19 seconds
Handbook of Validation
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5.3

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J.M.PROIX, I. CORMEAU, E. LECLERE Key
:
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\section*{7}

Summary of the results
With this choice of the limits of the field of calculation, we obtain variations of about \(1 \%\) on coefficients KI and KII, and on the rate of refund of energy \(G\).

With regard to method POST_K1_K2_K3, the results are further away from the reference with a standard grid (from \(1 \%\) to \(30 \%\) of variation), on the other hand, with a grid of the type Barsoum (nodes
mediums with the quarter on the sides), recommended for this type of method, the differences are included/understood between 3\% and
\(+1.2 \%\), which is relatively precise.
Handbook of Validation
V3.02 booklet: Linear statics of the plane systems
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.5

Titrate:
SSLS07-Thin cylinder under uniform axial loading

\section*{Date}

\section*{:}

01/10/03
Author (S):
X. DESROCHES Key
:
V3.03.007-A Page:
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Organization (S): EDF-R \& D /AMA

\title{
Handbook of Validation
}

V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.007

SSLS07-Thin cylinder under axial loading
uniform

\section*{Summary:}

The purpose of this test from guide VPCS (SSLS 07/89) is to validate a linear loading (FORCE_POUTRE) in axisymmetric modeling.
 (modeling B).

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.5

Titrate:
SSLS07-Thin cylinder under uniform axial loading
Date
01/10/03Author (S):
X. DESROCHES Key

:
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1
Problem of reference

\subsection*{1.1 Geometry}
Average radius: \(\mathbf{R o}=\)
1m
Thickness
: H
\(=0.02 \mathrm{~m}\)
Height
: L
\(=4 \mathrm{~m}\)
Internal ray: \(\mathbf{I H}=\)
Ro h/2

\section*{1.2}
Material properties
Young modulus
: E = 2.1 X 1011 Pa
Poisson's ratio: \(=0.3\)
1.3
Boundary conditions and loadings

Axial displacement no one at the low end \((U=0)+\) conditions of symmetry
Uniform axial loading per unit of length \(Q=10000 \mathrm{~N} / \mathrm{m}\), applied at the high end

\subsection*{1.4 Conditions} initial

Without object for the static analysis.
Handbook of Validation
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Titrate:
SSLS07-Thin cylinder under uniform axial loading
Date
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01/10/03
Author (S):
X. DESROCHES Key
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution

\section*{\(\boldsymbol{Q}\)}

Axial stress:

H
Circumferential constraint:
0
\(22=\)
\(q L\)
Lengthening of the cylinder: \(U X=\)
Eh
Q R

\section*{Radial displacement: \(\boldsymbol{U}\)}
```

O
$R=-$

```

Eh

\author{
2.2 \\ Results of reference \\ \(11=5 \times 105 \mathrm{~Pa}\) \\ \(U X=9.52 X 106 \mathrm{~m}\) \\ \(U R=7.14 X 107 \mathrm{~m}\)
}

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 Reference \\ bibliographical}

\section*{[1]}

Guide VPCS Edition 1990 (SSLS 07/89)
[2]
R.J. ROARK and W.C. YOUNG: Formulated for stress and strain, 5th edition, New York, Mc Graw-Hill, 1975
Handbook of Validation
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6.5

\section*{Titrate:}

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

AXIS, T6 meshs and Q8

Position of the points:
\(E, F\) with middle height
G, H, I remote Ro of the axis
Cutting:
100 elements according to the height
1 element in the thickness

Limiting conditions: \(D Y=0\)
on \(\boldsymbol{A B}\)

\section*{Loading:}

Force distributed \(=500000\)
on CD

Name of the nodes:
Not \(A=N 1\)
Not \(C=N 452\)
Not \(E=N 201\)
Not \(\boldsymbol{G}=\mathrm{N} 51\)
Not \(I=N 503\)
Not \(B=N 101\)
Not \(D=\) N504

Not \(F=N 203\)
Not \(\boldsymbol{H}=\) N202

\section*{3.2}

Characteristics of the grid
A number of nodes: 553
A number of meshs and types: 50 QUAD8, 100 TRIA6, 204 SEG3

\subsection*{3.3 Functionalities}
tested
Orders

\author{
"MECHANICAL"AFFE_MODELE "AXIS" \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_CONTOUR \\ GROUP_MA \\ CALC_CHAM_ELEM OPTION \\ "SIGM_ELNO_DEPL"
}

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls

\section*{HT-66/03/008/A}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.5

\section*{Titrate:}

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01/10/03
Author (S):
X. DESROCHES Key
:
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\title{
4 \\ Results of modeling \(A\)
}

\subsection*{4.1 Values}
tested
Standard localization
of
value Reference Aster
\% difference
Points G, H, I
ur (m)
-7.14 10-7
-7.14 10-7 0.
Points C, D, I
ux (m)
9.52 10-6
9.52 10-6 0.

Points A, B, C, D, E,
22 (Pa)
0.

10-6
-
F, G, H, I
Points A, B, C, D, E,
11 (Pa)
5. 10-5
5.001050.

F, G, H, I

\subsection*{4.2 Notice}

The Fy value provided corresponds to the pressure \(p=q / h\).

\author{
Handbook of Validation
}

V3.03 booklet: Linear statics of the plates and hulls HT-66/03/008/A

\author{
: \\ 01/10/03
}

Author (S):
X. DESROCHES Key

\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

AXIS, T6 meshs and Q8

Position of the points:
\(E\), \(F\) with middle height
G, H, I remote Ro of the axis
Cutting: 100 elements according to the height
1 element in the thickness
The load is broken up in the following way:
charge q1 varying linearly of 0 in D with \(10000 \mathrm{~N} / \mathrm{m}\) out of \(C\) field of \(\boldsymbol{U} 1\) displacements charge \(q 2\) varying linearly of \(10000 \mathrm{~N} / \mathrm{m}\) in \(D\) with 0 out of \(C\) field of \(\boldsymbol{U} \mathbf{2}\) displacements The results are given separately for each field \(\boldsymbol{U} 1\) and \(\boldsymbol{U} 2\).

Name of the nodes:
Not \(A=N 1\)
Not \(C=N 452\)
Not \(E=N 201\)
Not \(G=\) N51
Not \(I=\) N503
Not \(B=\) N101
Not \(D=\) N504
Not \(F=N 203\)
Not \(H=N 202\)

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 557
A number of meshs and types: 50 QUAD8, 100 TRIA6, 204 SEG3

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
"MECHANICAL" AFFE_MODELE "AXIS" \\ ALL \\ AFFE_CHAR_MECA_F FORCE_CONTOUR \\ GROUP_MA \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ CALC_CHAM_ELEM OPTION \\ "SIGM_ELNO_DEPL" \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.5

\section*{Titrate:}

SSLSO7 - Thin cylinder under uniform axial loading
Date
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Author (S):
X. DESROCHES Key
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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested

\section*{Fields Standard Localization \\ of Reference \\ Aster \\ \% difference \\ value \\ U1 \\ Not G (N51) \\ ur (m) \\ -3,583. 10-7 \\ -3,5833. 10-7 \\ 0,009 \\ Not H (N202) \\ -3,583. 10-7 \\ \(-3,5833.10-7\) \\ 0,009 \\ Not I (N503) \\ -1,012. 10-6 \\ -1,0116. 10-7 \\ -0,036 \\ Not C (N452) \\ ux (m) \\ 4,896. 10-6 \\ 4,8963. 10-6}

Not D (N504)
4,658. 10-6
4,6583. 10-6
0,006
Not I (N503)
4,777. 10-6
4,7774. 10-6
0,009
U2
Not \(G\)
ur (m)
-3,559. 10-7
\(-3,5595.10-7\)
0,015
Not H
-3,559. 10-7
\(-3,5595.10-7\)
0,015
Not I
2,973. 10-7
2,9735. 10-7
0,017
Not C (N452)
4,627. 10-6
4,6275. 10-6
0,001
Not D (N504)
4,865. 10-6
4,8655. 10-6
0,011
Not I (N503)
4,746. 10-6
4,7464. 10-6
0,008
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version

\section*{6.5}

Titrate:
SSLSO7 - Thin cylinder under uniform axial loading
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01/10/03
Author (S):

\section*{X. DESROCHES Key}
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\section*{7 \\ Summary of the results}

Key word FORCE_CONTOUR used starting from 2 orders AFFE_CHAR_MECA and AFFE_CHAR_MECA_F provides right results.
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Titrate:
SSLSO9 - Thin cylinder under actual weight
Date
:

02/1 1/05
Author (S):

\section*{X. DESROCHES Key}
:
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Organization (S): EDF-R \& D /AMA

\title{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ Document: V3.03.009
}

\section*{SSLS09- Thin cylinder under actual weight}

\section*{Summary:}

This test from guide VPCS (SSLS 09/89) aims to test a voluminal loading (here the weight clean), in axisymmetric analysis, by using key word FORCE_INTERNE.

One will use for that the two orders: AFFE_CHAR_MECA (modeling A) and AFFE_CHAR_MECA_F
(modeling B).
Modeling C tests the incompressible elements by using the key word GRAVITY on a loading equivalent.

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V3.03 booklet: Linear statics of the plates and hulls
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Code_Aster \({ }^{\circledR}\)
Version
7.4

Titrate:
SSLSO9 - Thin cylinder under actual weight
Date

\title{
X. DESROCHES Key
}
:

V3.03.009-B Page:

1

Problem of reference

\subsection*{1.1 Geometry}

Average radius: \(\boldsymbol{R}\)
\(=1 \mathrm{~m}\)
Thickness
: H
\(=0.02 \mathrm{~m}\)
Height
: L
\(=4 \mathrm{~m}\)

\author{
1.2 \\ Material properties \\ Young modulus \\ : E = 2.1 X 1011 Pa \\ Poisson's ratio: \(=0.3\) \\ Voluminal weight \\ \(:=7.85 \times 104 \mathrm{~N} / \mathrm{m} 3\)
}

\section*{1.3 \\ Boundary conditions and loadings}

Axial displacement no one at the low end \((U=0)+\) conditions of symmetry
Actual weight, according to the axis, direction \(+X\)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls

\section*{Code_Aster \({ }^{\circledR}\)}

\author{
Version
}
7.4

\section*{Titrate:}

SSLS09- Thin cylinder under actual weight
Date
```

:

```

02/11/05
Author (S):
X. DESROCHES Key

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2
Reference solution
2.1
Method of calculation used for the reference solution
In a point of co-ordinate \(X\) :
- X-ray
- radial displacement: \(\boldsymbol{U} \boldsymbol{R}=\)
E\(x 2\)
- axial displacement: \(\boldsymbol{U} X=\)
2nd\(\boldsymbol{R}\)- rotation of a generator:=
E
- axial stress:
\(=X\)
11
- circumferential constraint:

0
\(22=\)
2.2
Results of reference
- Axial Déplacement high end: \(U X=2.99 X 106 \mathrm{~m}\)
- Radial Déplacement low end: \(U R=4.49\) X 107 m
\(\cdot=1.12 \times 107 \mathrm{rad}\)
- \(11=3.14 \times 105\) Pa, at the low end
0
\(22=\)
. everywhere

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 Reference \\ bibliographical}

\section*{[1]}

Guide VPCS Edition 1990 (SSLS 09/89)
[2]
R.J. ROARK and W.C. YOUNG: Formulated for stress and strain, 5th edition, New York, Mc Graw-Hill, 1975

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.4
```

Titrate:
SSLS09 - Thin cylinder under actual weight
Date
:
02/11/05
Author (S):
X. DESROCHES Key
:
V3.03.009-B Page:
4/10

```

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

AXIS, T6 meshs and Q8

Position of the points:

E, \(F\) with middle height
G, H, I remote \(R\) of the axis
Cutting:
100 elements according to the height
1 element in the thickness

Limiting conditions: \(\boldsymbol{D Y}=0\) on \(A B\)

Loading:
Constant voluminal force equalizes to 78500.
Name of the nodes:
Not \(A=N 1\)
Not \(C=N 452\)
Not \(E=N 201\)

\title{
Not \(G=N 51\) \\ Not \(I=\) N503 \\ Not \(B=\) N101 \\ Not \(D=N 504\) \\ Not \(F=N 203\) \\ Not \(H=\) N202
}

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: 553 \\ A number of meshs and types: 50 QUAD8, 100 TRIA6, 204 SEG3}

\subsection*{3.3 Functionalities}
tested
Orders

\author{
"MECHANICAL" AFFE_MODELE "AXIS" \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_INTERNE \\ CALC_CHAM_ELEM OPTION \\ "SIGM_ELNO_DEPL" \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/05/005/A
}

Code_Aster \({ }^{\circledR}\)
Version
7.4

\section*{Titrate:}

SSLS09- Thin cylinder under actual weight
Date
:
02/11/05
Author (S):
X. DESROCHES Key
```

4
Results of modeling $A$

```

\subsection*{4.1 Values}
tested

\section*{Standard localization}
of
value Reference Aster
\% difference
Points C, D, I
ux (m)
2.99 10-6
2.99 10-6 0.

Not \(G\)
ur (m)
-4.49 10-7
-4.42 10-7 -1.5
Not \(G\)
11 (Pa)
-3.14 105
-3.14 1050.
Points A, B, G
22 (Pa)
0. <

40

\subsection*{4.2 Remarks}
- The values of 22 data are not significant.
- Taking into account the grid (1 element in the thickness), the results are completely satisfactory.

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.4
Titrate:
SSLS09- Thin cylinder under actual weight
Date
02/11/05
Author (S):
X. DESROCHES Key
V3.03.009-B Page:
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\section*{5 Modeling}
B

\section*{5.1}
Characteristics of modeling
AXIS, T6 meshs and Q8

Position of the points:
\(E, F\) with middle height
G, H, I remote \(R\) of the axis

\section*{Cutting:}

100 elements according to the height
1 element in the thickness

Limiting conditions: \(D Y=0\) on \(A B\)

\section*{Loading:}

Voluminal force in the form of a constant function defined in \(y=0,3,6\).
Name of the nodes:
Not \(A=N 1\)
Not \(C=N 452\)

\title{
Not \(E=N 201\) \\ Not \(G=N 51\) \\ Not \(I=\) N503 \\ Not \(B=\) N101 \\ Not \(D=\) N504 \\ Not \(F=N 203\) \\ Not \(H=N 202\)
}

\section*{5.2}

Characteristics of the grid
A number of nodes: 553
A number of meshs and types: 50 QUAD8, 100 TRIA6, 204 SEG3

\author{
5.3 Functionalities \\ tested \\ Orders \\ \author{
"MECHANICAL"AFFE_MODELE "AXIS" \\ \\ ALL \\ \\ AFFE_CHAR_MECA DDL_IMPO \\ \\ GROUP_NO \\ \\ AFFE_CHAR_MECA_F FORCE_INTERNE
}
}

\author{
CALC_CHAM_ELEM \\ "SIGM_ELNO_DEPL"
}

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
7.4

Titrate:
SSLS09- Thin cylinder under actual weight
Date
6
Results of modeling \(B\)

\subsection*{6.1 Values}
tested

\section*{Standard localization}
of
value Reference Aster
\% difference
Points C, D, I
ux (m)
2.99 10-6
2.99 10-6 0.

Not G
ur (m)
-4.49 10-7
-4.42 10-7 -1.5
Not \(G\)
11 (Pa)
-3.14 105
-3.14 1050.
Points A, B, G
22 (Pa)
0. <

40

\subsection*{6.2 Remarks}
- The values of 22 data are not significant.
- The results are identical to those of modeling \(A\).

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls

\section*{HT-66/05/005/A}

Code_Aster \({ }^{\circledR}\)
Version
7.4

Titrate:
SSLSO9 - Thin cylinder under actual weight
Date

02/11/05
Author (S):
X. DESROCHES Key

\title{
7 Modeling \\ C \\ \\ 7.1 \\ \\ 7.1 \\ \\ Characteristics of modeling
} \\ \\ Characteristics of modeling
}

AXIS_INCO, T6 meshs and Q8

Position of the points:

E, \(F\) with middle height
G, H, I remote \(R\) of the axis
Cutting:
100 elements according to the height
1 element in the thickness

Limiting conditions: \(D Y=0\) on \(A B\)

Loading:
Gravity
Name of the nodes:

Not \(A=N 1\)
Not \(C=N 452\)
Not \(E=N 201\)
Not \(G=\) N51
Not \(I=\) N503
Not \(B=\) N101
Not \(D=\) N504
Not \(F=N 203\)
Not \(H=N 202\)

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 553
A number of meshs and types: 50 QUAD8, 100 TRIA6, 204 SEG3

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

\author{
"MECHANICAL" AFFE_MODELE "AXIS_INCO" ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ AFFE_CHAR_MECA GRAVITY \\ STAT_NON_LINE \\ COMP_ELAS \\ RELATION \(=\) "ELAS" \\ CALC_ELEM OPTION \\ "SIEF_ELNO_ELGA" \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/05/005/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.4

\section*{Titrate:}

SSLSO9-Thin cylinder under actual weight
Date
\(\qquad\)
02/11/05
Author (S):
X. DESROCHES Key
:
V3.03.009-B Page:
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\section*{8 \\ Results of modeling C}

\subsection*{8.1 Values}
tested

\section*{Standard localization \\ of \\ value Reference Aster \\ \% difference \\ Points C, D, I \\ ux (m) \\ 2.99 10-6 \\ 2.99 10-6 0. \\ Not \(G\) \\ ur ( \(m\) ) \\ -4.49 10-7 \\ -4.42 10-7 -1.5 \\ Not \(G\) \\ 11 (Pa) \\ -3.14 105 \\ -3.14 1050. \\ Points A, B, G \\ 22 (Pa) \\ 0 . \\ 40}

\subsection*{8.2 Remarks}
- The found values of 22 are not significant.
- The results are identical to those of modeling \(A\) and \(B\).

\section*{Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/05/005/A}

Code_Aster \({ }^{\circledR}\)
Version
7.4

Titrate:
SSLS09- Thin cylinder under actual weight
Date
:
02/11/05
Author (S):
X. DESROCHES Key
:
V3.03.009-B Page:
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\section*{9}

Summary of the results
The use of a function for the definition of a constant density of volume charge is valid: results are identical, whether one uses one or the other of 2 orders AFFE_CHAR_MECA or AFFE_CHAR_MECA_F. An equivalent loading gravity gives the same results. Moreover, the incompressible elements give the same results (modeling C).
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/05/005/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
1/10
Organization (S): EDF/IMA/MMN, SAMTECH
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
Document: V3.03.020
SSLS20-Cylindrical hull free pinch on board
Summary:
The test from guide VPCS, makes it possible to check the effect of a specific loading on a thin cylindrical hull
in linear elasticity. One compares the arrows with the point of application of the loading compared to one
modeling of the thin cylindrical hull in elements DKT and two modelings COQUE_3D (1/8 cylinder is represented).
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
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1
Problem of reference
1.1 Geometry
\(y\)
F
F
D
With
D
C
With
\(X\)
```

C
B
B
L
F
Z
R
L
eighth of cylinder
Length
L=10.35 m
Ray
R=4.953 m
Thickness
T = 0.094 m
1.2
Material properties
E = 10.5 106 Pa
= 0.3125
1.3
Boundary conditions and loadings
Specific force: F = 100. NR
F
With
B
F
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

```
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA
Key:
V3.03.020-D Page:
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2

\section*{Reference solution}
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SSLS20/89 of guide VPCS.
It was established by average of results of several software packages of calculation by the method of finite elements.
2.2

Results of reference
Displacement of point A following \(\mathbf{y}: ~ v=0.1139\)
2.3

Uncertainty on the solution
2\%
2.4 References
bibliographical
[1]
G. HORRIGMOE, P.G. BERGAN "Not linear analysis of free from shells by flat finite elements " Mathematics Computer in Applied Mechanics and Amsterdam Engineering, North
Holland, vol. 16 (1978)
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
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3 Modeling
With
3.1

Characteristics of modeling
Element of hull DKT
\(y\)
D
L
longitudinal twinge
\(C\)
circumferential twinge \(0.5 R=8.3\)
\(10 T\)
Z
B
Modeling of a eighth of cylinder
Cutting: 10 on \(A D\) and BC 15 on \(A B\) and \(c d .==>300\) meshs TRIA3
Limiting conditions:
in all the nodes of arc \(A B\)
DDL_IMPO:
(GROUP_NO: AB DZ: 0. , DRX: 0. , DRY: 0.)
segment) AD)
(GROUP_NO: ADsansA DX: 0. , DRY: 0. , DRZ: 0.)
segment) BC)
(GROUP_NO: BCsansB DY: 0. , DRX: 0. , DRZ: 0.)
in \(A\)
(GROUP_NO: WITH DX: 0. , DRZ: 0.)
out of B
(GROUP_NO: B DY: 0. , DRZ: 0.)
Loading:
with node A
FORCE_NODALE:
(GROUP_NO: In Fy: 25. )
Name of the nodes:
Not \(A=\) NO176 Not \(C=N O 1\)
Not \(B=\) NO11 Not \(D=\) NO166

\section*{3.2}

Characteristics of the grid
A number of nodes: 176
A number of meshs and types: 300 TRIA3

\subsection*{3.3 Functionalities}
tested
Orders

\section*{Keys}

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
```

DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

```

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

\section*{Key:}

V3.03.020-D Page:
5/10
4
Results of modeling \(A\)
4.1 Values
tested
Not
Size and unit
Reference
Aster
\% difference
With
displacement \(v\) (m)
0.1139
0.1128
0.54
4.2 Parameters
of execution
Version: 4.00.02
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords

Time CPU To use:
5.1 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
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5 Modeling
B
5.1

Characteristics of modeling
Element of hull COQUE_3D MEC3QU9H
\(y\)
D
L
longitudinal twinge
\(=3.7\)
\(X\)
30t
With
C
circumferential twinge \(0.5 R=8.3\)
\(10 T\)
Z
B
Modeling of a eighth of cylinder
Cutting: 4 on AD and BC 4 on AB and cd. ==> 16 meshs QUAD9
Limiting conditions:
in all the nodes of arc \(A B\)
DDL_IMPO:
(GROUP_NO: AB DZ: 0., DRX: 0. , DRY: 0.)
segment) AD)
(GROUP_NO: ADsansA DX: 0. , DRY: 0. , DRZ: 0.)
segment) BC)
(GROUP_NO: BCsansB DY: 0. , DRX: 0. , DRZ: 0.)
in \(A\)
(GROUP_NO: WITH DX: 0., DRZ: 0.)
out of B
(GROUP_NO: B DY: 0. , DRZ: 0.)
Loading:
with node A
FORCE_NODALE:
(GROUP_NO: In Fy: 25. )
Name of the nodes:
Not \(A=\) NO3 Not \(C=\) NO1
Not \(B=\) NO4 Not \(D=N O 2\)
5.2

Characteristics of the grid
A number of nodes: 65
A number of meshs and types: 16 QUAD9
5.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"COQUE_3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]

\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HI-75/98/040 - Ind A
}

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
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6
Results of modeling B
6.1 Values
tested
Not
Size and unit
Reference
Aster
\% difference
With
displacement \(v\) (m)
0.1139
0.1136
0.26
6.2 Parameters
of execution
Version: 4.00.14
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
5.0 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
8/10
7 Modeling
C
7.1

Characteristics of modeling
Element of hull COQUE_3D MEC3TR7H
y
D
\(L\)
longitudinal twinge
\(=3.7\)
X
30t
With
C
circumferential twinge \(0.5 R=8.3\)
\(10 T\)
Z
B
Modeling of a eighth of cylinder
Cutting: 5 on AD and BC 12 on AB and cd. ==> 120 meshs TRIA7
Limiting conditions:
in all the nodes of arc AB
DDL_IMPO:
(GROUP_NO: AB DZ: 0. , DRX: 0. , DRY: 0.)
segment) AD)
(GROUP_NO: ADsansA DX: 0. , DRY: 0. , DRZ: 0.)
segment) BC)
(GROUP_NO: BCsansB DY: 0. , DRX: 0. , DRZ: 0.)
in A
(GROUP_NO: WITH DX: 0., DRZ: 0.)
out of B
(GROUP_NO: B DY: 0. , DRZ: 0.)
Loading:
with node A
FORCE_NODALE:
(GROUP_NO: In Fy: 25. )
Name of the nodes:
Not A = NO3
Not \(\mathrm{C}=\mathrm{NO} 1\)
Not B = NO4
Not D = N02
7.2

\section*{Characteristics of the grid}

A number of nodes: 275
A number of meshs and types: 120 TRIA7
7.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"COQUE_3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
9/10
8
Results of modeling C
8.1 Values
tested
Not
Size and unit
Reference
Aster
\% difference
With
displacement v (m)
0.1139
0.1112
1.76

\subsection*{8.2 Parameters}
of execution
Version: 4.00.14
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
12.6 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

\section*{Titrate:}

Cylindrical SSLS20 Hull pinch on free board
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.020-D Page:
10/10
9
Summary of the results
With regard to the elements:

\section*{- DKT:}

Suitable solution for a fine network.

To supplement later on:
by a less fine grid,
by an analysis of the constraints,
by 4 modelings (DKQ, DST, DSQ, Q4G).
- MEC3QU9H: very good solution obtained with a relatively coarse network.
- MEC3TR7H: to arrive at a suitable solution, that requires a fine grid, compared
with that for element MEC3QU9H. In the same way compared with element DKT, the number of nodes total is much more important.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster ©}

Version
4.0

Titrate:
Thin SSL127 bored or bent Section
Date:
19/01/98
Author (S):
P. MASSIN, J.R. LEVESQUE

Key:
V3.03.027-C Page:
1/6

\section*{Organization (S): EDF/IMA/MMN}

\section*{Handbook of Validation}

\section*{V3.03 booklet: Linear statics of the plates and hulls}

\section*{Document: V3.03.027}

SSLS27-Bored or bent thin section
Summary:
The test, taken again guide VPCS, makes it possible to check the behavior of an embedded plane plate subjected to sound
loose lead with two nodal forces of the same sign (inflection) or of opposite sign (torsion).
The first loading constitutes an extension of the initial test for which a reference solution is given in [bib3]. Only one modeling in element DKT.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/96/028 - Ind A
Code_Aster \({ }^{\circledR}\)
Version4.0
Titrate:
Thin SSL127 bored or bent Section
Date:
19/01/98
Author (S):
P. MASSIN, J.R. LEVESQUE
Key:
V3.03.027-C Page:
2/6
1
Problem of reference
1.1 Geometry
Z
y
D
C
C
C
E

\section*{L}
case 1
case 2
Length
\(\mathrm{L}=12 \mathrm{~m}\)
Width
\(\mathrm{B}=1 \mathrm{~m}\)
Thickness
\(\mathrm{T}=0.05 \mathrm{~m}\)
Co-ordinates of the points (in m):
With
B
C
D
E
0.
12.
12.

0 .
12.
y
0.5
0.5
0.5
0.5
0.

Z
0 .
0.
0.
0.
0.
1.2

Material properties
```

$\mathrm{E}=1.1011 \mathrm{~Pa}$

```
\(=0.25\)
1.3

\section*{Boundary conditions and loadings}

\section*{Embedded side AD:}
any point \(P\) such as \(x p=0(U=v=W=0 X=y=Z=0)\)
Loading: 2 loading cases
1) out of B and C: opposite forces parallel with axis \(Z\)
\(\mathrm{FzB}=1 \mathrm{NR}\)
\(\mathrm{FzC}=+1 \mathrm{NR}\)
2) out of \(B\) and \(C\) : of the same forces feel parallel with axis \(Z\)
\(\mathrm{FzB}=+1 \mathrm{NR}\)
\(\mathrm{FzC}=+1 \mathrm{NR}\)
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/96/028 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Thin SSL127 bored or bent Section
Date:
19/01/98
Author (S):
P. MASSIN, J.R. LEVESQUE

Key:
V3.03.027-C Page:
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2
Reference solution
2.1

Method of calculation used for the reference solution
- Opposite Forces perpendicular to the plate [bib1], [bib2]

The reference solution is that given in card SSLS27/89 of guide VPCS:
Displacement of the point \(\mathrm{C}: \mathrm{W}=35.37107 \mathrm{~m}\)
- Of the same Forces feel perpendicular to the plate [bib3]

The formulation in beam of Euler gives a solution approached for a Poisson's ratio

\section*{F}

3
Displacement of all the nodes on the side BC: \(W=\)
\(2 L\)
6th \(I z\)
2.2

\section*{Results of reference}

Displacement of the points B, C and E.

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References}

\section*{bibliographical}
[1]
J. ROBINSON "Element evaluation. With set of assessment shares and standard tests" Proceedings of Finite Methods Element in the commercial Environment, vol. 1, (October 1978).
[2]
J.L. BATOZ, M.B. Quadrilateral TAHAR "Evaluation of new thin punt boundary element" International Newspaper for Numerical Methods in Engineering, vol. 18, John Wiley \& Sounds (1982).
[3]
R.J. ROARK, W.C. YOUNG "Formulated for Stress and Strain" New York: Mc Graw-Hill, \(5^{\circ}\) edition, p 96.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/96/028 - Ind A

\section*{Code_Aster ©}

Version
4.0

Titrate:
Thin SSL127 bored or bent Section
Date:
19/01/98
Author (S):
P. MASSIN, J.R. LEVESQUE

Key:
V3.03.027-C Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling
Element of hull DKT
y
53
54
55
56
57
58
59
60
61

Twinge
1) FORCE_NODALE (GROUP_NO: B Fz: 1.) (GROUP_NO: C Fz: +1.)
2) FORCE_NODALE (GROUP_NO: B, C) Fz: +1.))

Name of the nodes:
Not \(\mathrm{A}=\mathrm{N} 1\)
Not \(\mathrm{C}=\mathrm{N} 65\)
Not \(\mathrm{E}=\mathrm{N} 39\)
Not \(\mathrm{B}=\mathrm{N} 13\)
Not \(\mathrm{D}=\mathrm{N} 53\)
3.2

Characteristics of the grid
A number of nodes: 65
A number of meshs and types: 96 TRIA3

\subsection*{3.3 Functionalities}
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
AFFE_MATERIAU
ALL
[U4.23.02]
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\section*{Code_Aster \({ }^{\circledR}\)}

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\section*{Reference}

\author{
Aster
}
\% difference

\author{
charge
}

B
3.537106
3.5243106
0.36

1
E
displacement W (m)
3. 1014

C
3.537106
3.5243106
0.36

B
1.0950103
0.99

2
E
displacement W (m)
1.1059103
1.0950103
0.99

C
1.0950103
0.99

\section*{Contents of the file results}

Displacements with the nodes for each loading case.

\subsection*{4.2 Parameters}
of execution
Version: 3.02.11
Machine: CRAY C98
System:

\section*{UNICOS 8.0}

Obstruction memory:
8 megawords
Time CPU To use:
8.5209 seconds

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
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Date:
19/01/98
Author (S):
P. MASSIN, J.R. LEVESQUE

Key:
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Summary of the results
- Suitable Solution for a fine network.
- To be supplemented later on by a less fine grid and to extend to other elements (DKQ, DST, DSQ, Q4G).
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Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

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\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ Document: V3.03.100
}

SSLS100-Plate circular embedded subjected with a uniform pressure

\section*{Summary:}

This problem allows a comparison between the solutions obtained and various elements of plate in elasticity
linear:
models of Coils-Kirchhoff (plate known as thin):
triangular surface mesh (TRIA3) DKT, quadrangular surface mesh (QUAD4) DKQ,
linear mesh (SEG3) COQUE_AXIS,
models of Mindlin-Reissner (plate known as thick):
quadrangular surface mesh (QUAD4) DSQ,
models of hulls thick: COQUE_3D (QUAD9 and TRIA7).
The same reference solution is treated with three forms of loadings: pressure, gravity and force-hull. The sizes observed are: displacements (translation/rotation), deformations and efforts generalized.

Modelings C and D were removed: resorption of modeling COQU_CYL.
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\section*{Code_Aster \({ }^{\circledR}\)}

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P. MASSIN, D. BUI, A. LAULUSA Key :
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{\(y\) \\ C \\ B \\ E \\ F \\ \(O\) \\ D}

With
\(X\)
1/4 of plate

\section*{Ray}
\(R=1 m\)
Thickness
\(T=0.1 \mathrm{~m}\)

\section*{Co-ordinates of the points:}

OABCDEF
\(X 0\).
1.

2 /2
0. 0.5

0 .

0.4
\(y 0\).
0.
\(2 / 2\)
1. 0 .
0.5
0.4
Z 0
0.
0.
0.
0.
0.
0 .
1.2
Material properties
\(E=1 P a\)
\(=0.3\)
\(=1 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3}
Boundary conditions and loadings
Embedding on the edge of the plate:
in all the points \(P\) such as \(C O p=R: U=v=W=0, X=y=Z=0\).

\section*{FORCE_COQUE}
Uniform pressure
\(P=1 \mathrm{~N} / \mathrm{m} 2\)
FORCE_COQUE
Charge distributed normal
\(F 3=1 \mathrm{~N} / \mathrm{m} 2\)
GRAVITY
\(G=10 \mathrm{~m} / \mathrm{s} 2\) according to \(Z\) from where
\(F Z=W P=1 \mathrm{~N} / \mathrm{m} 2\)
These three loadings lead to the same solution.

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Code_Aster \({ }^{\circledR}\)
Version
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\section*{2}

Reference solution
2.1

Method of calculation used for the reference solution
Two reference solutions are usable, for the calculation of the deformation:
the theory of LOVE-KIRCHHOFF, usually used for the plates known as "thin', that one will retain for modelings \(A, B, C, D, E\) and \(I\),
the theory of MINDLIN-REISSNER, including the effects of shearing for the plates known as "thick", that one will retain for modelings \(F, G, H\) and \(J\).
In any distant point of \(R\) of the center of the plate \((R R)\), the arrow is expressed:
4
2
2
3
(
\(\boldsymbol{P} \boldsymbol{R}\)
\(\boldsymbol{R}\)
\(\boldsymbol{R}\)
\(\boldsymbol{E} T\)
\(W \boldsymbol{R})=-\)
1-
1-
)
\(16 T 21\)
with \(=0(-\) KIRCHHOFF COILS) or \(=\)
(REISSNER.
)
5 R1-
For the calculation of the moments the two theories lead to the same expressions:
PR2
R 2
\(\boldsymbol{P} \boldsymbol{R} 2\)
R 2
\(M(\boldsymbol{R})\)
(3)
(1) \(M(R)=\)
(
1+3)
(1)
\(r r\)
=
\(+\)
16
R - +
16
\(\boldsymbol{R}\)

\section*{In the center of the plate:}
```

4

```
4
(
PR
PR
W)
0
=
(- KIRCHHOFF COILS) or (
W)
0
\(=\)
-
(1+) (REISSNER)
64D
64D
PR2
M()
0
\(=M()\)
0
= -
(1+
)
\(r r\)
16
Note:

Code_Aster calculates the moments with the nodes of each finite element in the reference mark of reference defined by the external normal and the reference axes defined on the hull (see AFFE_CARA_ELEM [U4.24.01]).
The value of moment Mxx (or Myy) in a node pertaining to several finite elements can to be regarded as being the average of the computed values on the elements which have this node jointly. This average can be obtained by procedure POST_RELEVE [U4.74.03].
For each node, one a: \((M+M)=(M+M)=S m\)
\(r r\)
\(x x\)
\(y y\)
```

for the point O
M
=M
= M
=M
x
yy
rr
for the points A and D
M
=M
and
M
= M
xx
rr
yy
for the points C and E
M
= M
and
M
= M
xx
yy
rr
for the points B and F
M
= M
xx
yy
=(M+M
rr
)/2
2.2
Results of reference

```

Arrow and moments at the points \(O, A, B, C, D, E, F\). Extraction of the average values of the components
M

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
TIMOSHENKO and WOINOWSKY-KRIEGER. Plates and hulls. Béranger edition, (1961).
[2]
BATOZ and DHATT. Modeling of the structures by finite elements. Hulls. Presses
Univ. Laval, 1992.
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Element of hull DKT (modeling of a quarter of plate)
C

\author{
B \\ E \\ \(F\) \\ O \\ With \\ D
}

Limiting conditions:
DDL_IMPO
in all the nodes of the arc \(A B C\)
(GROUP_NO: ABC DX: 0. , DY: 0. , DZ: 0.)
DRX: 0. , DRY: 0. , DRZ: 0.)
in all the nodes of segment] OA [
(GROUP_NO: OA DY: 0. , DRX: 0. , DRZ: 0.)
in all the nodes of segment] OC [
(GROUP_NO: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node \(O\)
(GROUP_NO: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Not \(O\)
meshs: M30, M33
Not A
meshs: M76
Not B
meshs: M39, M40, M51
Not C
meshs: M1
Not D
meshs: M55, M56, M65
Not E
meshs: M8, M17, M18
Not F
meshs: M34, M35, M37, M41, M46, M47, M48

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 50
A number of meshs and types: 76 TRIA3

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

\section*{Keys \\ AFFE_CARA_ELEM HULL \\ THICK}

\author{
[U4.24.01] \\ ANGL_REP \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_COQUE \\ NEAR \\ "MECHANICAL" AFFE_MODELE "DKT" \\ ALL \\ [U4.22.01] \\ CALC_CHAM_ELEM "EFGE_ELNO_DEPL" \\ [U4.61.01] \\ "SIGM_ELNO_DEPL" \\ POST_RELEVE ACTION \\ OPERATION \\ "EXTRACTION" \\ [U4.74.03] \\ Handbook of Validation
}

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Code_Aster \({ }^{\circledR}\)
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SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
```

4
Results of modeling $A$

```

\subsection*{4.1 Values}
tested
Not Reference Aster \% difference
tolerance
Coil-Kirchhoff
O(
\(W R\) )
-170.6251-169.32
-0.76
rel 1.102
D (
\(W\) R)
-95.9766-95.76
0.23 rel 0.5102

E(
\(W\) R)
-95.9766-95.73
0.25 rel 0.5102

F
W R)
-78.897-78.64
0.32 rel 0.5102

\section*{Reference}
Not
Mrr
M
Sm/2
\(O\)
\(-0.08125-0.08125-0.08125\)
With
\(+0.125+0.0375+0.08125\)
\(B\)
\(+0.125+0.0375+0.08125\)
```

C
+0.125 +0.0375 +0.08125
D
-0.02969 -0.05156 -0.04062
E
-0.02969 -0.05156 -0.04062
F
-0.01525 -0.04325 -0.02925
Aster
Not
Mxx
M yy
Sm/2
O
-0.08031-0.08033-0.08032
With
+0.1260 +0.0378 +0.0819
B
+0.08487+0.08507 +0.08497
C
+0.03778 +0.1259 +0.08184
D
-0.03166 -0.05328-0.04227
E
-0.05330-0.03164-0.04247
F
-0.02958 -0.02994-0.02974

```
\% difference (with eps 10-14 and \% difference on the absolute values)
Not
\(M x x\)
\(M y y\)
Sm/2
Relative tolerance
\(O\)
\(-1.15-1.14-1.15\)
1.5
With
\(+0.81+0.81+0.81\)
1.
\(B\)

C
\(+0.75+0.75+0.75\)
1.

D
\(+6.65+3.34+4.55\)
7./3.5

E
\(+3.38+6.58+4.55\)
3.5/7.

F
\(+1.14+2.35+1.71\)
1.5

\subsection*{4.2 Parameters}
of execution
Version: 4.00.02

\section*{Machine: CRAY C90}

System:
UNICOS 8.0
Obstruction memory:
16 MW
Time CPU To use:
5.7 seconds

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Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

\section*{5 Modeling \\ B}

\section*{5.1}

Characteristics of modeling
Element of hull DKT (modeling of a quarter of plate)
C
B
E
\(F\)
O
With
D

Limiting conditions:
DDL_IMPO
in all the nodes of the arc \(A B C\)
(GROUP_NO: ABC DX: 0. , DY: 0. , DZ: 0.)
DRX: 0. , DRY: 0. , DRZ: 0.)
in all the nodes of segment] OA [
(GROUP_NO: OA DY: 0. , DRX: 0. , DRZ: 0.)
in all the nodes of segment] OC [
(GROUP_NO: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node \(O\)
(GROUP_NO: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)

\section*{Not \(O\)}
meshs: M1 m2
Not A
meshs: M248 M255
Not B
meshs: M292 M293 M296
Not C
meshs: M74 M75
Not D
meshs: M76 M108 M109

Not \(E\)
meshs: M34 M40 M41
Not F
meshs: M122 M123 M124 M148 M152 M153

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 170
A number of meshs and types: 296 TRIA3

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
Keys \\ AFFE_CARA_ELEM HULL
}

\section*{[U4.24.01]}

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_COQUE
NEAR
F3
GRAVITY
"MECHANICAL" AFFE_MODELE "DKT"
ALL
[U4.22.01]
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
[U4.61.01]
"SIGM_ELNO_DEPL"
POST_RELEVE ACTION
OPERATION
"EXTRACTION"
[U4.74.03]

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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested
```

Not Reference Aster \%
difference
tolerance
Coil-Kirchhoff
O(
$W R$ )
170.6251170 .83840 .12 rel 0.5102
D (
$W R$ )
-95.9766 -96.1477 0.18
rel 0.5102
E(
$W R$ )
-95.9766 -96.2078 0.24
rel 0.5102
F (
W R)
-78.897-79.072 0.22

```
```

rel 0.5102

```

\section*{Reference}
\(\left.\begin{array}{l}\text { Not } \\ \text { Mrr } \\ M \\ \text { Sm/2 } \\ O \\ -0.08125-0.08125-0.08125 \\ \text { With } \\ +0.125+0.0375+0.08125 \\ B \\ +0.125+0.0375+0.08125 \\ C \\ +0.125+0.0375+0.08125 \\ D \\ -0.02969-0.05156-0.04062 \\ E \\ -0.02969-0.05156-0.04062 \\ F \\ -0.01525-0.04325-0.02925 \\ \text { Aster } \\ \text { Not } \\ \text { Mxx } \\ M y y \\ \text { Sm/2 } \\ O \\ -0.08151 \\ \text { With } \\ 0.0 .08131\end{array}\right]-0.08141\)
\% difference (with eps 10-14 and \% difference on the absolute values)
Not
\(M x x\)
\(M y y\)
Sm/2
Relative tolerance
\(O\)
0.330 .070 .19
0.5
With
2.628 .884 .02
3.19.
\(B\)
2.642 .642 .64
3.
\(C\)
8.952 .694 .13
9.13.
\(D\)
2.291 .811 .99
2.5
\(E\)
2.01
2.49
2.5
\(F\)
1.92
2.19
2.18
2.05
6.2 Parameters
of execution

Version: 4.00.02

Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
9.5 seconds

\author{
Handbook of Validation
}

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22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
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\section*{7 Modeling}

\section*{E}

\section*{7.1 \\ Characteristics of modeling}

Element of hull DKQ (modeling of a quarter of plate)

C
1.0

B
0.5

E
0.0

O
D
With
0.0
0.5
1.0

Limiting conditions:
DDL_IMPO
in all the nodes of the arc \(A B C\)
(GROUP_NO: ABC DX: 0. , DY: 0. , DZ: 0.)

DRX: 0. , DRY: 0. , DRZ: 0.)
in all the nodes of segment] OA [
(GROUP_NO: OA DY: 0. , DRX: 0. , DRZ: 0.)
in all the nodes of segment] OC [
(GROUP_NO: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node \(O\)
(GROUP_NO: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)

\author{
Not \(O\)
}
meshs: M1
Not A
meshs: M147
Not B
meshs: M98 M111
Not C
meshs: M14
Not D
meshs: M85 M99
Not E
meshs: M7 M8
Not F
meshs: M91 M92 M105

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 169
A number of meshs and types: 147 QUAD4

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

\author{
Keys \\ AFFE_CARA_ELEM HULL
}
[U4.24.01]
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
[U4.25.01]

\title{
NEAR
}

\section*{F3}

GRAVITY

\author{
"MECHANICAL" AFFE_MODELE "DKT" \\ ALL \\ [U4.22.01] \\ CALC_CHAM_ELEM "EFGE_ELNO_DEPL" \\ [U4.61.01] \\ "SIGM_ELNO_DEPL" \\ POST_RELEVE ACTION \\ OPERATION \\ "EXTRACTION" \\ [U4.74.03] \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
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\section*{Titrate:}

SSLS100 Plates circular embedded subjected to a pressure
Date:
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Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

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8
Results of modeling \(E\)

\subsection*{8.1 Values}
tested

\section*{Not Reference Aster \%}
tolerance
Coil-Kirchhoff
O (
\(W\) R)
-170.6251-171.00
0.22 rel 0.5102

D
W R)
-95.9766-96.198 0.23
rel 0.5102
E(
W R)
-95.9766-96.198 0.23
rel 0.5102
F (
W R)
-78.897-79.06 0.20
rel 0.5102
Reference
Not
Mrr
\(M\)
\(S m / 2\)
\(O\)
\(-0.08125-0.08125\)
-0.08125
With
\(+0.125+0.0375\)
+0.08125
\(B\)
\(+0.125+0.0375\)
+0.08125
\(C\)
\(+0.125+0.0375\)
+0.08125
\(D\)
\(-0.02969-0.05156\)
-0.04062
\(E\)
\(-0.02969-0.05156\)
F
-0.01525-0.04325
-0.02925
Aster
Not
Mxx
Myy
\(\mathrm{Sm} / 2\)
O
\(-0.08162-0.08162-0.08162\)
With
0.12560 .037680 .08164
B
0.081410 .081400 .08141
C
0.037670 .125570 .08162
D
-0.03030-0.05314-0.04172
E
-0.05314-0.03030-0.04172
F
-0.02903-0.02901-0.02902
\% difference (with eps 1014 and \% difference on the absolute values)
Not
\(M x x\)
\(M y y\)
Sm/2
Relative tolerance
\(O\)
0.460 .460 .46
0.5
With
0.49
0.49
0.5
O.49
B
0.20 .20 .2
0.5
\(C\)
0.45

\subsection*{8.2 Parameters \\ of execution}

Version: 4.00.02

\author{
Machine: CRAY C90
}

System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
8.4 seconds

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Code_Aster \({ }^{\circledR}\)
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4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure

\section*{Date:}

22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

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\section*{9 Modeling \\ F \\ 9.1 \\ Characteristics of modeling}

Element of hull DST (modeling of a quarter of plate)

C
B
E
F
O
With
D

Limiting conditions:
DDL_IMPO
in all the nodes of the arc \(A B C\)
(GROUP_NO: AB DX: 0. , DY: 0. , DZ: 0.)
DRX: 0. , DRY: 0. , DRZ: 0.)
in all the nodes of segment] OA [
(GROUP_NO: OA DY: 0. , DRX: 0. , DRZ: 0.)
in all the nodes of segment] OC [
(GROUP_NO: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node \(O\)
(GROUP_NO: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)

\section*{Not \(O\)}
meshs: M1 m2
Not A
meshs: M248 M255
Not B
meshs: M292 M293 M296
Not C
meshs: M74 M75
Not D
meshs: M76 M108 M109
Not E
meshs: M34 M40 M41
Not F
meshs: M122 M123 M124 M148 M152 M153

\section*{9.2 \\ Characteristics of the grid}

A number of nodes: 170
A number of meshs and types: 296 TRIA3

\subsection*{9.3 Functionalities}
tested

\section*{Orders}

\author{
Keys \\ AFFE_CARA_ELEM HULL
}

\section*{[U4.24.01]}

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_COQUE
NEAR
F3
GRAVITY

\author{
"MECHANICAL" AFFE_MODELE "DST" \\ ALL \\ [U4.22.01] \\ CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
}

\author{
[U4.61.01] \\ "SIGM_ELNO_DEPL" \\ POST_RELEVE ACTION \\ OPERATION \\ "EXTRACTION" \\ [U4.74.03] \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

V3.03.100-C Page:
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\section*{10 Results of modeling \(F\)}

\subsection*{10.1 Values}
tested

\section*{Not Reference Aster \% difference tolerance}

Reissner
O(
W R)
\(178.419179 .743+0.74 \mathrm{rel} 1.0102\)
D (
W R)
\(101.82102 .60+0.77\) rel 1.0102
E (
W R)
\(101.82102 .67+0.84\) rel 1.0102
F (
W R)
\(84.19884 .83+0.75\) rel 1.0102
Reference
\begin{tabular}{lll} 
Not \\
Mrr \\
\(\boldsymbol{M}\) \\
Sm/2 \\
\(O\) \\
-0.08125 & -0.08125 & -0.08125 \\
With
\end{tabular}
```

+0.125+0.0375+0.08125
B
+0.125 +0.0375 +0.08125
C
+0.125+0.0375+0.08125
D
-0.02969 -0.05156 -0.04062
E
-0.02969 -0.05156 -0.04062
F
-0.01525-0.04325-0.02925
Aster
Not
Mxx
M yy
Sm/2
O
-0.08216-0.08204-0.08210
With
+0.12515 +0.04591 +0.08553
B
+0.08289 +0.08289 +0.08289
C
+0.04594 +0.12524 +0.08559
D
-0.03108-0.05184-0.04146
E
-0.05195-0.03116-0.04155
F
-0.02956 -0.02963-0.02960

```
\% difference (with eps 1014 and \% difference on the absolute values)
Not
Mxx
Myy
Sm/2

\section*{Relative tolerance}

O
\(+1.12-0.97+1.04\)
1.5/1.

With
\(+0.12+22.44+5.26\)
0.5/23.

B
\(+2.02+2.02+2.02\)
2.5

C
\(+22.52+0.19+5.34\)
23./0.5

D
\(+4.67+0.54+2.07\)
5./1.

E
\(+0.76+4.94+2.29\)
1./5.

F
\(+1.07+1.33+1.19\)
1.5
10.2 Contents of the file results

Values at the points of observation of displacements and moments realised.
10.3 Parameters
of execution
Version: 4.00.02

Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
9.2 seconds

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

V3.03.100-C Page:
12/24

\section*{11 Modeling}

G
11.1 Characteristics of modeling

Element of hull DSQ (modeling of a quarter of plate)
C
1.0

B
0.5

E
0.0

O
D
With
0.0
0.5
1.0

Limiting conditions:
DDL_IMPO
in all the nodes of the arc \(A B C\)
(GROUP_NO: ABC DX: 0. , DY: 0. , DZ: 0.)
DRX: 0. , DRY: 0. , DRZ: 0.)
in all the nodes of segment] OA [
(GROUP_NO: OA DY: 0. , DRX: 0. , DRZ: 0.)
in all the nodes of segment] OC [
(GROUP_NO: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node 0
(GROUP_NO: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)

\section*{Not \(O\)}
meshs: M1
Not A
meshs: M147
Not B
meshs: M98 M111
Not C
meshs: M14
Not D
meshs: M85 M99
Not E
meshs: M7 M8
Not \(F\)
meshs: M91 M92 M105

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 169
A number of meshs and types: 147 QUAD4

\subsection*{11.3 Functionalities}
tested
Orders

\author{
Keys \\ AFFE_CARA_ELEM HULL
}

\section*{[U4.24.01]}

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_COQUE
NEAR
F3
GRAVITY

ALL
[U4.22.01]
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"

\author{
[U4.61.01] \\ "SIGM_ELNO_DEPL" \\ POST_RELEVE ACTION \\ OPERATION \\ "EXTRACTION" \\ [U4.74.03] \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:
13/24

12 Results of modeling \(G\)

\subsection*{12.1 Values}
tested
Not Reference Aster \% difference tolerance
Reissner
O(
W R)
\(178.419178 .754+0.19\) rel 0.3102
D (
W R

\section*{\(101.82102 .015+0.19\) rel 0.3102}

E (
W R)
\(101.82102 .014+0.19\) rel 0.3102
F (
W R)
\(84.19884 .318+0.14\) rel 0.3102

\section*{Reference}
Not
Mrr
\(\boldsymbol{M}\)
Sm/2
\(\boldsymbol{O}\)
\(-0.08125-0.08125-0.08125\)
With
\(+0.125+0.0375+0.08125\)
\(\boldsymbol{B}\)
\(+0.125+0.0375+0.08125\)
\(\boldsymbol{C}\)
\(+0.125+0.0375+0.08125\)
\(\boldsymbol{D}\)
\(-0.02969-0.05156-0.04062\)
\(\boldsymbol{E}\)
\(-0.02969-0.05156-0.04062\)
\(\boldsymbol{F}\)
\(-0.01525-0.04325-0.02925\)
Aster
Not
Mxx
\(\boldsymbol{M} \boldsymbol{y y}\)
Sm/2
\(\boldsymbol{O}\)
-0.08098
With
+0.08098
\(+0.12730+0.04136+0.08098\)
\(\boldsymbol{B}\)
\(+0.08261+0.08261+0.08261\)
\(\boldsymbol{C}\)
\(+0.04114+0.12727+0.08420\)
\(\boldsymbol{D}\)
-0.03029
```

E
-0.05217 -0.03029 -0.04078
F
-0.03449-0.03444-0.03447
% difference (with eps 10-14 and % difference on the absolute values)
Not
Mxx
Myy
Sm/2
Relative tolerance
O
-0.33-0.33+0.33
0.5
With
+1.84 +10.31 +3.79
2./11.
B
+1.68 +1.68 +1.69
2.
C
+9.07 +1.82 +3.64
10./2.
D
+2.05 +1.19 +0.4
2.5/1.5
E
+1.19 +2.05 +0.4
1.5/2.5
F
+17.91 +17.74 +17.80
18
12.2 Contents of the file results

```

Values at the points of observation of displacements and moments realised.
12.3 Parameters
of execution
Version: 4.00.02

\section*{Machine: CRAY C90}

\section*{System: \\ UNICOS 8.0}

Obstruction memory:
16 megawords
Time CPU To use:
8.6 seconds

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

V3.03.100-C Page:
14/24
13 Modeling
H

\subsection*{13.1 Characteristics of modeling}

Element of hull Q4G (modeling of a quarter of plate)
C
1.0

B
0.5

E
0.0

O
D
With

\section*{Limiting conditions:}

DDL_IMPO
in all the nodes of the arc \(A B C\)
(GROUP_NO: ABCDX: 0. , DY: 0. , DZ: 0.)
D DRX: 0. , DRY: 0. , DRZ: 0.)
in all the nodes of segment] OA [
(GROUP_NO: OA DY: 0. , DRX: 0. , DRZ: 0.)
in all the nodes of segment] \(O C\) [
(GROUP_NO: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node 0
(GROUP_NO: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Not \(O\)
meshs: M1
Not A
meshs: M147
Not B
meshs: M98 M111
Not C
meshs: M14
Not D
meshs: M85 M99
Not E
meshs: M7 M8
Not F
meshs: M91 M92 M105
13.2 Characteristics of the grid

A number of nodes: 169
A number of meshs and types: 147 QUAD4

\subsection*{13.3 Functionalities}
tested
Orders

Keys
AFFE_CARA_ELEM HULL

\author{
[U4.24.01] \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_COQUE \\ NEAR \\ F3 \\ GRAVITY
}
"MECHANICAL"AFFE_MODELE "DKT"
ALL
[U4.22.01]
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
[U4.61.01]
"SIGM_ELNO_DEPL"
POST_RELEVE ACTION
OPERATION
"EXTRACTION"
[U4.74.03]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:
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\section*{14 Results of modeling \(\boldsymbol{H}\)}

\subsection*{14.1 Values}

\section*{tested}
```

Not Reference Aster \%
difference tolerance
Reissner
O(
W R)
-178.419-178.282-0.08
rel 0.4102
D (
W R)
-101.82-101.532-0.28
rel 0.4102
E(
W R)
-101.82-101.531-0.28
rel 0.4102
F (
W R)
-84.198-83.91-0.34
rel 0.4102
Reference

```

```

Aster
Not
Mxx
M yy
Sm/2
O
-0.08119 -0.08119 -0.08119
With
+0.11589 +0.03348 +0.07468
B
+0.07236 +0.07234 +0.07235
C
+0.03349+0.11164+0.07256
D
-0.02961-0.05223-0.04092
E
-0.05222 -0.02961-0.04091
F
-0.02897 -0.02898-0.02897
% difference (with eps 10-14 and % difference on the absolute values)
Not
Mxx
Myy
Sm/2
Relative tolerance
O
-0.07-0.07-0.07
0 . 1
With
-10.73 -10.73 -10.73
11.
B
-10.94-10.96 -10.95
11.
C
-10.69 -10.69 -10.69
11.
D
-0.25+1.3+0.74
0.5/1.5
E

```

\subsection*{14.2 Contents of the file results}

Values at the points of observation of displacements and moments realised.

\subsection*{14.3 Parameters}
of execution
Version: 4.00.02

Machine: CRAY C90

\section*{System:}

UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:
8.4 seconds

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key :
V3.03.100-C Page:
16/24

\section*{15 Modeling}

I

\subsection*{15.1 Characteristics of modeling}

Axisymmetric element of hull SEG3, in theory of Coils-Kirchhoff: one does not consider modification of metric, coefficient A_CIS is worth 106.
\(y\)
p
O
D
With
\(X\)
0
0.5
1.0

Limiting conditions:
DDL_IMPO:
(NODE: WITH DX: 0. , DY: 0. , DRZ: 0.)
(NODE:
O
DRZ:
0.)
15.2 Characteristics of the grid

A number of nodes: 21
A number of meshs and types: 10 SEG3

\author{
15.3 Functionalities \\ tested \\ Orders
}

Keys
"MECHANICAL"AFFE_MODELE "COQUE_AXIS"

\author{
ALL \\ [U4.22.01] \\ AFFE_CARA_ELEM HULL \\ A_CIS \\ [U4.24.01] \\ MODI_METRIQUE \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_COQUE \\ ALL
}

\section*{CALC_CHAM_ELEM "DEGE_ELNO_DEPL"}
[U4.61.01]
"EFGE_ELNO_DEPL"
"SIEF_ELGA_DEPL"
"SIGM_ELNO_DEPL"
POST_RELEVE ACTION
NODE
"EXTRACTION"
[U4.74.03]

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:

\title{
16 Results of modeling I
}

\author{
16.1 Values
}
tested

\author{
Not Reference \\ Aster \% \\ difference \\ tolerance \\ Coil-Kirchhoff \\ O arrow ( \\ W R) \\ 170.6251169 .7590 .51 rel 102 \\ D arrow ( \\ W R) \\ -95.9765 -95.0383 \\ 0.98 rel 102 \\ D rotation (R) \\ 255.940257 .120 \\ 0.46 rel 102
}
Not
Réfé rence
Aster
\%
diff érence
Krr
maill
K
K \(x x\)
K yy
K xx
K yy
\(D\)
170.625511 .875 IJK
60.0456514 .124
0.44
0.44

\section*{KLM}
265.473
514.361
0.49
moy
162.760
514.242
4.61
0.46

\section*{Not}

Réfé rence
Aster
\%
diff érence
Mrr
maill
M
\(M \boldsymbol{x x}\)
\(M y y\)
\(M \boldsymbol{x x}\)
\(M y y\)
O 0.08125
-0.08125
STU
-0.08139
-0.08139
0.18
0.18

To +0.125
+0.0375
ABC
+0.10717
+0.03215
14.2
14.2

D 0.02969
-0.05156
IJK

Note:
One notes the good results obtained, except on Krr and M rr, which utilize derivative of a higher nature less better calculated by the element.
16.2 Contents of the file results

Generalized displacements, deformations and efforts and constraints with the nodes.

\subsection*{16.3 Parameters \\ of execution}

Version: 3.02.11

Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 MW
Time CPU To use:
5.11 seconds

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure

\author{
Date: \\ 22/12/98 \\ Author (S): \\ : \\ V3.03.100-C Page: \\ 18/24
}
P. MASSIN, D. BUI, A. LAULUSA Key

\section*{17 Modeling}
\(J\)

\subsection*{17.1 Characteristics of modeling}

Axisymmetric element of hull SEG3, in theory of Mindlin-Reissner: one does not consider modification of metric, coefficient A_CIS is worth \(5 / 6\).

\section*{Limiting conditions:}

DDL_IMPO:
(NODE: WITH DX: 0. , DY: 0. , DRZ: 0.)
(NODE:
O
DRZ:
0.\()\)
17.2 Characteristics of the grid

A number of nodes: 21
A number of meshs and types: 10 SEG3

\subsection*{17.3 Functionalities}
tested
Orders

Keys
"MECHANICAL"AFFE_MODELE "COQUE_AXIS"
ALL
[U4.22.01]
AFFE_CARA_ELEM HULL
A_CIS
[U4.24.01]
MODI_METRIQUE
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_COQUE
ALL

CALC_CHAM_ELEM "DEGE_ELNO_DEPL"
[U4.61.01]
"EFGE_ELNO_DEPL"
"SIEF_ELGA_DEPL"
"SIGM_ELNO_DEPL"
POST_RELEVE ACTION
NODE
"EXTRACTION"
[U4.74.03]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:
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18 Results of modeling J
18.1 Values
tested
```

Not Reference
Aster \%
difference
tolerance
REISSNER
O arrow (
W R)
-178.424-178.368
0.03
rel 102
D arrow (
W R)
-101.827-101.7769
0.05
rel 102
D rotation (R)
255.940256 .001
0.02
rel 102

```
Not
Réfé rence
Aster
\%
diff érence
Krr
maill
\(K\)
\(K \boldsymbol{x} x\)
\(K y y\)
\(K \boldsymbol{x x}\)
\(K y y\)
D
170.625511 .875 IJK
167.892 512.050-1.62 0.03
KLM
178.920
511.952
4.86
0.01
moy
173.406
512.001
1.60
0.02
Not
Réfé rence
Aster
\%
diff érence
M rr
maill
M
Mxx
\(M y y\)
Not
Réfe rence
Aster
\%
diff érence
Mrr
maill
M
\(M \boldsymbol{x x}\)
\(M y y\)

\section*{IJK}

\section*{Note:}

One notes the good results obtained, except on Krr and M rr, which utilize derivative of a higher nature less better calculated by the element.
18.2 Contents of the file results

Generalized displacements, deformations and efforts and constraints with the nodes.

\subsection*{18.3 Parameters}
of execution

Version: 3.02.11

Machine: CRAYC90

System:
UNICOS 8.0
Obstruction memory:
16 MW
Time CPU To use:
4.99 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:
20/24

19 Modeling
K
19.1 Characteristics of modeling

Modeling: Element of hull COQUE_3D MEC3QU9H

\section*{With}

\section*{Limiting conditions:}

\section*{DDL_IMPO:}
in all the nodes of the arc (Group_no: ABC DX: 0. , DY: 0. , DZ: 0.)
\(A B C\)
DRX: 0. , DRY: 0. , DRZ: 0.)
segment] OA]
(Group_no: OA DY: 0. , DRX: 0. , DRZ: 0.)
segment] OC]
(Group_no: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node O
(Group_no: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Names of the nodes:
Point 0
meshs: M1
Not A
meshs: M21
Not B
meshs: M25
Not C
meshs: M5
Not D
meshs: M11
Not E
meshs: M3

\subsection*{19.2 Characteristics of the grid}

A number of nodes: 96
A number of meshs and types: 25 QUAD9
19.3 Functionalities
tested
Orders

\author{
Keys \\ AFFE_CARA_ELEM HULL
}

\author{
[U4.24.01] \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_COQUE \\ NEAR
}

F3

GRAVITY

AFFE_MATERIAU ALL
[U4.23.02]
"MECHANICAL"AFFE_MODELE "COQUE_3D"
ALL
[U4.22.01]
CALC_CHAM_ELEM "EFFO_ELNO_DEPL"
[U4.61.01]
"SIGM_ELNO_DEPL"

\section*{DEFI_MATERIAU ELAS}
[U4.23.01]
POST_RELEVE ACTION
OPERATION "EXTRACTION"
[U4.74.03]
VALE_TEST

EPSI_TEST

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:
21/24

20 Results of modeling \(K\)
20.1 Values
tested
Not Reference Aster \%
difference
tolerance
REISSNER
O
-178.419-178.39
0.016 rel 102

With
0.eps
- ABS 1010

B
0. eps
- ABS 1010

C W
(R)
0.eps
- ABS 1010

D
-101.82-101.79
0.029 rel 102
```

E
-101.82 -101.79
0 . 0 2 9 ~ r e l ~ 1 0 2 ~

```

\section*{Not Reference \\ Aster}
Mrr
M
Sm/2
\(M x x\)
M yy
Sm/2
O 0.08125
-0.08125
-0.08125
-0.08342
-0.08342
0.08342
To +0.125
+0.0375
\(+0.08125\)
0.03675
0.1234
0.08007
C +0.125
+0.0375
\(+0.08125\)
0.1243
0.03582
0.08006
D 0.02969
-0.05156
-0.04062
-0.05241
-0.02971
0.04106
E 0.02969
-0.05156
-0.04062
-0.02958
-0.05254
0.04106

Not
\% difference
Relative tolerance
\(M x x\)
\(M y y\)
\(\mathrm{Sm} / 2\)
O 2.67
2.67
2.673.

To 2.
-1.28
-1.45 3 .
C 0.56
-4.48
-1.46 5 .
D 1.65
0.067
1.082.

E 0.37
1.9
0.952.
with eps 1014 and \% difference on the absolute values

\section*{Note:}

The test of the values is carried out automatically using the functionalities offered by procedure POST_RELEVE:
extraction on the nodes corresponding to the points observed of the average values of components Mxx and Myy; these values are extracted from field "EFGE_ELNO_DEPL", and the average is calculated for all the liquid assets on the meshs which contain it node observed,
calculation of the variation compared to the value of reference provided by observing the rules of correspondence between Mxx, Myy and Mrr, M given page 3.

\section*{Contents of the file results}

Values at the points of observation of displacements and moments realised.

\subsection*{20.2 Parameters \\ of execution}

Version: 4.00.14

Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 MW
Time CPU To use:
8.77 seconds

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

V3.03.100-C Page:
22/24

\section*{21 Modeling}

L

\subsection*{21.1 Characteristics of modeling}

Modeling: Element of COQUE_3D MEC3TR7H
\(C\)
\(C\)
\(E\)
\(O\)
\(D\)
With

Limiting conditions:

\section*{DDL_IMPO:}
in all the nodes of the arc (Group_no: ABC DX: 0. , DY: 0. , DZ: 0.)
ABC
DRX: 0. , DRY: 0. , DRZ: 0.)
segment] OA]
(Group_no: OA DY: 0. , DRX: 0. , DRZ: 0.)
segment] OC]
(Group_no: OC DX: 0. , DRY: 0. , DRZ: 0.)
with the node O
(Group_no: O DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
Names of the nodes:

\author{
Point 0 \\ meshs: M1 and m2 \\ Not A \\ meshs: M41 \\ Not B \\ meshs: M49 and M50 \\ Not C \\ meshs: M10 \\ Not D \\ meshs: M21 \\ Not \(E\) \\ meshs: M6 \\ \subsection*{21.2 Characteristics of the grid} \\ A number of nodes: 121 \\ A number of meshs and types: 50 TRIA7 \\ \subsection*{21.3 Functionalities} \\ tested \\ Orders \\ Keys \\ AFFE_CARA_ELEM HULL \\ [U4.24.01] \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_COQUE \\ NEAR
}

F3

\section*{GRAVITY}
\(A F F E \_M A T E R I A U A L L\)

\title{
[U4.23.02]
}
"MECHANICAL"AFFE_MODELE "COQUE_3D"

\section*{ALL}
[U4.22.01]
CALC_CHAM_ELEM "EFFO_ELNO_DEPL"
[U4.61.01]
"SIGM_ELNO_DEPL"

DEFI_MATERIAU ELAS
[U4.23.01]
POST_RELEVE ACTION
OPERATION "EXTRACTION"
[U4.74.03]
VALE_TEST

EPSI_TEST

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

\section*{Titrate:}

SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.100-C Page:
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\section*{22 Results of modeling \(L\)}

\subsection*{22.1 Values}
tested

\author{
Not Reference Aster \% difference \\ tolerance \\ REISSNER \\ O \\ -178.419-178.18 \\ -0.13 \\ rel 102 \\ With \\ 0. eps \\ - ABS 1010 \\ B \\ 0. eps \\ - ABS 1010 \\ C W \\ (R) \\ 0.eps \\ - ABS 1010 \\ D \\ -101.82-101.46 \\ 0.35 rel 102 \\ E \\ -101.82-101.46 \\ 0.35 rel 102
}

Not Reference
Aster
Mrr
M
Sm/2
\(M x x\)
Myy
\(\mathrm{Sm} / 2\)
O 0.08125
-0.08125
-0.08125
-0.08360
\[
C+0.125
\]
\[
+0.0375
\]
\[
+0.08125
\]
\[
0.12325
\]
\[
0.03711
\]
\[
0.08018
\]
\[
\text { D } 0.02969
\]
-0.05156
-0.04062
\[
-0.05117
\]
-0.02907
\[
0.04012
\]
\[
\text { E } 0.02969
\]
-0.05156
-0.04062
-0.02905
-0.05119
\[
0.04012
\]

\author{
Not \\ \% difference \\ Relative tolerance
}

\author{
\(M x x\)
}

M yy
Sm/2
O 2.89
2.89
2.893.

To 0.24
-1.65
-1.33 3.
C 1.4
-1.04
-1.32 3.
with eps 1014 and \% difference on the absolute values

\section*{Note:}

The test of the values is carried out automatically using the functionalities offered by procedure POST_RELEVE:
extraction on the nodes corresponding to the points observed of the average values of components Mxx and Myy; these values are extracted from field "EFGE_ELNO_DEPL", and the average is calculated for all the liquid assets on the meshs which contain it node observed,
calculation of the variation compared to the value of reference provided by observing the rules of correspondence between Mxx, Myy and Mrr, M given page 3.

Contents of the file results
Values at the points of observation of displacements and moments realised.

\subsection*{22.2 Parameters}
of execution
Version: 4.00.14

Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 MW
Time CPU To use:
9.29 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLS100 Plates circular embedded subjected to a pressure
Date:
22/12/98
Author (S):
P. MASSIN, D. BUI, A. LAULUSA Key

V3.03.100-C Page:
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23 Summary of the results
\% of the differences compared to the reference solutions

\section*{DKT DKQ}

DST
DSQ
Q4G
Modé. With
B
E
F
G
H
Coil Kirchhoff
Coil-Kirchhoff
Reissner
Reissner
Reissner
Point 50
nodes
170 nodes
169 nodes
170 nodes
169 nodes
169 nodes
76 TRIA3
296 TRIA3
147 QUAD4
296 TRIA3
147 QUAD4 147 QUAD4
\(\boldsymbol{O}(\)
\(\boldsymbol{W} \boldsymbol{R})\)
\(-0.76+0.12+0.22\)
\(+0.74+0.19-0.08\)
\(\boldsymbol{D}(\)
\(\boldsymbol{W} \boldsymbol{R})\)
\(-0.23+0.18+0.23\)
\(+0.77+0.19-0.28\)
\(\boldsymbol{E}(\)
\(\boldsymbol{W} \boldsymbol{R})\)
\(-0.25+0.24+0.23\)
\(+0.84+0.19-0.28\)
\(\boldsymbol{F}(\)
\(\boldsymbol{W} \boldsymbol{R})\)
\(-0.32+0.22+0.20\)
\(+0.75+0.14-0.34\)

\section*{COQU_AXIS MEC3QU9H MEC3TR7H}

Modé. I
\(\boldsymbol{J}\)
\(\boldsymbol{K}\)
\(\boldsymbol{L}\)

\section*{Coil-Kirchhoff}

Reissner
Point 21
nodes
96
nodes
121 nodes
10 SEG3
25 QUAD9
50 TRIA7
O (
W R)
+0.51 0.03
-0.16-0.13

\section*{D (}

W R)
+0.28 0.05
-0.029
-0.35
E (
W R)
-0.029
-0.35
F (
W R)
\(-+0.22\)

DKT DKQ
DST
DSQ
Q4G
Modé. With
B
\(\boldsymbol{E}\)
\(\boldsymbol{F}\)
\(\boldsymbol{G}\)
\(\boldsymbol{H}\)
Coil Kirchhoff
Coil-Kirchhoff
Reissner
Reissner
Reissner
Point 50
nodes
170 nodes
169 nodes
170 nodes
169 nodes
169 nodes
76 TRIA3
296 TRIA3
147 QUAD4
296 TRIA3
147 QUAD4 147 QUAD4

\section*{O Sm/2 1.15}
\(+0.19\)
\(+0.46\)
\(+1.04\)
-0.33
-0.07
To Sm/2 +0.81
\(+4.02\)
\(+0.49\)
+5.26
\(+3.79\)
-10.73
B Sm/2 +4.58
\(+2.64\)
\(+0.20\)
+2.02
\(+1.69\)
-10.95
C Sm/2 +0.75
\(+4.13\)
\(+0.45\)
\(+5.34\)
\(+3.64\)
-10.69
D Sm/2 +4.55
+1.99
\(+2.71\)
\(+2.07\)
\(+0.40\)
\(+0.74\)
\(E \operatorname{Sm} / 2+4.55\)
+2.19
\(+2.71\)
\(+2.29\)
\(+0.40\)
\(+0.74\)
F Sm/2 +1.71
+2.05
-0.79
\(+1.19\)
\(+17.80\)
-0.94

\author{
COQU_AXIS MEC3QU9H \\ MEC3TR7H \\ Modé. I \\ \(J\) \\ K \\ L \\ Coil-Kirchhoff \\ Reissner \\ Point 21 \\ nodes \\ 96 \\ nodes \\ 121 nodes \\ 10 SEG3 \\ 25 QUAD9 \\ 50 TRIA7 \\ O Sm/2 \\ \(+0.18+0.62\) \\ 2.672 .89 \\ To Sm/2 \\ +14.2-1.01 \\ \(-1.45-1.33\) \\ B Sm/2
}

\section*{C Sm/2}
-1.46
-1.32
D Sm/2
+0.84-0.85
1.08-1.23

E Sm/2
0.95
-1.23
F Sm/2

Note:

Concerning the efforts, direct calculation with the nodes leads to variations in several nodes, in particular at the point \(F\) in DSQ and on the edge \(A B C\) in \(Q 4\).

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

V3.03.101-D Page:
1/22
Organization (S): EDF-R \& D /AMA, SAMTECH

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
Document: V3.03.101

SSLS101 - Plate circular posed subjected with a uniform pressure

\section*{Summary:}

One treats the case of a circular plate posed on the edge in linear elasticity under 3 loadings (weight clean, pressure, effort distributed constant) which gives the same deformation.

The first two modelings make it possible to evaluate the influence of the grid.
The test gathers 9 modelings (model Coils-Kirchhoff, Mindlin-Reissner and COQUE_3D and SHB8).

\section*{Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/03/008/A}

\section*{Code_Aster \({ }^{\circledR}\)}

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Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

\section*{:}

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{With}
\(X\)
1/4 of plate
Ray
\(R=1 \mathrm{~m}\)
Thickness
\(T=0.1 \mathrm{~m}\)

\section*{Co-ordinates of the points:}

\section*{O WITH B C D E F}
\(X 0\).
1.
\(2 / 2\)
0.0 .5
0.0 .4
\(y 0\).
0.
\(2 / 2\)
1. 0.0 .5
0.4

Z
0.0.0.0.0.0.0.

\section*{1.2}

Material properties
\(E=1 . P a\)
\(=0.3\)
\(=1 . \mathrm{kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

Simple support on the edge of the plate:
in all the points \(P\) such as \(C O p=R: U=v=W=0\)
FORCE_COQUE
Uniform pressure
\(P=1 \mathrm{~N} / \mathrm{m} 2\)
FORCE_COQUE

\title{
Charge distributed normal \\ F3 = \(1 \mathrm{~N} / \mathrm{m} 2\) \\ GRAVITY \\ \(G=10 \mathrm{~m} / \mathrm{s} 2\) according to \(Z\) from where \\ \(F Z=W P=1 \mathrm{~N} / \mathrm{m} 2\)
}

These three loadings lead to the same solution.

\section*{Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/03/008/A}

Code_Aster \({ }^{\circledR}\)
Version
7.2

\section*{Titrate:}

SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
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2
Reference solution
2.1

Method of calculation used for the reference solution
Two reference solutions are usable, for the calculation of the deformation, according to the theory of plate
used:
\(\cdot\) the theory of Coils-Kirchhoff, usually used for the plates known as "thin", that one will retain for modelings \(A, B\) and \(E\),
\(\cdot\) the theory of Reissner, including the effects of shearing for the plates known as "thick", that one will retain for modelings \(F, G\) and \(H\).

In any distant point of \(R\) of the center of the plate \((\mathbb{R})\), one has for the calculation of the arrow:
R
2
R
(23+)
3
=-
And
1-
1+
- with \(D=\)
2
64
2
D
R
\(R\)
1+
```

12(1-2)
16T21
and=0(Coil -
or

```
Kirchhoff)
(Reissner).
5 R 1

For the calculation of the moments the two theories lead to the same expressions: PR2

\section*{R 2 \\ PR2}
\(1+3 R 2\)
\(M r r(R)=\)
\((3+)\)
1
\(M(R)\)
\(=\)
\((3+) 1\)
16
-
\(R-\)
16
\(3+\)
\(R\)

\section*{Coil - Kirchhoff)}
or

For each node, one \(a:(M+M)=(M+M)=S m\)
for the points B and \(F\)
M
\(=M\)
\(x \boldsymbol{x}\)
\(y y\)
\(=(M+M\)
\(r r\)
)/2

\section*{2.2}

Results of reference
Arrow and moments at the points: \(O, A, B C, D E, F\).

\section*{2.3}

Uncertainty on the solution
Analytical solution.
2.4 References
bibliographical
[1]
TIMOSHENKO and WOINOWSKY-KRIEGER, Plates and hulls, Béranger Edition - (1961). Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)

\section*{Version}
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.101-D Page:
\(4 / 22\)

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Element of hull DKT (modeling of a quarter of plate)

\author{
C \\ B \\ E \\ F \\ O \\ With \\ D \\ \subsection*{3.1.1 Conditions \\ \\ limits}
}
in all the nodes of the arc \(A B C\)
: DX: 0. , DY: 0. , DZ: 0.
in all the nodes of segment] OA [
: DY: 0. , DRX: 0. , DRZ: 0.
in all the nodes of segment] OC [: DX: 0. , DRY: 0. , DRZ: 0 . with the node \(O\)
: DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.
Point 0
meshs: M30 M33
Not A
meshs: M76
Not B
meshs: M39 M40 M51
Not C
meshs: M1
Not D
meshs: M55 M56 M65
Not E
meshs: M8 M17 M18
Not F
meshs: M34 M35 M37 M41 M46 M47 M48

\section*{3.2}

Characteristics of the grid
A number of nodes: 50
A number of meshs and types: 76 TRIA3

\subsection*{3.3 Functionalities}
tested

\author{
Orders
}

\author{
AFFE_CARA_ELEM HULL
}

THICK

\author{
ANGL_REP \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO
}

\author{
FORCE_COQUE \\ NEAR \\ AFFE_MATERIAU ALL
}

\author{
"MECHANICAL" AFFE_MODELE "DKT" ALL \\ CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
}

\title{
"SIGM_ELNO_DEPL" \\ DEFI_MATERIAU ELAS
}

\author{
POST_RELEVE ACTION OPERATION \\ "EXTRACTION" \\ VALE_TEST \\ EPSI_TEST \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/03/008/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.101-D Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
0 (
W R) 695.6256
-687.95
-1.10
D (

W R) 489.727
-484.76
-1.01
E
W R) 489.727
-484.69
-1.03
F (
W R) 435.8974
-431.33
-1.05

Reference
Not
\(M\)
\(S m /\)
\(r r\)
\(M\)
2
\(O 0.20625\)
-0.20625
-0.20625
To 0.
-0.0875
-0.04375
B 0.
-0.0875
-0.04375
C 0.
-0.0875
-0.04375
D 0.15469
-0.17656
-0.1656
E 0.15469
-0.17656
-0.1656
F 0.14025
-0.16825
-0.15425

Aster
Not
    M
    Sm/
\(\boldsymbol{x x}\)
\(M y y\)
2
O 0.20377
-0.20382
-0.20379
To 0.00992
-0.08265
-0.04628
B 0.03827
-0.03771
-0.03799
C 0.08263
0.00990
-0.04626
D 0.15516
-0.17680
-0.16597
E 0.17677
-0.15509
-0.16593
F 0.15307
-0.15342
-0.15324
\% difference (\% difference on the absolute values)
Not
Mxx
\(M y y\)
Sm/2
Relative tolerance
\(O\)
\(-1.20-1.18-1.19\)
1.5
With
- -5.54
\(+5.79\)
-/6.
B

\section*{-12.5 -13.8 -13.1}
13./14.

C
\(-5.56-+5.73\)
6./-

\section*{D}
0.300 .14
+0.2
0.5

E
0.120 .26
+0.19
0.3

F
+9.15-8.81-0.66
10.

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HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

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SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.101-D Page:
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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

Element of hull DKT (modeling of a quarter of plate)

\begin{abstract}
\(C\)
B
\(\boldsymbol{E}\)
F
O
With
D

\subsection*{5.1.1 Conditions \\ limits}
\end{abstract}
in all the nodes of the arc \(A B C\)
: DX: 0. , DY: 0. , DZ: 0.
in all the nodes of segment] OA [
: DY: 0. , DRX: 0. , DRZ: 0.
in all the nodes of segment] OC [
: DX: 0. , DRY: 0. , DRZ: 0.
with the node \(O\)
: DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.

Not \(O\)
meshs: M1 m2
Not A
meshs: M248 M255
Not B
meshs: M292 M293 M296
Not C
meshs: M74 M75
Not D
meshs: M76 M108 M109
Not E
meshs: M34 M40 M41
Not F
meshs: M122 M123 M124 M148 M152 M153
5.2

Characteristics of the grid
A number of nodes: 170
A number of meshs and types: 296 TRIA3

\subsection*{5.3 Functionalities}
tested
Orders

AFFE_CARA_ELEM HULL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

FORCE_COQUE
NEAR
F3
GRAVITY

AFFE_MATERIAU ALL
"MECHANICAL" AFFE_MODELE "DKT" ALL
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
"SIGM_ELNO_DEPL"
DEFI_MATERIAU ELAS

\author{
POST_RELEVE ACTION OPERATION \\ "EXTRACTION" \\ VALE_TEST \\ EPSI_TEST \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.101-D Page:
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6
Results of modeling B

\subsection*{6.1 Values}
tested
Identification Reference Aster \%
difference
O(
W R) 695.6256
-694.97
-0.09
D (
W R) 489.727
-489.24
-0.1
E (
W R) 489.727
-489.31
-0.09
F
W R) 435.8974
-435.48
-0.09
Reference
Not
Mrr
```

M
Sm/2
O 0.20625
-0.20625
-0.20625
To 0.
-0.0875
-0.04375
B 0.
-0.0875
-0.04375
C}0
-0.0875
-0.04375
D 0.15469
-0.17656
-0.1656
E 0.15469
-0.17656
-0.1656
F 0.14025
-0.16825
-0.15425
Aster
Not
Mxx
Myy
Sm/2
O 0.20631
-0.20609
-0.20620
To 0.00238
-0.08459
-0.04348
B 0.4132
-0.04134
-0.04133
C 0.08460
0 . 0 0 2 3 8
-0.04349
D 0.15516

```

\title{
-0.17727
}
-0.16621
E 0.17739
-0.15523
-0.16631
F 0.15461
-0.15468
-0.15464
\% difference (\% difference on the absolute values)
\begin{tabular}{l} 
Not \\
Mxx \\
M yy \\
Sm/2 \\
Relative tolerance \\
O \\
\(0.03-0.7\) \\
0.02 \\
0.1 \\
With \\
--3.32 \\
-0.06 \\
\(-/ 3.5\) \\
B \\
\(-5.56-5.51-5.53\) \\
6. \\
\(C\) \\
\(-3.31-0.06\) \\
\(3.5 /-\) \\
\(D\) \\
\(+0.30+0.40+0.35\) \\
0.5 \\
\(E\) \\
\(+0.47+0.35+0.42\) \\
0.5 \\
F \\
\(+0.23+0.28+0.25\) \\
0.3 \\
Handbook of Validation \\
V3.03 booklet: Linear statics of the hulls and the plates \\
HT \(-66 / 03 / 008 /\) a \\
\hline
\end{tabular}

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

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\section*{7 Modeling}

C

\section*{7.1 \\ Characteristics of modeling}

Element of hull SHB8 (modeling of a quarter of plate)
C
1.0

B
0.5

E
0.0

O
D
With
0.0
0.5
1.0

\subsection*{7.1.1 Conditions \\ limits}
in all the nodes of the arc \(A B C\)
: DX: 0. , DY: 0. , DZ: 0 .
in all the nodes of face] OA [
: DY: 0. ,
in all the nodes of face] OC [
: DX: 0 .,
with the node \(O\)
: DX: 0. , DY: 0.,
The grid is built starting from the surface grid of modeling E, by thickening, with assistance of CREA_MAILLAGE/COQU_VOLU. One builds 2 layers of meshs HEXA8.

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 338
A number of meshs and types: 147 HEXA8

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\author{
FORCE_FACE \\ F2 \\ FORCE_INTERNE \\ F2 \\ GRAVITY
}

AFFE_MATERIAU ALL
"MECHANICAL" AFFE_MODELE SHB8
ALL
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"

\title{
CREA_MAILLAGE COQU_VOLU
}

\section*{CALC_CHAM_ELEM OPTION}

COOR_ELGA
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
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7.2

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Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
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\section*{8}

Results of modeling C

\subsection*{8.1 Values}
tested
Identification Reference
Aster \%
difference
O(
W R) 695.6256
-698.6
0.4

D
W R) 489.727
491.4

W R) 489.727
-491.4
0.3

F (
W R) 435.8974
-436.6
0.2

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
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Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key :
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\section*{9 Modeling \\ E \\ 9.1 \\ Characteristics of modeling}

Element of hull DKQ (modeling of a quarter of plate)

C
1.0

B
0.5

E
0.0
\(O\)

\section*{D}

With
0.0
0.5
1.0

\subsection*{9.1.1 Conditions limits}
in all the nodes of the arc \(A B C\)
: DX: 0. , DY: 0. , DZ: 0 .
in all the nodes of segment] OA [
: DY: 0. , DRX: 0. , DRZ: 0 .
in all the nodes of segment] OC [
: DX: 0. , DRY: 0. , DRZ: 0 .
with the node \(O\)
: DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0 .
Not \(O\)
meshs: M1
Not A
meshs: M147
Not B
meshs: M98 M111
Not C
meshs: M14
Not D
meshs: M85 M99
Not E
meshs: M7 M8
Not \(F\)
meshs: M91 M92 M105

\section*{9.2 \\ Characteristics of the grid}

A number of nodes: 169
A number of meshs and types: 147 QUAD4

\subsection*{9.3 Functionalities \\ tested}

Orders

\title{
AFFE_CARA_ELEM HULL
}

\section*{AFFE_CHAR_MECA DDL_IMPO}

GROUP_NO

\section*{FORCE_COQUE \\ NEAR}

F3
GRAVITY

AFFE_MATERIAU ALL
"MECHANICAL" AFFE_MODELE "DKT" ALL
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"

\title{
POST_RELEVE ACTION
}

OPERATION
"EXTRACTION"
VALE_TEST
EPSI_TEST
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

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10 Results of modeling \(E\)

\subsection*{10.1 Values}
tested
Identification Reference
Aster \%
difference
O(
W R) 695.6256
-695.01
-0.09
D (
W R) 489.727
-489.20
-0.11
E(
W R) 489.727
-489.20
-0.12
F
W R) 435.8974
\(-435.38\)
-0.09
Reference
Not
Mrr
\(M\)
Sm/2

\section*{O}
-0.20625-0.20625-0.20625
To 0. -0.0875
-0.04375

\section*{B 0.}
-0.0875
-0.04375
C 0.
-0.0875
-0.04375
D
-0.15469-0.17656-0.1656
```

E
-0.15469 -0.17656-0.1656

```

\section*{F}
```

-0.14025-0.16825-0.15425

```

Aster
Not
Mxx
\(M y y\)
Sm/2
\(O\)
-0.20639-0.20639-0.20639
To \(\mathbf{+ 0 . 0 0 0 3 6}\)
-0.087104
-0.04353

\section*{B}
-0.04330-0.04332-0.04331
C 0.087133
+0.00034
-0.04355
```

D
-0.15506-0.17790-0.16648
E
-0.17790 -0.15507-0.16648
F
-0.15380-0.15377-0.15378

```

\section*{\% difference (\% difference on the absolute values)}
```

Not
Mxx
Myy
Sm/2
Relative tolerance
O
+0.07 +0.07 +0.07
0 . 1
With

- -0.45
-0.49
-/0.5
B
-1.02+0.98+1.0
1.1
C
-0.42--0.46
0.5/-
D
+0.24+0.76 +0.5

1. 

E
+0.76 +0.24 +0.5
1.
F
-0.29-0.31-0.3
0.5

```

\section*{Handbook of Validation}
```

V3.03 booklet: Linear statics of the hulls and the plates HT-66/03/008/A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:

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\section*{11 Modeling \\ F}
11.1 Characteristics of modeling

Element of hull DST (modeling of a quarter of plate)
C
B
E
F
\(O\)
With
D

\subsection*{11.1.1 Limiting conditions}
in all the nodes of the arc \(A B C\)
: DX: 0. , DY: 0. , DZ: 0.
in all the nodes of segment] OA [
: DY: 0. , DRX: 0. , DRZ: 0.
in all the nodes of segment] OC [
: DX: 0. , DRY: 0. , DRZ: 0.
with the node \(O\)
: DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.
Not \(O\)
meshs: M1 m2

\title{
Not A
}

meshs: M248 M255

Not B
meshs: M3292 M293 M296
Not C
meshs: M74 M75
Not D
meshs: M76 M108 M109
Not E
meshs: M34 M40 M41
Not \(\boldsymbol{F}\)
meshs: M122 M123 M124 M148 M152 M153

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 170
A number of meshs and types: 296 TRIA3

\subsection*{11.3 Functionalities}
tested
Orders

AFFE_CARA_ELEM HULL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\section*{FORCE_COQUE}

NEAR
F3
GRAVITY

AFFE_MATERIAU ALL
"MECHANICAL" AFFE_MODELE "DST" ALL

\title{
CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
}

\author{
"SIGM_ELNO_DEPL" \\ DEFI_MATERIAU ELAS
}

\author{
POST_RELEVE ACTION \\ OPERATION \\ "EXTRACTION" \\ VALE_TEST \\ EPSI_TEST \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
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Author (S):
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:
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12 Results of modeling \(F\)

\subsection*{12.1 Values}
tested
Identification Reference
Reissner Aster \%
difference
0 (
W R) 703.40
-704.25
+0.12
D
W R) 495.56
-495.96
+0,08
E (
W R) 495.56
-496.04
+0.09
F (
W R) 441.18
-441.50
+0.07

Reference
Not
Mrr
M
Sm/2
O 0.20625
-0.20625
-0.20625
To 0.
-0.0875
-0.04375
B 0.
-0.0875
-0.04375
C 0.
-0.0875
-0.04375
D 0.15469
-0.17656
-0.1656
E 0.15469
-0.17656

\section*{F 0.14025 \\ -0.16825 \\ -0.15425}
\begin{tabular}{|c|}
\hline Aster \\
\hline Not \\
\hline Mxx \\
\hline \(M y y\) \\
\hline Sm/2 \\
\hline \(O\) \\
\hline -0.20705-0.20692-0.20698 \\
\hline With \\
\hline -0.01143-0.07992-0.04567 \\
\hline B \\
\hline -0.03976-0.03977-0.03977 \\
\hline C \\
\hline -0.07993-0.01143-0.4568 \\
\hline D \\
\hline -0.15597-0.17665-0.16631 \\
\hline \(E\) \\
\hline -0.17678-0.15606-0.16642 \\
\hline \(F\) \\
\hline -0.15445-0.15452-0.15448 \\
\hline
\end{tabular}
\% difference (\% difference on the absolute values)
Not
Mxx
\(M y y\)
Sm/2
Relative tolerance
O
\(-0.39-0.32+0.07\)
```

0.4
With
--8.66
-4.40
-/9.
B
-9.12 -9.09 -9.10
9.5
C
-8.64--4.41
9./-
D
+0.83 +0.05 +0.43
0.9
E
+0.13 +0.89 +0.49
0 . 9
F
+0.13 +0.18 +0.15
0 . 2
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```

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
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\section*{13 Modeling}

G
13.1 Characteristics of modeling

\section*{Element of hull DSQ (modeling of a quarter of plate)}
\(C\)
1.0
\(B\)
0.5
\(E\)
0.0
\(O\)
\(D\)
With
0.0
0.5
1.0

\subsection*{13.1.1 Limiting conditions}

\section*{in all the nodes of the arc \(A B C\)}

\section*{:}

DX: 0. , DY: 0. , DZ: 0.
in all the nodes of segment] OA [
:
DY: 0. , DRX: 0. , DRZ: 0.
in all the nodes of segment] \(O C\) [

\section*{.}

DX: 0. , DRY: 0. , DRZ: 0.
with the node O
:
DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.

\author{
Not \(O\) \\ meshs: M1
}

Not A
meshs: M147
Not B
meshs: M98 M111
Not C
meshs: M14
Not D
meshs: M85 M99
Not E
meshs: M7 M8

\author{
Not \(\boldsymbol{F}\) meshs: M91 M92 M105 \\ \subsection*{13.2 Characteristics of the grid} \\ A number of nodes: 169 \\ A number of meshs and types: 147 QUAD4 \\ 13.3 Functionalities \\ tested \\ Orders \\ AFFE_CARA_ELEM HULL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_COQUE \\ NEAR \\ F3 \\ GRAVITY
}

AFFE_MATERIAU ALL

\author{
"MECHANICAL" AFFE_MODELE "DST" ALL \\ CALC_CHAM_ELEM "EFGE_ELNO_DEPL" \\ "SIGM_ELNO_DEPL" \\ DEFI_MATERIAU ELAS
}

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HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
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14 Results of modeling \(G\)
14.1 Values
tested
Identification Reference
Reissner Aster \%
difference
O(
W R) 703.40
-702.61
-0.11
D (
W R) 495.56
-494.90
-0.13
E (
W R) 495.56
-494.90
-0.13

W R) 441.18
-440.51
-0.15

\section*{Reference}

\author{
Not
}

Mrr
M
Sm/2
O 0.20625
-0.20625
-0.20625
To 0.
-0.0875
-0.04375
B 0.
-0.0875
-0.04375
C 0.
-0.0875
- 0.04375

D 0.15469
-0.17656
-0.1656
E 0.15469
-0.17656
-0.1656
F 0.14025
-0.16825
-0.15425

Aster
Not
Mxx

\section*{Myy}

Sm/2
O 0.20469
-0.20469
-0.20469
To +0.00654
-0.08547
-0.39465
B 0.04063
-0.04063
-0.04063
C 0.08553
+0.00629
-0.39620
D 0.15560
-0.17723
-0.16641
E 0.17723
-0.15563
-0.16643

\section*{F 0.18352}
-0.18349
-0.18350
\% difference (\% difference on the absolute values)
Not
\(M x x\)
\(M y y\)
Sm/2
Relative tolerance
\(O\)
\(-0.76-0.76-0.76\)
1.
With
--2.32
-9.80

\section*{-/2.5}

\section*{B}
-7.11-7.12-7.12
7.5

C
-2.24--9.44
2.5/-

D
\(+0.59+0.37+0.49\)
0.6

E
\(+0.38+0.60+0.50\)
0.7

F
\(+19.0+18.9+19\).
19.

\section*{Handbook of Validation}

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J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
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15 Modeling
H

\subsection*{15.1 Characteristics of modeling}

Element of hull Q4 (modeling of a quarter of plate)
C
O

\subsection*{15.1.1 Limiting conditions}

\section*{in all the nodes of the arc \(A B C\)}
:

DX: 0. , DY: 0. , DZ: 0. in all the nodes of segment] OA [
:

DY: 0. , DRX: 0. , DRZ: 0.
in all the nodes of segment] OC [
:

DX: 0. , DRY: 0. , DRZ: 0.
with the node O
:

DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.

Not \(O\)
meshs: M1
Not A
meshs: M147
Not B
meshs: M98 M111
Not C
meshs: M14
Not D
meshs: M85 M99
Not E
meshs: M7 M8
Not \(\boldsymbol{F}\)
meshs: M91 M92 M105
15.2 Characteristics of the grid

A number of nodes: 169 A number of meshs and types: 147 QUAD4

\subsection*{15.3 Functionalities}
tested
Orders

AFFE_CARA_ELEM HULL

\author{
AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO
}

\section*{FORCE_COQUE}

NEAR
F3
GRAVITY

AFFE_MATERIAU ALL

\title{
"MECHANICAL"AFFE_MODELE "DKT" ALL \\ CALC_CHAM_ELEM "EFGE_ELNO_DEPL"
}

\author{
"SIGM_ELNO_DEPL" \\ DEFI_MATERIAU ELAS
}

\author{
POST_RELEVE ACTION \\ OPERATION \\ "EXTRACTION" \\ VALE_TEST
}

\author{
EPSI_TEST
}

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Titrate:
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Date:
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Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

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16 Results of modeling \(H\)
16.1 Values
tested
Identification Reference
Reissner Aster \%
difference
O (
W R)
-703.40-702.18-0.15
D (
W R)
-495.56-494.53-0.20
E (
W R)
-495.56-494.53-0.20
F (
W R)
-441.18-440.23-0.21
Reference
Not

To 0. -0.0875
-0.04375

\section*{B 0.}
-0.0875
-0.04375
C 0.
-0.0875
-0.04375
D 0.15469
-0.17656
-0.1656
E 0.15469
-0.17656
-0.1656
F 0.14025
-0.16825
-0.15425

Aster
Not
Mxx
M yy
Sm/2
O 0.20595
-0.20595
-0.20595

To 0.01204

\title{
-0.05236
}
-0.05234

\section*{C 0.09107}
-0.01207
-0.05157
D 0.15438
-0.17699
-0.16568
E 0.17699
-0.15437
-0.16568

\section*{F 0.15374}
-0.15374
-0.15374
\% difference (\% difference on the absolute values)
Not
\(M x x\)
\(M y y\)
Sm/2
Relative tolerance
\(O\)
\(-0.14-0.14-0.14\)
0.2
With
-+4.08
+17.8
-14.5
\(B\)
\(+19.6+19.7+19.7\)
20.
\(C\)
\(+4.08-+17.9\)
\(4.5 /-\)
4.5/-
```

D
-0.2 +0.24 +0.05
0.25
E
+0.24-0.2 +0.05
0 . 2 5
F
-0.33-0.33-0.33
0 . 3 5

```

\section*{Handbook of Validation}
```

V3.03 booklet: Linear statics of the hulls and the plates HT-66/03/008/A

```

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Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

\section*{17 Modeling}

I

\subsection*{17.1 Characteristics of modeling}

Modeling: Element of COQUE_3D MEC3QU9H
\(C\)
\(C\)
\(E\)
\(O\)
\(D\)
With

\subsection*{17.1.1 Limiting conditions}
in all the nodes of arc DX: 0. , DY: 0. , DZ: 0.
ABC
DRX: 0. , DRY: 0. , DRZ: 0.
segment] OA]
DY: 0. , DRX: 0. , DRZ: 0.
segment] OC]
DX: 0. , DRY: 0. , DRZ: 0.
with the node \(O\)
DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.
Names of the nodes:
Point 0
meshs: M1

\title{
Not A
}
meshs: M21
Not B
meshs: M25
Not C
meshs: M5
Not D
meshs: M11
Not E
meshs: M3
17.2 Characteristics of the grid

A number of nodes: 96
A number of meshs and types: 25 QUAD9
17.3 Functionalities
tested

Orders

AFFE_CARA_ELEM HULL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\section*{FORCE_COQUE \\ NEAR}

F3

\section*{GRAVITY}

\title{
CALC_CHAM_ELEM "EFFO_ELNO_DEPL"
}
"SIGM_ELNO_DEPL"

DEFI_MATERIAU ELAS

\title{
POST_RELEVE ACTION \\ OPERATION "EXTRACTION"
}

VALE_TEST

EPSI_TEST

\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/03/008/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
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18 Results of modeling I
18.1 Values
tested
```

Not Reference Aster \%
difference
REISSNER
O
-703.40-703.41 1.42 10-3
With
0. eps -
B
0. eps -
C W
(R)
0. eps -
D
-495.56-496.57 2. 10-3
E
-495.56-496.57 2. 10-3

```

\section*{Not Reference}

Aster
Mrr
M
Sm/2
\(M \boldsymbol{x x}\)
\(M y y\)
Sm/2
O 0.20625
-0.20625
-0.20625
-0.20842
-0.20842
-0.20842
To 0. -0.0875
-0.04375
-0.08846
-0.00159
-0.04502
C 0.
-0.0875
-0.04375

\author{
-0.00067 \\ -0.08938 \\ -0.04502 \\ D 0.15469 \\ -0.17656 \\ -0.1656 \\ -0.17743 \\ -0.15472 \\ -0.16607 \\ E 0.15469 \\ -0.17656 \\ -0.1656 \\ -0.15459 \\ -0.17757 \\ -0.16608
}

Not
\% difference
Relative tolerance
M \(\boldsymbol{x x}\)
\(M y y\)
Sm/2
O 1.05
1.05
1.053

At 1.09
2.93

C
2.1
2.95

D 0.49
0.02
0.281

E 0.06
0.57
0.281
- with eps 1014 and \% difference on the absolute values

Note:

The test of the values is carried out automatically using the functionalities offered by procedure POST_RELEVE:
- extraction on the nodes corresponding to the points observed of the average values of components Mxx and Myy; these values are extracted from field "EFGE_ELNO_DEPL", and average is calculated for all the liquid assets on the meshs which contain it node observed, - calculation of the variation compared to the value of reference provided by observing the rules of correspondence between Mxx, Myy and Mrr, M given page 3.

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key
:
V3.03.101-D Page:
20/22

\section*{19 Modeling \\ \(J\)}
19.1 Characteristics of modeling

Modeling: Element of COQUE_3D MEC3TR7H
C
B
E
O
D
With
19.1.1 Limiting conditions
in all the nodes of arc \(D X: 0 ., D Y: 0 ., D Z: 0\).
\(A B C\)
DRX: 0., DRY: 0. , DRZ: 0.
segment] OA]
DY: 0. , DRX: 0. , DRZ: 0.
segment] OC]
DX: 0. , DRY: 0. , DRZ: 0.
with the node \(O\)
DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.
Names of the nodes:
Point 0
meshs: M1 and m2
Not A
meshs: M41
Not B
meshs: M49 and M50
Not C
meshs: M10
Not D
meshs: M21
Not E
meshs: M6
19.2 Characteristics of the grid

A number of nodes: 121
A number of meshs and types: 50 TRIA7
19.3 Functionalities
tested
Orders

AFFE_CARA_ELEM HULL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\section*{FORCE_COQUE}

NEAR

F3

\section*{GRAVITY}

AFFE_MATERIAU ALL
"MECHANICAL"AFFE_MODELE "COQUE_3D" ALL
CALC_CHAM_ELEM "EFFO_ELNO_DEPL"
"SIGM_ELNO_DEPL"

DEFI_MATERIAU ELAS

\author{
POST_RELEVE ACTION OPERATION "EXTRACTION" \\ VALE_TEST
}

EPSI_TEST

\author{
Handbook of Validation
}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

\section*{Titrate:}

SSLS101 Plates circular posed subjected to a uniform pressure

\section*{Date: \\ 05/01/04}

Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key :
V3.03.101-D Page:
21/22

20 Results of modeling \(J\)

\subsection*{20.1 Values}
tested
Not Reference Aster \% difference REISSNER
O
-703.40-703.19-0.03
With
0. eps -

B
0. eps -

C W
(R)
0. eps -

D
-495.56-495.22-0.07
E
-495.56-495.22-0.07

\section*{Not Reference}

Aster
Mrr
M
Sm/2
\(M \boldsymbol{x x}\)
\(M y y\)
Sm/2
O 0.20625

\author{
-0.20625 \\ -0.20625 \\ -0.20860 \\ -0.20860 \\ -0.20860 \\ To 0. \\ -0.0875 \\ -0.04375 \\ -0.08619 \\ -0.00154 \\ -0.04386 \\ \section*{C 0.} \\ -0.0875 \\ -0.04375 \\ -0.00134 \\ -0.08639 \\ -0.04386 \\ D 0.15469 \\ -0.17656 \\ -0.1656 \\ -0.17617 \\ -0.15409 \\ -0.16513 \\ E 0.15469 \\ -0.17656 \\ -0.1656 \\ -0.15407 \\ -0.17619 \\ -0.16513
}

\author{
Not \\ \% difference \\ Relative tolerance
}
\(M_{x x}^{x}\)
\(M y y\)
Sm/2
O 1.14
1.14
1.142

At 1.49
0.252

C
-1.27
0.252

D 0.22
-0.39
-0.28 1 .
E 0.40
-0.21
-0.28 1 .
- with eps 1014 and \% difference on the absolute values

Note:

The test of the values is carried out automatically using the functionalities offered by procedure POST_RELEVE:
- extraction on the nodes corresponding to the points observed of the average values of components Mxx and Myy; these values are extracted from field "EFGE_ELNO_DEPL", and average is calculated for all the liquid assets on the meshs which contain it node observed, - calculation of the variation compared to the value of reference provided by observing the rules of correspondence between Mxx, Myy and Mrr, M given page 3.

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

Titrate:
SSLS101 Plates circular posed subjected to a uniform pressure
Date:
05/01/04
Author (S):
J.M. PROIX, P. MASSIN, D. BUI, A. LAULUSA Key

\section*{.}

V3.03.101-D Page:
22/22

\section*{21 Summary of the results}
\% of the differences compared to the reference solutions
\begin{tabular}{l} 
DKT \\
DKQ \\
DST \\
DSQ \\
Q4G \\
WITH B \\
\(E\) \\
\(F\) \\
\(G\) \\
\(H\) \\
Coil-Kirchhoff \\
Coil-Kirchhoff \\
Coil-Kirchhoff \\
Reissner \\
Reissner \\
Reissner \\
50 \\
nodes \\
170 nodes \\
169 nodes \\
170 nodes \\
169 nodes \\
169 nodes \\
76 TRIA3 \\
296 TRIA3 \\
147 QUAD4 \\
296 TRIA3 \\
147 QUAD4 147 QUAD4 \\
O ( \\
W R) \\
-1.10 -0.09 - 0.09 \\
+0.12 \\
-0.11 \\
-0.15 \\
\(D\) ( \\
W R) \\
-1.01 -0.1 - 0.11 \\
+0.08 \\
-0.13 \\
\\
\hline
\end{tabular}
-0.13

\section*{MEC3QU9H MEC3TR7H}

\section*{SHB8}

\section*{I J C}

\section*{96}
nodes
121 nodes
338 nodes

25 QUAD9
50 TRIA7
147 HEXA8
O (
W R) 1.42
10-3-0.03 0.4
D (
W R) 2.
10-3-0.07 0.3
E (
W R) 2.
10-3-0.07 0.3
F (
W R ) -
```

0 . 2

```
DKT
DKQ
DST
DSQ
Q4G
WITH B
E
F
G
H
Coil-Kirchhoff
Coil-Kirchhoff
Coil-Kirchhoff
Reissner
Reissner
Reissner
50
nodes
170 nodes
169 nodes
170 nodes
169 nodes
169 nodes
76 TRIA3
296 TRIA3
147 QUAD4
296 TRIA3
147 QUAD4 147 QUAD4
O Sm/2
-1.19
+0.02
+0.07
+0.07
-0.76
-0.14

\section*{In Sm/2}
\(+5.79\)
-0.06
-0.49
-4.40
-9.80
+17.80
B Sm/2
-13.100
-5.53
+1.00
-9.10
-7.12
+19.70
C Sm/2
\(+5.73\)
-0.06
-0.46
-4.41
-9.44
+17.90
D Sm/2
+0.20
+0.35
\(+0.50\)
+0.43
+0.49
+0.05
E Sm/2
+0.19
+0.42
+0.50
+0.49
+0.50
+0.05
F Sm/2
-0.66
\(+0.25\)
-0.30
+0.15
\(+19.00\)
-0.33

\section*{MEC3QU9H MEC3TR7H}

\section*{I J}

96
nodes
121 nodes

25 QUAD9
50 TRIA7
O Sm/2
1.05
1.14

In Sm/2
2.9
0.25

\section*{B Sm/2}

\section*{C Sm/2}
2.9
0.25

\section*{D Sm/2}

\section*{F Sm/2}

Concerning the efforts:
\(\cdot\) the value on the supported edge is unacceptable for DSQ and Q4 and to a lesser extent for DST,
- for DSQ, one notes in more one important error at the point \(F\).

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
1/10
Organization (S): EDF/IMA/MMN, SAMTECH
Handbook of Validation

V3.03 booklet: Linear statics of the hulls and the plates
Document: V3.03.104
SSLS104-Cylindrical hull pinch
with diaphragm
Summary:
One treats in linear elasticity the case of a cylinder formed by two circular funds at the two ends and gripped
with semi-length.
This makes it possible to treat the modes of deformation inextensionnels and a complex membrane

\section*{behavior}
had with the diaphragms.
The value tested is the arrow at the point of application of the force.
Three modelings: DKT, COQUE_3D QUAD9 and COQUE_3D TRIA7.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates

\section*{HI-75/98/040 - Ind A}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):

\section*{P. MASSIN, A. LAULUSA}

Key:
V3.03.104-C Page:
2/10
1
Problem of reference
1.1 Geometry
\(y\)
F
F
B
B
C
With
C
D
With
\(X\)

Ray
\(R=300\)

\section*{Thickness}
\(T=3\)
Co-ordinates of the points:
With
B
C
D
\(X\)
300.
0.
0.
300.
\(y\)

\section*{0.}
300.
300.
0.

Z
0.
0.
300.
300.
1.2

Material properties
\(E=3.106 \mathrm{~Pa}\)
\(=0.3\)
\(A_{-} C I S=0.8333\)
1.3

Boundary conditions and loadings
Rigid diaphragm at each end:
\(U=v=0, Z=0\)
Specific force out of \(C\) :
```

$F=1 . N R$

```

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
3/10
2
Reference solution
2.1

Method of calculation used for the reference solution
The parameters of the dealt with problem and the results of reference are explicitly given in publication quoted below.
2.2

Results of reference
Displacement of the point C following \(Y\).

\subsection*{2.3 References}
bibliographical
[1]
Thomas J.R HUGHES, Ted BELYTSCHKO. Race notes for Recent advances in nonlinear finite element analysis. Volume III -p 238 and 239 (1990).

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):

\section*{P. MASSIN, A. LAULUSA}

Key:
V3.03.104-C Page:
4/10
3 Modeling
With
3.1

Characteristics of modeling
Element of hull DKT
\(y\)
B
C
With
\(X\)
Z
D
Modeling of a eighth of plate
Cutting:
10 on \(A D\) and \(B C\)
16 on \(A B\) and cd. 364 meshs TRIA3
Limiting conditions:
in all the nodes of:
DDL_IMPO:
\(\operatorname{arc}(A B)\)
(GROUP_NO: AB DX: 0. , DY: 0. , DRZ: 0. )
\(\operatorname{arc}\) (CD)
(GROUP_NO: CD DZ: 0. , DRX: 0. , DRY: 0. )
segment) BC (
(GROUP_NO: BCsansBC DX: 0. , DRY: 0. , DRZ: 0. )
segment) AD (
(GROUP_NO: ADsansAD DY: 0. , DRX: 0. , DRZ: 0. )
out of C
(GROUP_NO: C DX: 0., DRZ: 0. )
in \(D\)
(GROUP_NO: D DY: 0. , DRZ: 0. )
Loading:
with the node \(C\) :
(GROUP_NO: C FY: -0.25)
Names of the nodes:
Not A
N04
Not B
N02
Not CNot D
N03
3.2
Characteristics of the grid
A number of nodes: 209
A number of meshs and types: 364 TRIA3
3.3 Functionalities
tested
Orders
Keys
AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
AFFE_MODELE
"MECHANICAL"
"DKT"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A
Code_Aster \({ }^{\circledR}\)
Version
4.0
Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
5/10
4
Results of modeling \(A\)
4.1 Values
tested
Identification
Reference
Aster
\% difference
tolerance
Not C
1.8248105
1.8776105
2.89
3.102
displacement \(v\)
With 1.366 nodes: 1.8511 F out of \(C\).
4.2 Parameters
of execution
Version: 4.00.02
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use:

\section*{5.5 seconds}

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
6/10
5 Modeling
B
5.1

Characteristics of modeling
Element of hull 3D MEC3QU9H
y
B
C
With
X
Z
D
Modeling of a eighth of plate
Cutting:
4 on AD and BC
8 on AB and cd. 32 meshs QUAD9
Limiting conditions:
in all the nodes of:
DDL_IMPO:
\(\operatorname{arc}(\mathrm{AB})\)
(GROUP_NO: AB DX: 0. , DY: 0. , DRZ: 0. )
arc (CD)
(GROUP_NO: CD DZ: 0. , DRX: 0. , DRY: 0. )
segment) BC (
(GROUP_NO: BCsansBC DX: 0. , DRY: 0. , DRZ: 0. )
segment) AD (
(GROUP_NO: ADsansAD DY: 0. , DRX: 0. , DRZ: 0. )
out of C
(GROUP_NO: C DX: 0. , DRZ: 0. )
in D
(GROUP_NO: D DY: 0., DRZ: 0. )

Loading:
with the node C :
(GROUP_NO: C FY: -0.25)
Names of the nodes:
Not A
N01
Not B
N02
Not C
N03
Not D
N04
5.2

Characteristics of the grid
A number of nodes: 121
A number of meshs and types: 32 QUAD9
5.3 Functionalities
tested

\section*{Orders}

\section*{Keys}

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"COQUE_3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):

\section*{P. MASSIN, A. LAULUSA}

Key:
V3.03.104-C Page:
7/10
6
Results of modeling B
6.1 Values
tested

\section*{Identification}

\section*{Reference}

Aster
\% difference

\section*{tolerance}

Not C
1.8248105
1.7806105
2.42
3.102
displacement v

\subsection*{6.2 Remarks}

For a grid of 60 meshs QUAD9 and 213 nodes (corresponding to cutting 6 on AD and BC and 10 on AB and cd. .), displacement v at the point C is worth 1.8011105.

\subsection*{6.3 Parameters}
of execution
Version: 4.00.14
Machine: CRAY C90
System:

\section*{UNICOS 8.0}

Obstruction memory:
16 megawords
Time CPU To use:
6.52 seconds

Handbook of Validation

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
8/10
7 Modeling
C
7.1

Characteristics of modeling
Element of hull MEC3TR7H
y
B
C
With
X
Z
D
Modeling of a eighth of plate
Cutting:
10 on AD and BC
18 on AB and cd. 360 meshs TRIA7
Limiting conditions:
in all the nodes of:
DDL_IMPO:
\(\operatorname{arc}(\mathrm{AB})\)
(GROUP_NO: AB DX: 0. , DY: 0. , DRZ: 0. )
arc (CD)
(GROUP_NO: CD DZ: 0. , DRX: 0. , DRY: 0. )
segment) BC (
(GROUP_NO: BCsansBC DX: 0. , DRY: 0. , DRZ: 0. )
segment) AD (
(GROUP_NO: ADsansAD DY: 0. , DRX: 0. , DRZ: 0. )
out of C
(GROUP_NO: C DX: 0. , DRZ: 0. )
in D
(GROUP_NO: D DY: 0. , DRZ: 0. )
Loading:
with the node C :
(GROUP_NO: C FY: -0.25)
Names of the nodes:
Not A
N01
Not B
N02
Not C
N03
Not D
N04
7.2

Characteristics of the grid
A number of nodes: 777
A number of meshs and types: 360 TRIA7
7.3 Functionalities
tested
Orders

\section*{Keys}

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_NODALE
GROUP_NO
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"COQUE_3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
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8
Results of modeling C
8.1 Values
tested

\section*{Identification}

\section*{Reference}

Aster
\% difference
tolerance
Not C
1.8248105
1.7424105
4.51
5. 102
displacement v

\subsection*{8.2 Remarks}

For a grid with 500 meshs TRIA7 and 1071 nodes (cutting 10 on AD and BC, 25 on AB and
Cd .), one obtains a displacement v at the point C of 1.7723 105. The relative error on displacement v in C is then \(2.88 \%\). The results with this element for light grids is thus not very good and improves relatively little with an increase of the number of meshs.

\subsection*{8.3 Parameters}

\author{
of execution
}

Version: 4.00.14
Machine: CRAY C90
System:
UNICOS 8.0

Obstruction memory:
16 megawords
Time CPU To use:
89.25 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Cylindrical SSLS104 Hull pinch with diaphgrame
Date:
01/12/98
Author (S):
P. MASSIN, A. LAULUSA

Key:
V3.03.104-C Page:
10/10
9

\section*{Summary of the results}

With regard to the elements:

\section*{DKT:}

The result is better with a finer grid (1366 nodes) which leads to an error < \(1.5 \%\).
MEC3QU9H:
The result is acceptable with relatively few elements (compared with DKT). While increasing appreciably the number of elements (60 instead of 32), the error is < \(1.3 \%\).

\section*{MEC3TR7H:}

Result little satisfying even with a fine grid leading to a great total number of nodes for MEC3TR7H ( 777 for MEC3TR7H to be compared with 209 for DKT and 121 for MEC3QU9H). To arrive at an error lower than \(2.9 \%\), that requires a very great number of nodes (1071). It seems recognized that this element is worse than MEC3QU9H.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/98/040 - Ind A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

\section*{Titrate:}

SSLS105 - Doubly gripped hemisphere

Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, Key S. BAGUET
:
V3.03.105-D Page:
1/8
Organization (S): EDF-R \& D /AMA, INSA LYON

\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ Document: V3.03.105
}

\section*{SSLS105 - Doubly gripped hemisphere}

\section*{Summary:}

One treats the case of the hemisphere doubly gripped in linear elasticity, which makes it possible to evaluate the quality of
plane facets for the representation of a deep hull.
The values tested are the arrows at the points of application of the forces.

One has 3 modelings:

\author{
A: elements DKT \\ B: elements of COQUE_3D in QUAD9
}

C: elements SHB8

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS105 - Doubly gripped hemisphere

Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, Key S. BAGUET
:
V3.03.105-D Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{1.2 \\ Material properties}
\[
E=6.825107 \mathrm{~Pa},=0.3
\]

\section*{1.3}

Boundary conditions and loadings
On a quarter of the hemisphere:
Not C
no displacement in \(Z\)
Side AC
symmetry compared to the xz plan
Side BC
symmetry compared to the yz plan

Side \(A B\)
free
Specific force in a: \(F=2 . K N\)
Specific force in \(b: F=+2 . K N\)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS105 - Doubly gripped hemisphere

Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, Key S. BAGUET

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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is that given in the card "Test No LE3" of the tests of reference published by NAFEMS [bib1].

\section*{2.2}

\section*{Results of reference}

Displacement of point \(A\) following \(X\).

\subsection*{2.3 References}

\section*{bibliographical}
A. Morris. Dynamics Working Group - College of Aeronautics, Cranfield, the U.K. Free vibrations benchmarks. NAFEMS - Test No LE3 - (1986).

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
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Titrate:
SSLS105 - Doubly gripped hemisphere

Date:
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Author (S):
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

Element of hull DKT
Modeling of a quarter of the hemisphere in TRIA3.
Names of the nodes:

Not \(A\)
N03
Not B
N02
Not C

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 734
A number of meshs and types: 1373 TRIA3

\author{
3.3 Functionalities \\ tested \\ Orders
}

AFFE_CARA_ELEM HULL
ALL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\author{
FORCE_NODALE \\ GROUP_NO \\ "MECHANICAL" AFFE_MODELE "DKT" \\ ALL \\ DEFI_MATERIAU ELAS
}

4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Reference Aster \% difference
Not A

Not B
\(+0.185+0.1839-0.59\)
displacement \(v\)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
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\section*{Code_Aster \({ }^{\circledR}\)}

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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

Element of hull COQUE_3D MEC3QU9H

C
With
B
Modeling of a quarter of the hemisphere in QUAD9
Names of the nodes:

Not \(A\)
N01

\author{
Not B \\ N021 \\ Not C \\ N041 \\ \section*{5.2} \\ \title{
Characteristics of the grid
} \\ A number of nodes: 256 \\ A number of meshs and types: 75 QUAD9 \\ \subsection*{5.3 Functionalities} \\ tested \\ Orders
}

AFFE_CARA_ELEM HULL ALL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\author{
FORCE_NODALE \\ GROUP_NO \\ AFFE_MATERIAU ALL
}
"MECHANICAL" AFFE_MODELE "COQUE_3D"
ALL
DEFI_MATERIAU ELAS

\section*{6}

Results of modeling \(\boldsymbol{B}\)

\subsection*{6.1 Values}

\title{
Identification Reference Aster \%
} difference
Not A
-0.185-0.1844
-0.32
displacement \(U\)
Not B
\(+0.185+0.1844\)
-0.32
displacement v
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS105 - Doubly gripped hemisphere

Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, Key S. BAGUET

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\section*{7 Modeling \\ C \\ 7.1 \\ Characteristics of modeling}

Element of hull COQUE_MASSIF SHB8
C2
Cl
Al
B1
A2
B2

Modeling of a quarter of the hemisphere in SHB8

\section*{7.2}

Characteristics of the grid
Case 1:
A number of nodes: 662
A number of meshs and types: 300 SHB8
Names of the nodes:

Not Al

Not A2
N42
Not B1
N01
Not B2
N02

\section*{Not C1}

N662
Not C2
N658

\section*{Case 2:}

A number of nodes: 2522
A number of meshs and types: 1200 SHB8
Names of the nodes:

Not A1
N1239
Not A2
N1241
Not B1
N301
Not B2
N303
Not C1
N602
Not C2
N604

\section*{Case 3:}

A number of nodes: 5582
A number of meshs and types: 2700 SHB8
Names of the nodes:

\title{
Not Al
}

N329
Not A2
N331
Not B1
N3439
Not B2
N3441

Not Cl
N4760
Not C2
N4762

\subsection*{7.3 Functionalities}
tested
Orders

MODI_MAILLAGE ORIE_SHB8
GROUP_MA ALL
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FORCE_NODALE
GROUP_NO
AFFE_MATERIAU ALL

\section*{"MECHANICAL" AFFE_MODELE SHB8}

ALL
DEFI_MATERIAU ELAS

\section*{Handbook of Validation}

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HT-66/03/008/A

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7.2

\section*{Titrate:}

SSLS105 - Doubly gripped hemisphere

Date:
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:

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\section*{8}

Results of modeling \(C\)

\subsection*{8.1 Values}
tested

\section*{Identification Reference Aster \% \\ difference}

Not \(A\)
-0.185-0.1490
-19.44
Cas" 1
displacement \(U\)
Not
B
\(+0.185+0.1490\)
-19.44
displacement \(v\)
Not A
-0.185-0.1822
-1.49
Case 2
displacement \(U\)
Not
B
\(+0.185+0.1822\)
-1.49
displacement \(v\)

Not A
-0.185-0.1845
-0.24
Case 3
displacement \(U\)
Not
B
\(+0.185+0.1845\)
-0.24
displacement \(v\)

\subsection*{8.2 Notice}

For this modeling, convergence is relatively slower than for the MEC3QU9H, it is necessary in effect 1200 elements to obtain a relative error of about \(1 \%\). But time CPU is not
crippling (12s). This is undoubtedly due to the choice of the law of behavior specific to the element SHB8: law of plane constraints, and nonnull rigidity in the direction of the normal to the element. If one chose a law of elasticity in plane constraints, without modification, one one would obtain then correct solution ( \(0,2 \%\) of variation) as of the 1st case (300 elements). But this law would be then unsuited
for other tests, such that of the sphere under pressure (SSLS123).

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V3.03 booklet: Linear statics of the plates and hulls
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\section*{Code_Aster \({ }^{\circledR}\)}

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7.2

Titrate:
SSLS105 - Doubly gripped hemisphere

Date:
03/1 1/03
Author (S):
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\section*{9 \\ Summary of the results}

Severe test which requires a fine grid, in particular for element DKT. Results with the element
MEC3TR7H were not retained as test because it is necessary to have many elements (1801) and thus a time of convergence much longer to obtain correct values by report/ratio with other modelings (> 500 S for a relative error of about \(4 \%\) ).
For element SHB8, convergence is relatively slower than for the MEC3QU9H, it is necessary in effect 1200 elements to obtain a relative error of about \(1 \%\). But time CPU is not crippling (12s).

Results in conformity with the reference solution.
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS107 Panel subjected to its own weight
Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA

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Organization (S): EDF/IMA/MMN, SAMTECH

\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ Document: V3.03.107
}

\section*{SSLS107-Subjected cylindrical panel \\ with its own weight}

\section*{Summary:}

This test makes it possible to validate two finite elements of thick hull in linear elasticity. Modeling \(A\) tests it quadrangle, modeling B tests the triangle associated with the formulation. This problem of cylindrical panel under actual weight is a traditional test of hull.

The results of reference are analytical solutions.
One will note the good results obtained with the quadrangle and the results much worse obtained with triangle.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS107 Panel subjected to its own weight
Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA

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Problem of reference

\subsection*{1.1 Geometry}
```

Z
D
With
C
Free
X
y
B
L
R
Length
L=6.m
Ray
R=3.m
Thickness
T=0.03 m
Angular section = 40

```

\section*{Co-ordinates of the points:}

WITH B CD
\(X\)
3. \(\cos 40^{\circ}\)
3. \(\cos 40^{\circ}\)
3.
3.
\(y\)
3. \(\sin 40^{\circ}\)
3. \(\sin 40^{\circ}\)
0.
0.

Z 3.
0.
0.
3.
1.2

Material properties
\(E=3.1010 \mathrm{~Pa}\)
\(=0\).
\(=2.0833104 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3}

Boundary conditions and loadings
Rigid diaphragm at each end: \(U=v=0, Z=0\).
Loading 1: Force due to gravity \(G=10 \mathrm{~m} / \mathrm{s} 2\)
Loading 2: Force hull charges distributed vertical \(F x=6250 . N\)
Two loadings leading to the same solution are tested.
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS107 Panel subjected to its own weight

\section*{Date:}

05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA
:
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The parameters of the problem and the results of reference (analytical solutions) are given by BATOZ and DHATT [bib1].

\section*{2.2}

Results of reference
Displacement of the point B following \(X\)
Displacement of the point \(C\) following \(X\).

\subsection*{2.3 References \\ bibliographical \\ [1] \\ BATOZ J.L., DHATT G.: Modeling of the structures by finite elements. Volume 3 hulls, p445-448 (1992). \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HI-75/01/010/A}

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLS107 Panel subjected to its own weight
Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA

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3 Modeling
With

\section*{3.1 \\ Characteristics of modeling \\ Element of COQUE_3D MEC3QU9H}
\(Z\)
\(D\)
\(C\)
With
\(X\)
\(y\)
\(B\)

\section*{Modeling of a quarter of cylinder}

\section*{Cutting:}

6 on \(A B\) and \(c d\).
6 on AD and BC 36 meshs QUAD9
Limiting conditions:
in all the nodes of:
DDL_IMPO:
\(\operatorname{arc}(A D)\)
(GROUP_NO: AD DX: 0. , DY: 0. , DRZ: 0. )
segment] CD [
(GROUP_NO: CDsansCD DY: 0. , DRY: 0. , DRZ: 0. )
\(\operatorname{arc}(B C)\)
(GROUP_NO: BC DX: 0. , DRX: 0. , DRY: 0. )
out of C
(GROUP_NO: C DY: 0., DRZ: 0. )

\section*{Loading:}

FORCE_COQUE:
(FX: -6250.)
GRAVITY:
(10.-1. 0. 0.)

Names of the nodes:
```

Not A
N03
Not B
N02
Not C
N01
Not D
N04

```

\section*{3.2}
```

Characteristics of the grid
A number of nodes: 169
A number of meshs and types: 36 QUAD9

```

\subsection*{3.3 Functionalities}
tested
Orders

\author{
Keys \\ AFFE_MODELE AFFE \\ MODELING \\ : \\ "COQUE_3D" \\ [U4.22.01] \\ AFFE_CARA_ELEM HULL \\ ALL \\ [U4.24.01] \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_COQUE \\ FX \\ GRAVITY \\ DEFI_MATERIAU \\ ELAS \\ E, NAKED, RHO
}

\author{
[U4.23.01] \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
4.0

\section*{Titrate:}

Cylindrical SSLS107 Panel subjected to its own weight
Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA
:
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

Identification Reference Aster \% difference
Not B
-3.61 10-2 -3.619
10-2
-0.25
displacement DX
Not C
5.44 10-3 5.427

10-3
-0.31
displacement DX

\subsection*{4.2 Parameters}
of execution
Version: 4.00.14

\section*{Machine: CRAY}

\section*{System:}

UNICOS 8.0
Obstruction memory:
16 megawords
Time CPU To use: 7.34 seconds

\section*{Handbook of Validation}

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HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Cylindrical SSLSI07 Panel subjected to its own weight
Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA
:
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\section*{5 Modeling}

B

\section*{5.1}

Characteristics of modeling
Element of hull 3D MEC3TR7H
Z
D
```

C
With
X
y
B

```

\section*{Modeling of a quarter of cylinder}

\section*{Cutting:}

12 on \(A B\) and \(c d\).
12 on AD and BC 288 meshs TRIA7

\section*{Limiting conditions:}
in all the nodes of:
DDL_IMPO:
\(\operatorname{arc}(A D)\)
(GROUP_NO: AD DX: 0. , DY: 0., DRZ: 0.)
segment) CD (
(GROUP_NO: CDsansCD DY: 0. , DRY: 0. , DRZ: 0. )
\(\operatorname{arc}\) (BC)
(GROUP_NO: BC DX: 0. , DRX: 0. , DRY: 0. )
out of C
(GROUP_NO: C DY: 0., DRZ: 0. )
The grid is of directed type:
Loading:

FORCE_COQUE:
(FX: -6250.)
GRAVITY:
(10.-1. 0. 0.)

Names of the nodes:
Not A
N03
Not B
N02
Not C
N01

\section*{5.2}

Characteristics of the grid
A number of nodes: 913
A number of meshs and types: 288 TRIA7

\subsection*{5.3 Functionalities}
tested

\author{
Orders
}

\author{
Keys \\ AFFE_MODELE AFFE \\ MODELING \\ :
"COQ
[U4.2
AFF
ALL
}
[U4.24.01]
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_COQUE
FX
GRAVITY
DEFI_MATERIAU
ELAS
E, NAKED, RHO
[U4.23.01]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)

\title{
Version \\ 4.0
}

\section*{Titrate:}

Cylindrical SSLS107 Panel subjected to its own weight

Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA
:
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\section*{6 \\ Results of modeling \(B\)}

\subsection*{6.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
Not B
-3.61 10-2 -3.603
10-2
-0.19
displacement DX
Not C
5.41 10-3 5.396

10-3
-0.26
displacement DX

\subsection*{6.2 Parameters}
of execution
Version: 4.00.14

\author{
Machine: CRAY \\ \section*{System:} \\ UNICOS 8.0 \\ Obstruction memory: \\ 16 megawords \\ Time CPU To use: 54.54 seconds \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the hulls and the plates HI-75/01/010/A
}

Titrate:
Cylindrical SSLS107 Panel subjected to its own weight
Date:
05/10/98
Author (S):
P. Key MASSIN, A. LAULUSA

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\section*{7 \\ Summary of the results}

Element MEC3QU9H makes it possible to obtain a good solution with a coarse grid, while element MEC3TR7H requires a very fine grid to reach a satisfactory precision.

It is noted that the reference solution is the analytical solution obtained starting from the theory of "deep" hulls. The 2 elements of hull converge towards this solution and not towards the theory "not very deep" hulls.

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

\section*{Titrate:}

SSLS108-Helicoid hull under concentrated loadings
Date:
03/1 1/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls

\section*{Document: V3.03.108}

SSLS108 - Helicoid hull under load

\section*{Summary:}

This test in linear elasticity is very severe from its geometry (left hull). Two first modelings correspond one to the element of QUAD of modeling COQUE_3D (MEC3QU9H) and the other with element TRI7 of modeling COQUE_3D (MEC3TR7H). The two following ones relate to modeling SHB8.

The values of reference are computation results provided in the literature and the comparison calculation/reference relates to displacement in a point of the structure. One notes variations compared to
reference lower than \(0.15 \%\) for element MEC3QU9H and lower than \(0.4 \%\) for element MEC3TR7H. For modelings SHB8, the stupid maximum changes of \(1.4 \%\) with 24 elements SHB8 (modeling C) and
\(0.3 \%\) with 96 SHB8 (modeling D).
Handbook of Validation

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLSS108-Helicoid hull under concentrated loadings
Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET
:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{\(\boldsymbol{Y}\)}

FY
L
C
H
O
B
With
\(X\)
Z
B
FZ

The hull is thickness 0.32 m , length 12 m and of width 1.1 Mr.

\section*{1.2}

Material properties
\(E=29.106 \mathrm{~Pa}\)
\(=0.22\)

\section*{1.3}

Boundary conditions and loadings
Embedded on side \(O B C\) : \(U=v=W=0, X=y=Z=0\)
Two loading cases which corresponds to loadings concentrated at point a:

Force parallel with axis \(Z ; F z=1 \mathrm{NR}\)
Force parallel with the axis \(Y ; F y=1 N R\)
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HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS108-Helicoid hull under concentrated loadings
Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET
:
V3.03.108-B Page:
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\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
Parameters of the problem and results of reference (solutions obtained by finite elements of beam type) are given in the reference below [bib1].

\title{
2.2 \\ Results of reference \\ Displacement of point A following \(Y\) \\ Displacement of point A following Z.
}

\subsection*{2.3 References \\ bibliographical}
[1]
BATOZ J.L., DHATT G.: Modeling of the structures by finite elements. Volume 3 hulls, p456-459 (1992).

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
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Version
7.2

Titrate:
SSLSS108-Helicoid hull under concentrated loadings
Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET

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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
Element of hull 3D MEC3QU9H

\section*{Cutting:}

2 according to the width, 12 according to the length
24 meshs QUAD9, thickness: \(H=0.32\)
Names of the nodes:
Not \(O\)
N06
Not B
N01
Not C
N02
Not A
N032

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 125 \\ A number of meshs and types: 24 QUAD9
}

\author{
3.3 Functionalities \\ tested \\ Orders
}

\section*{AFFE_CARA_ELEM HULL}

ALL

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FORCE_NODALEGROUP_NOAFFE_MODELE AFFE
MODELING
:"COQUE_3D"
DEFI_MATERIAU ELAS
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
7.2
Titrate:
SSLS108 - Helicoid hull under concentrated loadings
Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET

:
V3.03.108-B Page:
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4
Results of modeling \(A\)
4.1 Values
tested
Case of
Not
Size in unit
Reference
Aster \%
difference

\author{
loads \\ \(F z=1 N R\) \\ With \\ displacement \(V(m)\) \\ -1.72 10-3 -1.717 \\ 10-3 \\ -0.15 \\ displacement \(W\) (m) \\ 5.42 10-3 5.411 \\ 10-3 \\ -0.16 \\ \(F y=1 N R\) \\ With \\ displacement V(m) \\ 1.75 10-3 1.750 \\ 10-3 \\ 0.0 \\ displacement \(W\) (m) \\ -1.72 10-3 -1.717 \\ 10-3 \\ -0.15
}

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
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Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS108 - Helicoid hull under concentrated loadings
Date:
03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET

V3.03.108-B Page:

\section*{5 Modeling}

B
5.1

Characteristics of modeling
Element of hull MEC3TR7H

\section*{Cutting:}

2 according to the width, 12 according to the length
48 meshs TRIA7, thickness: \(H=0.32\)
Names of the nodes:
Not \(O\)
N06
Not B
N01
Not C
N02
Not A
N032

\section*{5.2 \\ Characteristics of the grid \\ A number of nodes: 173 \\ A number of meshs and types: 48 TRIA7}

\subsection*{5.3 Functionalities}
tested
Orders

\section*{ALL}

\author{
AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_NODALE \\ GROUP_NO \\ AFFE_MATERIAU ALL
}

\title{
"MECHANICAL"AFFE_MODELE "COQUE_3D"
}

ALL
DEFI_MATERIAU ELAS

\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/03/008/A
}
```

Code_Aster ${ }^{\circledR}$
Version
7.2
Titrate:
SSLS108 - Helicoid hull under concentrated loadings

```

\section*{Date:}

03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET

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6
Results of modeling B

\subsection*{6.1 Values}
tested
```

Case of
Not
Size in unit
Reference
Aster %
difference
loads
Fz=1 NR
With
displacement V (m)
-1.72 10-3 -1.714
10-3
-0.34
displacement W (m)
5.42 10-3 5.399
10-3
-0.38
Fy=1NR
With
displacement V (m)
1.75 10-3 1.746
10-3
-0.23
displacement W (m)
-1.72 10-3 -1.714
10-3
-0.34

```

\section*{Handbook of Validation}
```

V3.03 booklet: Linear statics of the plates and hulls HT-66/03/008/A

```

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS108-Helicoid hull under concentrated loadings
Date:

\section*{7 Modeling}

C

\section*{7.1}

Characteristics of modeling

\section*{MASSIVE element of hull SHB8}

\section*{Cutting:}

2 according to the width, 12 according to the length 24 meshs SHB8, thickness: \(H=0.32\)

Names of the nodes:
Not O1
N03
Not O2
N05
Not B1
N04
Not B2
N06
Not C1
N01
Not C2
N02
Not A1

\section*{N78}

Not \(\mathbf{A 2}\)
N75

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 78
A number of meshs and types: 24 SHB8

\subsection*{7.3 Functionalities}
tested

Orders

\author{
MODI_MAILLAGE ORIE_SHB8 \\ GROUP_MA \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_NODALE \\ GROUP_NO \\ AFFE_MODELE AFFE \\ MODELING \\ : \\ SHB8 \\ DEFI_MATERIAU ELAS
}

\section*{Handbook of Validation}

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SSLSS108-Helicoid hull under concentrated loadings

\section*{Date:}

03/11/03
Author (S):
P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET :
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8
Results of modeling \(C\)

\subsection*{8.1 Values}
tested

Case of
Not
Size in unit
Reference
Aster
\% difference
loads
\(F z=1 N R\)
A1 and A2
displacement \(V(m)\)
-1.72 10-3
-1.717 10-3
0.16
displacement \(W\) (m)
5.42 10-3
5.408 10-3
-0.21
\(F y=1 N R\)
\(A 1\) and A2
displacement \(V(m)\)
1.75 10-3
1.726 10-3
-1.36
displacement \(W\) (m)
-1.72 10-3

\subsection*{8.2 Remarks}

A modeling 3D on the same grid (meshs HEXA8) revealed a blocking: results are very far away from the reference. For example, in the case of load 1, one obtains:

\author{
Not \\ Size in unit \\ Reference \\ Aster \\ A1 and A2 \\ displacement \(V(m)\) \\ -1.72 10-3 \\ -7.5 10-4 \\ displacement \(W\) (m)
}
5.42 10-3
5.408 10-3

This blocking does not appear any more with quadratic meshs HEXA20, since one obtains then:
Not
Size in unit
Reference
Aster
A1 and \(A 2\)
displacement \(V(m)\)
-1.72 10-3
-1.729 10-3
displacement \(W(m)\)
5.42 10-3
5.43 10-3

Modeling SHB8 makes it possible to avoid any numerical blocking, at a cost (in time CPU) similar to that of a grid HEXA8.
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\section*{9 Modeling}

D
9.1

Characteristics of modeling
MASSIVE element of hull SHB8
BlDIOIEI
C1
C2
A1
A2

\section*{Cutting:}

4 according to the width, 24 according to the length
96 meshs SHB8, thickness: \(H=0.32\)
Names of the nodes:
Not Ol
N245
Not O2
N249
Not B1
N224
Not B2
N226

Not C1
N239
Not C2
N241
Not D1
N236
Not D2
N238
Not E1
N250
Not E2
N246
Not A1
N05
Not A2
N06

\section*{9.2}

\section*{Characteristics of the grid}

A number of nodes: 250
A number of meshs and types: 96 SHB8

\author{
9.3 Functionalities \\ tested \\ Orders \\ MODI_MAILLAGE ORIE_SHB8 \\ GROUP_MA \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_NODALE \\ GROUP_NO \\ AFFE_MODELE AFFE \\ MODELING \\ : \\ SHB8 \\ DEFI_MATERIAU ELAS
}

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:
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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested

Case of
Not
Size in unit
Reference
Aster
\% difference
loads
\(F z=1 N R\)
A1 and A2
displacement V(m)
-1.72 10-3
\(-1.71810-3\)
0.09
displacement \(W\) (m)
```

5.42 10-3
5.41 10-3
-0.18
Fy=1NR
A1 and A2
displacement V (m)
1.75 10-3
1.744 10-3
-0.33

```

\section*{displacement \(W\) (m)}
```

-1.72 10-3
$-1.71810-3$
0.09

```

\subsection*{10.2 Remarks}

A modeling 3D on the same grid (meshs HEXA8) revealed a blocking: even with a grid with 96 elements. The results remain very far away from the reference. For example, for loading case 1, one obtains:

\author{
Not \\ Size in unit \\ Reference \\ Aster \\ A1 and A2 \\ displacement \(V(m)\) \\ -1.72 10-3 \\ -2.49 10-4 \\ displacement \(W\) (m) \\ 5.42 10-3 \\ 1.12 10-3
}

This blocking does not appear any more with quadratic meshs HEXA20, since one obtains then:

\author{
Not \\ Size in unit \\ Reference \\ Aster \\ A1 and A2 \\ displacement \(V(m)\)
}

Titrate:
SSLS108-Helicoid hull under concentrated loadings

\section*{Date:}

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P. MASSIN, J.M. PROIX, A. LAULUSA, Key S. BAGUET
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11 Summary of the results
This test is very severe because of geometry of the hull which is left.
Good solutions are obtained for four modelings.

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\section*{Titrate:}

SSLS109 Plates in traction made up of three tablecloths of Date reinforcements

\section*{: \\ 21/03/02}

Author (S):
C. CHAVANT, O. MERABET, Key Mr. DJERROUD
:
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Organization (S): EDF/AMA, INSA-Lyon

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
V3.03.109 document

SSLS109 - Plate in traction made up of three tablecloths of reinforcements (Model ROASTS)

\section*{Summary}

This test quasi-static 2D forced plane enters within the framework of the validation of the finite elements in elasticity
linear. A plate made up of three tablecloths of reinforcements offset compared to the average layer is subjected to a uniaxial traction. The orientations of the reinforcements and the mechanical

\section*{characteristics are}
different for the three tablecloths.
The principal interest of this test is to validate in linear elasticity the model "ROASTS" which is in fact an element of thin section (DKT) with a offsetting compared to the average layer.

The results provided by the model "ROASTS" (modeling B) are identical to those obtained by modeling "DEFI_COQU_MULT" of Code_Aster (modeling A).

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{\(\boldsymbol{Y}\)}

NO4
NO3
Xref
\(30^{\circ}\)
\(X\)
NO1
NO2

\section*{Xref}

\section*{Z}
\(Y\)
Xref
\(-75^{\circ}\)
\(-20^{\circ}\)
Xref
1
E
E
2
X
E
3

\section*{Handbook of Validation}

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1.2

Properties of materials

\section*{Solid 1}

E
1
1 =;
7

\section*{Solid 2}

E
2
\(2=\)
;
7
G2 10
=
\(2=\)
=1
L2
T 2
; offsetting \(=0,00\); épaisseur \(=0,01\)
Solid 3
E
3
3 =
;
7
3
G
10-
\(=\)
0
```

3=
;
=
= 1
; offsetting $=-0,01$; épaisseur $=0,01$
1.3
Boundary conditions and loadings

Embedded side NO1-NO4.
One imposes a tractive effort on side NO2-NO3: FX $=2$

## 2 <br> Reference solution

### 2.1 Solution

formal
Model DEFI_COQU_MULT of Code_Aster (modeling A) makes it possible to validate the model "GRID" proposed (modeling B).

## 2.2

Numerical values of reference
Values of:
$U, v, W, X$ and $y$ with node NO2,
membrane deformation: $x x, y y, x y$ in solid 1 calculated with node NO2, variation of curve: $x x, y y, x y$ in solid 1 calculated with node NO2, constraints 3D: xx, yy, zz, xy in solid 1 calculated with node NO2.

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3 Modeling
With

## 3.1 <br> Characteristics of modeling

The test-tube is with a grid with four elements "DKT"
$\boldsymbol{Y}$
NO4
NO3
1
1
NO5
$X$
1
1
NO1
NO2

Grid:
A number of nodes: 5
A number of meshs and type: 4 meshs TRIA3
The grid is symmetrical in order to avoid the parasitic modes of deformations.

## 3.2

Stages of calculation and functionalities tested

The principal stages of calculation correspond to the functionalities which one wishes to validate: Orders

AFFE_MODELE AFFE MODELING "DKT"

AFFE_CARA_ELEM HULL

DEFI_MATERIAU ELAS_ORTH

DEFI_COQU_MULT SLEEP

## MECA_STATIQUE

CALC_ELEM

CALC_NO

RECU_CHAMP

## Handbook of Validation

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5.0

Titrate:
SSLS109 Plates in traction made up of three tablecloths of Date reinforcements

```
:
21/03/02
```

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Author (S): <br> C. CHAVANT, O. MERABET, Key Mr. DJERROUD <br> : <br> V3.03.109-A Page: <br> 5/8 <br> ```
4 <br> Results of modeling $A$

```
}

\author{
Variables \\ DEFI_COQU_MULT
}

\author{
(modeling A) \\ Displacements node NO2
}

U 81.9373
v 18.6398
W 2098.66
\(X\)
-1239.88
\(y\)
-3820.42
Deformations node NO2

\section*{(Solid 1)}
\(x x\)
32.9433
\(y y\)
2.33011
\(x y\)
-34.3522
\(z z\)
0
Constraints node NO2
(Solid 1)
\(x x\)
42.7252
yy
63.0951
xy
-5.88029

Curves node NO2
(Solids 1, 2 and 3)
\(\boldsymbol{x x}\)
\(-531.043\)
\(y y\)
-2035.19
\(x y\)
2058.44

\subsection*{4.1 Remarks}

\section*{\(\boldsymbol{R}\)}

\section*{R}

The results presented are given in the reference mark of reference ( \(X\)
```

,Y

```
)
\(r e f\).
ref. forming an angle of \(30^{\circ}\)
\(\boldsymbol{R} \boldsymbol{R}\)
compared to ( \(X, Y\) ), concerning the deformations, constraints and curves.
Handbook of Validation

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\section*{HT-66/02/001/A}

Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

SSLS109 Plates in traction made up of three tablecloths of Date reinforcements
```

5
Modeling B (GRID)

```

\section*{5.1 \\ Characteristics of modeling}

The test-tube is with a grid with 12 elements "ROASTS" (4 elements by tablecloth of reinforcements) to three
nodes (thin section: formulation DKT with offsetting compared to the datum-line).
nodes are common to each tablecloth.
```

Y
NO4
NO3
I
1
NO5
X
I
I
NO1

```
NO2

Grid:
A number of nodes: 5
A number of meshs and type: 3x4 meshs TRIA3
The grid is symmetrical in order to avoid the parasitic modes of deformations.

\section*{5.2}

Stages of calculation and functionalities tested
The principal stages of calculation correspond to the functionalities which one wishes to validate:
Orders

\author{
AFFE_CARA_ELEM ROASTS
}

\section*{MECA_STATIQUE}

\section*{CALC_ELEM OPTION}

EPSI_ELNO_DEPL

SIGM_ELNO_DEPL

DEGE_ELNO_DEPL

\section*{EFGE_ELNO_DEPL}

SIEF_ELGA_DEPL

\author{
CALC_NO OPTION \\ FORC_NODA \\ RECU_CHAMP
}

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Titrate:

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C. CHAVANT, O. MERABET, Key Mr. DJERROUD

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6
Results of modeling \(B\)

\author{
Variables \\ Modeling A \\ Modeling B \\ \% difference \\ Displacements node NO2 \\ U 81.9373 \\ 81.9373 \\ 0 \\ v 18.6398 \\ -18.6398 \\ 0 \\ W 2098.66 \\ 2098.66 \\ 0 \\ X \\ \(-1239.88-1239.880\) \\ \(y\) \\ -3820.42-3820.42 0 \\ Deformations node NO2
}

\section*{(Solid 1)}
\(\boldsymbol{x} \boldsymbol{x}\)
```

32.9433 32.9433 0
yy
2.33011 2.330110
xy
-34.3522-34.3522 0
zz
0
O
Constraints node NO2

```
(Solid 1) \(x x\)
42.725242 .72520
\(y y\)
63.095163 .09510
\(x y\)
\(-5.88029-5.880290\)
\(z z\)
00
0
Curves node NO2
(Solids 1, 2 and 3)
\(x x\)
-531.043-531.043 0
\(y y\)
-2035.19-2035.19 0
\(x y\)
2058.442058 .440

\subsection*{6.1 Remarks}

\section*{R}
\(\boldsymbol{R}\)
The results presented are given in the reference mark of reference ( \(X\)
,
)
ref.
ref. forming an angle of \(30^{\circ}\)
R R
compared to ( \(X, Y\) ), concerning the deformations, constraints and curves.

\author{
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}

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Summary of the results and remarks general
The results of modeling \(A\) are identical to those of modeling \(B\).
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Titrate:
SSLS110 Stability of a compressed square plate

Date:
03/01/00
Author (S):
P. MASSIN, Key Mr. Al MIKDAD

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Organization (S): EDF/IMA/MMN, SAMTECH

\author{
Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ Document: V3.03.110
}

SSLS110 - Stability of a compressed square plate

\section*{Summary:}

A homogeneous isotropic linear plate square elastic simply pressed on its four sides is subjected with a linear compressive force acting on two on its sides.

One calculates the critical loads leading to the elastic buckling of the plate. The matrix of rigidity geometrical used in the resolution of the problem to the eigenvalues is that which is due to the constraints
initial.
- mechanical linear rubber band,
- buckling of a hull,
- interest of the test: calculation of the geometrical matrix of rigidity of elements COQUE_3D, - 2 modelings.

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\title{
Titrate: \\ SSLS110 Stability of a compressed square plate
}

Date:
03/01/00
Author (S):
P. MASSIN, Key Mr. Al MIKDAD

\section*{:}

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

Because of the geometrical and physical symmetry of the problem, only the quarter of the plate is modelled. By taking account the conditions of symmetry, one can collect only the only modes of buckling symmetrical.

L
\(L=250 \mathrm{~mm}\)
2
2
P3
\(Q\)
P4
P2
\(H=5 \mathrm{~mm}\)
1
\(P\)
Z
\(y\)
\(X\)

\section*{1.2}

Material properties
\(E=2.110+5 \mathrm{Mpa}\).

\section*{\(=0.3\)}

The transverse coefficient of shearing for the plate is worth \(A_{-} C I S=5 / 6\).

\section*{Handbook of Validation}

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\section*{1.3}

Boundary conditions and loadings
C.L. :

P2P3:
\(D Z=0\).

\section*{P3P4:}
\(D Z=0\).

\section*{Symmetry}

P1P2:
\(D Y=0\).
\(D R X=0\).
\(D R Z=0\).
P4P1:
\(D X=0\).
\(D R Y=0 . D R Z=0\).

\section*{Loading:}

Linear compressive force \(Q\) on P2P3

\subsection*{1.4 Remarks}

It is not possible to solve the problem of deformation of compression without introducing them conditions of symmetry. Indeed, to impose boundary conditions of symmetry for a quarter of plate amounts eliminating the modes from rigid body for the complete plate.

\section*{2 \\ Reference solution}

\section*{2.1 \\ Reference solution}

The analytical solution obtained with a thin theory of section in homogeneous linear elasticity isotropic [bibl] without taking into account of the transverse energy of shearing I ème load determines critical:

2
D
12
\(Q\)
\(=(I+)\)
Cr I
2
\(L\)
I
with:
3

H: the thickness
\(L\) : the length on the side of the square plate.

\section*{2.2 \\ Results of reference}

Certain modes corresponding to the critical loads of the analytical solution are not symmetrical and cannot be collected with the conditions of symmetry for a quarter of plate. The Values of the critical loads obtained thus correspond to the first 3 symmetrical modes of buckling:
- Mode 1 of the quarter of the plate \(=\) Mode 1 of all the plate
- Mode 2 of the quarter of the plate \(=\) Mode 3 of all the plate
- Mode 3 of the quarter of the plate \(=\) Mode 5 of all the plate

\section*{2.3 \\ Uncertainty on the solution}

Exact solution for a theory of plate without transverse shearing.

\subsection*{2.4 References \\ bibliographical}
[1]
J.G. EISLEY "Mechanics of Elastic Structures: Classical years Finite Methods Element".

Prentice Hall, Englewood Cliffs N.J. 07632 (19XX).
[2]
"Stability of Square Punt Biaxial Under Loading". The the SAMCEF software User' S Manuals V7.1. (1998).

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03/01/00
Author (S):

\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

MEC3QU9H (hull 3D)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 121
A number of meshs and types: 25 QUAD9

\subsection*{3.3 Functionalities}
tested
Orders

\section*{Keys}

AFFE_MODELE
"MECHANICAL"
"COQUE_3D"
ALL
[U4.22.01]
AFFE_CARA_ELEM
HULL

A_CIS
[U4.24.01]
THICK

\author{
AFFE_CHAR_MECA \\ DDL_IMPO \\ GROUP_NO \\ [U4.25.01] \\ FORCE_ARETE \\ GROUP_MA \\ CALC_MATR_ELEM \\ "RIGI_GEOM" \\ [U4.41.01] \\ MODE_ITER_SIMULT \\ "PLUS_PETITE" \\ [U4.52.02]
}

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Date:
03/01/00
Author (S):
P. MASSIN, Key Mr. Al MIKDAD

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference Aster \% difference}

\author{
mode 1 \\ \(3.79600 E+023.78520 E+020.987\) \\ mode 2 \\ \(1.05444 E+031.04904 E+030.512\) \\ mode 3 \\ \(2.56609 E+032.57466 E+03+0.334\)
}

\subsection*{4.2 Remarks}

Energy due to transverse shearing is not neglected.

\subsection*{4.3 Parameters \\ of execution}

Version: 5.02.20

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 13.30 seconds
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HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLS110 Stability of a compressed square plate

Date:

\section*{5 Modeling \\ B \\ 5.1 \\ Characteristics of modeling}

MEC3TR7H (COQUE_3D)
4
3
1
2
modeling COQUE_3D

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 641
A number of meshs and types: 200 TRIA7

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
Keys \\ AFFE_MODELE \\ "MECHANICAL" \\ "COQUE_3D" \\ ALL \\ [U4.22.01] \\ AFFE_CARA_ELEM
}

\section*{HULL}

A_CIS
[U4.24.01]
THICK
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_ARETE
GROUP_MA
CALC_MATR_ELEM
"RIGI_GEOM"
[U4.41.01]
MODE_ITER_SIMULT
"PLUS_PETITE"
[U4.52.02]
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLS110 Stability of a compressed square plate

Date:
03/01/00
Author (S):
P. MASSIN, Key Mr. Al MIKDAD
:
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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested

\section*{Identification Reference Aster \% difference}

\section*{Mode 1 \\ \(3.79600 E+023.77689 E+020.503\)}

Mode 2
\(1.05444 E+031.05744 E+03+0.284\)
mode 3
\(2.56609 E+032.58295 E+03+0.657\)

\subsection*{6.2 Remarks}

Energy due to transverse shearing is not neglected.

\subsection*{6.3 Parameters \\ of execution}

Version: 5.02.20

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 48.06 seconds
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Version
5.0

Titrate:
SSLS110 Stability of a compressed square plate

Date:
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03/01/00

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Author (S):
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\section*{7}

Summary of the results
The results obtained are very satisfactory for the two types of elements, QUAD9 and TRIA7, even if it is necessary to employ a greater number of elements triangles.

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Titrate:
SSLS111-Simple offsetting of plate

Date:
19/09/02
Author (S):
P. Key MASSIN, J.M. PROIX

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Organization (S): EDF/AMA

\title{
Handbook of Validation
}

V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.111

SSLS111-Simple Excentremement of plate

\section*{Summary:}

This test makes it possible to validate the offsetting of the simple plates (i.e it acts neither of multilayer, nor of one homogenized behavior).

The reference is given by a first resolution where one models double-layered made up of 2 materials.
The validation is done in the second calculation where one models the 2 layers of the preceding model by 2 plates offset compared to the average plan of the first calculation.

Three modelings are used: DKT, DST (meshs QUAD4) DST (meshs TRIA3).

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P. Key MASSIN, J.M. PROIX

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{A3}

\section*{5 subdivisions}
```

Zy
GRN011
0.2
m
A2
0.2
m
A1
10
subdivisions
X

```

The co-ordinates of the points are:
A1 \((0,0,0)\)
A3 \((10,5,0)\)
A2 \((10,0,0)\)
A4 \((0,5,0)\)

\section*{1.2 \\ Material properties}

The material is double-layered.
The material constituting the first layer is orthotropic and is characterized by the data following:
\[
\begin{aligned}
& E L=20000 . M P a \\
& A N D=20000 . M P a \\
& V L T=0.3 \\
& G L T=2000 . M P a .
\end{aligned}
\]

The material constituting the second layer is also orthotropic and is characterized by the data following:
\(E L=15000 . \mathrm{MPa}\)
\(A N D=15000 . \mathrm{MPa}\)
\(V L T=0.3\)
\(G L T=1500 . M P a\)
1.3

Boundary conditions and loadings
The A1 node is embedded:
\(d x=0\).
\(D y=0\).
\(d z=0\).
\(d R x=0\).
\(D R y=0\).
\(D R z=0\).
The A2 node is blocked according to following ddls:
\[
\begin{aligned}
& d x=0 . \\
& D y=0 .
\end{aligned}
\]

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One applies a nodal force \(F z=1000\) to the A3 node.
In addition, one applies to the meshs M1, m2 and m3 (see drawing) the loading distributed (key word Force_coque) according to:
\(F x=200 \mathrm{~N} / \mathrm{m} 2, F y=500 \mathrm{~N} / \mathrm{m} 2, F z=500 \mathrm{~N} / \mathrm{m} 2, M X=100 \mathrm{~N} / \mathrm{m}, M y=40 \mathrm{~N} / \mathrm{m}\)
in the plan of the grid.

2
Reference solution
2.1

Method of calculation used for the reference solution
Calculation with double-layered material is used as reference. Nonregression of the results obtained for it
the first calculation is checked.

\section*{2.2}

Results of reference
They are consisted of the values of the field of displacement DX, DY, DZ, DRX, DRY at the point A3 (node
N1 for ASTER) and at the point of co-ordinates (9,2,0).
One compares also the efforts with the A1 point.
In addition, the 4 smaller frequencies of the structure are calculated.

\section*{2.3 \\ Uncertainty on the solution}

Null since it is about the same calculation carried out by two different ways.
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\section*{3 Modeling}

With

\section*{3.1}

\section*{Characteristics of modeling}

The model consists of 2 plates corresponding to the average plan of the 2 layers of the model of reference.

To represent these 2 plates, one leaves the grid of the average plan of double-layered which one offsets distances 0.1 and 0.1.

PLAQ1
The elements used are elements of plate DKQ.

PLAQ 2
\(+0.1\)

\section*{3.2}

Characteristics of the grid
Z
\(y\)
5

10
\(X\)

The grid is regular. There are 10 subdivisions according to \(X\) and 5 subdivisions according to \(y\); that is to say on the whole 50 meshs
DKQ (quad4) and 66 nodes.

\subsection*{3.3 Functionalities}
tested

Order

Key word factor key Word

\author{
AFFE_CARA_ELEM \\ Offsetting \\ AFFE_CHAR_MECA \\ Force_coque \\ PLAN \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/02/001/A
}

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4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Reference
Aster \%
Difference
DX (N1)
-7.9818
-7.9818
0
DY (N1)
0
DZ (N1)
-6330
-6330
0
DRX (N1)
-454.433
-454.433
0
DRY (N1)
566.451
566.451
0
DX (N10)
-4.5814
-4.5814
0
DY (N10)
-1.8758
-1.8758
0
DZ (N10)
-4466
-4466
0
DRX (N10)
-430.09
-430.09
0
DRY (N10)
512.163
512.163
0
Frequency 1st mode
9.5393.10-4 9.5393.10-4 0
Frequency 2nd mode
3.7115.10-3 3.7115.10-3 0
Frequency 3rd mode
8.2208.10-3 8.2208.10-3 0
Frequency 4th mode
1.6837.10-2
```

1.6837.10-2
0
NXX
2.2024.104 2.2024.104
0
NYY
-2.4402.103-2.4402.103
0
NXY
1.0581.103 1.0581.103
0
MXX
4.1733.104 4.1733.104
0
MYY
1.8444.104 1.8444.104
0
MXY
6.3333.103 6.3333.103
0
QX
-3.193.104 -3.193.104
0
QY
-1.4346.104 -1.4346.104
O

```
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\section*{5 Modeling}

B

\section*{5.1}

Characteristic of modeling
The model is the same one as that of modeling A, with this close that instead of having elements of plate DKQ, one has elements DSQ.

\section*{5.2}

Characteristic of the grid

Z
\(y\)
5
\(X\)

The grid is regular. There are 10 subdivisions according to \(X\) and 5 subdivisions according to \(y\); that is to say on the whole 50 meshs
DSQ and 66 nodes.

\subsection*{5.3 Functionalities}
tested
Order
Key word factor key Word

\author{
AFFE_CARA_ELEM \\ Offsetting \\ AFFE_CHAR_MECA \\ Force_coque
}

\author{
PLAN \\ AFFE_MODELE \\ AFFE \\ MODELING \(=\) DST \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/02/001/A
}

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\section*{6}

Result of modeling B
6.1 Values
tested

\section*{Identification Reference}

Aster \%
Difference
DEPL DX N1
7.98940E06
8.01344E06
0.301

DEPL DY N1
1.17388E06
1.19166E06
1.514

\author{
DEPL DZ N1 \\ 6.35573E03 \\ \(6.37537 E 03\) \\ 0.309 \\ DEPL DRX N1 \\ 4.42621E04 \\ 4.39613E04 \\ -0.679 \\ DEPL DRY N1 \\ 5.72702E04 \\ 5.75565E04 \\ 0.500 \\ DEPL DX N10 \\ 4.62813E06 \\ 4.65034E06 \\ 0.480 \\ DEPL DY N10 \\ 1.88945E06 \\ 1.90341E06 \\ 0.739 \\ DEPL DZ N10 \\ 4.52107E03 \\ 4.54691E03 \\ 0.572 \\ DEPL DRX N10 \\ 4.17874E04 \\ 4.14602E04 \\ -0.783 \\ DEPL DRY N10 \\ 5.19527E04 \\ 5.22722E04 \\ 0.615 \\ EFGE NXX N60 \\ 1.62953E+04 \\ 1.53979E+04 \\ -5.507 \\ EFGE NYY N60 \\ \(4.50035 E+03\) \\ \(5.11922 E+03\) \\ 13.752 \\ EFGE NXY N60 \\ 9.91495E+02 \\ \(9.30980 E+02\)
}

\section*{-6.103}

EFGE MXX N60
3.63645E+04
\(3.56107 E+04\)
-2.073
EFGE MYY N60
\(1.59599 E+04\)
1.56220E+04
-2.118
EFGE MXY N60
6.31716E+03
6.32614E+03
0.142

EFGE QX N60
2.07352E+04
\(1.90500 E+04\)
-8.127
EFGE QY N60
1.04743E+04
\(1.01187 E+04\)
-3.395
MODE 1
9.50214E-01
9.48127E-01
-0.220
MODE 2
\(3.61805 E+00\)
3.58389E+00
-0.944
MODE 3
8.16228E+00
\(8.13462 E+00\)
-0.339
MODE 4
1.65440E+01
\(1.64359 E+01\)
-0.653

\subsection*{6.2 Remarks}

There is a difference (not explained) on this modeling between the value of reference (double-layered) and
two offset plates.

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\section*{7 Modeling}

C

\section*{7.1}

Characteristics of modeling
The model is the same one as that of modeling A, with this close that instead of having elements of plate DKQ, one has DST elements. (DST Modeling with meshs TRIA3).

\section*{7.2}

Characteristics of the grid
```

Z
y
5

The grid is regular. There are 10 subdivisions according to $X$ and 5 subdivisions according to $y$; that is to say on the whole 100
DST meshs and 66 nodes.
7.3 Functionalitiestested
Order
Key word factor key Word
AFFE_CARA_ELEM
Offsetting
AFFE_CHAR_MECA
Force_coque
PLAN
AFFE_MODELE
AFFE ..... MODELING $=$ DST
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# Result of modeling C 

8.1 Values

tested

## Identification Reference

Aster \%

Difference
DEPL DX N66
6.49678E06
6.50205E06
0.081

DEPL DY N66
$6.08932 E 07$
6.10332E07
0.230

DEPL DZ N66
5.33844E03
5.36043E03
0.412

DEPL DRX N66
4.29182E04
4.29587E04
0.095

DEPL DRY N66
4.75601E04
4.77482E04
0.395

DEPL DX N53
3.58293E06
3.58709E06
0.116

DEPL DY N53
1.18788E06
1.19013E06
0.190

DEPL DZ N53
3.63885E03
3.65793E03
0.524

DEPL DRX N53
4.05175E04

4.05324E04<br>0.037<br>DEPL DRY N53<br>4.23116E04<br>4.25311E04<br>0.519<br>EFGE NXX N6<br>$1.70005 E+04$<br>$1.68443 E+04$<br>-0.918<br>EFGE NYY N6<br>1.14438E+04<br>$1.12660 E+04$<br>-1.554<br>EFGE NXY N6<br>$3.53598 E+03$<br>$3.57111 E+03$<br>0.993<br>EFGE MXX N6<br>$2.14585 E+04$<br>$2.13070 E+04$<br>-0.706<br>EFGE MYY N6<br>1.53094E+04<br>$1.51378 E+04$<br>-1.121<br>EFGE MXY N6<br>$5.71331 E+03$<br>$5.76258 E+03$<br>0.862<br>EFGE QX N6<br>3.03380E +03<br>$2.81593 E+03$<br>-7.181<br>EFGE QY N6<br>1.76436E+03<br>$1.78725 E+03$<br>1.297<br>MODE 1<br>$1.01181 E+00$<br>1.00910E+00<br>-0.268<br>MODE 2

$4.26070 E+00$
-0.218
MODE 3
8.39151E+00
$8.36517 E+00$
-0.314
MODE 4
$1.72305 E+01$
$1.71358 E+01$
-0.549

### 8.2 Remarks

As for modeling B, one notes a difference between the solution obtained for a hull double-layered and that resulting from two offset full-course hulls, without it being possible at the time
drafting of the test to determine from which the variation comes.
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## 9

Summary of the results

With regard to modeling A (DKT) results obtained with two full-course hulls offset coincide perfectly with those obtained by a double-layered hull, as well in terms displacements, that generalized efforts or Eigen frequencies. This thus validates offsetting by hulls DKT.

On the other hand, modelings B and C, both with DST hulls, reveal one
difference between the two ways of carrying out calculation. This one is lower than 1,5\% with regard to
displacements and frequencies, which remains reasonable.
On the other hand, there are variations on the efforts going up to 7\% for modeling C (DST triangles) and
14\% for modeling B (DST quadrangles).
This variation is not explained and one cannot know a priori if it is due to offsetting for DST modelings or with the multi-layer hulls DST.

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Titrate:
SSLS112-Offsetting of composite plates

Date:
19/09/02
Author (S):
J. Mr. PROIX, G. BERTRAND Clé
:
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Organization (S): EDF/AMA, CS IF

# Handbook of Validation <br> V3.03 booklet: Linear statics of the plates and hulls <br> Document: V3.03.112 

SSLS112-Offsetting of composite plates

## Summary:

This test makes it possible to validate the offsetting of composite plates.
The reference is given by a first resolution where one models a quadricouche presenting one material not-symmetry compared to the average plan.

The validation is done in the second calculation where one models the quadricouche preceding model by
2 double-layered offset compared to the average plan of the first calculation.

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SSLS112- Offsetting of composite plates

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19/09/02
Author (S):
J. Mr. PROIX, G. BERTRAND Clé

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1
Problem of reference

### 1.1 Geometry

A4
A3
5 subdivisions

## 10 subdivisions

X

## 1.2 <br> Material properties

The material consists of 4 orthotropic layers thickness 0.1.
The first layers is characterized by:
$E L=20000.106$ Pa
$A N D=20000.106 \mathrm{~Pa}$
$V L T=0.3$
$G L T=2000.106 \mathrm{~Pa}$
the second layer by:
$E L=15000.106$ Pa
$A N D=15000.106 \mathrm{~Pa}$
$V L T=0.3$
$G L T=1500.106 \mathrm{~Pa}$
the third layer by:
$E L=20000.106$ Pa
$A N D=20000.106 \mathrm{~Pa}$
$V L T=0.3$
$G L T=2000.106 \mathrm{~Pa}$
and the fourth layer by:
$E L=15000.106$ Pa
$A N D=15000.106 \mathrm{~Pa}$
$V L T=0.3$
$G L T=1500.106 \mathrm{~Pa}$
1.3

Boundary conditions and loadings
The A1 node is embedded:
$d x=0$.
$D y=0$.
$d z=0$.
$d R x=0$.
$d R y=0$.
$d R z=0$.
The A2 node is blocked according to following ddls:
$d x=0$.
$D y=0$.

One applies a modal force $F z=-1000 . N$ to the $A 3$ node.
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## 2

Reference solution

## 2.1

Method of calculation used for the reference solution
The reference solution results from the first calculation with ASTER with the quadricouche describes in the problem of reference.

## 2.2

Results of reference
They are consisted of the values of the field of displacement $D X, D Y, D Z, D R X, D R Y$ at the A3 point (N1 node for ASTER) and with the N10 node of co-ordinates $(9,2,0)$.

## 2.3

Uncertainty on the solution
Null, since it is about the same calculation carried out by two different ways.

## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

The model consists of 2 double-layered plates corresponding to the average plan of the quadricouche of model of reference.

To represent these 2 plates, one leaves the grid of the average plan of the quadricouche which one offsets distances -0.1 and 0.1.

## PLAQ1

The elements used are elements of plate DKT.

PLAQ
2
$+$
0.1
-
0.1

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# Titrate: <br> SSLS112-Offsetting of composite plates 

## Date:

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J. Mr. PROIX, G. BERTRAND Clé
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## 3.2 <br> Characteristics of the grid

## Z

$y$

5

5
$X$

The grid is regular.
There are 10 subdivisions according to $X$ and 5 subdivisions according to $y$; that is to say on the whole 50 meshs DKQ (quad4) and 66 nodes.

3.3 Functionalities<br>tested<br>Order Mot<br>key<br>AFFE_CARA_ELEM<br>Offsetting

## 4

Result of modeling $A$

### 4.1 Values

tested
Identification Reference
(*10-6m) ASTER (*10-6m) \%
Difference
DX (N1)
-3.680419
-3.680419
0
DY (N1)
-0.493941
-0.493941
0
DZ (N1)
-5697.7635
-5697.7635
0
DRX (N1)
-436.1676
-436.1676
0
DRY (N1)
508.6670
508.6670
0
DX (N10)
-2.172360
-2.172360
0
DY (N10)
-0.783905
-0.783905
0
DZ (N10)
-3946.2632
-3946.2632
0

## DRX (N10)

-412.1209
-412.1209
0
DRY (N10)
455.0638
455.0638

0

5 Synthesis
The results obtained with offset multi-layer plates agree with the reference.
This test thus validates offsetting for the multi-layer plates.
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Code_Aster ${ }^{\circledR}$
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## Titrate:

SSLS113-Offsetting of homogénéisées plates

## Date:

23/09/02
Author (S):
J.M. PROIX, G. BERTRAND Clé
:
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Organization (S): EDF/AMA, CS IF

# Handbook of Validation <br> V3.03 booklet: Linear statics of the plates and hulls <br> Document: V3.03.113 

## SSLS113-Offsetting of plates <br> homogenized

## Summary:

This test makes it possible to validate the offsetting of the plates having a behavior "ELAS_COQUE".
The reference is given by a first resolution where one models double-layered orthotropic not having one
material symmetry compared to the average plan.
The validation is done in the second calculation where one models the behavior of the preceding plate by 2 offset full-course plates having a behavior "ELAS_COQUE".

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Titrate:
SSLS113-Offsetting of homogénéisées plates
Date:
23/09/02
Author (S):
J.M. PROIX, G. BERTRAND Clé

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1
Problem of reference

### 1.1 Geometry

## N60 GRNO13 <br> N87

0,7071m

## $Z$

N1
GRNO11
N28
0,1m
$y$

0,1m

0,7071m
$X$

Co-ordinates of the points:
N1 (0,0,0)
N87 (0,7071,0.7071, 0)

N28 (0,7071,0,0)
N60 (0,0.7071, 0)
1.2

## Material properties

The material is double-layered.
The material constituting the first layer is orthotropic and is characterized by the data following:
$E L=6800 . P a$
$A N D=6800 . P a$
$V L T=0.35$
$G L T=2530 . P a$.
The material constituting the second layer is also orthotropic and is characterized by following data:
$E L=14000 . P a$
$A N D=14000 . \mathrm{Pa}$
$V L T=0.144$
$G L T=2070 . P a$.

## 1.3

Boundary conditions and loadings
Side N1N28 (GRN011) is embedded:
$d x=0$.
$D y=0$.
$d z=0$.
$d R x=0$.
$d R y=0$.
$d \boldsymbol{R z}=0$.
One imposes the ddls dx and Dy of the nodes on the side N80N60 (GROUPNO GRN013) on the values
following:

$$
\begin{aligned}
& d x=0.07071 \mathrm{~m}, \\
& D y=0.07071 \mathrm{~m}
\end{aligned}
$$

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# Code_Aster ${ }^{\circledR}$ 

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2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution results from the first calculation with ASTER with the double-layered one describes in
problem of reference.
2.2

Results of reference
They are consisted of the values of the field of displacement to the N1 node of co-ordinates (0, .0. , 0.) (ddl
$D Z)$ and with the N10 node of co-ordinates (0.216760, 0.0764431, 0.).

## 2.3 <br> Uncertainty on the solution

Null, since it is about the same calculation carried out by two different ways.

## 3 Modeling

## 3.1 <br> Characteristics of modeling

The model consists of 2 plates corresponding to the average plan of the 2 layers of the model of
reference.
To represent these 2 plates, one leaves the grid of the average plan of double-layered which one offsets distances -0.05 m and 0.05 Mr .

PLAQ1
The elements used are elements of
plate DKT.

PLAQ
2
$+$
0.05
0.05

One assigns behavior "ELAS_COQUE" to each one of these plates corresponding to homogenized orthotropic behavior of the corresponding layer.

The values of the coefficients material introduced under "ELAS_COQUE" were calculated directly [U4.43.01], page 27.

## 3.2 <br> Characteristics of the grid

The model has 87 triangular nodes and 140 elements DKT.

### 3.3 Functionalities

tested

## Order Mot <br> key

AFFE_CARA_ELEM
Offsetting

## DEFI_MATERIAU

ELAS_COQUE

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## 4 <br> Result of modeling $A$

### 4.1 Values

tested

## Identification Reference

## ASTER \%

Difference
DZ (N1)
-0.169388-0.169388
0
DX (N10)
0.0089620 .008962

0
DY (N10)
0.0081700 .008170

0
DZ (N10)
0.1635980 .163598

0
DRX (N10)
4.1964304 .196428

0
DRY (N10)
-0.050793-0.050793
0

### 4.2 Remarks

No the error compared to double-layered orthotropic.

5 Synthesis
The results show the good taking into account of offsetting for ELAS_COQUE.
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SSLS114 - Not regression on cylindrical quarter of binding ring
Date:
03/11/03
Author (S):
P. Key MASSIN, J.R. LEVESQUE
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Organization (S): EDF-R \& D /AMA

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Document: V3.03.114

SSLS114- Not regression on quarter of ring cylindrical

## Summary:

It is about a test of mechanics in linear statics.
The goal is to test the setting under pressure of a cylindrical quarter of binding ring with the elements of hull and of plate.

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1
Problem of reference

1.1<br>Properties of materials<br>$E=200.000 \mathrm{MPa}$

```
= 0.3
```

3
=
$1234 \mathrm{~kg} / \mathrm{m}$ for modelings I, J and K.

### 1.2 Characteristics

geometrical
One notes:
$R=0975$

```
m the interior ray of the cylinder;
I
R=1025
m}\mathrm{ the ray external of the cylinder;
2
-R=1 m the average radius of the cylinder equal to the half the sum of the two preceding rays;
-E=00
. 5m the radial thickness of the cylinder;
-H=05
. m the height of the cylinder.
```


## 1.3

Boundary conditions and loadings mechanical

## Conditions of Dirichlet

DDL_IMPO, the nodes blocked depend on modeling.
pressure on the elements of hull and plate: $P=10 \mathrm{MPa}$ on the cylinder
PRES_REP
FORCE_COQUE (real or given by a function)
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2
Reference solution
Analytical solution.

## 2.1

Results of reference
Displacement of the average layer
Constraints of the average layer, layers superior and inferior.
In modelings $I, J$ and $K$, one calculates the mass, the co-ordinates of the centre of gravity and them terms of the matrix of inertia. The analytical expressions are given in documentation [R3.07.02].

### 2.1.1 Method of calculation used for the reference solution in displacements and constraints

Into incompressible:

```
B
U
=
(1+)
a2 b2
```

with $B=$
$\boldsymbol{P}$
2
2
$U=U$
E
$=0$
(B-has)
Z
B
$r r=-r 2$
B
$=+$
$R$
2
$R=z z=0$
$a 2$
b2
J
$=P$
$r r$
b2-a2
$r 2$

1+
b2-a2
$r 2$
$R=0$
$a 2$
$=2 P$
$z z$
b2-a2

## Passage in the Cartesian system:

2
$\boldsymbol{x} \boldsymbol{x}$
$=r r \cos +\sin -2 R \sin \cos$

2
2
$y y$
$=r r \sin +\cos +2 R \sin \cos$

2
2
$x y$
$=r r \sin \cos -\sin \cos -2 R(\cos \sin )$
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### 2.1.2 Determination of the masses, centre of gravity and tensor of inertia

For modeling I of hull type of revolution around an axis OZ
$\cdot$ the mass is worth: $M=H(R 2-R 2) 2$
2
1
$=H$
er

X 0
G

- the co-ordinates of the centre of gravity are: $y$

| $=$ |
| :--- |
| 0 |
|  |

G

## - the tensor of inertia compared to $O$ is worth:

MR2
1 E
M
1
$I+$
2
()$]$
$H 2$
0
0

$I$
$I$
$I$
2
$4 R$

3
$x x$
$x y$
$x z$

MR2
1 E
M
2
2

```
I
```

I
$I=$
0
1
$[+()]+$
H
0
$x y$
3
$I$
$I$
$I$
1

$x z$
$y z$
$z z$
$E$
0
0
$M R 21$
$[+$
2
()
$]$
$4 R$

## - the tensor of inertia compared to $G$ is worth:

$M R 2$
$1 E$
$M$
1
$[+$
2
()$]+$
$H 2$
0
0
$I$
$I$
$I$
2
$4 R$
12

$x x$
$x y$
$x z$
$M R 2$
$1 E E$
$M$
2
2
$I$
$I$
$I=$
0
1
$[+()]+$
$H$
0
$x y$
$y y$
$y z$
2
$4 R$

For modelings $J$ and $K$ where the trace of a quarter of cylinder of cross-section around an axis $O Z$ on one
plan perpendicular to this axis is represented
(R
)
$1+\boldsymbol{R} 2$

- the mass per unit height is worth: $M=$
$E=R e ;$
2
2
2
2
1 E 2
$X$
$\boldsymbol{R}$
1
$[+$
()
]

G
$12 R$
the co-ordinates of the centre of gravity are:
$y$
2
1 E
G
R 1
[ +
2
()]
$12 R$

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- the tensor of inertia compared to $O$ is worth:
$I=$
[
]
0
$x y$
$y y$
$y z$
$4 R$
$+$
$4 R$
$x z$
$y z$
$z z$
1
2
2
E
0
0
MR. 1
[ +
]
$4 R$


## - the tensor of inertia compared to $G$ is worth:

2
2
2
E
M
1 2
2
E

R 1
$[+$
] My
R 1
[
] $M X y$
0
G
$\boldsymbol{G} \boldsymbol{G}$
2
4 -
$+$
4
$I$
$I$
$I$
$R$
$R$
$x x$
$x y$
$x z$
M
1
2
2
2
E

| $M$ |
| :--- |
| 1 |
| 2 |
| $E$ |
| $I$ |
| $I$ |
| $I=$ |
| $R 1$ |
| $[+$ |
| $] M X y$ |
| $R 1$ |
| $[$ |
| $] M x 2$ |
| 0 |
| $x y$ |
| $y y$ |
| $y z$ |
| $4 R$ |
| $G G$ |

Note:

## E

In practice, one neglects the terms in () 2 in these expressions.

## R

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## 3 Modeling

With
3.1

## Characteristics of the grid

Pa
PB
PC
PD
Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Coor_z
Pa
-1.
0.
5.0E-01

PB
-1.
0.
0.

PC
0.
-1.
5.0E-01

PD
0.
-1.
0.

Characteristics of the elements:
Types of meshs:
24 COQUE_3D
24
TRIA7
Boundary conditions:
Group meshs AB:

- displacement following the axis Y: DY = 0
- rotation around axis $X: D R X=0$
- rotation around axis $Z: D R Z=0$

Group meshs CD:

- displacement following axis $X: D X=0$
- rotation around the axis $Y: D R Y=0$
- rotation around axis $Z: D R Z=0$

Group nodes PB:

- displacement following axis $Z: D Z=0$
with $A B$ the group of meshs connecting Pa and PB
and CD that connecting PC and PD

3.2<br>Functionalities tested<br>Orders Word<br>key<br>AFFE_CHAR_MECA PRES_REP<br>FORCE_COQUE<br>AFFE_CHAR_MECA_F FORCE_COQUE<br>Handbook of Validation<br>V3.03 booklet: Linear statics of the plates and hulls<br>HT-66/03/008/A

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## 4 <br> Results of modeling $A$ <br> 4.1 Values <br> tested <br> In a systematic way, one takes the values of displacements and the constraints on node Pa

Key word Identification Reference<br>Aster Difference PRES_REP Displacements

Average layer
9.819071010
-9.49753 10-10
-3.275\%

Constraints (SIXX)

Average layer<br>194.93754<br>196.22121<br>0.659\%<br>Higher layer<br>200.125<br>248.5679<br>24.206\%<br>Lower layer<br>190.125<br>143.8745<br>-24.326\%<br>FORCE_COQUE<br>Displacements

(REALITY)
Average layer
-9.81907 10-10
-9.49753 10-10
-3.275\%

Constraints (SIXX)

Average layer<br>194.93754<br>196.22121<br>0.659\%<br>Higher layer<br>200.125<br>248.5679<br>24.206\%<br>Lower layer<br>190.125<br>143.8745<br>-24.326\%<br>FORCE_COQUE<br>Displacements

## (FUNCTION)

Average layer
-9.81907 10-10
-9.49753 10-10
-3.275\%
Constraints
(SIXX)
Average layer
194.93754
196.22121
$0.659 \%$
Higher layer
200.125
248.5679
$24.206 \%$
Lower layer
190.125
143.8745
$-24.326 \%$
Handbook of Validation

Code_Aster ${ }^{\circledR}$
Version
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Titrate:
SSLS114 - Not regression on cylindrical quarter of binding ring

## Date:

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## 5 Modeling

B

## 5.1

Characteristics of the grid
Pa
PB
PC
PD
Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Coor_z
Pa
-1.
0.
5.0E-01

PB
-1.
0.
0.

PC
0.

## -1.

5.0E-01

## PD

0. 

-1.
0.

Characteristics of the elements:
Types of meshs:
12 COQUE_3D
12
QUAD9

## Boundary conditions:

Group meshs AB:

- displacement following the axis $Y: D Y=0$
- rotation around axis $X: D R X=0$
- rotation around axis $Z: D R Z=0$

Group meshs CD:

- displacement following axis $X: D X=0$
- rotation around the axis Y: DRY $=0$
- rotation around axis $Z: D R Z=0$

Group nodes PB:

- displacement following axis $Z: D Z=0$
with $A B$ the group of meshs connecting Pa and PB and CD that connecting PC and PD

5.2<br>Functionalities tested<br>Orders Word<br>key<br>AFFE_CHAR_MECA PRES_REP<br>\section*{FORCE_COQUE}<br>AFFE_CHAR_MECA_F FORCE_COQUE<br>\section*{Handbook of Validation}<br>V3.03 booklet: Linear statics of the plates and hulls<br>HT-66/03/008/A

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6
Results of modeling B
6.1 Values
tested

Key word
Identification
Reference
Aster Difference
PRES_REP
Displacements

Average layer
9.819071010
-9.9465 10-10
1.298\%

Constraints (SIXX)

Average layer
194.93754
199.36962
2.274\%

Higher layer<br>200.125<br>204.3322<br>2.102\%<br>Lower layer<br>190.125<br>194.406<br>2.252\%<br>FORCE_COQUE<br>Displacements

(REALITY)
Average layer
-9.81907 10-10
-9.9465 10-10
1.298\%

Constraints (SIXX)

Average layer<br>194.93754<br>199.36962<br>2.274\%<br>Higher layer<br>200.125<br>204.3322<br>2.102\%<br>Lower layer<br>190.125<br>194.406<br>2.252\%<br>FORCE_COQUE<br>Displacements

(FUNCTION)
Average layer
-9.81907 10-10
-9.9465 10-10

## Constraints

(SIXX)

```
Average layer
194.93754
199.36962
2.274%
Higher layer
200.125
204.3322
2.102%
Lower layer
190.125
194.406
2.252%
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```

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## 7 Modeling <br> C <br> 7.1 <br> Characteristics of the grid

Pa
PB
PC
PD

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Coor_z
Pa
-1.
0.
5.0E-01

PB
-1.
0.
0.

PC
0.
-1.
5.0E-01

PD
0.
-1.
0.

Characteristics of the elements:
Types of meshs:
12 DKT
12
DST

## Boundary conditions:

Group meshs AB:

- displacement following the axis $Y: D Y=0$
- rotation around axis $\mathrm{X}: \mathrm{DRX}=0$
- rotation around axis $Z: D R Z=0$

Group meshs CD:

- displacement following axis $X: D X=0$
- rotation around the axis Y: DRY = 0
- rotation around axis $Z: D R Z=0$

Group nodes PB:

- displacement following axis $Z: D Z=0$
with $A B$ the group of meshs connecting Pa and PB
and CD that connecting PC and PD


## 7.2

Functionalities tested
Orders Word
key
AFFE_CHAR_MECA PRES_REP

## FORCE_COQUE

AFFE_CHAR_MECA_F FORCE_COQUE
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8
Results of the modélistaion C
8.1 Values
tested

Key word
Identification
Reference
Aster Difference
PRES_REP
Displacements

Average layer<br>9.819071010<br>-9.91444 10-10<br>$0.971 \%$<br>Constraints (SIXX)

Average layer<br>194.93754<br>198.28897<br>1.719\%<br>Higher layer<br>200.125<br>198.28897<br>-0.917\%<br>Lower layer<br>190.125<br>198.28897<br>4.294\%<br>FORCE_COQUE<br>Displacements

(REALITY)
Average layer -9.81907 10-10
-9.91444 10-10
0.971\%

Constraints (SIXX)

Average layer<br>194.93754<br>198.28897<br>1.719\%<br>Higher layer<br>200.125<br>198.28897<br>-0.917\%<br>Lower layer<br>190.125<br>198.28897<br>4.294\%<br>FORCE_COQUE<br>Displacements<br>\section*{(FUNCTION)}<br>Average layer<br>-9.81907 10-10<br>-9.91444 10-10<br>$0.971 \%$<br>Constraints<br>(SIXX)

Average layer
194.93754
198.28897
1.719\%

Higher layer
200.125
198.28897
-0.917\%
Lower layer
190.125
198.28897
4.294\%

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## 9 Modeling

D

## 9.1

Characteristics of the grid
Pa
PB
PC
$P D$

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Coor_z
Pa
-1.
0.
5.0E-01

PB
-1.
0.
0.

PC
0.
-1.
0.

Characteristics of the elements:
Types of meshs:
6 DKQ
6
DSQ

## Boundary conditions:

Group meshs AB:

- displacement following the axis $Y: D Y=0$
- rotation around axis $X: D R X=0$
- rotation around axis $Z: D R Z=0$

Group meshs CD:

- displacement following axis $X: D X=0$
- rotation around the axis $Y: D R Y=0$
- rotation around axis $Z: D R Z=0$

Group nodes PB:

- displacement following axis $Z: D Z=0$
with AB the group of meshs connecting Pa and PB and $C D$ that connecting PC and PD


## 9.2

Functionalities tested

Orders Word
key
AFFE_CHAR_MECA PRES_REP

## FORCE_COQUE

AFFE_CHAR_MECA_F FORCE_COQUE

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:
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10 Results of modeling $D$
10.1 Values
tested
Key word
Identification
Reference
Aster Difference
PRES_REP
Displacements
Average layer
9.819071010
-9.91444 10-10
$0.971 \%$
Constraints (SIXX)
Average layer194.93754
198.28897
1.719\%Higher layer200.125

## (REALITY)

Average layer
-9.81907 10-10
-9.91444 10-10
$0.971 \%$

Constraints (SIXX)

Average layer
194.93754
198.28897
1.719\%

Higher layer
200.125
198.28897
-0.917\%
Lower layer
190.125
198.28897
4.294\%

FORCE_COQUE
Displacements
(FUNCTION)
Average layer
-9.81907 10-10
-9.91444 10-10
$0.971 \%$

## Constraints

(SIXX)

Average layer<br>194.93754<br>198.28897<br>1.719\%<br>Higher layer<br>200.125<br>198.28897<br>-0.917\%<br>Lower layer<br>190.125<br>198.28897<br>4.294\%<br>\section*{Handbook of Validation}<br>V3.03 booklet: Linear statics of the plates and hulls<br>HT-66/03/008/A

Code_Aster ${ }^{\circledR}$
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6.5

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## 11 Modeling

E

### 11.1 Characteristics of the grid

PA_sup
PA_inf
PB_inf
PC_inf
PC_sup
PD_inf
PD_sup

## Co-ordinates of the points:

```
GROUP_NO
Coor_x Coor_y
Coor_z
PA_inf
-9.75E-01
0.
5.0E-01
Pa
-1.
0.
5.0E-01
PA_sup
-1.025E+00
```

$$
0 .
$$

5.0E-01
PB_inf

$$
\text { -9.75E-01 } 0 .
$$

0. 

PB
-1.
0.
0.

PB_sup
$-1.025 E+00$
0.
0.

PC_inf
0.
-9.75E-01
5.0E-01

PC
0.
-1.
5.0E-01

PC_sup
0.
$-1.025 E+00$
5.0E-01

PD_inf
0.
-9.75E-01
0.

PD
0.
-1.
0 .
PD_sup
0. $-1.025 E+00$

0 .
Characteristics of the elements:
Types of meshs:
24 meshs linear Hexa_8 3D
Boundary conditions:

## Group meshs S_AB:

- displacement following the axis $Y: D Y=0$
- displacement following axis $Z: D Z=0$

Group meshs S_CD:

- displacement following axis $X: D X=0$
- displacement following axis $Z: D Z=0$


### 11.2 Functionalities tested

Orders Word<br>key<br>AFFE_CHAR_MECA PRES_REP<br>AFFE_CHAR_MECA_F PRES_REP<br>Handbook of Validation<br>V3.03 booklet: Linear statics of the plates and hulls<br>HT-66/03/008/A

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6.5

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12 Results of modeling $E$
12.1 Values
tested

Key word
Identification
Reference
Aster Difference

PRES_REP<br>Displacements

(REALITY)<br>Average layer<br>-9.81907 10-10 -9.67127098 10-10<br>-1.505\%<br>Lower layer<br>-9.90234 10-10 -9.78335202 10-10<br>-1.202\%<br>Higher layer<br>-9.81907 10-10 -9.567175895 10-10<br>-2.565\%

## Constraints (SIYY)

Average layer
194.93754
211.10227
8.292\%

Lower layer
200.125
217.9814
8.923\%

Higher layer
190.125
204.62504
7.627\%

PRES_REP
Displacements
(FUNCTION)
Average layer
-9.81907 10-10 -9.671270 10-10
-1.505\%
Lower layer
-9.90234 10-10 -9.78335202 10-10
-1.202\%
Higher layer

## Constraints (SIYY)

```
Average layer
194.93754
211.10227
8.292%
Lower layer
200.125
217.98146
8.923%
Higher layer
190.125
204.62504
7.627%
Handbook of Validation
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Code_Aster }\mp@subsup{}{}{\circledR
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```


## Titrate:

```
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```


## 13 Modeling <br> F

13.1 Characteristics of the grid

```
PA_sup
PA_inf
PB_inf
PC_inf
PC_sup
PD_inf
PD_sup
```

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Coor_z
PA_inf
-9.75E-01
0.
5.0E-01

Pa
-1.
0.
5.0E-01

PA_sup
$-1.025 E+00$
0 .
5.0E-01

PB_inf
-9.75E-01 0.
0.

PB
-1.
0.
0.

PB_sup
$-1.025 E+00$
0.
0.

PC_inf
0.
-9.75E-01
5.0E-01

PC
0.
-1.

```
5.0E-01
PC_sup
O.
-1.025E+00
5.0E-01
PD_inf
O.
-9.75E-01
0.
PD
0.
-1.
0.
PD_sup
0. -1.025E+00
0.
```

Characteristics of the elements:
Types of meshs: 48 linear meshs PENTA6 3D

Boundary conditions:
Group meshs S_AB:

- displacement following the axis $\boldsymbol{Y}: \mathbf{D Y}=\mathbf{0}$
- displacement following axis Z: DZ $=0$

Group meshs S_CD:

- displacement following axis X: DX $=0$
- displacement following axis Z: DZ $=0$


### 13.2 Functionalities tested

Orders Word<br>key<br>AFFE_CHAR_MECA PRES_REP<br>AFFE_CHAR_MECA_F PRES_REP<br>Handbook of Validation<br>V3.03 booklet: Linear statics of the plates and hulls HT-66/03/008/A

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## 14 Results of modeling $F$

### 14.1 Values

tested

Key word
Identification
Reference
Aster Difference
PRES_REP
Displacements

## (REALITY)

Average layer
9.819071010
-9.454821 10-10
-3.710\%
Lower layer
9.902341010
-9.5731464 10-10
-3.324\%
Higher layer
9.819071010
-9.34578869 10-10
-4.820\%

## Constraints (SIYY)

Average layer<br>194.93754<br>193.619011<br>-0.676\%<br>Lower layer 200.125<br>200.724933<br>0.300\%<br>Higher layer<br>190.125<br>186.858348<br>-1.718\%<br>PRES_REP<br>Displacements<br>\section*{(FUNCTION)}<br>Average layer<br>9.819071010<br>-9.454821 10-10<br>-3.710\%<br>Lower layer<br>9.902341010<br>-9.5731464 10-10<br>-3.324\%<br>Higher layer<br>9.819071010<br>-9.34578869 10-10<br>-4.820\%<br>Constraints (SIYY)

Average layer
194.93754
193.619011
-0.676\%
Lower layer
200.125
200.724933

```
0.300%
Higher layer
190.125
186.858348
-1.718%
Handbook of Validation
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```


## 15 Modeling

G
15.1 Characteristics of the grid
PA_sup
PA_inf
PB_inf
PC_inf
PC_sup
PD_inf
PD_sup

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y

```
Coor_z
PA_inf
-9.75E-01
0.
5.0E-01
Pa
-1.
0.
5.0E-01
PA_sup
-1.025E+00
0.
5.0E-01
PB_inf
-9.75E-01 0.
0.
PB
-1.
0.
0.
PB_sup
-1.025E+00
0.
0.
PC_inf
0.
-9.75E-01
5.0E-01
PC
0.
-1.
5.0E-01
PC_sup
O.
-1.025E+00
5.0E-01
PD_inf
0.
-9.75E-01
0.
PD
0.
-1.
```

24 quadratic meshs HEXA_20 3D
Boundary conditions:
Group meshs S_AB:

- displacement following the axis $Y: D Y=0$
- displacement following axis $Z: D Z=0$

Group meshs S_CD:

- displacement following axis $X$ : $D X=0$
- displacement following axis $Z: D Z=0$


### 15.2 Functionalities tested

## Orders Word

key
AFFE_CHAR_MECA PRES_REP
AFFE_CHAR_MECA_F PRES_REP
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
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Code_Aster ${ }^{\circledR}$
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# 16 Result of modeling G 

16.1 Values

tested

Key word<br>Identification<br>Reference<br>Aster Difference<br>PRES_REP<br>Displacements

(REALITY)
Average layer
9.819071010
-9.6611705 10-10
-1.608\%
Lower layer
9.902341010
-9.7808343 10-10
-1.227\%
Higher layer
9.819071010
-9.5508891 10-10
-2.731\%

## Constraints (SIYY)

Average layer
194.93754
219.59661
12.650\%

Lower layer
200.125
226.83032
13.344\%

Higher layer

190.125<br>211.09036<br>11.027\%<br>PRES_REP<br>Displacements

(FUNCTION)<br>Average layer<br>9.819071010<br>-9.6611705 10-10<br>-1.608\%<br>Lower layer<br>9.902341010<br>-9.7808343 10-10<br>-1.227\%<br>Higher layer<br>9.819071010<br>-9.5508891 10-10<br>-2.731\%<br>Constraints (SIYY)

Average layer
194.93754
219.59661
$12.650 \%$
Lower layer
200.125
226.83032
$13.344 \%$
Higher layer
190.125
211.09036
$11.027 \%$

Handbook of Validation
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## Titrate: <br> SSLS114 - Not regression on cylindrical quarter of binding ring

Date:
03/11/03
Author (S):
P. Key MASSIN, J.R. LEVESQUE

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## 17 Modeling

H
17.1 Characteristics of the grid

```
PA_sup
PA_inf
PB_inf
PC_inf
PC_sup
PD_inf
PD_sup
```

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Coor_z
PA_inf
-9.75E-01
0.
5.0E-01

Pa
-1.
0.
5.0E-01

PA_sup
$-1.025 E+00$

$$
0 .
$$

5.0E-01
PB_inf

$$
\text { -9.75E-01 } 0 .
$$

0. 

PB
-1.
0.
0.

PB_sup
$-1.025 E+00$
0.
0.

PC_inf
0.
-9.75E-01
5.0E-01

PC
0.
-1.
5.0E-01

PC_sup
0.
$-1.025 E+00$
5.0E-01

PD_inf
0.
-9.75E-01
0.

PD
0.
-1.
0 .
PD_sup
0. $-1.025 E+00$

0 .
Characteristics of the elements:
Types of meshs: 48 quadratic meshs PENTA15 3D

Boundary conditions:

## Group meshs S_AB:

- displacement following the axis $Y: D Y=0$
- displacement following axis $Z: D Z=0$

Group meshs S_CD:

- displacement following axis $X: D X=0$
- displacement following axis $Z: D Z=0$
17.2 Functionalities tested

Orders Word<br>key<br>AFFE_CHAR_MECA PRES_REP<br>AFFE_CHAR_MECA_F PRES_REP<br>Handbook of Validation<br>V3.03 booklet: Linear statics of the plates and hulls<br>HT-66/03/008/A

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18 Results of modeling $\boldsymbol{H}$
18.1 Values
tested

Key word
Identification
Reference
Aster Difference

PRES_REP<br>Displacements

(REALITY)<br>Average layer<br>9.819071010<br>-9.6609841 10-10<br>-1.610\%<br>Lower layer<br>9.902341010<br>-9.787884 10-10<br>-1.156\%<br>Higher layer<br>9.819071010<br>-9.5539546 10-10<br>-2.700\%<br>Constraints (SIYY)

Average layer
194.93754
214.780477
10.179\%

Lower layer
200.125
242.54015
21.194\%

Higher layer
190.125
186.1681
-2.081\%
PRES_REP
Displacements

## (FUNCTION)

Average layer
9.819071010
-9.6609841 10-10
-1.610\%

Lower layer
9.902341010
-9.787884 10-10
-1.156\%
Higher layer
9.819071010
-9.5539546 10-10
-2.700\%
Constraints (SIYY)

Average layer
194.93754
214.780477
$10.179 \%$
Lower layer
200.125
242.54015
$21.194 \%$
Higher layer
190.125
186.1681
$-2.081 \%$
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## 19 Modeling

I

### 19.1 Characteristics of the grid

Pa

PB

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Pa
1.0
0.5

PB
1.00.

Characteristics of the elements:
Types of meshs:
2 COQUE_AXI
Thus R1=1.025 $m$ and $R 2=0.975 \mathrm{Mr}$.
$h=0.5 \mathrm{~m}$
For this modeling and the following ones, one specifies the density $=1234 . \mathrm{kg} / \mathrm{m} 3$
Boundary conditions:
Group nodes PB: displacement following the axis $Y: D Y=0$
19.2 Functionalities tested
Orders Word
key
AFFE_CHAR_MECA PRES_REP
FORCE_COQUE
AFFE_CHAR_MECA_F FORCE_COQUE
Handbook of Validation
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20 Results of modeling I
20.1 Values
tested
Key word
Identification
Reference
Aster Difference
PRES_REP
Displacements
Average layer
-9.81907 10-10

## -1.000 10-9

1.843\%

## Constraints (SIYY)

Average layer<br>194.93754<br>200.000<br>2.597\%

## FORCE_COQUE <br> Displacements

(REALITY) with<br>Average layer<br>-9.81907 10-10<br>-1.000 10-9<br>1.843\%<br>MODI_METRIQUE

## Constraints (SIYY)

Average layer<br>194.93754<br>199.954<br>2.573\%<br>Higher layer<br>200.125<br>205.588<br>2.730\%<br>Lower layer<br>190.125<br>194.595<br>2.351\%<br>FORCE_COQUE

## Displacements

(FUNCTION) with
Average layer
-9.81907 10-10
-1.000 10-9
1.843\%

MODI_METRIQUE

Constraints (SIYY)

Average layer<br>194.93754<br>199.954<br>2.573\%<br>Higher layer<br>200.125<br>205.588<br>2.730\%<br>Lower layer<br>190.125<br>194.595<br>$2.351 \%$<br>FORCE_COQUE<br>Displacements

(FUNCTION) without
Average layer
-9.81907 10-10
-1.000 10-9
$1.843 \%$
MODI_METRIQUE
Constraints (SIYY)

Average layer
194.93754
MASS
$1.93836 E+02$
$1.93836 E+02$
$C D G_{-} X$
0.0
0.0
$C D G_{-} Y$
0.0
0.0
$C D G \_Z$
$2.5 E-01$
$2.5 E-01$
$I X_{-} G$
$1.00956 E+02$
$1.00956 E+02$
$I Y_{-} G$
$1.00956 E+02$
$1.00956 E+02$
$I Z_{-} G$
$1.93836 E+02$
$1.93836 E+02$
$I X Y_{-} G$
0.0
0.0
$I X Z_{-} G$
0.0
0.0
$I Y Z_{-} G$
0.0
0.0

## Handbook of Validation

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## 21 Modeling

$J$

### 21.1 Characteristics of the grid

Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Pa
1.0
0.

PC
0. 1.

## Characteristics of the elements:

Types of meshs:
10 COQUE_C_PLAN
R = 1 Mr .
$=1234 . \mathrm{Kg} / \mathrm{m} 3$

## Boundary conditions:

Group nodes Pa:

- displacement following the axis $Y=0$
- rotation around axis $Z: D R Z=0$

Group nodes PC:

- displacement following axis X: DX =0
- rotation around axis $\mathrm{Z}: \mathrm{DZ}=0$
21.2 Functionalities tested

Orders Word
key
AFFE_CHAR_MECA PRES_REP

## FORCE_COQUE

AFFE_CHAR_MECA_F FORCE_COQUE

## Handbook of Validation

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22 Results of modeling J
22.1 Values
tested

Key word
Identification
Reference

## Aster Difference <br> PRES_REP <br> Displacements

Average layer<br>-9.81907 10-10<br>-9.999974 10-10<br>1.842\%

Constraints (SIXX)

Average layer 194.93754
200.000
2.597\%

## FORCE_COQUE

Displacements

## (REALITY) with

Average layer
-9.81907 10-10
-9.999974 10-10
1.842\%

MODI_METRIQUE

## Constraints (SIXX)

Average layer
194.93754
199.959
2.576\%

Higher layer

# 200.125 

205.046
2.459\%

Lower layer
190.125
195.118
2.626\%

FORCE_COQUE
Displacements

## (FUNCTION) with

Average layer
-9.81907 10-10
-9.999974 10-10
1.842\%

MODI_METRIQUE

## Constraints (SIXX)

Average layer<br>194.93754<br>199.959<br>2.576\%<br>Higher layer<br>200.125<br>205.046<br>2.459\%<br>Lower layer<br>190.125<br>195.118<br>2.626\%<br>FORCE_COQUE<br>Displacements

(FUNCTION) without
Average layer
-9.81907 10-10
-9.999974 10-10

```
1.842%
MODI_METRIQUE
```


## Constraints (SIXX)

Average layer<br>194.93754<br>200.000<br>2.597\%<br>Higher layer<br>200.125<br>200.000<br>-0.062\%<br>Lower layer<br>190.125<br>200.000<br>5.194\%<br>MASS<br>9.69181 E+01<br>9.69181 E+01<br>CDG_X<br>6.36619 E-01<br>6.36619 E-01<br>CDG_Y<br>6.36619 E-01<br>6.36619 E-01<br>CDG_Z<br>0.0<br>0.0<br>IX_G<br>9.17961<br>9.17961<br>IY_G<br>9.17961<br>9.17961<br>IZ_G<br>$1.83592 E+01$<br>$1.83592 E+01$<br>IXY_G<br>-8.42942

-8.42942
IXZ_G
0.0
0.0
IYZ_G
0.0
0.0
Handbook of Validation
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Code_Aster ${ }^{\circledR}$
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Date:
03/11/03
Author (S):
P. Key MASSIN, J.R. LEVESQUE

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23 Modeling
K
23.1 Characteristics of the grid
Co-ordinates of the points:
GROUP_NO
Coor_x Coor_y
Pa1.0
0 .
PC
0. 1.

Characteristics of the elements:
Types of meshs:
10 COQUE_D_PLAN
R = 1 Mr .
$=1234 . \mathrm{Kg} / \mathrm{m} 3$

## Boundary conditions:

## Group nodes Pa:

- displacement following the axis $Y=0$
- rotation around axis $\mathrm{Z}: \mathrm{DRZ}=\mathbf{0}$

Group nodes PC:

- displacement following axis $\mathrm{X}: \mathrm{DX}=0$
- rotation around axis $\mathrm{Z}: \mathrm{DZ}=0$


### 23.2 Functionalities tested

\author{
Orders Word <br> key <br> AFFE_CHAR_MECA PRES_REP <br> ```
FORCE_COQUE <br> AFFE_CHAR_MECA_F FORCE_COQUE

```
}

\author{
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}

Code_Aster \({ }^{\circledR}\)
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Author (S):
P. Key MASSIN, J.R. LEVESQUE

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\section*{24 Results of modeling \(K\)}

\author{
24.1 Values \\ tested
}

Key word
Identification
Reference
Aster Difference
PRES_REP
Displacements

\author{
Average layer \\ -9.81907 10-10 \\ -9.099974 10-10 \\ -7.323\% \\ Constraints (SIXX)
}

Average layer
194.93754
200.000
2.597\%

\section*{FORCE_COQUE \\ Displacements}
(REALITY) with
Average layer
-9.81907 10-10
-9.099974 10-10

\title{
-7.323\%
}

\section*{MODI_METRIQUE}

Constraints (SIXX)

\author{
Average layer \\ 194.93754 \\ 199.959 \\ 2.576\% \\ Higher layer \\ 200.125 \\ 205.046 \\ 2.459\% \\ Lower layer \\ 190.125 \\ 195.118 \\ 2.627\% \\ FORCE_COQUE \\ Displacements
}
(FUNCTION) with
Average layer
-9.81907 10-10
-9.099974 10-10
-7.323\%
MODI_METRIQUE

Constraints (SIXX)

Average layer
194.93754
199.959
2.576\%

Higher layer
200.125
205.046
2.459\%

\author{
Lower layer
}
190.125
195.118
2.627\%

FORCE_COQUE
Displacements

\section*{(FUNCTION) without}

Average layer
-9.81907 10-10
-9.099974 10-10
-7.323\%
MODI_METRIQUE
Constraints (SIXX)

Average layer
194.93754
200.000
2.597\%

Higher layer
200.125
200.000
-0.062\%
Lower layer
190.125
200.000
5.194\%

\section*{MASS}
9.69181 E+01
9.69181 E+01

CDG_X
6.36619 E-01
6.36619 E-01

CDG_Y
6.36619 E-01
6.36619 E-01

CDG_Z
0.0
```

0 . 0
IX_G
9 . 1 7 9 6 1
9 . 1 7 9 6 1
IY_G
9.17961
9 . 1 7 9 6 1
IZ_G
1.83592 E+01
1.83592 E+01
IXY_G
-8.42942
-8.42942
IXZ_G
0.0
0 . 0
IYZ_G
0.0
0 . 0

```

\section*{Handbook of Validation}
```

V3.03 booklet: Linear statics of the plates and hulls HT-66/03/008/A

```

Code_Aster \({ }^{\circledR}\)
Version
6.5

Titrate:
SSLS114 - Not regression on cylindrical quarter of binding ring
Date:
03/11/03
Author (S):
P. Key MASSIN, J.R. LEVESQUE
.
V3.03.114-A Page:
28/28

25 Summary of the results
The key words PRES_REP (real or function) and FORCE_COQUE (real or function) can be indifferently used for the elements of hull and plate, the results obtained coincide.

\title{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
1/24
Organization (S): EDF-R \& D /AMA, CS IF

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
Document: V3.03.115

SSLS115-Composite square plate under pressure uniform

\section*{Summary:}

One treats the case of a square plate tri-layers, simply supported and subjected to a uniform pressure. The skins consist of an orthotropic homogeneous material, as well as the heart (same axes of orthotropism).
The modules \(\boldsymbol{E}\) and \(\boldsymbol{G}\) of the heart are ten times weaker than those of the skins.
One calculates displacement in the center as well as the constraints with the lower and higher interfaces of skins.

The test gathers eight modelings: with regard to the four first, the results obtained are compared for triangular surface meshs then quadrangular, in two reference marks user different. Four last modelings make it possible to measure the sensitivity of the results the orientation triangular meshs in the two reference marks user.

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
2/24

\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

Because of the geometrical and physical symmetry of the problem, only the quarter of the plate is
modelled.
\(L\)
\(L\)
With
\(=500 \mathrm{~mm}\)
2
2
\(B\)
\(D\)
\(Z\)
\(y\)
\(X\)
\(H=100 \mathrm{~mm}\)
\(C\)
10 mm
Material 1
Material 2
80 mm
Material 1
10 mm

L
Twinge:
= 10: the plate is relatively thick.
H
1.2

Material properties

\author{
Material 1 \\ Material 2 \\ \(E_{-} L\left(1011 \mathrm{~N} / \mathrm{m}^{2}\right)\) \\ 3.4156 \\ 0.34156 \\ \(E_{-} T\left(1011 \mathrm{~N} / \mathrm{m}^{2}\right)\) \\ 1.793 \\ 0.1793 \\ \(G_{-} L N\left(1011 \mathrm{~N} / \mathrm{m}^{2}\right)\) \\ 0.608 \\ 0.0608 \\ \(G_{-} T N\left(1011 ~ N / m^{2}\right)\) \\ 1.015
}
```

0.1015
G_LT (1011 N/m}\mp@subsup{}{}{2}
1.0
0.1
NU_T
0.44
0.44

```

\section*{Handbook of Validation}
```

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A

```

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
3/24

\section*{1.3}

Boundary conditions and loadings

Simple bearing plate
C.L. :

AB:
\(D Z=0\).
\(D R Y=0\).
```

AD:
$D Z=0$.
$D R X=0$.

```

Symmetry
BC:
\(D X=0\).
\(D R Y=0\).

DRZ=0.
CD:
\(D Y=0\).
\(D R X=0 . D R Z=0\).
Loading:
FORCE_COQUE
Uniform pressure
\(P=1 N / m^{2}\)

\section*{2}

Reference solution

\section*{2.1 \\ Reference solution}

The numerical solution obtained with a theory of plate multi-layer in linear elasticity orthotropic is given in the reference [bib1] page 341.

\section*{2.2 \\ Results of reference \\ At the point C, one calculates displacement according to \(Z\) of the point as well as the constraints with the interfaces \\ \(X\) \\ lower and higher of the skins.}

\section*{2.3 \\ Uncertainty on the solution}

Numerical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
BATOZ and DHATT. Modeling of the structures by finite elements. Beams and plates. Hermès, 1990.

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates

\section*{HT-66/02/001/A}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
Element of hull DST (modeling of a quarter of plate).
The reference mark user is confused with the reference mark of orthotropism.

\section*{D \\ C \\ With \\ B}

Limiting conditions:
DDL_IMPO
(GROUP_NO=' AB', DZ=0., DRY=0.)
(GROUP_NO=' BC', \(D X=0 ., D R Y=0\).
(GROUP_NO=' CD', DY=0., \(D R X=0\).
(GROUP_NO=' DA', \(D Z=0 ., D R X=0\).
Not C
net: 72

\section*{3.2}

Characteristics of the grid

\section*{A number of nodes: 56}

\section*{A number of meshs and types: 72 TRIA3}

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
5/24

\subsection*{3.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE "MECHANICAL" "DST"
}

\author{
MODI_MAILLAGE \\ ORIE_NORME_COQUE
}

\author{
AFFE_CARA_ELEM
}

HULL
THICK

\author{
ANGLE_REP
}

\author{
DEFI_MATERIAU \\ ELAS_ORTH \\ DEFI_COQU_MULT \\ SLEEP \\ THICK \\ ORIENTATION \\ AFFE_CHAR_MECA \\ FORCE_COQUE \\ NEAR \\ ALL \\ DDL_IMPO \\ GROUP_NO \\ MECA_STATIQUE \\ CALC_CHAM_ELEM \\ SIGM_ELNO_DEPL
}
```

4
Results of modeling $A$

```

\author{
4.1 Values
}
tested
Not C
Identification
Reference
Aster \%
Difference
on lower layer 3
\(4.7100 E+014.7662 E+01\)
1.194
\(X\)
on higher layer 3
\(5.8800 E+015.9577 E+01\)
1.323
\(X\)
Constraints
on lower layer 2
4.7100E+01 4.7662E+01
1.194
\(X\)
on higher layer 2
4.7100E+01 4.7662E+01
1.194
\(X\)
on lower layer 1
\(5.8800 E+015.9577 E+01\)
1.323
X
on higher layer 1 4.7100E+01 4.7662E+01
1.194
X
DX
0.00 .00 .0
Displacement DY
0.00 .00 .0
DZ
4.1920E+01 4.1851E+01
-0.163
4.2
Contents of the file results
Values at the point of observation of displacements and constraints.
\(X\)
4.3 Parametersof execution
Version: 6.0.29

\author{
Obstruction memory: \\ 32 Mo \\ Time CPU To use: 5.74 seconds \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key

V3.03.115-A Page:
6/24

\section*{5 Modeling}

B

\section*{5.1 \\ Characteristics of modeling}

Element of hull DST (modeling of a quarter of plate).
The reference mark user is confused with the reference mark of orthotropism.
```

D
C
With
B
Limiting conditions:
DDL_IMPO
(GROUP_NO='AB', DZ=0., $D R Y=0$.)
(GROUP_NO='BC', DX=0., DRY=0.)
(GROUP_NO='CD', $D Y=0 ., D R X=0$.)
(GROUP_NO=' DA', DZ=0., $D R X=0$.

```

Not C
net: 36

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 57
A number of meshs and types: 36 QUAD4

\author{
Handbook of Validation
}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
7/24

\subsection*{5.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE \\ "MECHANICAL" \\ "DST"
}

MODI_MAILLAGE
ORIE_NORME_COQUE
AFFE_CARA_ELEM
HULL
THICK
ANGLE_REP
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP

\section*{THICK}

\section*{ORIENTATION}

\author{
AFFE_CHAR_MECA \\ FORCE_COQUE \\ NEAR \\ ALL \\ DDL_IMPO \\ GROUP_NO \\ MECA_STATIQUE \\ CALC_CHAM_ELEM \\ SIGM_ELNO_DEPL
}

\section*{6}

Results of modeling B

\subsection*{6.1 Values}
tested

\author{
Not C \\ Identification \\ Reference \\ Aster \% \\ Difference
}
on lower layer 3
\(4.7100 E+015.0881 E+01\)
8.028

X
on higher layer 3
\(5.8800 E+016.3601 E+01\)
8.166

X
Constraints
on lower layer 2
\(4.7100 E+015.0881 E+01\)
8.028

\section*{\(X\)}
on higher layer 2
4.7100E+01 5.0881E+01
8.028

X
on lower layer 1
\(5.8800 E+016.3601 E+01\)
8.166

X
on higher layer \(14.7100 E+015.0881 E+01\)
8.028

X
DX
0.00 .00 .0

Displacement DY
0.00 .00 .0

DZ
4.1920E+01 4.2040E+01
0.29

\section*{6.2}

Contents of the file results
Values at the point of observation of displacements and constraints.
\(X\)

\subsection*{6.3 Parameters}
of execution
Version: 6.0.29

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 4.25 seconds
Handbook of Validation

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
8/24

\section*{7 Modeling}

C

\section*{7.1}

\section*{Characteristics of modeling}

Element of hull DST (modeling of a quarter of plate).
The model of plate associated with modeling \(A\) is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to beta.
C
D
B
With

Limiting conditions:
LIAISON_OBLIQUE
(GROUP_NO='AB', ANGL_NAUT=(20., 30. , 0.), \(D Z=0 ., D R Y=0\).
(GROUP_NO=' BC', ANGL_NAUT= (20. , 30. , 0.), \(D X=0 ., D R Y=0\).
(GROUP_NO=' CD', ANGL_NAUT= (20. , 30. , 0.), \(D Y=0 ., D R X=0\).)
(GROUP_NO=' DA', ANGL_NAUT \(=(20 ., 30 ., 0),. D Z=0 ., D R X=0\).
Not C
net: 72

\section*{7.2}

\section*{Characteristics of the grid}

A number of nodes: 56
A number of meshs and types: 72 TRIA3
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
9/24

\subsection*{7.3 Functionalities \\ tested}

Orders

\author{
AFFE_MODELE \\ "MECHANICAL" \\ "DST"
}

MODI_MAILLAGE
ORIE_NORME_COQUE
AFFE_CARA_ELEM
HULL
THICK

\author{
ANGLE_REP
}

DEFI_MATERIAU
```

ELAS_ORTH

```
DEFI_COQU_MULT
SLEEP
THICK
ORIENTATION
AFFE_CHAR_MECA
FORCE_COQUE
NEAR
ALL
LIAISON_OBLIQUE
ANGLE_NAUT
GROUP_NO
MECA_STATIQUE
CALC_CHAM_ELEM
SIGM_ELNO_DEPL

\section*{8}

\section*{Results of modeling \(C\)}

\subsection*{8.1 Values}
tested
Not C
Identification
Reference
Aster \%
Difference
on lower layer 3
4.7100E+01 4.7662E+01
1.194

X
on higher layer 3
\(5.8800 E+015.9577 E+01\)
1.323
XConstraints
on lower layer 2
\(4.7100 E+014.7662 E+01\)
1.194
X
on higher layer 2
4.7100E+01 4.7662E+01
1.194
X
on lower layer 1
\(5.8800 E+015.9577 E+01\)
1.323
X
on higher layer 1
4.7100E+01 4.7662E+01
1.194
X
DX
1.9696E+01 1.9663E+01
-0.163
Displacement DY
7.1687E+00 7.1570E+00
-0.162DZ
\(3.6304 E+013.6244 E+01\)
-0.163

\subsection*{8.2 Remarks}

The values of reference of displacement to the point \(C\) are obtained by projecting displacement theoretical bench for a plate not turned in the new reference mark user (displacement for a not turned plate being vertical, new displacement is a function of the projection of the axis \(Z\) ). In the local reference mark, the projection of axis \(Z\) is as follows:
\(\sin \cos\)
sin \(\sin\), with \(=20\). and \(=\)

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

\section*{Titrate:}

SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
10/24

\section*{8.3 \\ Contents of the file results \\ Values at the point of observation of displacements and constraints. \\ X}
8.4 Parameters
of execution
Version: 6.0.29

Machine: SGI - ORIGIN 2000

\section*{Obstruction memory:}

32 Mo
Time CPU To use: 4.52 seconds

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key

V3.03.115-A Page:
11/24

\section*{9 Modeling}

D

\section*{9.1}

Characteristics of modeling
Element of hull DST (modeling of a quarter of plate).
The model of plate associated with modeling \(B\) is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to beta.

\section*{C}

D
B
With

\section*{Limiting conditions:}

LIAISON_OBLIQUE
(GROUP_NO=' AB', ANGL_NAUT= (20. , 30. , 0.), \(D Z=0 ., D R Y=0\).
(GROUP_NO=' BC', ANGL_NAUT \(=(20 ., 30 ., 0),. D X=0 ., D R Y=0\).
(GROUP_NO=' CD', ANGL_NAUT \(=(20 ., 30 ., 0),. D Y=0 ., D R X=0\).
(GROUP_NO=' DA', ANGL_NAUT = (20. , 30. , 0.), DZ=0., DRX=0.)
Not C
net: 36

Characteristics of the grid
A number of nodes: 57
A number of meshs and types: 36 QUAD4
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
12/24

\subsection*{9.3 Functionalities}
tested
Orders

AFFE_MODELE
"MECHANICAL"
"DST"

\section*{MODI_MAILLAGE}

ORIE_NORME_COQUE

\section*{AFFE_CARA_ELEM}

HULL
THICK
ANGLE_REP

\author{
DEFI_MATERIAU \\ ELAS_ORTH \\ DEFI_COQU_MULT \\ SLEEP \\ THICK \\ ORIENTATION \\ AFFE_CHAR_MECA \\ FORCE_COQUE \\ NEAR \\ ALL \\ LIAISON_OBLIQUE \\ ANGLE_NAUT \\ GROUP_NO \\ MECA_STATIQUE \\ CALC_CHAM_ELEM \\ SIGM_ELNO_DEPL
}

10 Results of modeling \(D\)
10.1 Values
tested
Not C
Identification
Reference
Aster \%
Difference
on lower layer 3
4.7100E+01 5.0881E+01
8.028
\(X\)
on higher layer \(35.8800 E+016.3601 E+01\)
8.166
\(X\)

\section*{Constraints}
on lower layer 2
\(4.7100 E+015.0881 E+01\)
8.028

X
on higher layer 2 4.7100E+01 5.0881E+01
8.028

X
on lower layer 1
\(5.8800 E+016.3601 E+01\)
8.166

X
on higher layer \(14.7100 E+015.0881 E+01\)
8.028

X
DX
\(1.9696 E+011.9750 E+01\)
0.290

Displacement DY
7.1687E+00 7.1895E +00
0.291

DZ
3.6304E+01 3.6409E+01
0.289

\subsection*{10.2 Remarks}

The values of reference of displacement to the point \(C\) are obtained by projecting displacement theoretical bench for a plate not turned in the new reference mark user (displacement for a not turned plate being vertical, new displacement is a function of the projection of the axis \(Z\) ). In the local reference mark, the projection of axis \(Z\) is as follows:
\(\sin \cos\)
sin \(\sin\), with \(=20\). and \(=\)
30

\title{
Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
13/24
10.3 Contents of the file results

Values at the point of observation of displacements and constraints.
\(X\)
10.4 Parameters
of execution
Version: 6.0.29

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 6.11 seconds

\section*{11 Modeling}

E
11.1 Characteristics of modeling

Element of hull DST (modeling of a quarter of plate).
The reference mark user is confused with the reference mark of orthotropism. Compared to modeling \(A\), the model is characterized here by an orientation different from the surface meshs.

\author{
D \\ C \\ With \\ B
}

\section*{Limiting conditions:}

DDL_IMPO
(GROUP_NO=' \(\left.A B^{\prime}, D Z=0 ., D R Y=0.\right)\)
(GROUP_NO=' BC', DX=0., DRY=0.)
(GROUP_NO=' CD', DY=0., \(D R X=0\).
(GROUP_NO=' DA', DZ=0., DRX=0.)
Not C
net: 72

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key

V3.03.115-A Page:
14/24
11.2 Characteristics of the grid

A number of nodes: 56
A number of meshs and types: 72 TRIA3

\subsection*{11.3 Functionalities}

\author{
AFFE_MODELE "MECHANICAL" "DST"
}

\author{
MODI_MAILLAGE \\ ORIE_NORME_COQUE
}

\author{
AFFE_CARA_ELEM
}

HULL
THICK
ANGLE_REP
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
ORIENTATION
AFFE_CHAR_MECA
FORCE_COQUE
NEAR
ALL
DDL_IMPO
GROUP_NO
MECA_STATIQUE
CALC_CHAM_ELEM
SIGM_ELNO_DEPL

\section*{12 Results of modeling \(E\)}

\subsection*{12.1 Values}
tested

\author{
Not C Identification \\ Reference \\ Aster \% \\ Difference \\ on lower layer 3 \\ 4.7100E+01 5.2430E+01 \\ 11.317 \\ \(X\)
}
on higher layer \(35.8800 E+016.5537 E+0111.459\)
\(X\)
Constraints
on lower layer 2
4.7100E+01 5.2430E+01
11.317

X
on higher layer 2 4.7100E \(+015.2430 E+0111.317\)
\(X\)
on lower layer 1
\(5.8800 E+016.5537 E+01\)
11.459
\(X\)
on higher layer \(14.7100 E+015.2430 E+01\)
11.317
\(X\)
DX
0.00 .00 .0

Displacement DY
0.00 .00 .0

DZ
4.1920E+01 4.2024E+01
0.248
12.2 Contents of the file results

Values at the point of observation of displacements and constraints.
\(X\)
12.3 Parameters
of execution
Version: 6.0.29

Machine: SGI - ORIGIN 2000

\section*{Obstruction memory:}

32 Mo
Time CPU To use: 4.14 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
15/24

13 Modeling
F

\subsection*{13.1 Characteristics of modeling}

Element of hull DST (modeling of a quarter of plate).
The model of plate associated with modeling \(E\) is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to beta. Compared to modeling C, the model is here characterized by an orientation different from the meshs.
```

C
D
B
With

```

\section*{Limiting conditions:}
(GROUP_NO=' AB', ANGL_NAUT= (20. , 30. , 0.), \(D Z=0 ., D R Y=0\).

(GROUP_NO=' CD', ANGL_NAUT \(=(20 ., 30 ., 0),. D Y=0 ., D R X=0\).
(GROUP_NO=' DA', ANGL_NAUT = (20. , 30. , 0.), DZ=0., DRX=0.)
Not C
net: 72

\subsection*{13.2 Characteristics of the grid}

A number of nodes: 56
A number of meshs and types: 72 TRIA3
Handbook of Validation
V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
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\author{
13.3 Functionalities \\ tested \\ Orders
}

\author{
AFFE_MODELE \\ "MECHANICAL" \\ "DST" \\ MODI_MAILLAGE \\ ORIE_NORME_COQUE \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ ANGLE_REP \\ DEFI_MATERIAU \\ ELAS_ORTH \\ DEFI_COQU_MULT \\ SLEEP \\ THICK \\ ORIENTATION
}

\author{
AFFE_CHAR_MECA \\ FORCE_COQUE \\ NEAR \\ ALL \\ LIAISON_OBLIQUE \\ ANGLE_NAUT \\ GROUP_NO \\ MECA_STATIQUE \\ CALC_CHAM_ELEM \\ SIGM_ELNO_DEPL
}

\section*{14 Results of modeling \(F\)}

\subsection*{14.1 Values}
tested

\author{
Not C Identification \\ Reference \\ Aster \% \\ Difference
}
on lower layer 3
4.7100E+01 5.2430E+01
11.317
\(X\)
on higher layer 3
5.8800E+01 6.5537E+01
11.459

X
Constraints
on lower layer 2
4.7100E+01 5.2430E+01
11.317
\(X\)
on higher layer 2

\subsection*{4.7100E \(+015.2430 E+01\)}
11.317
\(X\)
on lower layer 1
\(5.8800 E+016.5537 E+01\)
11.459
\(X\)
on higher layer 1
4.7100E+01 5.2430E+01
11.317
\(X\)
DX
\(1.9696 E+011.9744 E+01\)
0.248

Displacement DY
\(7.1687 E+007.1865 E+00\)
0.249

DZ
3.6304E+01 3.6393E+01
0.248

\subsection*{14.2 Remarks}

The values of reference of displacement to the point \(C\) are obtained by projecting displacement theoretical bench for a plate not turned in the new reference mark user (displacement for a not turned plate being vertical, new displacement is a function of the projection of the axis \(Z\) ). In the local reference mark, the projection of axis \(Z\) is as follows:
\(\sin \cos\)
sin \(\sin\), with \(=20\). and \(=\)

\section*{Handbook of Validation}

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6.0

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Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
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14.3 Contents of the file results

Values at the point of observation of displacements and constraints.
\(X\)
14.4 Parameters
of execution
Version: 6.0.29

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 4.22 seconds
15 Modeling
G

\subsection*{15.1 Characteristics of modeling}

Element of hull DST (modeling of a quarter of plate).
The model of plate is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to
beta, without reference to a model not turned.
The orientation of the meshs is here identical to that of the reference [bib1].
C

\section*{Limiting conditions:}

LIAISON_OBLIQUE
(GROUP_NO =' \(A B^{\prime}, A_{2}\) (GL_NAUT \(\left.=(20 ., 30 ., 0),. D Z=0 ., D R Y=0.\right)\)
(GROUP_NO=' BC', ANGL_NAUT = (20. , 30. , 0.), DX=0., DRY=0.)
(GROUP_NO=' CD', ANGL_NAUT \(=(20 ., 30 ., 0),. D Y=0 ., D R X=0\).
(GROUP_NO =' DA', ANGL_NAUT \(=(20 ., 30 ., 0),. D Z=0 ., D R X=0\).
Not C
net: 72
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Version
6.0

Titrate:
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Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
V3.03.115-A Page:
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15.2 Characteristics of the grid

A number of nodes: 56
A number of meshs and types: 72 TRIA3

\subsection*{15.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE "MECHANICAL" "DST" \\ MODI_MAILLAGE ORIE_NORME_COQUE \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ ANGLE_REP \\ DEFI_MATERIAU \\ ELAS_ORTH \\ DEFI_COQU_MULT \\ SLEEP \\ THICK \\ ORIENTATION \\ AFFE_CHAR_MECA \\ FORCE_COQUE \\ NEAR \\ ALL \\ LIAISON_OBLIQUE \\ ANGLE_NAUT \\ GROUP_NO \\ MECA_STATIQUE \\ CALC_CHAM_ELEM \\ SIGM_ELNO_DEPL
}

16 Results of modeling \(G\)
16.1 Values
tested
Not C
Identification
Reference
Aster \%
Difference
on lower layer 3
4.7100E+01 4.7920E+01
1.742
\(X\)
on higher layer 3
\(5.8800 E+015.9900 E+01\)
1.872
X
Constraints
on lower layer 2
4.7100E+01 4.7920E+01
1.742
\(X\)
on higher layer 2
4.7100E+01 4.7920E+01
1.742
\(X\)
on lower layer 1
\(5.8800 E+015.9900 E+01\)
1.872
X
on higher layer 1
4.7100E+01 4.7920E+01
1.742
X
DX
\(1.9696 E+011.9882 E+01\)
0.946
Displacement DY
\(7.1687 E+007.2365 E+00\)
0.947
DZ
\(3.6304 E+013.6647 E+01\)

\subsection*{16.2 Remarks}

The values of reference of displacement to the point \(C\) are obtained by projecting displacement theoretical bench for a plate not turned in the new reference mark user (displacement for a not turned plate being vertical, new displacement is a function of the projection of the axis \(Z\) ). In the local reference mark, the projection of axis \(Z\) is as follows:
\(\sin \cos\)
sin \(\sin\), with \(=20\). and \(=\)
30
\(\cos\)
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6.0

Titrate:

\section*{SSLS115 - Composite square plate under uniform pressure}

Date:
23/10/02
Author (S):
J.M. PROIX, NR. RAHNI Key
:
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16.3 Contents of the file results

Values at the point of observation of displacements and constraints.
\(X\)
16.4 Parameters
of execution

Version: 6.0.29

Machine: SGI - ORIGIN 2000

\section*{Obstruction memory:}

32 Mo
Time CPU To use: 4.22 seconds
17 Modeling
H

\subsection*{17.1 Characteristics of modeling}

Element of hull DST (modeling of a quarter of plate).
The model of plate is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to
beta, without reference to a model not turned.
C
D
B
With

\section*{Limiting conditions:}

LIAISON_OBLIQUE
(GROUP_NO=' AB', ANGL_NAUT= (20. , 30. , 0.), DZ=0., DRY=0.)
(GROUP_NO=' BC', ANGL_NAUT \(=(20 ., 30 ., 0),. D X=0 ., D R Y=0\).
(GROUP_NO=' CD', ANGL_NAUT = (20. , 30. , 0.), \(D Y=0 ., D R X=0\).
(GROUP_NO=' DA', ANGL_NAUT \(=(20 ., 30 ., 0),. D Z=0 ., D R X=0\).
Not C
net: 142

\section*{Handbook of Validation}

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6.0

Titrate:
SSLS115 - Composite square plate under uniform pressure
Date:

\section*{23/10/02}

Author (S):
J.M. PROIX, NR. RAHNI Key
:
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\subsection*{17.2 Characteristics of the grid}

A number of nodes: 101
A number of meshs and types: 144 TRIA3

\subsection*{17.3 Functionalities}
tested
Orders

AFFE_MODELE
"MECHANICAL"
"DST"

MODI_MAILLAGE
ORIE_NORME_COQUE
AFFE_CARA_ELEM
HULL
THICK
ANGLE_REP
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
ORIENTATION
AFFE_CHAR_MECA

\title{
FORCE_COQUE
}

NEAR

\section*{ALL}

\section*{LIAISON_OBLIQUE}

ANGLE_NAUT
GROUP_NO
MECA_STATIQUE
CALC_CHAM_ELEM
SIGM_ELNO_DEPL

\section*{18 Results of modeling \(\boldsymbol{H}\)}

\subsection*{18.1 Values}
tested
Not C

\section*{Identification}

Reference
Aster \%
Difference
on lower layer 3
4.7100E \(+015.0957 E+01\)
8.19
\(X\)
on higher layer 3
\(5.8800 E+016.3691 E+01\)
8.32

X
Constraints
on lower layer 2
4.7100E+01 5.0957E+01
8.19
\(X\)
on higher layer 2
4.7100E+01 5.0957+01
8.19

\section*{\(X\)}
on lower layer 1
5.8800E+01 6.3696E+01
8.32

X
on higher layer 1
4.7100E+01 5.0957E+01
8.19

X
DX
1.9696E \(+011.9735 E+01\)
0.199

Displacement DY
7.1687E+00 7.1830E+00
0.200

DZ
3.6304E +01 3.6376E +01
0.200

\subsection*{18.2 Remarks}

The values of reference of displacement to the point \(C\) are obtained by projecting displacement theoretical bench for a plate not turned in the new reference mark user (displacement for a not turned plate being vertical, new displacement is a function of the projection of the axis \(Z\) ). In the local reference mark, the projection of axis \(Z\) is as follows:
\(\sin \cos\)
sin \(\sin\), with \(=20\). and \(=\)

30
\(\cos\)

\section*{Handbook of Validation}

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\section*{Titrate:}

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18.3 Contents of the file results

Values at the point of observation of displacements and constraints.
\(X\)
18.4 Parameters
of execution
Version: 6.0.29

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 4.22 seconds

\section*{Handbook of Validation}

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Titrate:
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J.M. PROIX, NR. RAHNI Key

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}

19 Synthesis graph

\section*{Modeling C \\ Modeling D}

\section*{Modeling \(F\) \\ Modeling \(\boldsymbol{G}\)}

\author{
Modeling H \\ \% Forced \\ \% Displacement \\ C
}

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

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\section*{Titrate:}

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\section*{20 Summary of the results}

The results obtained show that:
Identical \(\cdot\) A grid (standard of surface meshs and orientation of the meshs), change of reference mark user does not influence the constraints;
- Because of orthotropism of the problem, there exists a considerable sensitivity to the orientation triangular surface meshs (the precision of calculations passes from 1 to \(11 \%\) for constraints and from 0.17 to \(0.95 \%\) for displacements). This sensitivity does not disappear in refining the grid. This point is thus to take into account at the time of the comparison of

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates

\section*{HT-66/02/001/A}

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6.0

\section*{Titrate:}

SSLS115-Composite square plate under uniform pressure
Date:
23/10/02
Author (S):

\section*{J.M. PROIX, NR. RAHNI Key}
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V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
Membrane SSLS116 Loading of an offset plate
Date:
19/08/02
Author (S):
Key P.MASSIN, D. NUNEZ GUAJARDO

\section*{:}

V3.03.116-A Page:
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Organization (S): EDF/AMA, CS IF

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.116

SSLS116 - Membrane loading of a plate
offset

\section*{Summary:}

This test relates to the offsetting of a plate compared to the plan of the grid or plan of diagram. The

\section*{loading}
is purely membranous.
The reference is given by a first resolution where a not offset plate is modelled. It validates it the second calculation where one models a plate offset compared to the plan of the grid. Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
Membrane SSLS116 Loading of an offset plate
Date:
19/08/02
Author (S):
Key P.MASSIN, D. NUNEZ GUAJARDO
:
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1
Problem of reference

\subsection*{1.1 Geometry}
0.2

A4
Plate
A3
offset
L14
Plan purifies
L12

Z

A1
A2

Plate reference
not offset

One represented here the plate offset compared to the plan of diagram (which is confused here with the plan
inferior of the offset plate).
To avoid overloading the diagram, one does not trace the plate of not offset reference, for which the plan of diagram is also the average plan.

\section*{1.2 \\ Properties of materials}

The material constituting plate is characterized by the following data:
\(E L=20000 \cdot P a \quad E T=20000 \cdot P a \quad E N=20000 \cdot P a\)
\(L N=0 . L T=0 T N=0 . G L T=2000 . P a G L N=0 . G T N=0\).
\(R H O=1000 . \mathrm{kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loadings}

The \(L 14\) mesh is embedded \(D X=D Y=D Z=0\).
\(D R X=D R Y=D R Z=0\).
One applies the forces to the L12 mesh
\(F X=1000 . \mathrm{N} \mathrm{MY}=100 . \mathrm{N} . \mathrm{m}\)
on the offset plate
\(F X=1000 . \mathrm{N}\)
on the not offset plate
These loadings are applied by means of FORCE_ARETE of AFFE_CHAR_MECA in the plan of diagram.

\author{
Note: \\ The fact of applying a force FX to the plate offset to the level of the plan of diagram generates one moment MY which should be compensated for to find itself under conditions purely \\ membrane on the level of the offset plate. \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/02/001/A
}

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

The calculation of the membrane type with the not offset plate is used as reference. Nonregression by report/ratio with the results obtained by this first calculation is checked.

\section*{2.2 \\ Results of reference}

They are consisted of the values of the field of displacement \(D X, D Y, D Z, D R X, D R Y\) with nodes 66 and 52
(for the DKT and DST) and with the nodes N1 and N16 (for the DKQ and the DSQ) and of the calculation of the frequencies
of the first 4 modes.

\section*{2.3 \\ Uncertainty on the solution}

Uncertainty is null since it is about the same calculation carried out by two different ways.

\subsection*{2.4 Bibliography}
[1]
[R3.07.03]: Elements of plate DKT, DST, DKQ, DSQ and Q4.

\section*{[2]}
[R3.07.06]: Treatment of offsetting for the elements of plate DKT, DST, DKQ, DSQ and Q4G.

\author{
Handbook of Validation
}

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Key P.MASSIN, D. NUNEZ GUAJARDO
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Average plan Plates}
\(d=0.1\)
offset
With
With
3
4

Plan purifies

\section*{A1}

A2

The elements used are elements of plate DKT.

\section*{3.2 \\ Characteristics of the grid}

Co-ordinates of the nodes:

\author{
Node \\ Coor_X (m) \\ Coor_Y (m) \\ Coor_Z (m) \\ Al 0.0. 0 . \\ A2 10. o. 0. \\ A3 10. 5. 0 . \\ A4 0. 5. 0. \\ N66 10. 5. 0 . \\ N52 8. 2. 0. \\ 66 Nodes \\ 100 meshs DKT (TRIA3)
}

\subsection*{3.3 Functionalities}
tested

\section*{Orders Key word}
factor
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_ARETE
Handbook of Validation
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Version
6.0

\section*{Titrate:}

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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{Identification Reference}
(m) Aster (m) \%
difference
DX (N66)
\(2.5 m 2.51 .78\)
E15
DY (N66)
0.m
4.615 E14
4.615 E14

DZ (N66)
0.m
1.158 E12
1.158 E12

DRX (N66)
O.rad
2.76 E13
2.76 E13

DRY (N66)
O.rad
7.86 E14
7.86 E14

0 .
2.72 E13
2.72 E13
DRX (N52)

0 .
2.34 E13
2.34 E13

DRY (N52)
0.
1.84 E14
1.84 E14

Frequency lst mode
\(1.4439 \mathrm{E03Hz}\)
1.4465 E03
0.182

Frequency 2nd mode
3.71554 E03
3.7984 E03
2.231

Frequency 3rd mode
9.01537 E03
9.1305 E03
1.277

\subsection*{4.2 Parameters of execution}

Version: 6.0.21

Machine: SGI Origin 2000

Obstruction memory:
16 MW
Time CPU To use:
6 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.0

Titrate:
Membrane SSLS116 Loading of an offset plate
Date:
19/08/02
Author (S):
Key P.MASSIN, D. NUNEZ GUAJARDO
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\section*{5 Modeling \\ B}

\title{
5.1 \\ Characteristics of modeling
}

Average plan Plates
\(d=0.1\)
offset

Plan purifies

The elements used are elements of plate DKQ.

\section*{5.2 \\ Characteristics of the grid}

Co-ordinates of the nodes:
\begin{tabular}{|c|}
\hline Node \\
\hline Coor_X (m) \\
\hline Coor_Y (m) \\
\hline Coor_Z (m) \\
\hline Al 0.0.0. \\
\hline A2 10.0.0. \\
\hline A3 10. 5. 0. \\
\hline A4 0. 5. 0. \\
\hline N1 1. 5. 0. \\
\hline N16 8. 2. 0. \\
\hline
\end{tabular}

66 Nodes
50 meshs DKQ (QUAD4)

\subsection*{5.3 Functionalities}

\title{
Orders Key word
}
factor
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_ARETE
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HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
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Date:
19/08/02
Author (S):
Key P.MASSIN, D. NUNEZ GUAJARDO

\section*{:}

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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested

\section*{Identification Reference}
(m) Aster (m) \%
difference
DX (N1)
\(2.5 m 2.53 .42\)
E14
DY (N1)
0.m
2.52 E14
2.52 E14

\author{
DZ (N1) \\ 0.m \\ 1.521 E12 \\ 1.521 E12 \\ DRX (N1) \\ O.rad \\ 1.54 E14 \\ 1.54 E14 \\ DRY (N1) \\ O.rad \\ 2.63 E13 \\ 2.63 E13
}

DX (N16)
2. 2. 1.49

E08
DY (N16)
0 .
2.03 E14
2.03 E14

DZ (N16)
0 .
1.12 E12
1.12 E12

DRX (N16)
0.
4.29 E14
4.29 E14

DRY (N16)
0.
2.19 E13
2.19 E13

Frequency 1st mode
1.44474E03 Hz
1.446841 E03
0.145

Frequency 2nd mode
3.69339 E03
3.703038 E03
0.261

Frequency 3rd mode
9.04773 E03
9.14141 E03
1.023

Frequency 4th mode
1.33393 E02
1.34463 E02
0.802

\subsection*{6.2 Parameters}
of execution
Version: 6.0.21

Machine: SGI Origin 2000

Obstruction memory:
16 MW
Time CPU To use:
6 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

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Author (S):
Key P.MASSIN, D. NUNEZ GUAJARDO

\section*{:}

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\section*{7 Modeling \\ C}

\section*{7.1}

Characteristics of modeling

\section*{Average plan Plates}
\(d=0.1\)
offset

Plan purifies

The elements used are elements of DST plate.

\section*{7.2 \\ Characteristics of the grid}

Co-ordinates of the nodes:
Coor_X node
Coor_Y
Coor_Z
Al 0.0.0.

A2 10. 0. 0.
A3 10. 5. 0 .
A4 0. 5. 0.
N66 10. 5. 0.
N52 8. 2. 0.

66 Nodes
100 meshs DKT (TRIA3)

\subsection*{7.3 Functionalities}
tested
Orders Key word
factor
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_ARETE
Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
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\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested
0.
4.48 E12
4.48 E12
DRX (N66)
0.
2.19 E13
2.19 E13
DRY (N66)
0.
6.69 E13
6.69 E13
DX (N52)
2. 2.
1.49
E08
DY (N52)
0 .
4.08 E15
4.08 E15
DZ (N52)
0.
3.62 E12
3.62 E12
DRX (N52)
0.
1.60 E13
1.60 E13
0.
7.34 E13
7.34 E13

Frequency 1st mode
1.4439E03
1.4465 E03
0.182

Frequency 2nd mode
3.71554 E03
3.7984 E03
2.231

Frequency 3rd mode
9.01537 E03
9.1305 E03
1.277

Frequency 4th mode
1.34708 E02
1.4077 E02
4.501

\subsection*{8.2 Parameters \\ of execution}

Version: 6.0.21

Machine: SGI Origin 2000

Obstruction memory:
16 MW
Time CPU To use:
6 seconds
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Version
6.0

Titrate:
Membrane SSLS116 Loading of an offset plate
Date:
19/08/02
Author (S):
Key P.MASSIN, D. NUNEZ GUAJARDO
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\section*{9 Modeling}

D

\title{
9.1 \\ Characteristics of modeling
}

\section*{Average plan Plates}
\(d=0.1\)
offset

\section*{Plan purifies}

The elements used are elements of plate DSQ.

\section*{9.2 \\ Characteristics of the grid}

Co-ordinates of the nodes:

\section*{Coor_X node}

Coor_Y
Coor_Z
Al 0.0. 0 .
A2 10. 0. 0.
A3 10. 5. 0 .
A4 0. 5. 0.
N1 1.5. 0.
N16 8. 2. 0.

66 Nodes
50 meshs DSQ (QUAD4)

\subsection*{9.3 Functionalities}
tested

\author{
Orders Key word \\ factor \\ AFFE_CARA_ELEM \\ OFFSETTING \\ AFFE_CHAR_MECA \\ FORCE_ARETE \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/02/001/A
}

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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested
Identification Reference Aster \% difference

\author{
DX (N1)
}
2.52 .5
-1.33
E14
DY (N1)
0.
2.61 E14
2.61 E14

DZ (N1)
0 .
6.26 E13
6.26 E13

DRX (NI)
0.
8.18 E15
8.18 E15

DRY (N1)
0.
9.57 E14
9.57 E14

\section*{DX (N16)}
2. 2.
1.49

E08

\author{
DY (N16) \\ 0. \\ 1.79 E14 \\ 1.79 E14 \\ DZ (N16) \\ 0 . \\ 3.97 E13 \\ 3.97 E13 \\ DRX (N16) \\ 0. \\ 1.71 E14 \\ 1.71 E14 \\ DRY (N16) \\ 0 . \\ 9.03 E14 \\ 9.03 E14
}

Frequency 1st mode
1.44474E03
1.446841 E03
0.145

Frequency 2nd mode
3.69339 E03
3.703038 E03
0.261

Frequency 3rd mode
9.04773 E03
9.14141 E03
1.023

Frequency 4th mode 1.33393 E02

\subsection*{10.2 Parameters}
of execution
Version: 6.0.21

Machine: SGI Origin 2000

\section*{Obstruction memory:}

16 MW
Time CPU To use:
6 seconds
Handbook of Validation
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11 Summary of the results
For each modeling, DKT, DKQ, DST and DSQ, results found for the offset plate coincide with the reference solution.
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Date:
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Author (S):
P. Key MASSIN, D. NUNEZ-GUAJARDO
:
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Organization (S): EDF/AMA, CS IF

\title{
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}

SSLS117-Offsetting of plates
nonsymmetrical

\section*{Summary:}

This test validates the offsetting of nonsymmetrical simple plates compared to the plan of the grid or plan
of diagram (key word OFFSETTING of order AFFE_CARA_ELEM).
The reference is given by a first resolution where one models double-layered made up of two layers various thicknesses and of two materials.

It is used to validate the second calculation where one models two layers offset compared to the plan of the grid.

It differs from test SSLS111 only by the fact that the 2 layers are different thicknesses.
Four modelings implement elements DKT, DKQ, DST, DSQ.
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1
Problem of reference
1.1 Geometry

10 m
0.4 m

5 m

A4
A3
0.2 m

A1
A2
\(m 1\) m2 m3

\(X\)
1.2

Properties of materials
The material constituting the first layer is orthotropic and is characterized by the data following:
EL=20000. 106 Pa
ET=20000. 106 Pa
\(L T=0.3\)
GLT=2000. 106 Pa
The material constituting the second layer is also orthotropic and has the following characteristics:
\(E L=15000\). 106 Pa
ET=15000. 106 Pa
\(L T=0.3\)
\(G L T=1500.106 \mathrm{~Pa}\)

\section*{1.3}

Boundary conditions and loadings
The A1 node is embedded
\(D X=D Y=D Z=0\).
\(D R X=D R Y=D R Z=0\).
The \(A 2\) node is blocked according to following ddls: \(D X=D Y=0\).
One applies nodal forces \(F Z=1000\) NR to the \(A 3\) node, and one applies the loading distributed (key word FORCE_COQUE) on the meshs m1, m2 and m3:
\(F X=200\) NR
\(F X=500 . \mathrm{N} / \mathrm{m} 2 \mathrm{FZ}=500\).
N/m2
\(M X=100 . \mathrm{N} / \mathrm{m}\)
\(M Y=40 . \mathrm{N} / \mathrm{m}\)
The selected loading utilizes requests out of membrane and inflection.
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
Calculation with double-layered material (order DEFI_COQU_MULT) is used as reference. Not regression compared to the results obtained by this first calculation is checked. Two plates of the second modeling are offset compared to the average plan of the double-layered one.

\section*{2.2 \\ Results of reference}

They are consisted of the values of the field of displacement \(D X, D Y, D Z, D R X, D R Y\) at the A3 point and A4, efforts at the A1 point and of the first 4 Eigen frequencies.

\section*{2.3 \\ Uncertainty on the solution}

Uncertainty is null since it is about the same calculation carried out by two different ways.
2.4 Bibliography
[1]
[R3.07.03]: Elements of plate DKT, DST, DKQ, DSQ and Q4.
[2]
[R3.07.06]: Treatment of offsetting for the elements of plate DKT, DST, DKQ, DSQ and Q4G.

\author{
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}

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

The model consists of two plates corresponding to the average plan of each of both layers of the model of reference. To represent these two plates, one leaves the grid of the plan of diagram which one offsets distances 0.1 and 0.2.

\section*{Plan average Plaq2}
\(d=0.1 \mathrm{~m}\)
\(d=-0.2 \mathrm{~m}\)

\section*{Plan of diagram}

\section*{Plan average Plaq1}

The elements used are elements of plate DKT.
3.2
Characteristics of the grid
Co-ordinates of the nodes:
Node Coor_X Coor_Y Coor_Z
A1 0. 0.0.
A2 10.
0.

0.
A3 10.
5.
0.
A4 0.5.0.
94 Nodes
100 meshs DKT (TRIA3)
3.3 Functionalities ..... tested
Orders Key word
factor Key word
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_COQUE
PLAN
DEFI_COQU_MULT
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Author (S): \\ P. Key MASSIN, D. NUNEZ-GUAJARDO \\ : \\ V3.03.117-A Page: \\ 5/12 \\ \section*{4 \\ \\ Results of modeling \(A\)} \\ \subsection*{4.1 Values} \\ tested \\ \section*{Identification Reference} \\ Aster \% \\ difference \\ Displacement
}
```

DX (A4)
-1.9426 10-6 -1.93937
10-6 -0.166
DY (A4)
-1.1438 10-6 -1.138746
10-6 -0.442
DZ (A4)
-2.1832 10-4 -2.169149 10-4 -0.644
DRX (A4)
-6.0633 10-5 -6.03126
10-5 -0.528
DRY (A4) }1.281
10-4 1.28078
10-4-0.056

```
10-7 -0.04
DZ (A3)
-1.5584 10-3 -1.5563
10-3 -0.133
DRX (A3)
-1.2578 10-4 -1.2543
10-4-0.276
DRY (A3) 1.3909 104 1.3903 104 0.038
```


## Eigen frequencies

Frequency 1st mode<br>1.5303<br>1.53038<br>0.006

Frequency 2nd mode
6.41979
6.41979
1.07 E-04

Frequency 3rd mode
1.2698981011 .269898
1012.81

E-05

## Frequency 4th mode

2.615971012 .61597
1011.23

E-04

## Efforts

$N X X$<br>N6<br>$1.12134 E+04$<br>$1.11996 E+04$<br>-0.124<br>NYY<br>N6<br>$7.52699 E+03$<br>7.48953 E+03<br>-0.498<br>NXY<br>N6<br>1.89850 E +03<br>1.89286 E+03<br>-0.297<br>MXX<br>N6<br>$2.33596 E+04$<br>$2.33480 E+04$<br>-0.050<br>MYY<br>N6<br>$1.67905 E+04$<br>$1.66988 E+04$<br>-0.546<br>MXY<br>N6<br>5.43976 E +03<br>5.42402 E+03<br>-0.289<br>QX<br>N6<br>4.60472 E+03<br>4. $63427 E+03$<br>0.642<br>QY<br>N6<br>1.58798 E +03<br>1.61207 E+03<br>1.517

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## 5 Modeling <br> B

5.1

Characteristics of modeling
The model consists of two plates corresponding to the average plan of each of both layers of the model of reference. To represent these two plates, one leaves the grid of the plan of diagram which one offsets distances 0.1 and 0.2

## Plan average Plaq2

$d=0.1 \mathrm{~m}$
$d=-0.2 \mathrm{~m}$

Plan of diagram

## Plan average Plaq1

The elements used are elements of plate DKQ.
5.2

Characteristics of the grid
Co-ordinates of the nodes:
Node Coor_X Coor_Y Coor_Z
A1 0. 0.0.
A2 10.
0.
0.

A3 10.
5.
0.

A4 0.5.0.
67 Nodes
50 meshs DKQ (QUAD4)

### 5.3 Functionalities

tested
Orders Key word
factor Key word
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_COQUE
PLAN

## Handbook of Validation

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6
Results of modeling B
6.1 Values
tested
Identification Reference
Aster \%
difference
Displacement
DX (A4)
-2.354 10-6 -2.341
10-6 -0.520
DY (A4)
-2.059 10-6 -2.039
10-6 -0.940
DZ (A4)
-2.50 10-4 -2.45
10-4-1.924
DRX (A4)
-6.93 10-5 -6.823
10-5 -1.538
DRY (A4) 1.541
10-4 1.541
10-4-0.005

DX (A3)<br>-2.9951 10-6 -2.9835<br>10-6 -0.384<br>DY (A3)<br>-4.4646 10-7 -4.4648<br>10-7 0.006<br>DZ (A3)<br>-1.8604 10-3 -1.8550<br>10-3-0.287<br>DRX (A3)<br>-1.3346 10-4 -1.3217<br>10-4-0.963<br>DRY (A3) 1.6649<br>10-4 1.6649<br>10-4 0.002

## Eigen frequencies

## Frequency 1st mode

1.4362
1.4362
0.

## Frequency 2nd mode

5.57419
5.57419
0.

Frequency 3rd mode
1.235241011 .23524
1010.

## Frequency 4th mode

 2.525731012 .52573 1010.
## Efforts

NXX<br>N60<br>$1.30762 E+04$<br>1.31749 E+04<br>0.755<br>NYY<br>N60<br>2.68634 E+03<br>2.82116 E +03<br>5.019<br>NXY<br>N60<br>$4.33581 E+02$<br>$4.32313 E+02$<br>-0.292<br>MXX<br>N60<br>$4.17329 E+04$<br>$4.19250 E+04$<br>0.460<br>MYY<br>N60<br>1.84443 E+04<br>1.79620 E+04<br>-2.615<br>MXY<br>N60<br>6.33337 E+03<br>$6.29774 E+03$<br>-0.563<br>QX<br>N60

```
3.19128 E+04
3.20627 E+04
0 . 4 7 0
QY
N60
1.43089 E+04
1.39373 E+04
-2.597
```


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```
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```

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## 7 Modeling

C

## 7.1 <br> Characteristics of modeling

The model consists of two plates corresponding to the average plan of each of both layers of the model of reference. To represent these two plates, one leaves the grid of the plan of diagram which one offsets distances 0.1 and 0.2

[^1]$d=-0.2 m$

Plan of diagram

Plan average Plaq1

The elements used are elements of DST plate.

7.2<br>Characteristics of the grid<br>Co-ordinates of the nodes:<br>Node Coor_X Coor_Y Coor_Z<br>A1 0.0.0.<br>A2 10.<br>0.<br>0.<br>A3 10.<br>5.<br>0.<br>A4 0. 5. 0.<br>66 Nodes<br>100 meshs DST (TRIA3)

### 7.3 Functionalities

tested
Orders Key word
factor Key word
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_COQUE
PLAN

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## 8

Results of modeling C

### 8.1 Values

tested
Identification Reference
Aster \%
difference
Displacement

DX (A4)
-1.939 10-6 -1.93217
10-6-0.352
DY (A4)
-1.149 10-6 -1.1358
10-6-1.145
DZ (A4)
-2.2091 10-4 -2.199799
10-4-0.421
DRX (A4)
-6.09302 10-5 -6.06238

```
10-5 -0.503
DRY (A4) 1.297279
10-4 1.30383
10-4 0.505
```

```
DX (A3)
-2.4385 10-6 -2.4305
10-6-0.328
DY (A3)
-2.3382 10-7 -2.4272
10-7 -3.806
DZ (A3)
-1.5864 10-3 -1.5952
10-3 0.558
DRX (A3)
-1.2639 10-4 -1.2621
10-4-0.138
DRY (A3) }1.412
10-4 1.4215
10-4 0.627
```


## Eigen frequencies

Frequency 1st mode<br>1.512356<br>1.50555<br>-0.450

Frequency 2nd mode
6.373398
6.343485
-0.469

# Frequency 3rd mode 

1.250111011 .24249

101-0.610

Frequency 4th mode
2.5467261012 .518863

101-1.094

## Efforts

$N X X$<br>N1<br>9.85902 E+03<br>$1.09606 E+04$<br>11.173<br>NYY<br>N1<br>$6.36055 E+03$<br>7.04694 E+03<br>10.791<br>NXY<br>N1<br>$2.07601 E+03$<br>$2.07850 E+03$<br>0.120<br>MXX<br>N1<br>$2.11639 E+04$<br>2.10507 E+04<br>-0.535<br>MYY<br>N1<br>1.49410 E+04<br>1.46867 E+04<br>-1.701<br>MXY

```
N1
5.82623 E+03
5.86877 E+03
0 . 7 3 0
QX
N1
2.56538 E+03
2.35368 E+03
-8.252
QY
N1
1.79286 E+03
1.81901 E+03
1.458
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```

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## 9 Modeling

D
9.1

Characteristics of modeling
The model consists of two plates corresponding to the average plan of each of both layers of the model of reference. To represent these two plates, one leaves the grid of the plan of diagram which one offsets distances 0.1 and 0.2

## Plan average Plaq2

$d=0.1 \mathrm{~m}$
$d=-0.2 \mathrm{~m}$

Plan of diagram

Plan average Plaq1

The elements used are elements of plate DSQ.

## 9.2 <br> Characteristics of the grid

Co-ordinates of the nodes:

## Node Coor_X Coor_Y Coor_Z

Al 0.0. O.
A2 10.
0.
0.

A3 10.
5.
0.

A4 0. 5. 0 .
67 Nodes
50 meshs DSQ (QUAD4)

### 9.3 Functionalities

tested

## Orders Key word

factor Key word
AFFE_CARA_ELEM
OFFSETTING
AFFE_CHAR_MECA
FORCE_COQUE
PLAN

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### 10.1 Values

tested

## Identification Reference

Aster \%
difference
Displacement

## DX (A4)

-2.34539 10-6 -2.3241
10-6-0.904
DY (A4)
-1.9694 10-6 -1.9363
10-6-1.680
DZ (A4)
-2.2428 10-4 -2.1154
10-4-5.680
DRX (A4)
-6.2983 10-5 -5.9831
10-5-5.003
DRY (A4) 1.5823
10-4 1.5991
10-4-1.062

DX (A3)
-3.0023 10-6 -2.9771
10-6-0.839
DY (A3)
-4.6612 10-7-4.6487
10-7-0.267

DZ (A3)<br>-1.8842 10-3 -1.8923<br>10-3 0.430<br>DRX (A3)<br>-1.2768 10-4 -1.2478<br>10-4-2.264<br>DRY (A3) 1.7064<br>10-4 1.724<br>10-4 1.054

## Eigen frequencies

## Frequency 1st mode

1.4219
1.4155
-0.445

## Frequency 2nd mode

5.2995
5.21015
-1.686

## Frequency 3rd mode

1.2151011 .206

101-0.723

## Frequency 4th mode

2.43851012 .407

101-1.252

## Efforts

$N X X$<br>N1<br>8.68372 E+03<br>1.64692 E+04<br>89.657<br>NYY<br>N1<br>$4.10693 E+03$<br>$2.34359 E+03$<br>-42.936<br>NXY<br>N1<br>3.90190 E+02<br>$4.54002 \mathrm{E}+02$<br>16.354<br>MXX<br>N1<br>3.47663 E+04<br>$3.43655 E+04$<br>-1.153<br>MYY<br>N1<br>$1.52451 E+04$<br>$1.45102 \mathrm{E}+04$<br>-4.821<br>MXY<br>N1<br>6.34489 E+03<br>$6.33555 E+03$<br>-0.147<br>QX<br>N1<br>1.70439 E+04<br>$1.56565 E+04$<br>-8.140<br>QY<br>N1<br>9.82819 E+03<br>9.49952 E+03<br>-3.344

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## 11 Summary of the results

With regard to displacements for the modelings DKT and DKQ, the results obtained with 2 offset hulls differ from to the more $2 \%$ compared to the reference solution. For the others modelings, one obtains to the maximum of the errors of $4 \%$ for DST and $6 \%$ for the DSQ. For these two last modelings, the error is more important because the calculation of transverse shearing is not not are equivalent between the double-layered one and the two offset plates.

Indeed, transverse shearing is supposed to be constant in the thickness of each DST element or DSQ; this transverse shearing is an average shearing. An average value is thus obtained for each plate offset, overall different from average transverse shearing on the double-layered plate.

This is marked even more for the efforts, where the differences remain lower than $5 \%$ for modelings DKT (A and B) but reach $11 \%$ for modeling $C$ and $89 \%$ for modeling
D. Being given the nature of the test, one cannot know a priori if the problem comes from the hulls and plates
multi-layer or of offsetting. An anomaly is in the course of treatment on this problem.
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6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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Organization (S): EDF/MTI/MMN, CS IF

Handbook of Validation<br>V3.03 booklet: Linear statics of the plates and hulls<br>V3.03.118 document

SSLS118 - Square plate posed subjected
with a sinusoidal pressure

## Summary:

One treats the case of a multi-layer full-course square plate then, simply supported and subjected to one
sinusoidal pressure.
One calculates displacement in the center, the constraints, with the lower interfaces
$\boldsymbol{x x}$
$y y$
$x y$
$x z$
$y z$
averages and higher, efforts of membrane $N R, N R, N R$, efforts sharp $T, T$ and them
$x x$
$y y$
$x y$
$X$
$y$
moments M, M and Mr.
$x x$
$y y$
$x y$
The test gathers 14 modelings: with regard to modelings $A$ with $F$, the results obtained are compared for modelings DKQ, DSQ, DKT, DST, COQUE_3D with triangular meshs and COQUE_3D with rectangular meshs.
Modelings $G$ and $H$ make it possible to test the results in a reference mark user different from the total reference mark.
Modelings I and J measure the sensitivity of the results to the smoothness of the grid, for the configuration
DSQ.
Modelings $K$ with NR relate to the multi-layer plate, for modelings DST and DSQ, in reference mark total and user. They make it possible to estimate the distribution of the plane constraints and transverse shearing
inside the plate.
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## 1

Problem of reference

### 1.1 Geometry

### 1.2 Properties

of
materials

$$
\begin{aligned}
& E=25 \mathrm{~Pa} \\
& =0.25 \\
& =1 \mathrm{~kg} / \mathrm{m} 3 \\
& 1.3 \\
& \text { Boundary conditions and loadings }
\end{aligned}
$$

## Simple bearing plate

$$
\begin{aligned}
& C . L .: \\
& A B \\
& D X=0 . \\
& D Z=0 . \\
& D R Y=0 . \\
& M Y=0 .
\end{aligned}
$$

$$
\begin{aligned}
& B C \\
& D Y=0 . \\
& D Z=0 . \\
& D R X=0 . \\
& M X=0 .
\end{aligned}
$$

$$
\begin{aligned}
& C D \\
& D X=0 . \\
& D Z=0 . \\
& D R Y=0 . \\
& M Y=0 .
\end{aligned}
$$

$$
\begin{aligned}
& D A \\
& D Y=0 . \\
& D Z=0 . \\
& D R X=0 . \\
& M X=0 .
\end{aligned}
$$

Not $O$
$D X=0$.
$D Y=0$.
$D R X=0$.
$D R Y=0$.
$D R Z=0$.
Loading:
FORCE_COQUE
Sinusoidal pressure
$P$
$X$
$y$
With $P=F$ sin
where $F=1$ and has $=1$
0
sin
has
has
0
1.4 Conditions
initial

Without object for the static analysis.
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2
Reference solution

## 2.1

Method of calculation used for the reference solution
The analytical solution of reference is based on the theory of Coils-Kirchhoff, usually used for the plates known as "thin" [bib1].

Taking into account the problem and in any point of the plate, one has for the calculation of the arrow:
Fat
0
$X$
$y$
$W=$
$\sin \sin$
$44 D$
has
has
with:
3
$=$
Eh
D
, $F=1$, has $=1$ and $=0.25$
12(
2
1 -) 0
For the calculation of the moments, the theory leads to the following expressions:
$M=1+\sin \sin$

```
)
X
y
has
has
M
=M
yy
xx
M
=-1-cos cos
xy
(
)
X
y
has
has
2
F
with
O
has
\(=\)
2
4
For the sharp efforts, one obtains:
F has
0
X
T=
cos sin
X
2
has
has
F has
```

```
y
T
0
=
sin cos
y
2
has
has
```

For a homogeneous plate, the plane constraints are given by:

M
$\boldsymbol{x} \boldsymbol{x}$
$x x$
$\boldsymbol{Z} \boldsymbol{A} \boldsymbol{M}$
$y y$
= [ ]
$y y$

M
$x y$
$x y$
12
with []
With $=$
and Z the position in the thickness of the plate
3 [I]
H
and stresses shear transverse by:

```
xz=[D
,
I (Z)] X
```

```
T
yz
y
6 H2
```

with [D
1 (Z)]
=
$-z 2$
3
[I]
H
2

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## 2.2

Results of reference
For each modeling, one calculates:

- in the center of the plate, displacement,
- in the center of the plate and in the middle of side $A B$, the constraints, on
$x x$
$y y$
$x y$
$x z$
$y z$
plans:
inferior, means and superior of the plate in the full-course case,
inferior, means and superior of each section in the multi-layer case (5 layers),
$\cdot$ in the center, the corners and in the middle of sides $A B$ and $A D$, the efforts of membrane $N R, N R$, NR,
$x x$
$y y$
$x y$
sharp efforts T, T and the moments M, M and Mr.
$X$
$y$
$x x$
$y y$
$x y$
The expression of these quantities to the points $O, A, B, C, D$ gives:
$\boldsymbol{W} \boldsymbol{M}$
M
M
$T$
$T$
$x x$
$y y$
$x y$
$X$
$y$
O

```
(1+)
(1+)
00
4
3
Eh
With -
0
0-(1-)
0
B
0
0(1-)
0
B1 -
0
0
0
0
1/
2
C
0
0-(1-)
0
D -
0
0 (1-)
0
```


## Numerical application:

(31 2

- ) =1.1549,
4
3
Eh
$(1+)=0.031662$,
$(1-)=0.018997$
$1 / 2=0.1591$
The distribution of the plane constraints and shearing at the points $O$ and B1 inside the plate is the following one:
$O$

```
xx
yy
xy
xz
yz
H/2
18.9972 18.9972
0
0
0
3h /10
11.3983 11.3983
0
0
0
H /10
3.79943.799400
0
0
0
0
0
H /10
-3.7994 -3.7994
0
0
0
3h /10
-11.3983 -11.3983
O
0
H/2
-18.9972 -18.9972
0
0 0
```


## Handbook of Validation

```
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```

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## :

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$h / 2$
5 sleep
3h/10
4 sleep
h/10
3 sleep
H
0
2 sleep
Lay down 1

B1
$x \boldsymbol{x}$
$y y$
$x y$
$x z$
$y z$
H/2
000
0
0
3h /10
000

```
0
1.5278
H /10
00
0
2.3777
0
0
0
2.3873
H /10
00
0
2.3777
3h /10
00
0
1.5278
H/2
00
0
0
2.3
Uncertainty on the solution
```

Analytical solution.

### 2.4 References bibliographical

## [1]

BATOZ and DHATT. Modeling of the structures by finite elements. Beams and Plates. Volume 2 HERMES, 1990.
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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Quadrangular element of hull DKQ.
The reference mark user is confused with the reference mark of orthotropism.

## Limiting conditions:

DDL_IMPO
(GROUP_NO: AB, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0., DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Not $O$Meshs: M66, M67, M78, M79
Not ANet: M1Not B
Net: M12
Not C
Net: M144
Not D
Net: M133
Not B1
Meshs: M6, M7
Not D1
Meshs: M73, M61
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Version
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Date:
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Author (S):
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:
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3.2 Characteristics
grid
A number of nodes: 171
A number of meshs and type: 144 QUAD4
3.3 Functionalitiestested

## Orders

```
AFFE_MODELE
"MECHANICAL"
"DKT"
MODI_MAILLAGE ORIE_NORME_COQUE
```

AFFE_CARA_ELEM HULL THICK

ANGLE_REP<br>DEFI_MATERIAU ELAS_ORTH

## DEFI_COQU_MULT SLEEP

THICK

ORIENTATION

AFFE_CHAR_MECA FORCE_COQUE
NEAR
ALL
LIAISON_OBLIQUE
ANGLE_NAUT

GROUP_NO

MECA_STATIQUE

CALC_CHAM_ELEM SIGM_ELNO_DEPL

## Handbook of Validation <br> V3.03 booklet: Linear statics of the plates and hulls HI-75/01/010/A

## Code_Aster ${ }^{\circledR}$

Version
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Titrate:
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## 4 <br> Results of modeling A

4.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not $O$

## M78

on lower layer
18.99018 .871
-0.626\%
1.0\%
$x x$
on lower layer
18.99018 .871
-0.626\%
1.0\%
yy
on lower layer
0.000 2.03E08 2.03E08

```
yy
```

on higher layer
0.000 2.03E08
2.03E08
0.01
$x y$
on higher layer
0.0000 .0000 .000
0.01
$x z$
on higher layer
0.0000 .0000 .000
0.01
$y z$
Displacement DZ
-1.1549
-1.1406
-1.232\%
1.25\%
Not B1
M6
on lower layer
0.000 1.052E03
-0.001 0.01
$x x$
on lower layer
0.000 4.208E03
-0.004 0.01
yy
on lower layer
0.000 7.326E01
-0.733 0.74
$x y$
on lower layer
on layer medium
0.0000 .0000 .0000 .01

## yy

Constraints
on layer medium
0.0000 .0000 .0000 .01
xy
on layer medium
0.000 2.690E05
2.
$690 E 050.26$
$x z$
on layer medium
-2.39732-1.90731-20.44\% 21.0\%
$y z$
on higher layer
0.000 1.052E03 0.0010 .26
xx
on higher layer
0.000 4.208E03 0.0040 .26
yy
on higher layer
0.000 7.326E01 0.7330 .74
xy
on higher layer
0.0000 .0000 .0000 .01
$x z$

Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
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Author (S):
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## Reference

Not
NR
NR
NR
$M$
$M$
$M$
$T$
$T$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
O M78
0.0000 .0000 .000 3.1662E02
3.1662E02
0.0000 .000
0.0000 .0000 .000
0.000
0.000 1.8997E02
0.000
0.000

C
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000

D
0.0000 .0000 .000
0.000
0.000 1.8997E02
0.000
0.0000 .0000 .000
0.000
0.0000 .0000 .000 1.5915E01

B1 M7
0.0000 .0000 .000
0.000
0.0000 .0000 .000 1.5915E01

D1 M61
1.5915E01 0.000

D1 M73 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000

Aster
Not
NR
NR
NR
M
M
M
$Q$
$Q$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
y
O M78
0.0000 .0000 .000 3.1451E02
3.1451E02
3.3849E11
1.6740E02 1.6740E02

```
Difference
Not
NR
NR
NR
M
M
M
Q
Q
xx
yy
xy
xx
yy
xy
X
y
O M78
0.000 0.000 0.000-0.666%
0.666% 3.38E11 0.017
0.017
O M79
```


## With

0.0000 .0000 .000 1.07E17 3.53E18 2.363\% 1.36E05 1.36E05

B
0.0000 .000 0.000 2.00E17 9.39E17 2.363\% 1.36E05 1.36E05

C
0.0000 .000 0.000 8.61E18 4.15E18 2.363\% 1.36E05 1.36E05

D
0.0000 .0000 .000 1.35E17 1.16E18 2.363\% 1.36E05 1.36E05

B1 M6
0.0000 .0000 .000 1.75E06
7.01E06 0.001 1.79E06 20.106\%

B1 M7
0.0000 .0000 .000 1.75E06
7.01E06 0.001 1.79E06 20.106\%

D1 M61
20.106\% 1.79E06

D1 M73 0.000 0.000 0.000 7.01E06 1.75E06 0.001 20.106\% 1.79E06
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### 4.2 Parameters <br> of execution

Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use: 5.74 seconds

## 5 Modeling

B

## 5.1 <br> Characteristics of modeling

Quadrangular element of hull DSQ.
The reference mark user is confused with the reference mark of orthotropism.

Limiting conditions:
DDL_IMPO
(GROUP_NO: AB, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: O.)
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Not $O$
Meshs: M66, M67, M78, M79
Not A
Net: M1
Not B
Net: M12
Not C
Net: M144
Not D
Net: M133
Not B1
Meshs: M6, M7
Not D1
Meshs: M73, M61

### 5.2 Characteristics

## grid

A number of nodes: 171
A number of meshs and type: 144 QUAD4

### 5.3 Functionalities

tested

## Orders

```
AFFE_MODELE
"MECHANICAL"
"DST"
MODI_MAILLAGE ORIE_NORME_COQUE
```

AFFE_CARA_ELEM HULL THICK

ANGLE_REP<br>DEFI_MATERIAU ELAS_ORTH<br>DEFI_COQU_MULT SLEEP THICK<br>\section*{ORIENTATION}<br>AFFE_CHAR_MECA FORCE_COQUE<br>NEAR<br>ALL<br>LIAISON_OBLIQUE<br>ANGLE_NAUT<br>GROUP_NO<br>MECA_STATIQUE

CALC_CHAM_ELEM SIGM_ELNO_DEPL

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## 6

Results of modeling B
6.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not $O$

M78
on lower layer
18.99018 .959
-0.158\%
1.0\%
$x x$
on lower layer
18.99018 .959


#### Abstract

yy


on lower layer
0.000 4.73E08 4.73E08
0.01
xy
on lower layer
0.0000 .0000 .000
0.01
$x z$
on lower layer
0.0000 .0000 .000
0.01
$y z$
Constraints
on layer medium
0.0000 .0000 .000
0.01
$x x$
on layer medium
0.0000 .0000 .000
0.01
yy
on layer medium
0.0000 .0000 .000
0.01
xy
on layer medium
$0.000-0.306$
-0.306 0.31
$x z$
on layer medium
0.0000 .3060 .306
0.31
$y z$

## on higher layer

-18.990-18.959 -0.158\% 1.0\%
$x x$
on higher layer
-18.990-18.959-0.158\% 1.0\%
yy
on higher layer
0.000 4.73E08
4.73E08
0.01
xy
on higher layer
0.0000 .0000 .000
0.01
$x z$
on higher layer
0.0000 .0000 .000
0.01
$y z$
Displacement DZ
-1.1549
-1.2012 4.017\%
4.1\%

Not B1

## M6

on lower layer
0.000 1.695E01 0.170 .7
$x x$
on lower layer
0.000 6.933E01
-0.693 0.7
yy
on lower layer
0.000 7.316E01

## on lower layer

0.0000 .0000 .0000 .01
$x z$

```
on lower layer
0.000 0.000 0.000 0.01
yz
on layer medium
0.000 0.000 0.000 0.01
xx
on layer medium
0.000 0.000 0.000 0.01
yy
Constraints
on layer medium
0.000 0.000 0.000 0.01
xy
on layer medium
0.000 5.256E04
5.26E04 0.26
xz
on layer medium
-2.39732-2.32183-3.149% 3.2%
```

$y z$
on higher layer
0.000 1.695E01 0.17
0.7
$x x$
on higher layer
$0.0006 .933 E 010.693$
0.7
yy
on higher layer
0.000 7.316E01 0.7320 .74
xy
on higher layer
0.0000 .0000 .0000 .01
$x z$
on higher layer
0.0000 .0000 .0000 .01
$y z$
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## Reference

Not
NR
NR
$N R$
M
$M$
$M$
$T$
$T$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
0.0000 .0000 .000
1.5915E01

B1 M7
0.0000 .0000 .000
0.000
0.0000 .0000 .000
1.5915E01

D1 M61
1.5915E01 0.000

D1 M73 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000

Aster
Not
NR
NR
NR
M
M
M
$Q$
$Q$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$

```
y
O M78
0.000 0.000 0.000 3.1599E02
3.1599E02 7.878E11 2.0378E02 2.0378E02
O M79
2.0378E02 2.0378E02
O M66
2.0378E02
2.0378E02
O M67
2.0378E02
2.0378E02
With
0.000 0.000 0.000 6.3887E05 6.3887E05 1.8524E02 2.6619E04 2.6619E04
B
0.000 0.000 0.000 6.3887E05 6.3887E05 1.8524E02
2.6619E04 2.6619E04
C
0.000 0.000 0.000 6.388E05 6.388E05 1.8528E02 2.6619E04 2.6619E04
D
0.000 0.000 0.000 6.388E05 6.388E05 1.8548E02
2.6619E04 2.6619E04
B1 M6
0.000 0.000 0.000 2.8258E04 1.1555E03 1.2194E03 3.5045E05 1.5478E01
B1 M7
0.000 0.000 0.000 2.8258E04 1.1555E03 1.2194E03
3.5045E05 1.5478E01
D1 M61
```

3.5045E05
D1 M73 0.000 0.000 0.000 1.1555E03 2.8258E04 1.2194E03
1.5478E01 3.5045E05

```

\section*{Difference}
```

Not
NR
NR
NR
M
M
$0.0000 .0000 .0006 .39 E 05$ 6.39E05 2.491\% 2.66E04 2.66E04 B
$0.0000 .0000 .0006 .39 E 05$ 6.39E05 2.491\% 2.66E04 2.66E04
C
$0.0000 .0000 .0006 .39 E 056.39 E 052.491 \%$ 2.66E04 2.66E04
D
0.0000 .0000 .000 6.39E05 6.39E05 2.491\% 2.66E04 2.66E04

B1 M6
0.0000 .000 0.000 2.83E04
0.0010 .001
3.50E05
-2.743\%
B1 M7
$0.0000 .0000 .0002 .83 E 04$
0.001-0.001
3.50E05
-2.743\%
D1 M61

## $2.743 \% 3.50 E 05$

D1 M73 0.0000 .0000 .000
0.001 2.83E04 $0.0012 .743 \%$ 3.50E05

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V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/1 1/01
Author (S):

V3.03.118-A Page:
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### 6.2 Parameters of execution

Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
5.74 seconds

## 7 Modeling <br> C <br> 7.1 <br> Characteristics of modeling

Triangular element of hull DKT.
The reference mark user is confused with the reference mark of orthotropism.

Limiting conditions:
DDL_IMPO
(GROUP_NO: AB, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)

## FORCE_ARETE

(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

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Not $O$
Meshs: M132, M156, M134, M158
Not A
Net: M1
Not B
Net: M266
Not C
Net: M288
Not D
Net: M23
Not B1
Meshs: M122, M146
Not D1
Meshs: M14, M11

### 7.2 Characteristics

grid
A number of nodes: 170
A number of meshs and type: 288 TRIA3

### 7.3 Functionalities tested <br> Orders <br> ```AFFE_MODELE \\ "MECHANICAL" \\ "DKT" \\ MODI_MAILLAGE ORIE_NORME_COQUE```

AFFE_CARA_ELEM HULL
THICK

ANGLE_REP

DEFI_MATERIAU ELAS

## FORMULATE

## AFFE_CHAR_MECA FORCE_COQUE

 NEARDDL_IMPO
ANGLE_NAUT

# CALC_CHAM_ELEM SIGM_ELNO_DEPL 

DEGE_ELNO_DEPL

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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8
Results of modeling $C$
8.1 Values
tested
Identification

## Reference

Aster Difference
Tolerance
Not $O$

## M134

on lower layer
$18.99019 .0030 .073 \%$
1.0\%
$x x$
on lower layer
18.99018 .899
-0.475\%
1.0\%
yy
on lower layer
0.000 4.428E02
-0.044 0.01
xy
on lower layer
0.0000 .0000 .000
0.01
$x z$
on lower layer
0.0000 .0000 .000
0.01
$y z$
Constraints
on layer medium
0.0000 .0000 .000
0.01
xx
on layer medium
0.0000 .0000 .000
0.01

```
yy
```

on layer medium
0.0000 .0000 .000
0.01
$x y$
on layer medium
$0.0003 .8507 E 01$
-0.385 0.4
$x z$
on layer medium
0.0003 .2121 E01 0.3210 .4
$y z$
on higher layer
-18.990-19.003 0.073\% 1.0\%
$x x$
on higher layer
-18.990-18.899-0.475\% 1.0\%
yy
on higher layer
0.000 4.4282E02 0.0440 .05
$x y$
on higher layer
0.0000 .0000 .000
0.01
$x z$
on higher layer
0.0000 .0000 .000
0.01
$y z$
Displacement DZ
-1.1549
-1.1362
$-1.62 \%$
2.0\%

Not B1

## M122

on lower layer
0.000 1.7562E01
-0.176 0.2
$x x$
on lower layer
0.000 7.0248E01
-0.702 0.8
yy
on lower layer
0.000 7.4243E01
-0.742
0.8
xy
on lower layer
0.0000 .0000 .0000 .01
$x z$
on lower layer
0.0000 .0000 .0000 .01
$y z$
on layer medium
0.0000 .0000 .0000 .01
$x x$
on layer medium
0.0000 .0000 .0000 .01
yy
Constraints
on layer medium
0.0000 .0000 .0000 .01
$x y$
on layer medium
0.000 2.9390E01
-0.294
0.3
$x z$
on layer medium
-2.39732-2.3762-0.878\% 2.0\%
$y z$
on higher layer
0.000 1.7562E01 0.176
0.26
$x x$
on higher layer
0.0007 .0248

E01 0.702
0.8
yy
on higher layer
0.0007 .4243

E01 0.742
0.8
xy
on higher layer
0.0000 .0000 .0000 .01
$x z$
on higher layer
0.0000 .0000 .0000 .01
$y z$
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
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Code_Aster ${ }^{\circledR}$
Version
6.0

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16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key

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## Reference

Not
NR NR
NR
M
M
M
$T$
$T$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
y
O M134 0.000 0.000 0.000 3.1662E02 3.1662E02
0.0000 .0000 .000

O M158
0.000
0.000

O M132
0.000
0.000

O M156
0.000
0.000

With<br>0.0000 .0000 .000<br>0.000<br>0.000 1.8997E02 0.000<br>0.000<br>B<br>0.0000 .0000 .000<br>0.000<br>0.000 1.8997E02 0.000<br>0.000<br>C<br>0.0000 .0000 .000<br>0.000<br>0.000 1.8997E02 0.000<br>0.000<br>D<br>0.0000 .0000 .000<br>0.000<br>0.000 1.8997E02 0.000<br>0.000<br>B1 M122 0.0000 .0000 .000<br>0.000<br>0.0000 .0000 .000<br>1.5915E01<br>B1 M146 0.0000 .0000 .000<br>0.000<br>0.0000 .0000 .000<br>1.5915E01<br>\section*{D1 M11}<br>1.5915E01 0.000<br>D1 M14 0.0000 .0000 .000<br>0.000<br>0.0000 .000<br>1.5915E01<br>0.000<br>Aster<br>Not<br>NR NR<br>NR

```
M
M
M
Q
Q
xx
yy
xy
xx
yy
xy
X
y
O M134 0.000 0.000 0.000 3.1673E02 3.1499E02
7.38E05
2.567E02 2.1414E02
O M158
```


## With

0.0000 .0000 .0008 .33

E04
2.08 E04
1.8758 E02
3.4239 E02
6.153 E03

B
0.0000 .0000 .0003 .07

## E18

3.26 E18
1.8431 E02
1.17 E04
1.17 E04

C
0.0000 .000 0.000-1.18

## E16

1.13 E16
1.8431 E02
1.17 E04
1.17 E04

D
0.0000 .0000 .0008 .33

E04
2.08 E04
1.8758 E02
3.423 E02
6.153 E03

B1 M122 0.000 0.000 0.0002 .93
E04
1.170 E03
1.237 E03
1.9593 E02
1.5841 E01

B1 M146 0.000 0.000 0.000 9.19
E04
3.677 E03
1.0159 E03
1.8524 E02 1.5581 E01

D1 M11
1.2826 E01
2.3404 E02

D1 M14 0.0000 .0000 .0002 .289
E03
6.58 E04
8.50 E04
1.9585 E01
5.3227 E02

Difference
Not
NR NR
NR
M
M
M
$Q$
$Q$
$x x$
$y y$
$x y$
$x x$
$y y$
xy
$X$
y
O M134 $0.0000 .0000 .0000 .033 \%$
-0.515\% 7.38
E05 0.026
0.021

O M158
0.02
0.007

O M132
0.0000 .0000 .0008 .33
E04
2.08 E04
-1.26\%
-0.034
-0.006
B
0.0000 .0000 .0003 .07

E18 3.26
E18 2.98\%
1.17E04
1.17E04

C
$0.0000 .0000 .000-1.18$
E16 1.13
E16
-2.98\%
1.17E04
1.17E04

D
0.0000 .0000 .0008 .33

E04
2.08 E04
-1.26\%
-0.034
0.006

B1 M122 0.0000 .0000 .0002 .93
E04
0.001 0.001-0.02-0.463\%

B1 M146 0.0000 .0000 .0009 .19
E04
$0.004-0.001-0.019-2.101 \%$

### 23.059\%

0.053

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# Code_Aster ${ }^{\circledR}$ 

Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key

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### 8.2 Parameters <br> of execution

Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
7.38 seconds

## 9 Modeling

D
9.1

## Characteristics of modeling

Triangular element of hull DST.
The reference mark user is confused with the reference mark of orthotropism.

Limiting conditions:
(GROUP_NO: AB, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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Not $O$
Meshs: M132, M156, M134, M158
Not A
Net: M1
Not B
Net: M266

Not C
Net: M288
Not D
Net: M23
Not B1
Meshs: M122, M146
Not D1
Meshs: M14, M11

### 9.2 Characteristics

grid
A number of nodes: 170
A number of meshs and type: 288 TRIA3

### 9.3 Functionalities

tested
Orders

```
AFFE_MODELE
    "MECHANICAL"
    "DST"
MODI_MAILLAGE ORIE_NORME_COQUE
```


## AFFE_CARA_ELEM HULL

 THICKANGLE_REP

DEFI_MATERIAU ELAS

# AFFE_CHAR_MECA FORCE_COQUE 

 NEARDDL_IMPO

ANGLE_NAUT

GROUP_NO

FORCE_ARETE

MECA_STATIQUE

## CREA_CHAMP

CALC_CHAM_ELEM SIGM_ELNO_DEPL

DEGE_ELNO_DEPL

Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls
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Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

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10 Results of modeling $D$

### 10.1 Values

tested
Identification
Reference
Aster Difference
Tolerance
Not O

## M134

on lower layer
18.99018 .872
-0.62\% 1.0\%
xx
on lower layer
18.99018 .896
-0.49\% 1.0\%
$y y$
on lower layer
0.000
2.61 E04
2.61 E04
0.01
xy
on lower layer
0.0000 .0000 .000
0.01
$x z$
on lower layer
0.0000 .0000 .000
0.01
$y z$
Constraints
on layer medium
0.0000 .0000 .000
0.01
$x \boldsymbol{x}$
on layer medium
0.0000 .0000 .000
0.01
$y y$
on layer medium
0.0000 .0000 .000
0.01
$x y$
on layer medium
0.000-2.8134

E01
-0.281
0.3
$x z$
on layer medium
0.0003 .0148

E01 0.3010 .4
$y z$
on higher layer
-18.990-18.872-0.62\% 1\%
$\boldsymbol{x} \boldsymbol{x}$
on higher layer
-18.990-18.896-0.49\% 1\%
$y y$
on higher layer
0.000
2.61 E04
2.61 E04
0.01

```
xy
```

on higher layer
0.0000 .0000 .000
0.01
$x z$
on higher layer
0.0000 .0000 .000
0.01
$y z$
Displacement DZ
-1.1549
-1.1951 3.487\%
4.1\%
Not B1
M122
on lower layer
0.000 -5.2518
E01
-0.525 0.8
$x x$
on lower layer
0.000 -6.8948
E01
-0.689 0.8
$y y$
on lower layer
0.000-7.305
E01
-0.731
0.8
$x y$
on lower layer
0.0000 .0000 .0000 .01
$x z$
on lower layer
0.0000 .0000 .0000 .01
$y z$
on layer medium
0.0000 .0000 .0000 .01
$x x$
on layer medium
0.0000 .0000 .0000 .01
$y y$
Constraints
on layer medium
0.0000 .0000 .0000 .01
$x y$
on layer medium
0.000-4.900

E02
-0.049
0.05
$x z$
on layer medium
-2.39732-2.3421-2.3\%
3\%
$y z$
on higher layer
0.0005 .2518

E01 0.525
0.8
$x x$
on higher layer
0.0006 .8948

E01 0.689
0.8
$y y$
on higher layer
0.0007 .3051

E01 0.731
0.8

```
xy
```

on higher layer
0.0000 .0000 .0000 .01

$x z$
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HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0
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SSLS118 - Square plate subjected to a sinusoidal pressure
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16/11/01
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P. MASSIN, NR. RAHNI Key

:
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on higher layer
0.0000 .0000 .0000 .01

$y z$
Reference
Not
NR NR
NR
M
$M$$M$

## O M158

```
---
-
-
-
0 . 0 0 0
0 . 0 0 0
O M132
```

0.000
0.000
O M156
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000
D
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000

## B1 M122 0.0000 .0000 .000

### 0.000

0.0000 .0000 .000
1.5915E01

B1 M146 0.0000 .0000 .000
0.000
0.0000 .0000 .000
$1.5915 E 01$
D1 M11

## --

- 
- 

1.5915E01 0.000

D1 M14 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000

Aster
Not
NR NR
NR
M
M
$M$
$Q$
$Q$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
O M134 0.0000 .0000 .0003 .1453
E02 3.1494 E02
4.36 E07
1.8756 E02
2.009 E02

O M158

### 1.8415 E02

1.87 E04
1.87E04

D
0.0000 .0000 .0007 .61

E04
1.6 E05
1.8869 E02
3.849 E02
9.68 E04

B1 M122 0.0000 .0000 .0008 .75
E04
1.149 E03
1.217 E03
3.266 E03
1.5614 E01

B1 M146 0.0000 .0000 .0002 .21
E04
3.149 E03
9.47 E04
3.148 E03
1.5117 E01

D1 M11

### 1.4437 E01

4.336 E03

D1 M14 0.0000 .0000 .0001 .446
E03
4.34 E04
4.30 E04
1.6235 E01
9.371 E03

Difference
Not
NR NR
NR
M
M

```
M
Q
Q
xx
yy
xy
xx
yy
xy
X
y
O M134 0.000 0.000 0.000
-0.66%
-0.531% 4.36
E07 0.019
0 . 0 2
O M158
-
0 . 0 1 8
O M132
-0.019
-0.02
O M156
0.022
-0.018
With
0.0000 .0000 .0007 .61
E04 1.6
E05 0.673\% 0.0389 .68
E04
B
```


### 0.0000 .000 0.000-2.69

E05 2.69
E05 3.065\%
1.87 E04
1.87 E04

C
0.0000 .000 0.000-2.69

E05 2.69
E05 3.065\%
1.87 E04
1.87E04

D
0.0000 .0000 .0007 .61

E04 1.6
E05 0.673\% 0.0389 .68
E04
B1 M122 0.0000 .0000 .0008 .75
E04
0.001 0.001-0.003-1.891\%

B1 M146 0.0000 .0000 .0002 .21
E04
0.003 -9.47

E04 0.003 5.013\%
D1 M11
--
-
-
-
-9.29\%
0.004

D1 M14 0.0000 .0000 .000
0.001
4.34

E04 4.30
E04 2.013\%
0.009

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Code_Aster ${ }^{\circledR}$
Version
6.0

## Titrate:

SSLS118 - Square plate subjected to a sinusoidal pressure

# Date: 

16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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### 10.2 Parameters

of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
7.38 seconds

## 11 Modeling

E

### 11.1 Characteristics of modeling

Element of hull COQUE_3D triangle.
The reference mark user is confused with the reference mark of orthotropism.

## Limiting conditions:

DDL_IMPO
(GROUP_NO: AB, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
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SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

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Not $O$
Meshs: M132, M156, M134, M158
Not A
Net: M1
Not B
Net: M266
Not C
Net: M288
Not D
Net: M23

Not B1<br>Meshs: M122, M146<br>Not D1<br>Meshs: M14, M11<br>11.2 Characteristics<br>grid<br>A number of nodes: 626<br>A number of meshs and type: 288 TRIA6

### 11.3 Functionalities

tested
Orders

AFFE_MODELE
"MECHANICAL"
"COQUE_3D"

CREA_MAILLAGE MODI_MAILLE
"TRIA6_7"
MODI_MAILLAGE ORIE_NORME_COQUE

AFFE_CARA_ELEM HULL
THICK

## ANGLE_REP

## DEFI_MATERIAU ELAS

## FORMULATE

AFFE_CHAR_MECA FORCE_COQUE NEAR<br>DDL_IMPO<br>ANGLE_NAUT<br>\section*{GROUP_NO}<br>FORCE_ARETE<br>\section*{MECA_STATIQUE}<br>\section*{CREA_CHAMP}<br>\section*{CALC_CHAM_ELEM SIGM_ELNO_DEPL}<br>DEGE_ELNO_DEPL

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A

## Code_Aster ${ }^{\circledR}$

Version
6.0

## Titrate:

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

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## 12 Results of modeling $E$

### 12.1 Values

tested
Identification
Reference
Aster Difference
Tolerance
Not $O$

## M134

on lower layer
18.99018 .920
-0.364\%
1.0\%
$x x$
on lower layer
$18.99019 .0990 .5791 .0 \%$
$y y$
on lower layer
0.000-1.2540

E01
-0.125
0.2
$x y$
on lower layer
0.000-1.688

E02
-0.017 0.02
$x z$
on lower layer
0.0007 .210
$y z$
Constraints
on layer medium
0.000

4 E15
4 E15
0.01
$x x$
on layer medium
0.000

2 E14
2 E14
0.01
$y y$
on layer medium
0.000

4 E15
4 E15
0.01
$x y$
on layer medium
0.000-1.688

E02
-0.017 0.02
$x z$
on layer medium
0.0007 .210

E02 0.0720 .08
$y z$
on higher layer
-18.990-18.920
$-0.364 \% 1.0 \%$
$x x$
on higher layer
-18.990-19.099 0.579\% 1.0\%
$y y$
on higher layer
0.0001 .2540

E01 0.1250 .2
xy
on higher layer
0.000-1.688

E02
-0.017 0.02
$x z$
on higher layer
0.0007 .210

E02 0.0720 .08
$y z$
Displacement DZ
-1.1549
-1.2029 4.164\%
4.2\%

Not B1

M122
on lower layer
0.0006 .291

E02 0.0630 .08
$x x$
on lower layer
0.0001 .448

E01 0.1450 .2
$y y$
on lower layer
0.000-2.025

E02 0.02
0.03
$x y$
on lower layer
$0.000-4.916$
$x z$
on lower layer
0.000-1.542
-1.543
1.6
$y z$
on layer medium
0.000
5 E16
5 E16
0.01
$\boldsymbol{x x}$
on layer medium
0.000
2 E15
2 E15
0.01
$y y$
Constraints
on layer medium
0.000
4 E15
4 E15
0.01
$x y$
on layer medium
0.000-4.916
E02
-0.049
0.06
$x z$
on layer medium
-2.39732-1.5426-35.651\%
36.0\%
$y z$
on higher layer
0.000-6.291

E02
-0.063
0.08
$x x$
on higher layer
0.000-1.448

E01
-0.145
0.2
$y y$
on higher layer
0.0002 .025

E02 0.02
0.03
$x y$
on higher layer
0.000-4.916

E02
-0.049
0.05
$x z$
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on higher layer
0.000 -1.5426
-1.543
1.6
$y z$
Reference
Not
NR NR
NR
M
M
M
$T$
$T$
$x \boldsymbol{x}$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
O M134 0.000 0.000 0.000 3.1662E02 3.1662E02
0.0000 .0000 .000

O M158
--
-
-
-
0.000
0.000

O M132
0.000
0.000

O M156
0.0000.000
With
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000
B
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000
C
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000
D0.0000 .0000 .0000.000
0.000 1.8997E02 0.000
0.000
B1 M122 0.0000 .0000 .000
0.000
0.0000 .0000 .000
1.5915E01
B1 M146 0.0000 .0000 .000
0.000
0.0000 .0000 .000
1.5915E01
D1 M11
-- -
1.5915E01 0.000
D1 M14 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000
Aster

```
Not
NR NR
NR
M
M
M
Q
Q
xx
yy
xy
xx
yy
xy
X
y
O M134 0.000 0.000 0.000 3.1534
E02 3.1833 E02
2.09 E04
1.688 E03
7.210 E03
O M158
1.494 E03
8.008 E03
O M132
```


### 1.910 E03

### 3.90 E04

## With

0.0000 .0000 .0003 .69

E04

### 2.18 E04

1.9133 E02

### 5.387 E03

7.027 E03

## B

0.0000 .0000 .0001 .71

## E04

1.61 E04
1.8932 E02
1.511 E03
$1.513 \mathrm{E03}$
$C$
0.0000 .0000 .0001 .70

## E04

1.56 E04
1.8815 E02
1.463 E03
1.508 E03

D
0.0000 .0000 .0003 .66

E04
$2.13 \mathrm{E04}$
1.8998 E02
5.066 E03
6.800 E03

B1 M122 0.0000 .0000 .0001 .04
E04
2.41 E04
3.4 E05
4.916 E03
1.5426 E01

B1 M146 0.0000 .0000 .0002 .9

## E05

### 1.64 E04

6.5 E07

### 2.7681 E01

1.8932 E01

## D1 M11

1.8488 E01

2.7113 E02

D1 M14 0.0000 .0000 .0004 .30
E04
3.07 E04
4.2 E05
1.4791 E01
1.1165 E02

Difference
Not
NR NR
NR
M
M
$M$
$\underset{Q}{Q}$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
O M134 $0.0000 .0000 .0000 .404 \%$
$0.538 \% 2.09$
E04 0.002
0.007

O M158
-0.001
0.008
$O$ M132
---
-
-
-
-0.001
0.001
$O$ M156

```
-0.002-3.90
```

E04

With
0.0000 .0000 .0003 .69

E04
2.18 E04
$0.716 \%$
-0.005
0.007

B
0.0000 .0000 .0001 .71

E04
1.61 E04
-0.344\%
-0.002
0.002

C
0.0000 .0000 .0001 .70

E04
1.56 E04
-0.958\%
-0.001
-0.002
D
0.0000 .0000 .0003 .66

E04
2.13 E 04
0.003\%
-0.005

B1 M122 0.0000 .0000 .0001 .05
E04 2.41
E04 3.4
E05 0.005 3.072\%
B1 M146 0.0000 .0000 .0002 .9
E05
1.65 E04
6.5 E07
0.028
18.955\%

D1 M11
$16.17 \%$
-0.027
D1 M14 0.0000 .0000 .0004 .31
E04
3.07 E04
4.2 E05
-7.06\%
-0.011
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### 12.2 Parameters

of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
11.05 seconds

## 13 Modeling

F

### 13.1 Characteristics of modeling

Element of hull COQUE_3D quadrangle.
The reference mark user is confused with the reference mark of orthotropism.

Limiting conditions:
DDL_IMPO
(GROUP_NO: AB, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
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Not $O$
Meshs: M66, M67, M78, M79
Not A
Net: M1
Not B
Net: M12
Not C
Net: M144
Not D
Net: M133
Not B1
Meshs: M6, M7
Not D1
Meshs: M73, M61

### 13.2 Characteristics

grid
A number of nodes: 482
A number of meshs and type: 144 QUAD8

### 13.3 Functionalities

tested
Orders

AFFE_MODELE "MECHANICAL" "COQUE_3D"<br>\section*{CREA_MAILLAGE MODI_MAILLE} "QUAD8_9"<br>AFFE_CARA_ELEM HULL THICK

ANGLE_REP<br>DEFI_MATERIAU ELAS

## FORMULATE

AFFE_CHAR_MECA FORCE_COQUE NEAR<br>DDL_IMPO<br>ANGLE_NAUT<br>\section*{GROUP_NO}<br>FORCE_ARETE

## MECA_STATIQUE

## CREA_CHAMP

## CALC_CHAM_ELEM SIGM_ELNO_DEPL

## DEGE_ELNO_DEPL

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls HI-75/01/010/A

## Code_Aster ${ }^{\circledR}$

Version
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Titrate:
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Date:
16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key
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## 14 Results of modeling $F$

### 14.1 Values

tested
Identification
Reference
Aster Difference
Tolerance
Not $O$

## M78

on lower layer
$18.99019 .3061 .665 \%$
2.0\%
$\boldsymbol{x} \boldsymbol{x}$
on lower layer
$18.99019 .7634 .073 \%$
5.0\%
$y y$
on lower layer
0.000 -5.19

E01
-0.52

```
0.6
xy
on lower layer
0.000-4.85
E02
-0.049 0.06
xz
on lower layer
0.000 1.4700 0.147 0.2
yz
Constraints
on layer medium
0.000
8.9 E15
8.9 E15
0.01
xx
on layer medium
0.000
1.0 E14
1.0 E14
0.01
yy
on layer medium
0.000
1.8 E14
1.8 E14
0.01
xy
on layer medium
0.000-4.85
E02
-0.049 0.1
xz
on layer medium
0.000 1.470
E01 0.147 0.2
yz
```


## on higher layer

-18.990 -19.306 1.665\% 2.0\%
$x x$
on higher layer
-18.990-19.763 4.073\% 5.0\%
$y y$
on higher layer
0.0005 .199

E01 0.5200 .6
$x y$
on higher layer
0.000-4.853

E02
-0.049 0.06
$x z$
on higher layer
0.0001 .470

E01 0.1470 .2
$y z$
Displacement DZ
-1.1549
-1.2038
4.24\%
4.5\%

Not B1

## M6

on lower layer
0.0001 .924

E02 0.0190 .05
$x x$
on lower layer
0.0003 .190

E02 0.0320 .05
$y y$
on lower layer
0.000-3.526

E02
-0.035
0.05
$x y$
on lower layer
0.000-3.425

E03
-0.003
0.01
$x z$
on lower layer
0.000-1.671
-1.671
2.0
$y z$
on layer medium
0.000
4.2 E015
4.2 E15
0.01
$\boldsymbol{x} \boldsymbol{x}$
on layer medium
0.000
1.6 E14
1.6 E14
0.01
$y y$
Constraints
on layer medium
0.000
1.2 E14
1.2 E14
0.01
$x y$
on layer medium
0.000-3.425
on layer medium
-2.39732-1.671-30.274\%
35.0\%
$y z$
on higher layer
0.000-1.924

## E02

-0.019
0.05
$\boldsymbol{x} \boldsymbol{x}$
on higher layer
0.000-3.190

E02
-0.032
0.05
$y y$
on higher layer
0.0003 .526

E02 0.035
0.05
$x y$
on higher layer
0.000-3.425

E03
-0.003
0.01
$x z$
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Code_Aster ${ }^{\circledR}$
Version
6.0

```
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```

on higher layer
0.000-1.671
-1.671
2.0
$y z$
Reference
Not
NR
NR
NR
M
$M$
M
$T$
$T$
$x \boldsymbol{x}$
$y y$
$x y$
$\boldsymbol{x} \boldsymbol{x}$
$y y$
$x y$
$X$
$y$
O M78
0.0000 .0000 .000 3.1662E02
3.1662E02
0.0000 .0000 .000
O M79

```
0.000
0 . 0 0 0
O M66
```


## - -

```
0.000
0 . 0 0 0
With
0.000 0.000 0.000
0 . 0 0 0
0.000 1.8997E02 0.000
0.000
B
0.000 0.000 0.000
0 . 0 0 0
0.000 1.8997E02 0.000
0 . 0 0 0
C
0.000 0.000 0.000
0 . 0 0 0
0.000 1.8997E02 0.000
0 . 0 0 0
D
0.000 0.000 0.000
0 . 0 0 0
0.000 1.8997E02 0.000
0 . 0 0 0
B1 M6
0.000 0.000 0.000
0 . 0 0 0
0.000 0.000 0.000
1.5915E01
B1 M7
0.000 0.000 0.000
```


### 1.5915E01 0.000

D1 M73 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000

Aster
Not
NR
NR
NR
$M$
M
$M$
$\boldsymbol{Q}$
$\boldsymbol{Q}$
$x x$
$y y$
$x y$

## $x x$

$y y$
$x y$
$X$
$y$
O M78
0.0000 .0000 .000 -3.2177

E02 3.2939 E02

### 8.66 E04

4.853 E03
1.4700 E02

O M79
1.63 E04
1.26 E04
B
0.0000 .0000 .0007 .7
E08
8.5 E08
1.70 E04

D
0.0000 .0000 .0006 .7

E08
6.0 E08
1.8705 E02
2.49 E05
2.17 E05

B1 M6
0.0000 .000 0.000-3.20

E05
5.31 E05
5.87 E05
3.42 E04
1.6715 E01

B1 M7
0.000 0.000 0.000-3.20

E05
5.31 E05
2.24 E05
3.32 E04
1.6714 E01

D1 M61
-1.5743
E01
-2.81
E04
D1 M73 0.0000 .0000 .0001 .72
E05
2.15 E05
2.34 E04
1.5743 E01
3.12 E04

Difference
Not
NR
NR
NR
$y$
O M78
0.0000 .0000 .000
1.624\%
4.031\% 8.67
E04 0.005
0.015
O M79
-0.002
0.014
O M66
-0.002
0.014
O M67
-0.005
0.015
With
0.000 0.000 0.000-6.2

# E08 1.192\% 1.63 E04 

1.26 E04

B
0.0000 .0000 .0007 .7

## E08 8.5

E08 1.672\% 2.11 E04
8.0 E05

C
$0.0000 .0000 .000-8.7$
E08 7.7
E08 0.742\% 3.0 E07
1.70 E04

D
0.0000 .0000 .0006 .7

E08 6.0
E08 1.541\% 2.49 E05
2.17 E05

B1 M6
0.0000 .000 0.000-3.20

E05
5.31 E05
8.87 E05 3.42

E04 5.026\%
B1 M7
0.0000 .000 0.000-3.20

E05
5.31 E05
2.24 E05 3.32

E04 5.023\%
D1 M61

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### 14.2 Parameters

of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
9.83 seconds

15 Modeling
G

### 15.1 Characteristics of modeling

Triangular element of hull DST.
The model of plate associated with modeling $D$ is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to beta. The classification of the meshs is identical to that of modeling $D$.

## Limiting conditions:

## LIAISON_OBLIQUE

(GROUP_NO: AB, ANGL_NAUT= (20. , 30. , 0.), DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, ANGL_NAUT= (20. , 30. , 0.), DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, ANGL_NAUT= (20. , 30. , 0.), DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, ANGL_NAUT= (20., 30. , 0.), DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, ANGL_NAUT= (20. , 30. , 0.), DX: 0. , DY: 0. , DRX: 0., DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
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Not $O$
Meshs: M132, M156, M134, M158
Not A
Net: M1

# Not B <br> Net: M266 <br> Not C <br> Net: M288 <br> Not D <br> Net: M23 <br> Not B1 <br> Meshs: M122, M146 <br> Not D1 <br> Meshs: M14, M11 <br> <br> 15.2 Characteristics <br> <br> 15.2 Characteristics <br> grid <br> A number of nodes: 170 <br> A number of meshs and type: 288 TRIA3 

### 15.3 Functionalities

tested
Orders

AFFE_MODELE "MECHANICAL" "DST"

MODI_MAILLAGE ORIE_NORME_COQUE

AFFE_CARA_ELEM HULL
THICK

## ANGLE_REP

DEFI_MATERIAU ELAS

## FORMULATE

AFFE_CHAR_MECA FORCE_COQUE
NEAR

## LIAISON_OBLIQUE <br> ANGLE_NAUT

## GROUP_NO

FORCE_ARETE

## MECA_STATIQUE

## CREA_CHAMP

## CALC_CHAM_ELEM SIGM_ELNO_DEPL

## DEGE_ELNO_DEPL

### 15.4 Remarks

The value of reference of displacement to the point $O$ is obtained by projecting calculated displacement
for modeling $D$ in the reference mark turned (displacement for modeling $\boldsymbol{D}$ being vertical, it new displacement is a function of the projection of axis $Z$ ).
In the local reference mark, the projection of axis $Z$ is as follows:
$\sin \cos$

In addition, the expression of the sinusoidal pressure in the turned reference mark becomes:
Error! Objects cannot be created starting from the field codes of working.
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16 Results of modeling G
16.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not $O$
on lower layer
0.0000 .0000 .000
0.02
$x z$
on lower layer
0.0000 .0000 .000
0.08
$y z$
Constraints
on layer medium
0.000
3. E14
3. E14
0.01
$x x$
on layer medium
0.000
7. E14
7. E14
0.01
$y y$
on layer medium
0.000
8. E14
8. E14
0.01
xy
on layer medium
0.000 -2.8134
E01
-0.281
0.02
$x z$
on layer medium
0.0003 .0148
E01 0.3010 .08
$y z$
on higher layer-18.990-18.872-0.62\% 1.0\%
xx
on higher layer
-18.990-18.896-0.49 1.0\%
$y y$
on higher layer
0.000
2.61 E04
2.61 E04
0.2
$x y$
on higher layer
0.0000 .0000 .000
0.02
$x z$
on higher layer
0.0000 .0000 .000
0.08
$y z$
Displacement DZ
-1.0002
-1.0350 3.484\%
4.2\%

## Not B1

## M122

on lower layer
0.000 -5.2518

E01
-0.525 0.08
$x x$
on lower layer
0.000 -6.8948

E01
-0.689 0.2
$y y$
on lower layer
0.000-7.305

E01
-0.731
0.03
xy
on lower layer
0.0000 .0000 .0000 .05
$x z$
on lower layer
0.0000 .0000 .0001 .6
$y z$
on layer medium
0.000

1. E15
2. E15
0.01
$x x$
on layer medium
0.000
3. E15
4. E15
$y y$
Constraints
on layer medium
0.000
5. E15
6. E15
0.01
$x y$
on layer medium
0.000-4.900

E02
-0.049
0.06
$x z$
on layer medium
-2.39732-2.3421-2.3\% 36.0\%
$y z$
on higher layer
0.0005 .2518

E01 0.525
0.08
$x x$
on higher layer
0.0006 .8948

E01 0.689
0.2
$y y$
on higher layer
0.0007 .3051

E01 0.731
0.03
$x y$
on higher layer
0.0000 .0000 .0000 .05
$x z$
on higher layer
0.0000 .0000 .0001 .6
$y z$
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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## Reference

Not
NR NR
NR
M
M
M
$T$
$T$
$x x$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
O M134 0.0000 .000 0.000 3.1662E02
3.1662E02 0.000
0.000
0.000

O M158

```
---
```

--
-
-
0 . 0 0 0
0.000
With
0.000 0.000 0.000
0 . 0 0 0
0.000 1.8997E02 0.000
0 . 0 0 0
B
0.000 0.000 0.000
0.000
0.000 1.8997E02 0.000
0.000
C
0.000 0.000 0.000
0.000
0.000 1.8997E02 0.000
0.000
D
0.000 0.000 0.000
0.000
0.000 1.8997E02 0.000
0.000
B1 M122 0.000 0.000 0.000
0 . 0 0 0
0.000 0.000 0.000
1.5915E01

```

\title{
B1 M146 0.0000 .0000 .000
}
0.000
0.0000 .0000 .000
1.5915E01

D1 M11

\subsection*{1.5915E01 0.000}

D1 M14 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000

Aster
Not
NR NR
NR
M
M
M
\(Q\)
\(Q\)
\(x x\)
\(y y\)
\(x y\)
\(x x\)
\(y y\)
\(x y\)
\(X\)
\(y\)
O M134 0.0000 .0000 .0003 .1453
E02
3.1494 E02
4.36 E07
1.8756 E02
2.009 E02

O M158

\author{
D \\ 0.0000 .0000 .0007 .61 \\ E04 \\ 1.6 E05 \\ 1.8869 E02 \\ 3.849 E02 \\ 9.68 E04 \\ B1 M122 0.0000 .0000 .0008 .75 \\ E04 \\ 1.149 E03 \\ 1.217 E03 \\ 3.266 E03 \\ 1.5614 E01 \\ B1 M146 0.0000 .0000 .0002 .21 \\ E04 \\ 3.149 E03 \\ 9.47 E04 \\ 3.148 E03 \\ 1.5117 E01 \\ D1 M11
}
1.4437 E01
4.336 E03

D1 M14 0.000 0.000 0.000 1.446
E03
4.34 E04
4.30 E04
1.6235 E01
9.371 E03

Difference
Not
NR
\(N R\)
NR
M
M
M
\(Q\)
```

Q
xx
yy
xy
xx
yy
xy
X
y
O M134 0.000 0.000 0.000 0.66%
-0.531% 4.36
E07 0.019
0 . 0 2
O M158
0.022
0 . 0 1 8
O M132
-
-0.019
-0.02
O M156
0.022
-0.018
With
0.000 0.000 0.000 7.61
E04 1.6
E05 0.673% 0.038
-9.68
E04
B
0.000 0.000 0.000-2.69
E05 2.69

```

\section*{E05 3.065\% 1.87 E04}
1.87 E04

C
0.0000 .000 0.000-2.69

E05 2.69
E05 3.065\% 1.87 E04
1.87 E04

D
0.0000 .0000 .0007 .61

E04 1.6
E05 0.673\% 0.0389 .68
E04
B1 M122 0.0000 .0000 .0008 .75
E04
0.0010 .001 -0.003
-1.891\%
B1 M146 0.0000 .0000 .0002 .21
E04
0.003 -9.47

E04 0.003 5.013\%
D1 M11
- -
-
-
-9.29\%
0.004

D1 M14 0.0000 .0000 .000
0.001
4.34

E04 4.30
E04 2.013\%
0.009

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure

\section*{Date:}

16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
34/62

\subsection*{16.2 Parameters}
of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
7.98 seconds

\section*{17 Modeling \\ H}

\subsection*{17.1 Characteristics of modeling}

Triangular element of hull COQUE_3D.
The model of plate associated with modeling \(E\) is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to beta. The classification of the meshs is identical to that of modeling \(E\).

\section*{Limiting conditions:}

\section*{LIAISON_OBLIQUE}
(GROUP_NO: AB, ANGL_NAUT = (20. , 30. , 0.), DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, ANGL_NAUT= (20. , 30. , 0.), DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, ANGL_NAUT= (20. , 30. , 0.), DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, ANGL_NAUT= (20., 30. , 0.), DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, ANGL_NAUT= (20. , 30. , 0.), DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

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Not \(O\)
Meshs: M132, M156, M134, M158
Not A
Net: M1
Not B
Net: M266
Not C
Net: M288
Not D
Net: M23
Not B1

Meshs: M122, M146
Not D1
Meshs: M14, M11

\subsection*{17.2 Characteristics}
grid
A number of nodes: 626
A number of meshs and type: 288 TRIA6

\subsection*{17.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE "MECHANICAL" "COQUE_3D" \\ CREA_MAILLAGE MODI_MAILLE \\ "TRIA6_7" \\ MODI_MAILLAGE ORIE_NORME_COQUE
}

AFFE_CARA_ELEM HULL THICK

\section*{ANGLE_REP}

\section*{DEFI_MATERIAU ELAS}

FORMULATE

\title{
AFFE_CHAR_MECA FORCE_COQUE \\ NEAR
}

\section*{LIAISON_OBLIQUE \\ ANGLE_NAUT}

\section*{GROUP_NO}

FORCE_ARETE

MECA_STATIQUE

CREA_CHAMP

\section*{CALC_CHAM_ELEM SIGM_ELNO_DEPL}

\author{
DEGE_ELNO_DEPL
}

\subsection*{17.4 Remarks}

The value of reference of displacement to the point \(O\) is obtained by projecting calculated displacement
for modeling \(E\) in the reference mark turned (displacement for modeling \(E\) being vertical, it new displacement is a function of the projection of axis Z).
In the local reference mark, the projection of axis \(Z\) is as follows:
\(\sin \cos\)
sin \(\sin\), with \(=20\). and \(=\)

In addition, the expression of the sinusoidal pressure in the turned reference mark becomes:
\(\cos \cos X+\sin \cos y-\sin Z\)
\(\cos y-\sin X\)
\(P=F \sin\)

0
\(\sin\)
has
has
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

\section*{Titrate:}

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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18 Results of modeling H
18.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not \(O\)

\section*{M134}
on lower layer
18.99018 .920
-0.364\%
1.0\%
\(x x\)
on lower layer
\(18.99019 .0990 .5791 .0 \%\)
\(y y\)
on lower layer
0.000-1.2540

E01
-0.125
0.01
\(x y\)
on lower layer
0.000-1.688

E02
-0.017 0.01
\(x z\)
on lower layer
0.0007 .210

E02 0.0720 .01
\(y z\)
Constraints
on layer medium
0.000 1.E13 1.

E13
0.01
\(x x\)
on layer medium
0.000
3. E14
3. E14
0.01
\(y y\)
on layer medium
0.000
2. E13
2. E13
0.01
\(x y\)
on layer medium
0.000-1.688
E02
-0.017 0.02
\(x z\)
on layer medium
0.0007 .210
E02 0.0720 .08
\(y z\)
on higher layer
-18.990-18.920
-0.364\% 1\%
\(x x\)
on higher layer
-18.990-19.099 0.579\% 1\%
\(y y\)
on higher layer
0.0001 .2540
E01 0.1250 .2
\(x y\)
on higher layer
0.000-1.688
E02
-0.017 0.02
\(x z\)
on higher layer
0.0007 .210
E02 0.0720 .08
\(y z\)
Displacement DZ

\section*{M122}
on lower layer
0.0006 .291

E02 0.0630 .08
\(x x\)
on lower layer
0.0001 .448

E01 0.1450 .2
\(y y\)
on lower layer
0.000-2.025

E02 0.02
0.03
\(x y\)
on lower layer
0.000-4.916

E02
-0.049
0.05
\(x z\)
on lower layer
0.000-1.542

E01
-1.543
1.6
\(y z\)
on layer medium
0.000
2. E14
2. E14
0.01

\section*{Constraints}
on layer medium
0.000
2. E15
2. E15
0.01
\(x y\)
on layer medium
0.000-4.916

E02
-0.049
0.06
\(x z\)
on layer medium
-2.39732-1.5426-35.651\%
36\%
\(y z\)
on higher layer
0.000-6.291

E02
-0.063
0.08
\(\boldsymbol{x} \boldsymbol{x}\)
on higher layer
0.000-1.448

E01
-0.145
0.2
\(y y\)
on higher layer
0.0002 .025
\(x y\)
on higher layer
0.000-4.916

E02
-0.049
0.05
\(x z\)
on higher layer
\(0.000-1.5426\)
-1.543
1.6
\(y z\)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0
Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author \((\) S):
P. MASSIN, NR. RAHNI Key
\(:\)
V3.03.118-A Page:
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\section*{Reference}

Not
NR NR
NR
M
\(M\)
M
\(T\)
\(T\)

\title{
0.000 1.8997E02 0.000
}
0.000

D
0.0000 .0000 .000
0.000
0.000 1.8997E02
0.000
0.000

B1 M122 0.0000 .0000 .000
0.000
0.0000 .0000 .000 1.5915E01

B1 M146 0.000 0.000 0.000
0.000
0.0000 .0000 .000 1.5915E01

\section*{D1 M11}
--
-
-
\(1.5915 E 01\)
0.000
D1 M14 0.0000 .0000 .000
0.000
\(0.0000 .0001 .5915 E 01\)
0.000
Aster
Not
NR \(N R\)
\(N R\)
\(M\)
\(M\)
\(M\)
\(Q\)
\(Q\)
\(x x\)
\(y y\)
\(x y\)
\(x x\)
\(y y\)
\(x y\)
\(X\)
```

y
O M134 0.000 0.000 0.000 3.1534
E02 3.1833 E02
2.09 E04
1.688 E03
7.210 E03
O M158
1.494 E03
8.008 E03
O M132
1.283 E03
1.248 E03
O M156
1.910 E03
3.90 E04
With
0.000 0.000 0.000 3.69
E04
2.18 E04
1.9133 E02
5.387 E03
7.027 E03
B
0.000 0.000 0.000 1.71
E04

```
1.61 E04
1.8932 E02
1.511 E03
1.513 E03

C
0.0000 .0000 .0001 .70

E04
1.56 E04
1.8815 E02
1.463 E03
1.508 E03

D
0.0000 .0000 .0003 .66

E04
2.13 E04
1.8998 E02
5.066 E03
6.800 E03

B1 M122 0.000 0.000 0.0001 .04
E04
2.41 E04
3.4 E05
4.916 E03
1.5426 E01

B1 M146 0.0000 .0000 .0002 .9
E05
1.64 E04
6.5 E07
2.7681 E01
1.8932 E01

D1 M11
1.8488 E01
2.7113 E02

D1 M14 0.0000 .0000 .0004 .30
E04
3.07 E04
4.2 E05

\subsection*{1.4791 E01}
1.1165 E02

\section*{Difference}

Not
NR NR
NR
M
M
\(M\)
\(Q\)
\(Q\)
\(x x\)
\(y y\)
\(x y\)
\(x x\)
\(y y\)
\(x y\)
\(X\)
\(y\)
O M134 \(0.0000 .0000 .0000 .404 \%\)
\(0.538 \% 2.09\)
E04 0.002
0.007

O M158
-0.001
0.008

O M132
-0.001
0.001

O M156
-0.002
-3.90
E04
With
0.0000 .0000 .0003 .69

E04
2.18 E04
0.716\%
-0.005
0.007

B
0.0000 .0000 .0001 .71

E04
1.61 E04
-0.344\%
-0.002
0.002

C
0.0000 .0000 .0001 .70

E04
1.56 E04
-0.958\%
-0.001
-0.002
D
0.0000 .0000 .0003 .66

E04
2.13 E04
0.003\%
-0.005
-0.007
B1 M122 0.0000 .0000 .0001 .05
E04 2.41
E04 3.4
E05 0.005 3.072\%
B1 M146 0.0000 .0000 .0002 .9
E05 1.65 E04
6.5 E07
0.028
18.955\%

D1 M11
```

- 

16.17%
-0.027
D1 M14 0.000 0.000 0.000 4.31
E04 3.07
E04 4.2
E05 7.06%
-0.011
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A

```

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key :
V3.03.118-A Page:
38/62

\subsection*{18.2 Parameters}
of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
11.9 seconds

\section*{19 Modeling \\ I \\ 19.1 Characteristics of modeling}

Quadrangular element of hull DSQ. The plate is modelled with a grid \(24 \times 24\). The reference mark user is confused with the reference mark of orthotropism.

\section*{Limiting conditions:}

DDL_IMPO
(GROUP_NO: AB, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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Not \(O\)
Meshs: M276, M277, M300, M301
Not A
Net: M1
Not B
Net: M24
Not C
Net: M576

\author{
Not D \\ Net: M553 \\ Not B1 \\ Meshs: M12, M13 \\ Not D1 \\ Meshs: M289, M265
}

\subsection*{19.2 Characteristics}
grid
A number of nodes: 626
A number of meshs and type: 576 QUAD4

\subsection*{19.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE "MECHANICAL" "DST"
}

AFFE_CARA_ELEM HULL THICK

\section*{ANGLE_REP}

\section*{DEFI_MATERIAU ELAS}

\section*{FORMULATE}

AFFE_CHAR_MECA FORCE_COQUE
NEAR

\title{
DDL_IMPO \\ ANGLE_NAUT
}

\section*{GROUP_NO}

FORCE_ARETE

\section*{MECA_STATIQUE}

\section*{CREA_CHAMP}

\section*{CALC_CHAM_ELEM SIGM_ELNO_DEPL}

DEGE_ELNO_DEPL

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
```

Code_Aster ${ }^{\circledR}$
Version
6.0
Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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```

20 Results of modeling I

\subsection*{20.1 Values}
tested

\section*{Identification}

Reference
Aster Difference

\section*{Tolerance}

\author{
Not O
}

\section*{M300}
on lower layer
18.99018 .989
-0.005\%
1.0\%
\(\boldsymbol{x} \boldsymbol{x}\)
on lower layer
18.99018 .989
-0.005\%
1.0\%
\(y y\)
on lower layer
\(0.000-1.0\)
E14
1.0E14
0.01
\(x y\)
on lower layer
0.0000 .0000 .000
0.01
\(x z\)
on lower layer
0.0000 .0000 .000
0.01
\(y z\)
Constraints
on layer medium
0.0000 .0000 .000
\(x x\)
on layer medium
0.0000 .0000 .000
0.01
\(y y\)
on layer medium
0.0000 .0000 .000
0.01
\(x y\)
on layer medium
0.000 -0.1554
-0.155 0.26
\(x z\)
on layer medium
0.0000 .15540 .1550 .26
\(y z\)
on higher layer
-18.990 -18.989 -0.005 1.0\%
xx
on higher layer
-18.990 -18.989-0.005 1.0\%
\(y y\)
on higher layer
0.0001 .0

E14 1.0E14
0.01
\(x y\)
on higher layer
0.0000 .0000 .000
0.01
\(x z\)
on higher layer
0.0000 .0000 .000
0.01
\(y z\)

\section*{Displacement DZ}
-1.1549
-1.2120
4.95\%
5.0\%

Not B1

\section*{M12}
on lower layer
0.000 -9.1146

E02
-0.091 0.1
\(\boldsymbol{x} \boldsymbol{x}\)
on lower layer
0.000-3.6678

E01
-0.367 0.4
\(y y\)
on lower layer
0.000 -3.7119

E01
-0.371
0.74
\(x y\)
on lower layer
0.0000 .0000 .0000 .01
\(x z\)
on lower layer
0.0000 .0000 .0000 .01
\(y z\)
on layer medium
0.0000 .0000 .0000 .01
\(x \boldsymbol{x}\)
on layer medium
0.0000 .0000 .0000 .01
\(y y\)
Constraints
on layer medium
0.0000 .0000 .0000 .01
\(x y\)
on layer medium
0.000
3.80 E05
3.80 E05
0.26
\(x z\)
on layer medium
-2.39732-2.3712 1.086\% 21\%
\(y z\)
on higher layer
0.0009 .1146
E02 0.091
0.1
\(\boldsymbol{x x}\)
on higher layer
0.0003 .6678
E01 0.367
0.4
\(y y\)
on higher layer
0.0003 .7119
E01 0.371
0.74
\(x y\)
on higher layer
0.0000 .0000 .0000 .01
\(x z\)
on higher layer
0.0000 .0000 .0000 .01
\(y z\)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls

\author{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 6.0
}

\section*{Titrate:}

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
```

V3.03.118-A Page:

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\author{
Reference
}

Not
NR
NR
NR
M
M
M
\(T\)
\(T\)
\(x x\)
\(y y\)
\(x y\)
\(x x\)
\(y y\)
\(x y\)
\(X\)
\(y\)
O M300
0.0000 .0000 .000 3.1662E02
3.1662E02
0.0000 .0000 .000

O M301
-
0.000
0.000

O M276
```

- 

0.000

```
0.000

O M277
0.000
0.000

With
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000

B
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000

C
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000

D
0.0000 .0000 .000
0.000
0.000 1.8997E02 0.000
0.000

B1 M12
0.0000 .0000 .000
0.000
0.0000 .0000 .000
1.5915E01

B1 M13
0.0000 .0000 .000
0.000
```

---

```
D1 M289 0.0000 .0000 .000
0.000
0.0000 .000
1.5915E01
0.000
Aster
Not
NR
NR
NR
M
M
M
\(\boldsymbol{Q}\)
\(\boldsymbol{Q}\)
\(x \boldsymbol{x}\)
\(y y\)
\(x y\)
\(x x\)
\(y y\)
\(x y\)
\(X\)
\(y\)
\(y\)
O M300
0.0000 .000
0.000 -3.1648
E02 3.1648 E02
1.0 E17
1.0361 E02
1.0361 E02
O M301

\author{
1.0361 E02
}
1.0361 E02

O M276
```

1.0361 E02

```
1.0361 E02

O M277

\author{
- \\ 1.0361 E02 \\ 1.0361 E02 \\ With \\ 0.0000 .000 \\ 0.0001 .86 \\ E05 \\ 1.86 E05 \\ 1.8877 E02 \\ 3.87 E05 \\ 3.87 E05 \\ B \\ 0.0000 .000 \\ 0.0001 .86 \\ E05 \\ 1.86 E06 \\ 1.8877 E02 \\ 3.87 E05 \\ 3.87 E05 \\ C \\ 0.0000 .000 \\ 0.0001 .86 \\ E05 \\ 1.86 E05
}

\title{
0.0001 .52
}
```

2.56 E06
Difference
Not
NR
NR
NR
M
M
M
Q
Q
xx
yy
xy
xx
yy
xy
X
y
O M300
0.000 0.000
0.000-0.046%
-0.046% 1.0
E17
-0.01
0 . 0 1
O M301

```
0.01
0.01
O M276
-0.01
-0.01
O M277

\section*{-0.672\% 2.54}

E06
D1 M289
0.0000 .000
0.0006 .11

E04
1.51 E04
6.18 E04 0.672\% 2.54

E06
20.2 Parameters
of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
9.88 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

V3.03.118-A Page:

\section*{21 Modeling}
\(J\)

\subsection*{21.1 Characteristics of modeling}

Quadrangular element of hull DSQ. The plate is modelled with a grid \(48 \times 48\).
The reference mark user is confused with the reference mark of orthotropism.

Limiting conditions:
DDL_IMPO
(GROUP_NO: AB, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0., DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0., DZ: 0., DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Not O
Meshs: M1128, M1129, M1176, M1177
Not A
Net: M1
Not B
Net: M48
Not C
Net: M2304

\author{
Not D \\ Net: M2257 \\ Not B1 \\ Meshs: M24, M25 \\ Not D1 \\ Meshs: M1153, M1105 \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HI-75/01/010/A \\ ```
Code_Aster \({ }^{\circledR}\) \\ Version \\ 6.0 \\ Titrate: \\ SSLS118 - Square plate subjected to a sinusoidal pressure \\ Date: \\ 16/11/01 \\ Author (S): \\ P. MASSIN, NR. RAHNI Key \\ : \\ V3.03.118-A Page: \\ 43/62
```

}

### 21.2 Characteristics

grid
A number of nodes: 2402
A number of meshs and type: 2304 QUAD4

### 21.3 Functionalities

tested
Orders

AFFE_MODELE "MECHANICAL" "DST"

# AFFE_CARA_ELEM HULL 

 THICKANGLE_REP<br>DEFI_MATERIAU ELAS

FORMULATE

## AFFE_CHAR_MECA FORCE_COQUE

 NEARDDL_IMPO<br>ANGLE_NAUT

## GROUP_NO

FORCE_ARETE

## MECA_STATIQUE

CREA_CHAMP

CALC_CHAM_ELEM SIGM_ELNO_DEPL

DEGE_ELNO_DEPL

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls

## Code_Aster ${ }^{\circledR}$

Version
6.0

## Titrate:

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:

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22 Results of modeling J
22.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not $O$

## M1176

on lower layer
$18.99018 .9950 .029 \%$
1.0\%
$x x$
on lower layer
$18.99018 .9950 .029 \%$
1.0\%
$y y$
on lower layer
0.000
2.7E12

# 2.7 E12 <br> 0.01 <br> $x y$ 

on lower layer
0.0000 .0000 .000
0.01
$x z$
on lower layer
0.0000 .0000 .000
0.01
$y z$
Constraints
on layer medium
0.0000 .0000 .000
0.01
$x \boldsymbol{x}$
on layer medium
0.0000 .0000 .000
0.01
$y y$
on layer medium
0.0000 .0000 .000
0.01
$x y$
on layer medium
0.000-7.802

E02
-0.078 0.26
$x z$
on layer medium
0.0007 .802

E02 0.0780 .26
$y z$
on higher layer
-18.990-18.995 0.029\% 1.0\%
$\boldsymbol{x} \boldsymbol{x}$
on higher layer
-18.990-18.995 0.029 1.0\%
$y y$
on higher layer
0.000
2.7 E12
2.7E12
0.01
$x y$
on higher layer
0.0000 .0000 .000
0.01
$x z$
on higher layer
0.0000 .0000 .000
0.01
$y z$
Displacement DZ
-1.1549
-1.2148 5.187\%
5.2\%

Not B1

## M24

on lower layer
0.000-4.6366

E02
-0.046 0.1
$x x$
on lower layer
0.000-1.8574

E01
-0.186 0.4
$y y$
on lower layer
0.000-1.8628
on lower layer
0.0000 .0000 .0000 .01
$y z$
on layer medium
0.0000 .0000 .0000 .01
$x x$
on layer medium
0.0000 .0000 .0000 .01
$y y$
Constraints
on layer medium
0.0000 .0000 .0000 .01
$x y$
on layer medium
0.000
2.47 E06
2.47 E07
0.26
$x z$
on layer medium
-2.39732-2.3833-0.583\% 21\%
$y z$
on higher layer
0.0004 .6366

E02 0.046
0.1
$x x$
on higher layer
0.0001 .8574Handbook of ValidationV3.03 booklet: Linear statics of the plates and hulls
Code_Aster ${ }^{\circledR}$
Version6.0
Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key:
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Reference
NotNR

E02 0.000<br>0.000<br>C<br>0.0000 .0000 .000<br>0.000<br>0.000 1.8997E02 0.000<br>0.000<br>D<br>0.0000 .0000 .000<br>0.000<br>0.000 1.8997E02 0.000<br>0.000<br>B1 M24<br>0.0000 .0000 .000<br>0.000<br>0.0000 .0000 .000<br>1.5915E01<br>B1 M25<br>0.0000 .0000 .000<br>0.000<br>0.0000 .0000 .000<br>1.5915E01<br>D1 M1105

1.5915E01 0.000

D1 M1153 0.0000 .0000 .0000 .000
0.0000 .000
1.5915E01
0.000

Aster
Not
NR
NR
NR
M

M
$M$
$\underset{\sim}{Q}$
$\boldsymbol{x} \boldsymbol{x}$
$\boldsymbol{y y}$

```
xy
xx
yy
xy
X
y
O M1176
0.000 0.000 0.000
-3.1659
E02 3.1659 E02
4.5 E15
5.201 E03
5.201 E03
O M1177
-
5.201 E03
5.201 E03
O M1128
```


### 5.201 E03

```
5.201 E03
O M1129
5.201 E03
5.201 E03
With
0.0000 .0000 .0004 .83 E06
```

4.83 E06<br>1.8967 E02<br>5.03 E06<br>5.03 E06<br>B<br>0.0000 .0000 .0004 .83<br>E06<br>4.83 E06<br>1.8967 E02<br>5.03 E06<br>5.03 E06<br>C<br>0.0000 .0000 .0004 .83<br>E06<br>4.83 E06<br>1.8967 E02<br>5.03 E06<br>5.03 E06<br>D<br>0.0000 .0000 .0004 .83<br>E06<br>4.83 E06<br>1.8967 E02<br>5.03 E06<br>5.03 E06<br>B1 M24<br>0.0000 .0000 .0007 .73<br>E05<br>3.10 E04<br>3.10 E04<br>1.65 E07<br>1.5889 E01<br>B1 M25<br>0.0000 .0000 .0007 .73<br>E05<br>3.10 E04<br>3.10 E04<br>1.65 E07<br>1.5889 E01<br>D1 M1105

# 1.5889 E01 

1.6 E07

D1 M1153 0.0000 .0000 .0003 .10
E04
7.73 E05
3.10 E04
1.5889 E01
1.6 E07

Difference
Not
NR
NR
NR
M
M
M
$Q$
$Q$
$\boldsymbol{x x}$
$y y$
$x y$
$x x$
$y y$
$x y$
$X$
$y$
O M1176
$0.0000 .0000 .000-0.011$
-0.011 4.5
E15 0.0050 .005
O M1177

## - -

-0.005-0.005-

O M1129

C
0.0000 .0000 .0004 .83

E06 4.83
E06 0.159\% 5.03 E06
5.03 E06

D
0.0000 .0000 .0004 .83

E06 4.83
E06 0.159\% 5.03 E06
5.03 E06

B1 M24
0.0000 .0000 .0007 .73

E05
3.10 E04
3.10 E04 1.65

E07 0.166\%
B1 M25
0.0000 .0000 .0007 .73

E05
3.10 E04
3.10 E04 1.65

E07 0.166\%
D1 M1105

$$
-0.166 \% 1.6
$$

E07
D1 M1153

$$
0.0000 .0000 .0003 .10
$$

E04
7.73E05

$$
\text { 3.10 E04 0.166\% } 1.6
$$

E07
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0
Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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### 22.2 Parameters

of execution

Version: 6.0.37

Machine: SGI - ORIGIN 2000

## Obstruction memory:

32 Mo
Time CPU To use:
21.89 seconds

## 23 Modeling

K

### 23.1 Characteristics of modeling

Isotropic multi-layer plate (5 layers in the thickness). Triangular element of hull DST.
The reference mark user is confused with the reference mark of orthotropism.

## Limiting conditions:

## DDL_IMPO

(GROUP_NO: AB, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0., DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure

Date:<br>16/11/01<br>Author (S):<br>P. MASSIN, NR. RAHNI Key<br>:<br>V3.03.118-A Page:<br>47/62

Not $O$<br>Meshs: M132, M156, M134, M158<br>Not A<br>Net: M1<br>Not B<br>Net: M266<br>Not C<br>Net: M288<br>Not D<br>Net: M23<br>Not B1<br>Meshs: M122, M146<br>Not D1<br>Meshs: M14, M11

### 23.2 Characteristics

grid
A number of nodes: 170
A number of meshs and type: 288 TRIA3

### 23.3 Functionalities

tested
Orders

AFFE_MODELE "MECHANICAL" "DST"

AFFE_CARA_ELEM HULL<br>THICK

\author{
ANGLE_REP <br> DEFI_MATERIAU ELAS_ORTH E_L <br> $E_{-} T$ <br> $E_{-} N$ <br> G_TN <br> G_LT <br> G_LN <br> ```
$N U_{-} L T$ <br> DEFI_COQU_MULT SLEEP

```
}

\author{
FORMULATE
}

\section*{AFFE_CHAR_MECA FORCE_COQUE}

NEAR

\section*{DDL_IMPO}

ANGLE_NAUT

\section*{GROUP_NO}

\section*{FORCE_ARETE}

\section*{MECA_STATIQUE}

\author{
CREA_CHAMP
}

\section*{CALC_CHAM_ELEM SIGM_ELNO_DEPL}

DEGE_ELNO_DEPL

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls HI-75/01/010/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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\section*{24 Results of modeling \(K\)}

\subsection*{24.1 Values}
tested
Identification
Reference
Aster Difference
Tolerance
Not \(O\)

\section*{M134}

Lay down 1
on lower layer
18.99018 .872
-0.62\% 1.0\%
\(x x\)
on lower layer
18.99018 .896
-0.49\% 1.0\%

\section*{\(y y\)}
on lower layer
0.000
2.61 E04
2.61 E04

\author{
on lower layer
}
0.0000 .0000 .000
0.01
\(x z\)
on lower layer
0.0000 .0000 .000
0.01
\(y z\)
3 sleep
on layer medium
0.0000 .0000 .000
0.01
\(x x\)
on layer medium
0.0000 .0000 .000
0.01
\(y y\)
on layer medium
0.0000 .0000 .000
0.01
\(x y\)
on layer medium
0.000-2.8134

E01
-0.281
0.4
\(x z\)
on layer medium
0.0003 .0148

E01 0.3010 .4
\(y z\)
5 sleep
on higher layer
-18.990-18.872-0.62\% 1\%
\(x x\)

\section*{on higher layer}
-18.990-18.896-0.49\% 1\%
\(y y\)
on higher layer
0.000
2.61 E04
2.61 E04
0.01
\(x y\)
on higher layer
0.0000 .0000 .000
0.01
\(x z\)
on higher layer
0.0000 .0000 .000
0.01
\(y z\)
Displacement DZ
-1.1549
-1.1951 3.487\%
4.1\%

Not B1

\section*{M122}

Lay down 1
on lower layer

\subsection*{0.000 -5.2518}

E01
\(-0.5250 .8\)
\(\boldsymbol{x x}\)
on lower layer
0.000-6.8948

E01
-0.689 0.8
\(y y\)
on lower layer
0.000-7.305

\section*{E01}
-0.731
0.8
\(x y\)
on lower layer
0.0000 .0000 .0000 .01
\(x z\)
on lower layer
0.0000 .0000 .0000 .01
\(y z\)
3 sleep
on layer medium
0.0000 .0000 .0000 .01
\(x x\)
on layer medium
0.0000 .0000 .0000 .01
\(y y\)
on layer medium
0.0000 .0000 .0000 .01
xy
on layer medium
\(0.000-4.900\)
E02
-0.049
0.05
\(x z\)
on layer medium
-2.39732-2.3421-2.3\%
3\%
\(y z\)
5 sleep
on higher layer
0.0005 .2518

E01 0.525
0.8
\(x x\)
on higher layer
```

0.0006.8948
E01 0.689
0.8
yy
on higher layer
0.000 7.3051
E01 0.731
0.8
xy
on higher layer
0.000 0.000 0.000 0.01
xz
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster }\mp@subsup{}{}{\circledR
Version
6.0

```

\section*{Titrate:}
```

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
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```
on higher layer
0.0000 .0000 .0000 .01
\(y z\)
Evolution of the constraints in the plate
Not \(O\)
```

xx
yy
xy
xz
yz
Lower Couche1
18.8724 18.8969
-2.61426
E04
0
0
Higher Couche1
11.3234 11.3381
-1.56856
E04 1.80063 E01
1.92951 E01
Lower Couche2
11.3234 11.3381
-1.56856
E04 1.80063 E01
1.92951 E01
Higher Couche2
3.77447 3.77937
-5.22853
E05 2.70094 E01
2.89426 E01
Lower Couche3
3.77447 3.77937
-5.22853
E05 2.70094 E01
2.89426 E01
Average Couche3
6.54766 E16
6.55617 E16 9.07005 E21 2.81348 E01
3.01485 E01
Higher Couche3
-3.77447 -3.77937
5 . 2 2 8 5 3
E05
2.70094 E01
2.89426 E01
Lower Couche4
-3.77447-3.77937

```

\author{
5.22853 \\ E05 \\ 2.70094 E01 \\ 2.89426 E01 \\ Higher Couche4 \\ -11.3234-11.3381 \\ 1.56856 \\ E04 \\ 1.80063 E01 \\ 1.92951 E01 \\ Lower Couche5 \\ -11.3234-11.3381 \\ 1.56856 \\ E04 \\ 1.80063 E01 \\ 1.92951 E01 \\ Higher Couche5 \\ -18.8724-18.8969 \\ 2.61426 \\ E04 \\ 3.33183 E17 \\ \subsection*{3.57030 E17}
}

Not B1
\(x x\)
\(y y\)
\(x y\)
\(x z\)
\(y z\)
Lower Couche1
5.25182 E01 6.89486 E01
7.30513 E01

0
0
Higher Couche1 3.15109 E01 4.13691 E01 4.38308 E01 3.13609 E02
-1.49899
Lower Couche 2
3.15109 E01 4.13691 E01
4.38308 E01 3.13609 E02

\section*{-1.49899}

Higher Couche2 1.05036 E01 1.37897 E01 1.46103 E01 4.70414 E02
-2.24848
Lower Couche3
1.05036 E01 1.37897 E01
1.46103 E01 4.70414 E02
-2.24848
Average Couche 3
1.82209 E17 2.39231 E17
2.53448 E17 4.90014 E02
2.34217

Higher Couche3
1.05036 E01
1.37897 E01
1.46103 E01 4.70414 E02
-2.24848
Lower Couche4
1.05036 E01
1.37897 E01
1.46103 E01 4.70414 E02
-2.24848
Higher Couche4
3.15109 E01
4.13691 E01
4.38308 E01 3.13609 E02
-1.49899
Lower Couche5
3.15109 E01
4.13691 E01
4.38308 E01 3.13609 E02
-1.49899
Higher Couche5
5.25182 E01
6.89486 E01
7.30513 E01
5.80293 E18
2.77369 E16

\subsection*{24.2 Parameters}
of execution
Version: 6.0.37

Obstruction memory:
32 Mo
Time CPU To use:
seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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\section*{25 Modeling}

L

\subsection*{25.1 Characteristics of modeling}

Isotropic multi-layer plate (5 layers in the thickness). Triangular element of hull DST.
The model of plate associated with modeling K is turned of 20 degrees according to the nautical angle alpha and of 30 degrees according to beta. The classification of the meshs is identical to that of modeling K.

\section*{Limiting conditions:}

DDL_IMPO
(GROUP_NO: AB, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
51/62
Not O
Meshs: M132, M156, M134, M158
Not A
Net: M1
Not B
Net: M266
Not C
Net: M288
Not D

\title{
Net: M23
}

Not B1
Meshs: M122, M146
Not D1
Meshs: M14, M11

\subsection*{25.2 Characteristics}
grid
A number of nodes: 170
A number of meshs and type: 288 TRIA3
25.3 Functionalities
tested
Orders

\author{
AFFE_MODELE "MECHANICAL" "DST" \\ MODI_MAILLAGE ORIE_NORME_COQUE
}

\author{
AFFE_CARA_ELEM HULL
}

THICK

\section*{ANGLE_REP}

DEFI_MATERIAU ELAS_ORTH
\(\boldsymbol{E}_{-} \boldsymbol{L}\)
\(E_{-} T\)
\(E_{-} N\)

\title{
\(N U_{-} L T\) \\ DEFI_COQU_MULT SLEEP
}

\section*{FORMULATE}

AFFE_CHAR_MECA FORCE_COQUE
NEAR

LIAISON_OBLIQUE
ANGLE_NAUT

\section*{GROUP_NO}

FORCE_ARETE

\section*{MECA_STATIQUE}

CREA_CHAMP

CALC_CHAM_ELEM SIGM_ELNO_DEPL
\(D E G E \_E L N O \_D E P L\)

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

\section*{Titrate:}

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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26 Results of modeling \(L\)
26.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not \(O\)

\section*{M134}

Lay down 1
on lower layer
18.99018 .872
-0.62\% 1.0\%
\(x x\)
on lower layer
18.99018 .896
\[
-0.49 \% 1.0 \%
\]
\[
y y
\]
on lower layer
0.000
2.61 E04
2.61 E04
0.01
\(x y\)
on lower layer
0.0000 .0000 .000
0.01
\(x z\)
on lower layer
0.0000 .0000 .000
0.01
\(y z\)
3 sleep
on layer medium
0.0000 .0000 .000
0.01
\(x x\)
on layer medium
0.0000 .0000 .000
0.01
\(y y\)
on layer medium
0.0000 .0000 .000
0.01
\(x y\)
on layer medium
0.000-2.8134

E01
-0.281
0.4
\(x z\)
on layer medium
0.0003 .0148

\title{
E01 0.3010 .4
}
\(y z\)
5 sleep
on higher layer
-18.990-18.872-0.62\% 1\%
\(x x\)
on higher layer
-18.990-18.896-0.49\% 1\%
\(y y\)
on higher layer
0.000
2.61 E04
2.61 E04
0.01
\(x y\)
on higher layer
0.0000 .0000 .000
0.01
\(x z\)
on higher layer
0.0000 .0000 .000
0.01
\(y z\)
Displacement DZ
-1.1549
-1.1951 3.487\%
4.1\%

Not B1

M122
Lay down 1
on lower layer
0.000-5.2518

E01
-0.525 0.8
xx
on lower layer

\subsection*{0.000-6.8948}

\section*{E01}
-0.689 0.8
\(y y\)
on lower layer
0.000-7.305

E01
-0.731
0.8
\(x y\)
on lower layer
0.0000 .0000 .0000 .01
\(x z\)
on lower layer
0.0000 .0000 .0000 .01
\(y z\)
3 sleep
on layer medium
0.0000 .0000 .0000 .01
\(\boldsymbol{x} \boldsymbol{x}\)
on layer medium
0.0000 .0000 .0000 .01
\(y y\)
on layer medium
0.0000 .0000 .0000 .01
\(x y\)
on layer medium
0.000-4.900

E02
-0.049
0.05
\(x z\)
on layer medium
-2.39732-2.3421-2.3\%
3\%
\(y z\)
5 sleep
on higher layer
0.0005 .2518

E01 0.525
0.8
\(x x\)
on higher layer
0.0006 .8948

E01 0.689
0.8
\(y y\)
on higher layer
0.0007 .3051

E01 0.731
0.8
xy
on higher layer
0.0000 .0000 .0000 .01
\(x z\)
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HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)Version
\[
6.0
\]
Titrate:
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Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key

:
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on higher layer
0.0000 .0000 .0000 .01

Evolution of the constraints in the plate

\section*{Not \(O\)}

\author{
\(\boldsymbol{x} \boldsymbol{x}\) \\ \(y y\) \\ \(x y\) \\ \(x z\) \\ \(y z\) \\ Lower Couche1 \\ 18.872418 .8969 \\ -2.61426 \\ E04 \\ 0 \\ 0 \\ Higher Couche1 \\ 11.323411 .3381 \\ -1.56856 \\ E04 1.80063 E01 \\ 1.92951 E01 \\ Lower Couche2 \\ 11.323411 .3381 \\ -1.56856 \\ E04 1.80063 E01 \\ 1.92951 E01 \\ Higher Couche2 \\ 3.774473 .77937 \\ -5.22853 \\ E05 2.70094 E01 \\ 2.89426 E01 \\ Lower Couche3 \\ 3.774473 .77937 \\ -5.22853 \\ E05 2.70094 E01 \\ 2.89426 E01 \\ Average Couche3 \\ 2.70652 E14 2.69295 E14 \\ 1.22884 E13 2.81348 E01 \\ 3.01485 E01 \\ Higher Couche3
}

\title{
5.22853
}

\section*{E05}
2.70094 E01
2.89426 E01

Lower Couche4
-3.77447-3.77937
5.22853

E05
2.70094 E01
2.89426 E01

Higher Couche4
-11.3234-11.3381
1.56856

E04
1.80063 E01
1.92951 E01

Lower Couche5
-11.3234-11.3381
1.56856

E04
1.80063 E01
1.92951 E01

Higher Couche5
-18.8724-18.8969
2.61426

E04
3.33183 E17
3.57030 E17

Not B1
\(x x\)
\(y y\)
\(x y\)
\(x z\)
\(y z\)

\section*{Lower Couche1}

\subsection*{5.25182 E01 6.89486 E01}
7.30513 E01

0
0
Higher Couche1 3.15109 E01 4.13691 E01 4.38308 E01 3.13609 E02
-1.49899
Lower Couche2
3.15109 E01 4.13691 E01
4.38308 E01 3.13609 E02
-1.49899
Higher Couche2 1.05036 E01 1.37897 E01 1.46103 E01 4.70414 E02
-2.24848
Lower Couche3
1.05036 E01 1.37897 E01
1.46103 E01 4.70414 E02
-2.24848
Average Couche3
8.54411 E16 5.45131 E16
2.48746 E14 4.90014 E02
-2.34217
Higher Couche3
1.05036 E01
1.37897 E01
1.46103 E01 4.70414 E02
-2.24848
Lower Couche4
1.05036 E01
1.37897 E01
1.46103 E01 4.70414 E02
-2.24848
Higher Couche4
3.15109 E01
4.13691 E01
4.38308 E01 3.13609 E02
-1.49899
Lower Couche5
3.15109 E01
4.13691 E01
4.38308 E01 3.13609 E02
-1.49899
Higher Couche5
5.25182 E01
6.89486 E01
7.30513 E01
5.80293 E18

\subsection*{26.2 Parameters}
of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
26.17 seconds

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Code_Aster \({ }^{\circledR}\)
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6.0

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Author (S):
P. MASSIN, NR. RAHNI Key
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27 Modeling
M

\subsection*{27.1 Characteristics of modeling}

Isotropic multi-layer plate (5 layers in the thickness). Quadrangular element of hull DSQ. The reference mark user is confused with the reference mark of orthotropism.

\section*{Limiting conditions:}

DDL_IMPO
(GROUP_NO: AB, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0., DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
Not \(O\)
Meshs: M66, M67, M78, M79
Not A
Net: M1
Not B
Net: M12
Not C
Net: M144
Not D
Net: M133
Not B1
Meshs: M6, M7
Not D1
Meshs: M73, M61
Handbook of Validation
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HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
Version

\section*{Titrate:}

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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27.2 Characteristics
grid
A number of nodes: 171
A number of meshs and type: 144 QUAD4

\subsection*{27.3 Functionalities}
tested
Orders

\section*{AFFE_MODELE "MECHANICAL" "DST"}

\section*{MODI_MAILLAGE ORIE_NORME_COQUE}

\section*{AFFE_CARA_ELEM HULL}

THICK

\section*{ANGLE_REP}

\section*{DEFI_MATERIAU ELAS_ORTH} E_L
```

E_T
E_N
G_TN
G_LT
G_LN
NU_LT
DEFI_COQU_MULT SLEEP

```
DEFI_COQU_MULT SLEEP
THICK
ORIENTATION
AFFE_CHAR_MECA FORCE_COQUE
NEAR
ALL
LIAISON_OBLIQUE
ANGLE_NAUT
GROUP_NO
MECA_STATIQUE

\title{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
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28 Results of modeling \(M\)
28.1 Values
tested
Identification
Reference
Aster Difference
Tolerance
Not \(O\)

\section*{M134}

Lay down 1
on lower layer
18.99018 .959
-0.158\%
1.0\%
\(x \boldsymbol{x}\)
on lower layer
18.99018 .959
-0.158\%

\section*{\(1.0 \%\)}
\(y y\)

\author{
on lower layer \\ 0.000 4.73E08 4.73E08 \\ 0.01
}
\(x y\)
on lower layer
0.0000 .0000 .000
0.01
\(x z\)
on lower layer
0.0000 .0000 .000
0.01
\(y z\)
3 sleep
on layer medium
0.0000 .0000 .000
0.01
\(\boldsymbol{x x}\)
on layer medium
0.0000 .0000 .000
0.01
\(y y\)
on layer medium
0.0000 .0000 .000
0.01
\(x y\)
on layer medium
0.000 -0.306
-0.306 0.31
\(x z\)
on layer medium
0.0000 .3060 .306
0.31
\(y z\)
5 sleep
on higher layer
on higher layer
-18.990-18.959 -0.158\% 1.0\%
\(\boldsymbol{y y}\)
on higher layer
0.000 4.73E08
4.73E08
0.01
\(x y\)
on higher layer
0.0000 .0000 .000
0.01
\(x z\)
on higher layer
0.0000 .0000 .000
0.01
\(y z\)
Displacement DZ
-1.1549
-1.2012 4.017\%
4.1\%

Not B1

\section*{M122}

Lay down 1 on lower layer 0.000 1.695E01 0.170 .7
\(\boldsymbol{x} \boldsymbol{x}\)
on lower layer
0.000 6.933E01
-0.693 0.7
\(y y\)
on lower layer
0.000 7.316E01
-0.732 0.74
```

xy

```
on lower layer
0.0000 .0000 .0000 .01
\(x z\)
on lower layer
0.0000 .0000 .0000 .01
\(y z\)
3 sleep
on layer medium
0.0000 .0000 .0000 .01
\(\boldsymbol{x x}\)
on layer medium
0.0000 .0000 .0000 .01
\(y y\)
on layer medium
0.0000 .0000 .0000 .01
\(x y\)
on layer medium
0.000 5.256E04
5.26E04 0.26
\(x z\)
on layer medium
-2.39732-2.32183 -3.149\% 3.2\%
\(y z\)
5 sleep
on higher layer
0.000 1.695E01 0.17
0.7
\(\boldsymbol{x} \boldsymbol{x}\)
on higher layer
0.000 6.933E01 0.693
0.7
\(y y\)
on higher layer0.000 7.316E01 0.7320 .74
\(x y\)
on higher layer
0.0000 .0000 .0000 .01
\(x z\)
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Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
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Date:
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Author (S):
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on higher layer
0.0000 .0000 .0000 .01
\(y z\)
Evolution of the constraints in the plate
Not \(O\)

\section*{Higher Couche1}
11.376011 .3760
2.83603

E08
1.95632 E01
1.95632 E01

Lower Couche 2
11.376011 .3760
2.83603

E08
1.95632 E01
1.95632 E01

Higher Couche2
3.791993 .79199
9.45342

E09
2.93448 E01
2.93448 E01

Lower Couche3
3.791993 .79199
9.45342

E09
2.93448 E01
2.93448 E01

Average Couche3
6.57805 E16
6.57805 E16
1.63991 E24 3.05675 E01
3.05675 E01

Higher Couche3
-3.79199-3.79199
-9.45342
E09 2.93448 E01
2.93448 E01

Lower Couche4
-3.79199-3.79199
-9.45342
E09 2.93448 E01
2.93448 E01

Higher Couche4
-11.3760 -11.3760
-2.83603
E08 1.95632 E01

\subsection*{1.95632 E01}

Lower Couche5
-11.3760 -11.3760
-2.83603
E08 1.95632 E01
1.95632 E01

Higher Couche5
-18.9599 -18.9599
4.72681

E08
3.61992 E17
3.61992 E17

Not B1
\(x x\)
\(y y\)
\(x y\)
\(x z\)
\(y z\)
Lower Couche1
1.69553 E01 6.93351 E01
7.31643 E01

0
0
Higher Couche1 1.01732 E01 4.16010 E01 4.38986 E01 3.36438 E04
-1.48597
Lower Couche 2
1.01732 E01 4.16010 E01
4.38986 E01
3.36438 E04
-1.48597
Higher Couche2 3.39106 E02 1.38670 E01 1.46329 E01 5.04657 E04
-2.22896
Lower Couche3
3.39106 E02 1.38670 E01
1.46329 E01
5.04657 E04
-2.22896
Average Couche3

\subsection*{5.88254 E18 2.40554 E17}

\subsection*{2.53840 E17 \\ 5.25684 E04}
-2.32184
Higher Couche3
3.39106 E02
1.38670 E01
1.46329 E01
5.04657 E04
-2.22896
Lower Couche4
3.39106 E02
1.38670 E01
1.46329 E01
5.04657 E04
-2.22896
Higher Couche4
1.01732 E01
4.16010 E01
4.38986 E01
3.36438 E04
-1.48597
Lower Couche5
1.01732 E01
4.16010 E01
4.38986 E01
3.36438 E04
-1.48597
Higher Couche5
1.69553 E01
6.93351 E01
7.31643 E01 6.22535 E20
2.74961 E16

\subsection*{28.2 Parameters}
of execution
Version: 6.0.37

Machine: SGI - ORIGIN 2000

\author{
Obstruction memory: \\ 32 Mo \\ Time CPU To use: \\ 14.91 seconds \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HI-75/01/010/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/1 1/01
Author (S):
P. MASSIN, NR. RAHNI Key

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\section*{29 Modeling}

NR
29.1 Characteristics of modeling

Isotropic multi-layer plate (5 layers in the thickness). Quadrangular element of hull DSQ.
The model of plate associated with modeling \(M\) is turned of 20 degrees according to the nautical angle
alpha and of 30 degrees according to beta. The classification of the meshs is identical to that of modeling Mr.

Limiting conditions:
DDL_IMPO
(GROUP_NO: AB, DX: 0., DZ: 0. , DRY: 0.)
(GROUP_NO: BC, DY: 0. , DZ: 0. , DRX: 0.)
(GROUP_NO: CD, DX: 0. , DZ: 0. , DRY: 0.)
(GROUP_NO: DA, DY: 0. , DZ: 0., DRX: 0.)
(GROUP_NO: O, DX: 0. , DY: 0. , DRX: 0. , DRY: 0. , DRZ: 0.)
FORCE_ARETE
(GROUP_NO: AB MY: 0.)
(GROUP_NO: BC MX: 0.)
(GROUP_NO: CD MY: 0.)
(GROUP_NO: DA MX: 0.)
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Code_Aster \({ }^{\circledR}\)
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Not \(O\)
Meshs: M66, M67, M78, M79
Not A
Net: M1
Not B
Net: M12
Not C
Net: M144
Not D
Net: M133
Not B1
Meshs: M6, M7
Not D1
Meshs: M73, M61
29.2 Characteristics
grid
A number of nodes: 171
A number of meshs and type: 144 QUAD4
```

29.3 Functionalities
tested

```

\author{
Orders
}
```

AFFE_MODELE

```
AFFE_MODELE
"MECHANICAL"
"MECHANICAL"
"DST"
"DST"
MODI_MAILLAGE ORIE_NORME_COQUE
```


## AFFE_CARA_ELEM HULL THICK

```
ANGLE_REP
```

DEFI_MATERIAU ELAS_ORTH
E_L
$E_{-} T$
$E_{-} N$
G_TN
G_LT
G_LN
$N U \_L T$

# FORMULATE 

AFFE_CHAR_MECA FORCE_COQUE<br>NEAR

LIAISON_OBLIQUE
ANGLE_NAUT

GROUP_NO

FORCE_ARETE

MECA_STATIQUE

CREA_CHAMP

## CALC_CHAM_ELEM SIGM_ELNO_DEPL

DEGE_ELNO_DEPL

## Handbook of Validation

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Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS118 - Square plate subjected to a sinusoidal pressure
Date:

16/11/01<br>Author (S):<br>P. MASSIN, NR. RAHNI Key<br>:<br>V3.03.118-A Page:<br>60/62

## 30 Results of modeling $N R$

### 30.1 Values

tested
Identification
Reference
Aster Difference
Tolerance
Not $O$

M134
Lay down 1
on lower layer
18.99018 .959
-0.158\%
1.0\%
$x x$
on lower layer
18.99018 .959
-0.158\%
1.0\%
$y y$
on lower layer
0.0000 .0000 .000
0.01
$x y$
on lower layer
0.0000 .0000 .000
0.01
$x z$

```
on lower layer
0.000 0.000 0.000
0.01
yz
3 sleep
on layer medium
0.000 0.000 0.000
0.01
x
on layer medium
0.000 0.000 0.000
0.01
yy
on layer medium
0.000 0.000 0.000
0.01
xy
on layer medium
0.000-0.306
-0.306 0.31
xz
on layer medium
0.000 0.306 0.306
0.31
yz
5sleep
on higher layer
-18.990-18.959 -0.158% 1.0%
x
on higher layer
-18.990-18.959 -0.158% 1.0%
yy
on higher layer
0.000 0.000 0.000
0.01
xy
```

on higher layer
0.0000 .0000 .000
0.01
$x z$
on higher layer
0.0000 .0000 .000
0.01
$y z$
Displacement DZ
-1.1549
-1.2012 4.017\%
4.1\%

Not B1

## M122

Lay down 1
on lower layer
0.000 1.695E01 0.170 .7
xx
on lower layer
0.000 6.933E01
-0.693 0.7
$y y$
on lower layer
0.000 7.316E01
-0.732 0.74
$x y$
on lower layer
0.0000 .0000 .0000 .01
$x z$
on lower layer
0.0000 .0000 .0000 .01
$y z$
3 sleep
on layer medium
0.0000 .0000 .0000 .01
xx
on layer medium
0.0000 .0000 .0000 .01
$y y$
on layer medium
0.0000 .0000 .0000 .01
$x y$
on layer medium
0.000 5.256E04
5.26E04 0.26
$x z$
on layer medium
-2.39732-2.32183-3.149\% 3.2\%
$y z$
5 sleep
on higher layer
0.000 1.695E01 0.17
0.3
$\boldsymbol{x x}$
on higher layer
0.000 6.933E01 0.693
0.8
$y y$
on higher layer
0.000 7.316E01 0.732
0.8
$x y$
on higher layer
0.0000 .0000 .0000 .01
$x z$Handbook of ValidationV3.03 booklet: Linear statics of the plates and hulls
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Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:<br>SSLS118 - Square plate subjected to a sinusoidal pressure<br>Date:<br>16/11/01<br>Author (S):<br>P. MASSIN, NR. RAHNI Key<br>:<br>V3.03.118-A Page:<br>61/62

on higher layer
0.0000 .0000 .0000 .01
$y z$
Evolution of the constraints in the plate
Not $O$

$x x$<br>$y y$<br>$x y$<br>$x z$<br>$y z$<br>Lower Couche1<br>18.959918 .9599<br>-2.21998<br>E13<br>0<br>0<br>Higher Couche1<br>11.376011 .3760<br>-1.64964<br>E13 1.95632 E01<br>1.95632 E01<br>Lower Couche2<br>11.376011 .3760<br>-1.64964<br>E13 1.95632 E01<br>1.95632 E01<br>Higher Couche2

3.791993 .79199
-1.07931
E13 2.93448 E01
2.93448 E01

Lower Couche3
3.791993 .79199
-1.07931
E13 2.93448 E01
2.93448 E01

Average Couche3
1.26875 E13
1.05848 E13 7.94143 E14 3.05675 E01
3.05675 E01

Higher Couche3
-3.79199 -3.79199
-5.08976
E14 2.93448 E01
2.93448 E01

Lower Couche4
-3.79199 -3.79199
-5.08976
E14 2.93448 E01
2.93448 E01

Higher Couche4
-11.3760 -11.3760
6.13577

E15
1.95632 E01
1.95632 E01

Lower Couche5
-11.3760-11.3760
6.13577

E15
1.95632 E01
1.95632 E01

Higher Couche5
-18.9599 -18.9599
6.31692

E14
3.61992 E17
3.61992 E17

Not B1

Higher Couche1 1.01732 E01 4.16010 E01 4.38986 E01 3.36437 E04 -1.48597

Lower Couche2
1.01732 E01 4.16010 E01
4.38986 E01
3.36437 E04
-1.48597
Higher Couche2 3.39106 E02 1.38670 E01 1.46329 E01 5.04656 E04
-2.22896
Lower Couche3
3.39106 E02 1.38670 E01
1.46329 E01
5.04656 E04
-2.22896
Average Couche3
4.87415 E15 4.93108 E14
3.39767 E15
5.25683 E04
-2.32184
Higher Couche3
3.39106 E02
1.38670 E01
1.46329 E01
5.04656 E04
-2.22896
Lower Couche4
3.39106 E02
1.38670 E01
1.46329 E01

5.04656 E04<br>-2.22896<br>Higher Couche4<br>1.01732 E01<br>4.16010 E01<br>4.38986 E01<br>3.36437 E04<br>-1.48597<br>Lower Couche5<br>1.01732 E01<br>4.16010 E01<br>4.38986 E01<br>3.36437 E04<br>-1.48597<br>Higher Couche5<br>1.69553 E01<br>6.93351 E01<br>7.31643 E01 6.22534 E20<br>2.74961 E16

30.2 Parameters
of execution
Version: 6.0.37

Machine: SGI-ORIGIN 2000

Obstruction memory:
32 Mo
Time CPU To use:
15.32 seconds

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:

SSLS118 - Square plate subjected to a sinusoidal pressure
Date:
16/11/01
Author (S):
P. MASSIN, NR. RAHNI Key
:
V3.03.118-A Page:
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## 31 Summary of the results

The results obtained show that:

- modeling DSQ provides a more precise estimate of the constraints and efforts edges ( $<5 \%$ ) that modeling DKQ $(=20 \%)$ ). The estimate of the moments is equivalent for two modelings,
$\cdot$ the estimate of the constraints is more precise with modeling DKT (<2\%) compared to modeling DST ( $<3,5 \%$ ). The shearing action is on the other hand better estimated by DST (<10\% for DST and <20\% for DKT), $\cdot$ for the configurations COQUE_3D triangle or quadrangle, the estimate of the constraint of transverse shearing is constant in the thickness of the plate, in accordance with assumptions of modeling,
results expressed in the reference mark user for the DST configurations and COQUE_3D are identical to those expressed in the total reference mark,
the refinement of the grid for configuration DSQ improves the estimate of constraints, of the sharp efforts and the moments; the tendency is reversed in what relate to displacement,
- 

the multi-layer configuration makes it possible to visualize the distribution of the plane constraints and
of transverse shearing in the thickness of the plate, and to confirm the theory, with to know a linear for the plane constraints and parabolic distribution for shear stresses; in addition, the rotation of the reference mark does not influence the values constraints,
in a general way, with the nodes where one awaits constraints or efforts analytically null, the numerical results obtained are not correct owing to the fact that the estimates are made mesh by mesh then extrapolated with the nodes. Even them values realised with the nodes in question are not inevitably null.

Handbook of Validation

## Titrate:

SSLS119 - Embedded hook subjected to a sharp effort
Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
:
V3.03.119-A Page:
1/10
Organization (S): EDF/AMA

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.119

SSLS 119-Subjected embedded hook with a sharp effort at its end

## Summary:

This test represents a static calculation of an embedded hook subjected to a shearing force, consisted of one
elastic material. This test makes it possible to validate following modelings finite elements:

DST (QUAD4),

## DKT (QUAD4),

COQUE_3D (QUAD9),

## COQUE_3D (TRIA7),

linear 3D (HEXA8) and quadratic (HEXA20).
One studies blocking in transverse shearing particularly there.
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLS119 - Embedded hook subjected to a sharp effort
Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
:
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## 1

Problem of reference
1.1 Geometry

Height $\boldsymbol{H}=0.508 \mathrm{~m}$
$t=0.0508 \mathrm{~m}$
$R=0.3536 \mathrm{~m}$
$R=1.1684 \mathrm{~m}$
WITH, B
$X$
$30^{\circ}$
C
1.2

Properties of material
The properties of material constituting the beam are:
$E=22752510 P a$
Young modulus
$=0.35$
Poisson's ratio

## 1.3

Boundary conditions and loadings

## C.L. : Embedded side AB

Linear force $F z=8.7594 \mathrm{~N} / \mathrm{m}$ for the hulls.
Surface force $F z=172.4307$ N/m2 for the $3 D$.

### 1.4 Conditions

initial
Without object
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version

## Date:

05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE

## 2

Reference solution
This test makes it possible to test blocking in transverse shearing as well as the effects of the rigidity of rotation around the normal. It makes it possible to validate the choice COEF_RIGI_DRZ = 1. E05, value by
defect of this coefficient. This multiplicative factor makes it possible to affect a fictitious rigidity around
normal of the elements of plate by multiplying minimal rigidity according to the other directions by it coefficient in order to avoid the singular matrices of rigidity.

## 2.1

Results of reference
The results of reference result from a calculation by finite elements voluminal:
Value of the deflection out of C: 4.93 in is 1.252 E-01 Mr.

## 2.2 <br> Uncertainties on the solution

A few \% following the refinement of the grid.

### 2.3 References <br> bibliographical

[1]
Raasch Challenge for Shell Elements, N.F. Knight Jr., AIAA Newspaper, vol. 35, N², 1997,

# pp 375-381. 

Handbook of Validation
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Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SSLS119-Embedded hook subjected to a sharp effort

## Date:

05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
:
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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

## DST modeling

Boundary conditions: side $A B: U=v=w=0$

$z=X=y=0$<br>$F z=8.7594 \mathrm{~N} / \mathrm{m}$

## 3.2

Characteristics of the grid
A number of nodes: 2877
A number of meshs and type: 20 (according to Z) x136 (length) QUAD4

### 3.3 Functionalities

tested
Orders Key word factor
Key word
AFFE_MODELE MODELINGDSTAFFE_CARA_ELEM HULL
THICK
COEF_RIGI_DRZ
1.E-05
MECA_STATIQUE
4Results of modeling $A$
4.1 Values
tested
grid Identification Reference
Aster \%
difference
5 X 34
DZ
1.252 E-01
1.48268 E-01
18.42 \%$10 \times 68$
DZ
1.252 E-01
2.38825 E-01
$90.75 \%$
$20 \times 136$
DZ
1.252 E-01
4.45694 E-01
256.00 \%
4.2 Parametersof execution
Version: NEW 6.01.08
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 15.67 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0
Titrate:
SSLS119-Embedded hook subjected to a sharp effort
Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE

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## 5 Modeling <br> B

## 5.1

Characteristics of modeling

## Modeling DKT

Boundary conditions: side $A B: U=v=w=0$

$$
z=X=y=0
$$

5.2<br>Characteristics of the grid<br>A number of nodes: 2877<br>5.3 Functionalities<br>tested<br>Orders Key word<br>factor<br>Key word<br>AFFE_MODELE MODELING DKT<br>AFFE_CARA_ELEM HULL<br>\section*{THICK}<br>COEF_RIGI_DRZ<br>1.E-05<br>MECA_STATIQUE

A number of meshs and type: 20 (according to Z) x136 (length) QUAD4

6
Results of modeling $B$

### 6.1 Values

tested
grid Identification Reference
Aster \%
difference
5 X 34
DZ
1.252 E01
1.19617 E01

10 X 68
DZ
1.252 E01
1.16733 E01
-6.76 \%
$20 \times 136$
DZ
1.252 E01
1.06726 E01
-14.75 \%

### 6.2 Parameters

of execution
Version: NEW 6.01.08
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 13.23 seconds

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLS119-Embedded hook subjected to a sharp effort
Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
.
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## 7 Modeling

C

## 7.1

Characteristics of modeling
Modeling COQUE_3_D
Boundary conditions: side $A B$ : $U=v=w=0$
$=0$
$z=X=y$
$F z=8.7594 \mathrm{~N} / \mathrm{m}$

## 7.2 <br> Characteristics of the grid

A number of nodes: 11193
A number of meshs and type: 20 (according to Z) x136 (length) QUAD9

### 7.3 Functionalities

 tested
## Orders Key word

 factor
## Key word

## AFFE_MODELE MODELING COQUE_3D

AFFE_CARA_ELEM HULL

## THICK

## COEF_RIGI_DRZ

1.E-05

MECA_STATIQUE

## 8 <br> Results of modeling $C$

### 8.1 Values

tested

## grid Identification Reference

Aster \% difference
5 X 34
DZ
1.252 E-01
1.31964 E-01
$5.40 \%$
$10 \times 68$
DZ
1.252 E-01
1.31574 E-01
$5.10 \%$
$20 \times 136$
DZ
1.252 E-01
1.31195 E-01
4.79 \%

### 8.2 Parameters

Version: NEW 6.01.08
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 83.67 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLS119-Embedded hook subjected to a sharp effort
Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE

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## 9 Modeling

D

## 9.1

Characteristics of modeling
Modeling 3D CUB8
Boundary conditions: side $A B$ : $U=v=w=0$
$F z=172.4307 \mathrm{~N} / \mathrm{m} 2$

## 9.2

Characteristics of the grid
A number of nodes: 6072

# A number of meshs and type: 10 (according to Z) X 68 (length) X 1 (thickness) HEXA8 

9.3 Functionalities<br>tested<br>Orders Key word<br>factor<br>Key word<br>\section*{AFFE_MODELE MODELING}<br>3D<br>MECA_STATIQUE

## 10 Results of modeling $D$

### 10.1 Values

tested
grid Identification Reference
Aster \%
difference
5 X 34
DZ
1.252 E01
1.23233 E01
-1.57\%
$10 \times 68$
DZ
1.252 E01
1.28808 E01
2.88 \%

20 X 136 X 2
DZ
1.252 E01
1.32292 E01
5.66 \%

### 10.2 Parameters

of execution

Version: NEW 6.01.08
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 8.47 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
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Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
:
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11 Modeling
E

### 11.1 Characteristics of modeling

Modeling 3D CU20
Boundary conditions: side $A B: U=v=w=0$
$F z=172.4307 \mathrm{~N} / \mathrm{m} 2$
11.2 Characteristics of the grid

A number of nodes: 5160
A number of meshs and type: 10 (according to Z) X 68 (length) X 1 (thickness) HEXA20

### 11.3 Functionalities

## Orders Key word

factor
Key word
AFFE_MODELE MODELING
3D

MECA_STATIQUE

12 Results of modeling $E$

### 12.1 Values

tested

## grid Identification Reference

Aster \%
difference
5 X 34
DZ
1.252 E-01
1.32077 E-01
5.49 \%

10 X 68
DZ
1.252 E-01
1.33518 E-01
6.64 \%
$20 \times 136 \times 2$
DZ
1.252 E-01
1.34315 E-01
7.28 \%

### 12.2 Parameters

of execution
Version: NEW 6.01.08
Machine: SGI-Origin2000 R12000

Obstruction memory: 16 megabytes
Time CPU To use: 22.54 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLS119-Embedded hook subjected to a sharp effort

## Date:

05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
:
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## 13 Modeling

F
13.1 Characteristics of modeling

Modeling COQUE_3_D
Boundary conditions: side $A B: U=v=w=0$
$=0$
$z=X=y$
$F z=8.7594 \mathrm{~N} / \mathrm{m}$

### 13.2 Characteristics of the grid

A number of nodes: 11193
A number of meshs and type: 40 (according to Z) $x 272$ (length) TRIA7
13.3 Functionalities tested
Orders Key wordfactorKey word
AFFE_MODELE MODELINGCOQUE_3D
AFFE_CARA_ELEM HULL
THICK
COEF_RIGI_DRZ
1.E-05
MECA_STATIQUE
14 Results of modeling $F$
14.1 Values
tested
grid Identification ReferenceAster \%
difference
10 X 68
DZ
1.252 E-01
1.3224 E-01
5.62 \%
20 X 136
DZ
1.252 E-01
1.31835 E-01
$5.30 \%$
40 X 272
DZ
$1.252 \mathrm{E}-01$

### 1.31536 E-01

5.06 \%

### 14.2 Parameters <br> of execution

Version: NEW 6.01.08
Machine: SGI-Origin2000 R12000
Obstruction memory: 16 megabytes
Time CPU To use: 103.52 seconds
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLS119-Embedded hook subjected to a sharp effort
Date:
05/02/02
Author (S):
P. Key MASSIN, A LACHAIZE
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15 Summary of the results
The DST element with taking into transverse account of shearing does not seem to converge on it specific case-test. The elements COQUE_3D triangles and quadrangles with taking into account of transverse shearing do not present the same behavior and behave rather well on it test.
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version

## Titrate:

SSLS120 - Thin hull under hydrostatic pressure
Date:
12/12/02
Author (S):
J. Key Mr. PROIX
:
V3.03.120-A Page:
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Organization (S): EDF-R \& D /AMA

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and the hulls
Document: V3.03.120

SSLS120 - Cylindrical thin hull under pressure hydrostatic

## Summary:

This test represents a static calculation of thin cylindrical tank filled with water. It makes it possible to validate the maid
taking into account of the pressures function of the geometry, as well as the orthotropic materials rubber band. 3
modelings finite elements are used: AXIS, COQUE_3D with meshs QUAD9, COQUE_3D with meshs TRIA7 and DKT with meshs QUAD4. Displacements and the constraints obtained are compared with
an analytical reference solution.

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and the hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS120 - Thin hull under hydrostatic pressure
Date:
12/12/02
Author (S):
J. Key Mr. PROIX

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1
Problem of reference

### 1.1 Geometry

Cylindrical reserve of average radius $R=5.7 \mathrm{~m}$, thickness $E=0.04 \mathrm{~m}$ and height $L=16 \mathrm{~m}$, simply supported at its base (rotation is free), and subjected to an internal hydrostatic pressure.
Average radius: $\boldsymbol{R}$
$=5.7 \mathrm{~m}$Thickness:
E $=0.04 \mathrm{~m}$
Height:
$L=16 \mathrm{~m}$
1.2Material propertiesThe properties of materials constituting the plate are:
Material 1: isotropic rubber band:
Young modulus
E = 2.1 1011 Pa
Poisson's ratio ..... $=0.3$
Material 2: orthotropic rubber band: ..... $E_{-} L=1 . E 10 \mathrm{~Pa}$

$$
E_{-} T=2.1 E 11 P a
$$

$$
G_{-} L T=0.45 E 10 \mathrm{~Pa}
$$

$$
G_{-} T N=0.35 E 10 \mathrm{~Pa}
$$

$$
N U_{\_} L T=0.075
$$

The axis $L$ is confused with axis $Z$.

## 1.3

Boundary conditions and loadings
Base Z = 0 simply supported
internal pressure varying linearly according to Z: $p(Z)=P 0 .(L-z) / L$ with $P 0=15000$ Pa.

### 1.4 Conditions <br> initial

## Without object.

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## Date:

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2
Reference solution

## 2.1 <br> Method of calculation used for the reference solution

- Isotropic Matériau: Analytical solution [bib1], obtained with the mean assumption of hull:

```
U=
1-
R
Ee
L
PoRLvz
Z
U=
1-
Z
Ee
```

$2 L$
PoR2
Radial displacement at the base of the cylinder: $\boldsymbol{U}(\mathrm{Z}=)$
0 =
R
Ee
PoRLv
Vertical displacement in top of the cylinder: $U(Z=L)=-$
Z
2Ee
PoR
Circumferential constraint in bottom of the cylinder ( $\mathrm{Z}=$ )
0 =

- Orthotropic Matériau: The solution can be deduced from the preceding one: constraints being statically determined, it is enough to amend the law of behavior, and to integrate them deformations.


## PoR2

Raidal displacement at the base of the cylinder: $\boldsymbol{U}(\mathrm{Z}=)$
0 =
R
E E

$T$

## PoRLv

Vertical displacement in top of the cylinder: $U(Z=L$

## LT

) = -
Z
2nd $E$
$T$

## PoR

Circumferential constraint in bottom of the cylinder ( $\mathrm{Z}=$ )
0 =

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and the hulls HT-66/02/001/A

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## 2.2

Results of reference
Isotropic material:

Radial displacement at the base of the cylinder:
$\boldsymbol{U r}(\mathrm{A} 1)=5.8017857 \mathrm{E} 05 \mathrm{~m}$
Vertical displacement in top of the cylinder:
$U z(A 3)=2.442857 E 05 \mathrm{~m}$
Circumferential constraint in bottom of the cylinder:
$\boldsymbol{S T T}(A 1)=2.1375 E+06 ~ P a$

## Orthotropic material:

Radial displacement at the base of the cylinder:
$\boldsymbol{U r}(\mathrm{A} 1)=5.8017857 \mathrm{E} 05 \mathrm{~m}$
Vertical displacement in top of the cylinder:
$U z(A 3)=6.107143 E 06 \mathrm{~m}$
Circumferential constraint in bottom of the cylinder:
$\boldsymbol{S T T}(A 1)=2.1375 E+06 ~ P a$

## 2.3 <br> Uncertainty on the solution

- Analytical Solution.


### 2.4 References <br> bibliographical

## [1]

PILKEY W.D.: "Formulated for stress, Strain and Structural Matrices". Wiley \& Idiots, New York, 1994.

## Handbook of Validation

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Date:

Author (S):<br>J. Key Mr. PROIX<br>V3.03.120-A Page:<br>5/12

3 Modeling
With

## 3.1

Characteristics of modeling
Modeling AXIS. One nets only one generator of the cylinder. 2 meshs QUAD8 in the thickness and 400 on the height.

## 3.2 <br> Characteristics of the grid <br> A number of nodes: 3206 <br> A number of meshs and types: 800 QUAD8

3.3 Functionalities<br>tested<br>Orders Key word<br>factor<br>Key word<br>AFFE_MODELE<br>AFFE<br>MODELING = `AXIS `<br>MASSIVE AFFE_CARA_ELEM<br>ANGL_REP<br>AFFE_CHAR_MECA_F FORCE_CONTOUR<br>FX<br>DEFI_MATERIAU ELAS_ORTH

### 3.4 Values <br> tested

## Isotropic material

## Value Identification

Reference
Aster \%
difference
Ur ( $z=0$ )
DX (PM)
5.8018E05
5.7875E05
0.25
$U z(z=L)$
DY (A3)
2.4429E05
2.433E05
0.4
$U z(z=L)$
DY (A4)
$2.4429 E 05$
2.4185E05

1
SigmaTT ( $z=0$ )
SIZZ (PM)
$2.1375 E+06$
2.13 E6
0.4

## Orthotropic material

## Value Identification

Reference
Aster \%
difference
Ur ( $z=0$ )
DX (A1)
5.8018 E 05
5.7828E05
0.33
$U z(z=L)$
DY (A3)
6.10714E06
5.992E06
1.9

```
Uz (z=L)
DY (A4)
2.4429E05
6.1367E06
0 . 5
SigmaTT (z=0)
SIZZ (PM)
2.1375E+06
2.13E6
0 . 4
```


## Handbook of Validation

```
V3.03 booklet: Linear statics of the plates and the hulls HT-66/02/001/A
```


## 4 Modeling

B

## 4.1 <br> Characteristics of modeling

Modeling COQUE_3D. One nets only half of the cylinder (symmetry compared to the $y=0$ plan)
10 meshs QUAD 9 in the height and 20 on the semicircumference.

## 4.2 <br> Characteristics of the grid

A number of nodes: 664
A number of meshs and type: 200 QUAD9

### 4.3 Functionalities

tested

Orders Key word<br>factor<br>Key word<br>AFFE_MODELE<br>AFFE<br>MODELING = COQUE_3D<br>AFFE_CARA_ELEM HULL

ANGL_REP<br>$A F F E \_C H A R \_M E C A \_F$ FORCE_COQUE<br>NEAR<br>DEFI_MATERIAU ELAS_ORTH<br>Handbook of Validation<br>V3.03 booklet: Linear statics of the plates and the hulls<br>HT-66/02/001/A<br>Code_Aster ${ }^{\circledR}$<br>Version<br>6.0<br>Titrate:<br>SSLS120 - Thin hull under hydrostatic pressure<br>Date:<br>12/12/02<br>Author (S):<br>J. Key Mr. PROIX

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### 4.4 Values

tested

Isotropic material

## Value Identification

Reference
Aster \%
difference
$\operatorname{Ur}(z=0)$
DX (PM)
5.8018E05
5.7982E05
0.06
$\operatorname{Ur}(z=0)$
DX (Al)
5.8018E05
5.7982E05
0.06

Orthotropic material

Value Identification<br>Reference<br>Aster \%<br>difference

$\operatorname{Ur}(z=0)$
DX (PM)
5.8018E05 5.8018E05 2.E5
$\operatorname{Ur}(z=0)$
DX (A1)
5.8018E05 5.8018E05
2.E5
$\operatorname{Ur}(z=0)$
DX (A2)
5.8018E05 5.8018E05
2.E5
$U z(z=L)$
DZ (A3)
6.10714E06 6.10716E06
3.E4
$U z(z=L)$
DZ (A4)
6.10714E06 6.10716E06
3.E4
$\operatorname{SigmaTT}(z=0)$

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## 5 Modeling

C
5.1

Characteristics of modeling
Modeling COQUE_3D. One nets only half of the cylinder (symmetry compared to the $y=0$ plan) 10 meshs TRIA7 in the height and 20 on the semicircumference.

## 5.2

Characteristics of the grid
A number of nodes: 864
A number of meshs and types: 400 TRIA7

### 5.3 Functionalities

tested

## Orders Key word

factor

# Key word 

AFFE_MODELE
AFFE
MODELING $=$ COQUE_3D
AFFE_CARA_ELEM HULL
ANGL_REP
AFFE_CHAR_MECA_F FORCE_COQUE
NEAR
DEFI_MATERIAU ELAS_ORTH

Handbook of Validation
V3.03 booklet: Linear statics of the plates and the hulls
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLS120 - Thin hull under hydrostatic pressure
Date:
12/12/02
Author (S):
J. Key Mr. PROIX
:
V3.03.120-A Page:
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### 5.4 Values <br> tested <br> Isotropic material <br> Value Identification Reference <br> Aster \% <br> difference <br> $\operatorname{Ur}(z=0)$ <br> $D Y$ (PM) <br> 5.8018E05 <br> 5.799E05 <br> 0.4 <br> $\operatorname{Ur}(z=0)$

## DX (Al)

5.8018E05
5.793E05
0.15
$\operatorname{Ur}(z=0)$
$D X$ (A2)
5.8018E05
$5.806 E 05$
0.06
$U z(z=L)$
DZ (A3)
2.4429E05
2.4428E05
0.004
$U z(z=L)$
DZ (A4)
2.4429E05
2.4428E05
0.004

SigmaTT ( $z=0$ )
$\operatorname{SIZZ}(P M)$ 2.1375E+06
2.1377E+06 0.008

Orthotropic material

## Value Identification Reference

Aster \%
difference
$\operatorname{Ur}(z=0)$
DX (PM)
5.8018E05
5.8013E05
0.008
$\operatorname{Ur}(z=0)$
DX (A1)
5.8018E05
5.7904E05
0.2
$\operatorname{Ur}(z=0)$
DX (A2)
5.8018E05
5.8122E05

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## 6 Modeling

D

## 6.1

## Characteristics of modeling

Modeling DKT. One nets only half of the cylinder (symmetry compared to the $y=0$ plan) 30 meshs QUAD4 in the height and 60 on the semicircumference.

## 6.2 <br> Characteristics of the grid <br> A number of nodes: 1894 <br> A number of meshs and types: 1800 QUAD4

### 6.3 Functionalities

tested

## Orders Key word

factor
Key word
AFFE_MODELE
AFFE
MODELING $=D K T$
AFFE_CARA_ELEM HULL
ANGL_REP
AFFE_CHAR_MECA_F FORCE_COQUE
NEAR
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## DEFI_COQU_MULT SLEEP

ANGL_REP
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### 6.4 Values

tested
Isotropic material

## Value Identification

Reference
Aster \%
difference
$\operatorname{Ur}(z=0)$
DY (PM)
5.8018E05
5.7916E05
0.18
$\operatorname{Ur}(z=0)$
DX (Al)
5.8018E05
5.7916E05
0.18
$\operatorname{Ur}(z=0)$
DX (A2)
5.8018E05
5.7916E05
0.18
$U z(z=L)$
DZ (A3)
2.4429E05
2.4420E05
0.03
$U z(z=L)$
DZ (A4)
2.4429E05
2.4420E05
0.03
$\operatorname{SigmaTT}(z=0)$
$\operatorname{SIZZ}(P M) 2.1375 E+062.1371 E+060.02$

Orthotropic material
Value Identification
Reference
Aster \%

```
difference
Ur (z=0)
DY (PM)
5.8018E05
5.798E05
0 . 0 6
Ur (z=0)
DX (Al)
5.8018E05
5.798E05
0.06
Ur (z=0)
DX (A2)
5.8018E05
5.798E05
0 . 0 6
Uz (z=L)
DZ (A3)
6.10714E06
6.105E06
0.03
Uz (z=L)
DZ (A4)
6.10714E06
6.105E06
0 . 0 3
SigmaTT (z=0)
SIZZ (PM) 2.1375E+06 2.1371E+06 0.02
```


### 6.5 Remarks

To obtain a correct result, it is necessary to take guard with the convention adopted for NU_LT for hulls multi layers (DEFI_COQU_MULT), which is different from that used for ELAS_ORTH:

Here, it is necessary to take $N u \_L T$ such as $N u T L=E T / E L * N u L T$, that is to say $N U_{-} L T=$ $0.014285714)$.

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## 7

Summary of the results
The results of four modelings are very close to the analytical solution: to the maximum 0.4\% variation for modelings COQUE_3D and DKT, and less than 2\% of variation for modeling axisymmetric, which is explained by the fact why the analytical solution is a mean solution hull.

This test thus validates on the one hand the efforts of pressure varying linearly with the geometry, for thin hulls, and in addition the taking into account of orthotropic elasticity.

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6.2

Titrate:
SSLS121 - Plate laminated subjected to elementary loadings
Date:
19/08/02
Author (S):
Key J.M. PROIX
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Organization (S): EDF/AMA

# Handbook of Validation 

V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.121

SSLS121 - Plate laminated subjected with elementary loadings

## Summary:

This test represents the quasi-static calculation of a laminated plate, composed of 3 layers of material orthotropic, subjected to 4 elementary loadings.

The plate is modelled in finite elements DST (meshs QUAD4), it is located in a plan XZ and is inclined
48,5 degrees compared to $X$ (to check the changes of reference mark).
In this test, the plane constraints and stresses shear transverse, are compared with one analytical reference solution.

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## Titrate:

SSLS121 - Plate laminated subjected to elementary loadings

Date:<br>19/08/02<br>Author (S):<br>Key J.M. PROIX

## :

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## 1 <br> Problem of reference

### 1.1 Geometry

## $Z, W$

$y, v$
B
C
p
Stacking
With
$T$
D
$X, U$
Z
$0^{\circ}$
$90^{\circ}$
has
$0^{\circ}$
L
Width $a=100 \mathrm{~mm}$, thickness $h=1 \mathrm{~mm}$.

## 1.2

Properties of material
The properties of material constituting each of the three layers of the plate are as follows:
Orthotropic material:
$E=25 \mathrm{MPa}$
$E=1 \mathrm{MPa}$

$$
\begin{aligned}
& L \\
& T \\
& G=G=0.5 \mathrm{MPa} \\
& G=0.2 \mathrm{MPa} \\
& l t \\
& l z \\
& t z \\
& =0.25 \\
& \text { lt } \\
& \text { Stacking: } \\
& \text { - orientation: } \\
& \text { [ } 0 / 90 / 0 \text { ] } \\
& \text { - thickness: } \\
& \text { [h/4/h/2/h/4] }
\end{aligned}
$$

## 1.3

Boundary conditions and loadings
The loadings are applied in order to obtain uniform states of stresses in plate:

- Loading case 1: Mxx=1 in the plate


## Embedding on AD

Moment distributed on $B C$ : $M X=1$

- Loading case 2: Myy=1 in the plate


## Embedding on AB

Moment distributed on CD: $M Y=1$

- Loading case 3: QX=1 in the plate


## Embedding on AD

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$:$
$V$
3
2
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Reference solution

## 2.1

Method of calculation used for the reference solution
Analytical solution [bib1].
2.2

Results of reference
The results of reference are as follows:
Forced loading cases
Value (Mpa) Comments
$M x x=1 \operatorname{SIXX}(z=h / 2)$
-6.82
Constraint $x x$ on the lower skin of
Lay down 1
lay down 1
SIXX ( $z=h / 4$ ) layer 2
-0.135
Constraint $x x$ on the lower skin of

## 2 sleep

Myy=1 SIYY ( $z=h / 2$ )
-1.5
Constraint yy on the lower skin of
Lay down 1
lay down 1
SIYY ( $z=h / 4$ ) layer 2
-18.76
Constraint yy on the lower skin of
2 sleep
QX=1 SIXZ
( $z=h / 4$ )
1.279

Constraint $x z$ on the lower skin of
2 sleep
2 sleep
SIXZ
$(z=0) 1.296$
Constraint $x z$ on the average skin of
2 sleep
2 sleep
$Q Y=1 S I Y Z$
( $z=h / 4$ )
0.28125

Constraint $y z$ on the lower skin of
2 sleep
2 sleep
SIYZ
$(z=0) 2.62625$
Constraint $y z$ on the average skin of
2 sleep
2 sleep
The pace of the distribution of the constraints in the thickness of the plate is as follows:
0.135
1.296
2.626
1.279
0.281

## 2.3

# Uncertainties on the solution 

Null (analytical solution).

### 2.4 References <br> bibliographical

[1]
Dhatt-Batoz "Modeling of the structures by finite elements, Volume 2" Pages 246-250 Hermes edition.
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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

## Z

Modeling DST (QUAD4)
C

- The plate is located in the plan $Y=0.5$
- Not A (0.4; 0.5; 0.25)
${ }^{y}$
X
B
D
Y
$48^{\circ} 5$
With
X


## 3.2

Characteristics of the grid
A number of nodes: 49
A number of meshs and type: 36 QUAD4

### 3.3 Functionalities

tested

## Orders Key word

factor
Key word
AFFE_MODELE
AFFE
"DST"
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
MATER
ORIENTATION
AFFE_CARA_ELEM
HULL
THICK
ANGL_REP
AFFE_CHAR_MECA
FORCE_CONTOUR
NEAR
CALC_CHAM_ELEM
NUME_COUCHE
NIVE_COUCHE
"SUP" "MOY"
OPTION
"SIGM_ELNO_DEPL"
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## 4

Results of modeling $A$

### 4.1 Values

tested

## Loading case

Identification
Reference
Aster Difference
(\%)
Mxx=1 SIXX $(z=h / 2)$ layer 1
-6.82
-6.818
0.03

SIXX ( $z=h / 4$ ) layer 2
-0.135
-0.13636
1
Myy=1 SIYY ( $z=h / 2$ ) Layer 1
-1.5
-1.5
0
$\operatorname{SIYY}(z=h / 4)$ layer 2
-18.76
-18.75
0.05
$Q X=1$
SIXZ ( $z=h / 4$ ) Layer 2
1.279
1.278
0.05

SIXZ (z=0) Layer 2
1.296
1.295
0.09
$Q Y=1$
SIYZ ( $z=h / 4$ ) Layer 2
0.28125
0.2812
0.09

SIYZ (z=0) Layer 2
2.62625
2.625
0.04

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## 5

## Summary of the results

The very good agreement of the results with the analytical solution validates the calculation of the constraints
for a composite plate in an unspecified reference mark, at various levels thickness.
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## Titrate:

SSLS123 - Sphere under uniform external pressure

Date:
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Author (S):
J.M. PROIX, S. BAGUET

Key
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Organization (S): EDF-R \& D /AMA, INSA-LYON

## Handbook of Validation

V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.123

SSLS123-Sphere under uniform external pressure

## Summary:

One treats the case of the sphere under uniform pressure external in linear elasticity, which makes it possible to evaluate
quality of the modeling of the compressive forces.
The values tested are radial displacements at the points of intersection with the axes.
One has 2 modelings:

A: elements 3D in HEXA8
B: elements SHB8
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## 1

Problem of reference

### 1.1 Geometry

$B$
$B$
$y$
With
With
$X$
$X$
Ray
$R=10 . m$
Thickness
$T=0.04 \mathrm{~m}$

## Co-ordinates of the points:

With
B
C
X 10.
0.
0.
$y 0$.
10.
0.

Z 0 .
0.
10.

## 1.2 <br> Material properties

$E=6.825107 \mathrm{~Pa},=0.3$

## 1.3

Boundary conditions and loadings
On a quarter of the hemisphere:

## Side AC

symmetry compared to the xz plan
Side BC
symmetry compared to the $y z$ plan

# Side $A B$ <br> symmetry compared to the xy plan <br> External pressure uniform P=1.Pa <br> Handbook of Validation <br> V3.03 booklet: Linear statics of the plates and hulls <br> HT-66/03/008/A 

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2
Reference solution

## 2.1

Method of calculation used for the reference solution
Radial displacement in any node of the sphere under external pressure is given by:
C
$U=$.
B R +

## R

2
R
With
1-2
R 3
1+

## R 3 r 3

B

## E

$=$
P
$P$ . and C
I
I
E
$T$
$T$
where $R=R$ -
and $\boldsymbol{R}=\boldsymbol{R}+$
I
2
E
2

## 2.2

Results of reference
Displacement of point $A$ following $X$, displacement of the point $B$ following $y$, displacement of the point C following
Z.

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

## Element hull SHB8

C2
C1
A1
B1
A2
B2

Modeling of a quarter of the sphere in SHB8.
Names of the nodes:

Not A1
N40
Not $A 2$
N42

## Not B1

N01
Not B2
N02
Not C1
N662
Not C2
N658

## 3.2

Characteristics of the grid
A number of nodes: 662
A number of meshs and types: 300 SHB8 for the sphere and 300 QUAD4 for external surface.

### 3.3 Functionalities

tested
Orders

MODI_MAILLAGE ORIE_SHB8<br>GROUP_MA<br>ALL<br>MODI_MAILLAGE ORIE_PEAU_3D GROUP_MA<br>SEXT<br>AFFE_CHAR_MECA DDL_IMPO<br>GROUP_NO<br>PRES_REP<br>GROUP_MA<br>"MECHANICAL" AFFE_MODELE<br>SHB8 ALL<br>DEFI_MATERIAU ELAS

4
Results of modeling $A$

### 4.1 Values

tested

```
Identification Reference Aster %
difference
Not A2
-1.28279.10-5 -1.27928.10-5
0 . 2 7
displacement U
Not B2
-1.28279.10-5 -1.27929.10-5
0 . 2 7
displacement v
Not C2
-1.28279.10-5 -1.3034.10-5
1.7
displacement W
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```

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## 5 Modeling

B
5.1

Characteristics of modeling
Voluminal element 3D HEXA8

Modeling of a quarter of the sphere in HEXA8.
Names of the nodes:

Not A1
N40
Not $A 2$
N42
Not B1
N01
Not B2
N02
Not C1
N662
Not C2
N658

## 5.2

Characteristics of the grid
A number of nodes: 662
A number of meshs and types: 300 HEXA8 for the sphere and 300 QUAD4 for external surface.

### 5.3 Functionalities

tested
Orders

MODI_MAILLAGE ORIE_PEAU_3D
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
PRES_REP

GROUP_MA

"MECHANICAL" AFFE_MODELE "3D"

ALL
DEFI_MATERIAU ELAS
6
Results of modeling B
6.1 Values
tested
Identification Reference Aster \% difference
Not A2
-1.28279.10-5 -1.28298.10-5
0.015
displacement $\mathbf{U}$
Not B2
-1.28279.10-5 -1.28298.10-5
displacement v
Not C2
-1.28279.10-5 -1.28662.10-5
0.30
displacement $W$
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## Titrate:

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```


## 7

```
Summary of the results
Results in conformity with the reference solution.
One could expect to find exactly same displacement to the three point \(A, B\) and \(C\). difference at the point C comes from not-symmetry from the grid. The grid is slightly more distorted around this point, which explains the fall of precision, which remains nevertheless very good, as well for element HEXA8 for the SHB8.
```

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SSLS124-Beam in inflection with various twinges
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Author (S):
J.M. PROIX, Key S. BAGUET

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SSLS124-Beam in inflection with various
twinges

## Summary:

This test represents a calculation quasi-static of a beam in inflection, embedded at an end, and subjected to
a vertical force at the other end. This test makes it possible to validate for a linear elastic design, and four values of twinge (variable thicknesses) in each of two modelings:

- Finite elements SHB8 for a regular grid (modeling A)
- Finite elements SHB8 for a nonregular grid (modeling B)

Displacements obtained are compared with the elastic analytical solution of a beam in inflection. This test allows to assemble the limits of the elements in term of twinge, on the one hand, and to show their good

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## 1

Problem of reference

### 1.1 Geometry

## Z <br> C <br> $y$ <br> O <br> H <br> $L$ <br> F <br> With <br> $X$ <br> B <br> $L$

$L=$ length 100 m , width $l=10 \mathrm{Mr}$.
Thickness: case $1 \mathrm{~h}=10 \mathrm{~m}$, cases $2 \mathrm{~h}=1 \mathrm{~m}$, cases $\mathbf{3} \mathrm{h}=0.1 \mathrm{~m}$, cases $4 \mathrm{~h}=0.05 \mathrm{~m}$, cases $5 \mathrm{~h}=0.02 \mathrm{~m}$

## 1.2 <br> Material properties

$E=2.1011 \mathrm{~Pa}$

## 1.3

Boundary conditions and loadings
Embedded on side $O C: U=v=W=0, X=y=Z=0$
At end AB, a load uniformly distributed of resultant:
Force parallel with axis $Z ; F z=1$ NR

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2
Reference solution

## 2.1

Method of calculation used for the reference solution
The results of reference are obtained by the theory of the elastic beams.
Vertical displacement at end $A B$ is given by:
Uy = F.L3/3.E.Iz
With
$I z=l . h 3 / 12$

# 2.2 <br> Results of reference <br> Displacement of points $\boldsymbol{A}$ and $B$ following $Z$. 

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

## Element SHB8

$Z$
$X$
$y$

Cutting: a regular grid is considered in this modeling.

## Regular grid:

10 meshs SHB8: 1 according to the width, 10 according to the length, 1 according to the thickness thickness: case $1 \mathbf{h = 1 0} \mathbf{m}$, cases $\mathbf{2 h = 1} \mathbf{m}$, cases $3 h=0.1 \mathrm{~m}$, cases $4 h=0.05 \mathrm{~m}$, cases $5 h=0.02 \mathrm{~m}$

Boundary conditions:
In all the nodes on the side OC: blocked displacement following $X$
in C1: blocked displacement following Y and Z
in C2: blocked displacement following $Y$
in O1: blocked displacement following Z
Loading:
in A2: nodal force according to $X: F X=0,5$
in B2: nodal force following $Y$ : $F Y=0,5$
Name of the nodes:
Not O1
N40
Not O2
N44
Not A1
N03
Not $\mathbf{A 2}$
N01
Not B1
N04
Not B2
N02
Not C1
N43
Not C2
N39

## 3.2

Characteristics of the grid
A number of nodes: 44
A number of meshs and types: 11 SHB8
In the case of the regular grid, each element is a perfect square on side length 10 m

### 3.3 Functionalities

tested

\author{
MODI_MAILLAGE ORIE_SHB8 <br> GROUP_MA <br> ALL <br> AFFE_CHAR_MECA DDL_IMPO <br> GROUP_NO <br> FORCE_NODALE <br> GROUP_NO <br> AFFE_MODELE AFFE <br> MODELING <br> ```
:

``` \\ SHB8 \\ MECA_STATIQUE SOLVEUR \\ NPREC \\ \section*{Handbook of Validation} \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS124-Beam in inflection with various twinges

\section*{Date}
:
06/11/03
Author (S):
J.M. PROIX, Key S. BAGUET

\section*{:}

V3.03.124-A Page:
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4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Regular grid:}

\section*{Thickness}

Not
Size in unit
Reference
Aster
\% difference
Case 1
A2
displacement \(W\) (m)
2. 10-9
2.008 10-9
+0.41
\(H=10 m\)
B2
displacement \(W\) (m)
2. 10-9
2.008 10-9
+0.41
Case 2
A2
displacement \(W\) (m)
2. 10-6
1.995 10-6
-0.27
\(H=1 m\)
B2
displacement \(W\) (m)
2. 10-6
1.995 10-6
-0.27
Case 3
A2
displacement \(W\) (m)
2. 10-3
1.994 10-3
-0.28
\(H=0.1 m\)
B2
displacement \(W\) (m)
2. 10-3
\[
H=0.05 m
\]
B2displacement \(W\) (m)
1.6 10-2
1.595 10-2
-0.3
Case 5
A2
displacement \(W\) (m)
0.25
2.380 10-1\(+4.8\)
\(H=0.02 m\)
B2
displacement \(\boldsymbol{W}\) (m)
0.252.416 10-1
+3.4

\subsection*{4.2 Remarks}

For the strong twinges, (case 3, 4, 5), it is necessary to increase the number of decimals lost with the resolution, using key word NPREC. This does not prevent from obtaining a correct solution
(at least for cases 3 and 4).
The twinges are ( \(h / m\) in report/ratio \((I, L / 10)\) ) :
Case 1: 1
Case 2: 0.1
Case 3: 0.01
Case 4: 0.005
Case 5: 0.002
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V3.03 booklet: Linear statics of the plates and hulls

Code_Aster \({ }^{\circledR}\)
Version
7.2

\section*{Titrate:}

SSLS124-Beam in inflection with various twinges
Date
:
06/11/03
Author (S):
J.M. PROIX, Key S. BAGUET
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5 Modeling
B
5.1

Characteristics of modeling
Element SHB8

Cutting: an irregular grid is considered in this modeling.
Not-regular grid:
10 meshs SHB8: 1 according to the width, 10 according to the length, 1 according to the thickness thickness: case \(1 \mathrm{~h}=10 \mathrm{~m}\), cases \(2 \mathrm{~h}=1 \mathrm{~m}\), cases \(\mathbf{3} \mathrm{h}=0.1 \mathrm{~m}\), cases \(\mathbf{4} \mathrm{h}=0.05 \mathrm{~m}\), cases \(5 \mathrm{~h}=0.02 \mathrm{~m}\)

Boundary conditions:
In all the nodes on the side OC: blocked displacement following \(X\)
in C1: blocked displacement following Y and Z
in C2: blocked displacement following \(Y\)
in O1: blocked displacement following Z

\section*{Loading:}
in A2: nodal force according to \(X\) : \(F X=0,5\)
in B2: nodal force following \(Y: F Y=0,5\)

Names of the nodes:
Not O1
N40
Not O2
N44
Not A1
N03
Not \(\mathbf{A 2}\)
N01
Not B1
N04
Not B2
N02
Not C1
N43
Not C2
N39

\section*{5.2}

Characteristics of the grid
A number of nodes: 44
A number of meshs and types: 11 SHB8

\subsection*{5.3 Functionalities}
tested
Orders

MODI_MAILLAGE ORIE_SHB8
GROUP_MA
ALL
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FORCE_NODALE
GROUP_NO
AFFE_MODELE AFFE
MODELING
:
SHB8

\section*{MECA_STATIQUE SOLVEUR \\ NPREC}

\section*{Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/03/008/A}

Code_Aster \({ }^{\circledR}\)
Version
7.2

Titrate:
SSLS124-Beam in inflection with various twinges

\author{
Date
}
:
06/11/03
Author (S):
J.M. PROIX, Key S. BAGUET
:
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\section*{6}

Results of modeling B
6.1 Values
tested
Not-regular grid:
Thickness
Not
Size in unit
Reference
Aster
\% difference
Case 1
A2
displacement \(W\) (m)
2. 10-9
1.939 10-9
-3.03
\(H=10 \mathrm{~m}\)
B2
displacement \(W\) (m)
2. 10-9
1.921 10-9
-3.94
Case 2
A2
displacement \(W\) (m)
2. 10-6
1.925 10-6
-3.73
\(H=1 m\)
B2
displacement \(W\) (m)
2. 10-6
1.907 10-6
-4.64
Case 3
A2
displacement \(W\) (m)
2. 10-3
1.925 10-3
-3.75
\(H=0.1 m\)
B2
displacement \(W\) (m)
2. 10-3
1.907 10-3
-4.65
Case 4
A2
displacement \(W\) (m)
1.6 10-2
1.542 10-2
-3.60
\(H=0.05 m\)
B2
displacement \(W\) (m)
1.6 10-2
1.528 10-2
-4.51
Case 5

\section*{A2}
displacement \(W\) (m)
0.25
2.479 10-1
-0.83
\(H=0.02 m\)
B2
displacement \(W\) (m)
0.25
2.45710-1
-1.72

\subsection*{6.2 Remarks}

For the strong twinges, (case 3, 4, 5), it is necessary to increase the number of decimals lost with the resolution, using key word NPREC. This does not prevent from obtaining a correct solution
(at least for cases 3 and 4).
The twinges are (h/min report/ratio (I, L/10)) :
Case 1: 1
Case 2: 0.1
Case 3: 0.01
Case 4: 0.005
Case 5: 0.002
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SSLS124-Beam in inflection with various twinges
Date
:
06/11/03
Author (S):
J.M. PROIX, Key S. BAGUET

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\section*{7}

Summary of the results
In the case of the regular grid, from good solutions are obtained. The quality of the solution declines nevertheless when the twinge of the element (side ratio/thickness) reached 200.

In the case of the not-regular grid, whatever the twinge of the element, one tends to underestimate the rigidity of the beam from approximately \(4 \%\).

This test can be supplemented by a modeling 3D and to be applied to the elements of COQUE_3D MEC3QU9H like with elements DKT.

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Code_Aster \({ }^{\circledR}\)
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Titrate:
SSLS125-Buckling of a free cylinder under external pressure
Date:
28/10/03
Author (S):
J.M. PROIX, Key S. BAGUET
:
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Organization (S): EDF-R \& D /AMA, INSA LYON

\title{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ Document: V3.03.125
}

SSLS125-Buckling of a free cylinder under external pressure

\section*{Summary:}

This test represents a calculation of stability of a free thin cylindrical envelope at its ends subjected to an external pressure. One calculates the critical loads leading to the elastic buckling of Euler. The matrix
of geometrical rigidity used in the resolution of the problem to the eigenvalues is that which is due to initial constraints.

He makes it possible to validate modeling finite elements SHB8.
The critical load and the clean mode obtained are compared with an analytical reference solution.
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V3.03 booklet: Linear statics of the plates and hulls
HT-66/03/008/A
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Titrate:
SSLS125-Buckling of a free cylinder under external pressure
Date:
28/10/03
Author (S):
J.M. PROIX, Key S. BAGUET

\section*{V3.03.125-A Page:}

\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}
=
Z, W
Free cylinder at the ends
Z
\(O^{\prime}\)
C
D
L/2
O
B
\(y\)
With
\(y, v\)
\(X\)
\(X, U\)

The symmetry of the problem makes it possible to model a quarter of cylinder length L, with conditions
of symmetry specific to the lower edge.
\(L=2 m\)
Average radius \(R=2 m\)
Thickness \(E=0.02 \mathrm{~m}\)

\section*{1.2}

Properties of material
The properties of material constituting the plate are:
\(E=2.1011 \mathrm{~Pa}\)
Young modulus
\(=0.3\)
Poisson's ratio

\title{
1.3 \\ Boundary conditions and loadings
}

\section*{Loading:}
- pressure uniformly distributed of pcr= 1. Pa on the cylindrical part.
- Conditions of symmetry:
- on \(A B: D Z=0\)
- on \(B C: D X=0\)
- on DA: \(D Y=0\)

\subsection*{1.4 Conditions \\ initial}

Without object
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2
2.1

Method of calculation used for the reference solution
The critical pressure is given in [bib1] or [bib2] by the following expression:

Pcr \(=\) E/12/(1-naked \(\left.{ }^{2}\right) . n^{2} \cdot(e / R) 3\) with \(N\) number of the mode (here \(N=2,4,6\) )

\section*{2.2}

Results of reference
The pressures criticize (out of Pa) are:

Mode (N)
Reference
273260
4293040
6659340

\section*{2.3}

Uncertainties on the solution
Analytical solution

\subsection*{2.4 References \\ bibliographical}
[1]
S.P. TIMOSHENKO, J.M. MANAGES: Theory of elastic stability, page 500, second edition, DUNOD 1966.
[2]
BO O. ALMROTH, D.O. BRUSH: Buckling of bars, punts and shells, page 173, Mc GrawHill, New York, 1975.
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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Z, W
Modeling SHB8
Z
G
48
H
\(O^{\prime}\)
\(F\)

E
4
L/2
C
D
O
B
\(y\)
P1,
\(y, v\)
P12
X
\(X, U\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 882
A number of meshs and types: 400 HEXA8

\subsection*{3.3 Functionalities}
tested

\section*{Orders Key word}
factor
Key word

\section*{AFFE_MODELE AFFE \\ MODELISATION=SHB8}

\author{
AFFE_CHAR_MECA PRES_REP NEAR \\ CALC_MATR_ELEM OPTION \\ "RIGI_MECA" \\ "RIGI_GEOM" \\ MODE_ITER_SIMULT METHOD \\ "SORENSEN" \\ OPTION \\ "PLUS_PETITE" \\ \section*{4} \\ Results of modeling \(A\) \\ \subsection*{4.1 Values} \\ tested \\ Identification Mode \\ ( \(N\) )
}

\title{
Reference
}

Aster \%
difference
Pressure criticizes (Pa)
2
73260
72492
1
4
293040
293481
0.2

6
659340
673600
2
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\section*{5}

Summary of the results
The results obtained are satisfactory. Uncertainties on the critical pressure do not exceed \(2 \%\).

The modal deformation obtained corresponds well to the awaited circumferential mode: \(n=2\) for both modelings.

This test made it possible to test modeling SHB8 in linear buckling of Euler of a mean structure subjected to an external pressure.

\section*{Handbook of Validation}

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Date:
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Author (S):

\section*{J.M. PROIX, Key S. BAGUET}

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Titrate:
SSLS128 - Validation of option CRIT_ELNO_RUPT of CALC_ELEM
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05/03/04
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Key J.M. PROIX
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.128

SSLS128-CALC_ELEM: Validation of the option
CRIT_ELNO_RUPT

\section*{Summary:}

This option allows, in the case of the multi-layer hulls to calculate in a layer the constraints in locate this one as well as the corresponding criterion of Tsai-Hill.
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SSLS128 - Validation of option CRIT_ELNO_RUPT of CALC_ELEM
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Key J.M. PROIX
:
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1
Problem of reference
1.1 Geometry

LEFT
RIGHT-HAND SIDE
N1

It is about a composite material made up of 16 superimposed layers of the same material and of fibre directions different forming plate a 4.48 m thickness. The longitudinal direction or feel fibres of each layer is defined by the first direction of orthotropism.

\section*{1.2}

Property of material
The properties of material are:
\(\cdot\) longitudinal modulus Young: \(E_{-} L=59000 \mathrm{MPa}\)
- transverse modulus Young: E_T \(=59000 \mathrm{MPa}\)
- modulus of rigidity in plan LT: \(G_{-} L T=3700 \mathrm{MPa}\)
- Poisson's ratio in plan LT: NU_LT = 0.08
- criterion of rupture in traction in the longitudinal direction: \(X T=560 \mathrm{MPa}\)
\(\cdot\) criterion of rupture in compression in the longitudinal direction: \(X C=-475 \mathrm{MPa}\)
- criterion of rupture in traction in the transverse direction: YT \(=560 \mathrm{MPa}\)
- criterion of rupture in compression in the transverse direction: YC=-475 MPa
- criterion of rupture in shearing in plan LT: S_LT = 48 MPa

The orientation of the first layer is \(0^{\circ}\) compared to the reference mark of reference, for the second layer
\(45^{\circ}\), for the third \(0^{\circ}\) and so on.

\section*{1.3}

Boundary conditions and loadings
-N1
\(D Y=0, D Z=0, D R X=0, D R Y=0, D R Z=0\)
- LEFT
\(D X=0\)
- RIGHT
\(F X=-784 N R\)

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
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Key J.M. PROIX
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2
Reference solution given by the software "Plates"

In the first layer with \(0^{\circ}\)

\section*{Sxx Syy Szz}
-242-67 0
Constraints in the reference mark of the plate
SLST
SLT
-242-670
Constraints in the reference mark of the layer
Criterion of Tsai-Hill C
\(=0.344\)
TH
In the second with \(45^{\circ}\)

Sxx Syy Szz
-108-67 0
Constraints in the reference mark of the plate
SL ST
SLT
-88-88 21
Constraints in the reference mark of the layer

\section*{Criterion of Tsai-Hill C}
\(=0.223\)
TH

SL is the constraint in the first direction of orthotropism of the layer, ST the second and SLT shear stress.
Sxx, Syy, Szz are the constraints in the reference mark of the user.

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls

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\section*{Code_Aster \({ }^{\circledR}\)}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

The hull is modelled by elements DKT. Its characteristics are defined in AFFE_CARA_ELEM:
\(\cdot\) thickness: 16 X \(0.28=4.48 \mathrm{~m}\)
\(\cdot\) reference mark of reference of the hull defined by ANGL_REP \(=0\).
The various layers are defined by the operator DEFI_COQU_MULT who gives for each its thickness, its material and its orientation compared to the reference mark of reference sleep defined in
AFFE_CARA_ELEM.

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 624 \\ A number of meshs and types: 48 SEG2 and 576 QUA4
}

\author{
3.3 Functionalities \\ tested \\ Orders \\ THICK DEFI_COQU_MULT
}

\author{
MATER \\ ORIENTATION \\ CALC_ELEM SIGM_ELNO_DEPL \\ \section*{CRIT_ELNO_RUPT} \\ \section*{4} \\ Results of modeling \(A\) \\ \subsection*{4.1 Values} \\ tested
}

For the layer with \(0^{\circ}\)
Identification Reference
Aster \%
difference
Sxx-242-2.41623E+02
0.15

Syy 67
6.66229E+01
0.56

SL - 242
-2.41623E+02
0.15

ST 67
\(6.66229 E+01\)
0.56
|SLT| 0
2.82232E-12

0
CTH 0.344
3.44256E-01
0.06

For the layer with \(45^{\circ}\)
Identification Reference
Aster \%
difference
Sxx-108-1.08377E+02
```

0.35
Syy -67
-6.66229E+01
0.56
SL -88
-8.75000E+01
0.57
ST -88
-8.75000E+01
0 . 5 7
|SLT| 21
2.08771E+01
0.58
CTH 0.223
2.23106E-01
0.05
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```

\section*{5 Synthesis}

The results obtained are satisfactory. The maximum of difference is approximately \(0.6 \%\) and it is due to fact that the results resulting from the software "Plate" are given with little precision. Handbook of Validation
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Code_Aster \({ }^{\circledR}\)
Version
8.2

\author{
Titrate: \\ DEMO006 Optimization of the radius of curvature of a bent piping \\ Date: \\ 05/09/05 \\ Author (S): \\ J. LAVERNE, F. LEBOUVIER \\ Key: V3.03.131-A \\ Page: \\ 1/6 \\ Organization (S): EDF-R \& D /AMA, DeltaCAD
}

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
V3.03.131 document

DEMO006 Optimization of the radius of curvature of a bent piping

\section*{Summary:}

This test of demonstration illustrates the use of the language python in Code_Aster.
The objective of this case-test is to optimize the value of the radius of curvature of a piping via a loop python.

The radius of curvature is modified repeatedly. The optimal value is obtained when the maximum constraint
of Von Mises is lower than a threshold.
The language python, in this case-test allows repeatedly:
to modify the file describing the geometry of the elbow, of launching GMSH in order to net the elbow,
to recover the maximum constraint of Von Mises,
to evaluate a criterion of stop of the iterations,
of launching GMSH to carry out an interactive postprocessing.
This case-test takes again the problem of the case-test forma01f: bent piping, made up of an elastic material
linear, subjected to a force applied at its end, modelled by elements of hulls DKT.
Note:
To have the interactive functions of visualization, to put interactive \(=1\) at the beginning of the file of order.
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Titrate:
DEMO006 Optimization of the radius of curvature of a bent piping
Date:
05/09/05
Author (S):
J. LAVERNE, F. LEBOUVIER

Key: V3.03.131-A
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1
Problem of reference

\subsection*{1.1 Geometry}

The study relates to a piping including/understanding two right pipes and an elbow [Figure 1.1-a].
The geometrical data of the problem are as follows:
length LG of the two right pipes is \(\mathbf{3} \mathbf{~ m}\),
initial the Rc ray of the elbow is 0.3 m ,
the angle of the elbow is 90 degrees,
the thickness of the right pipes and the elbow is 0.02 m , and the ray external Re of the right pipes and the elbow is of 0.2 Mr .

LG
D
B
section D
section B
RC
C
0
section C

\section*{Z \\ Y \\ E \\ L \\ Z \\ Re \\ X}

With
section A
Appear 1.1-a
Note:
The geometry of the problem has a symmetry compared to the plan (A, X, Y).

\section*{1.2}

Material properties
Isotropic linear elastic material. The properties of material are those of A42 steel:
the Young modulus:
\(\mathrm{E}=1.81011 \mathrm{~Pa}\),
the Poisson's ratio:
\(=0.3\),
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Code_Aster \({ }^{\circledR}\)
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8.2

Titrate:
DEMO006 Optimization of the radius of curvature of a bent piping
Date:
05/09/05
Author (S):
J. LAVERNE, F. LEBOUVIER

\section*{1.3}

Boundary conditions and loadings
- Boundary conditions: embedding on the level of section A,
- Chargement: force constant FY directed according to the axis Y and applied to the section B.

The value of FY is calculated to leave:
average radius: \(R M O Y=0.19\),
total force applied:
2
\(F T O T=500000 \mathrm{NR} / \mathrm{m}\),
Its expression is as follows:
\(F Y=F T O T(2 R M O Y)(418828.8)\)
- The limiting constraint of Von Mises is of \(2.0 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)

\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is obtained numerically. It is thus only about one test of not regression.

\section*{2.2}

Results of modeling
The optimal value of the radius of curvature, respecting the criterion put < \(2.109 \mathrm{~N} . \mathrm{m}-\mathbf{2}\) is of \(\mathbf{1 . 1 m}\), obtained after 5 iterations. With each iteration the radius of curvature is increased by 0.2 m .

Rays of
Constraint max of
Iterations
curve
Von Mises
1

\section*{0.3 m}
\(3.3315 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
2
0.5 m
\(2.7647 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
3
0.7 m
\(2.4256 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
4
0.9 m
\(2.1727 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
5
1.1 m
\(1.9670 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)

\section*{2.3 \\ Uncertainty on the solution}

Solution of nonregression.
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HT-66/05/005/A

Code_Aster \({ }^{\circledR}\)
Version
8.2

Titrate:
DEMO006 Optimization of the radius of curvature of a bent piping
Date:
05/09/05
Author (S):
J. LAVERNE, F. LEBOUVIER

Key: V3.03.131-A
Page:
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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling

Modeling in elements hulls (DKT).

Appear 3.1-a: geometry and grid of the half roll (initial RC)

\section*{3.2 \\ Characteristics of the grid}

The grid is regenerated with each iteration because of the modification of the radius of curvature of
geometry. However the topological characteristics of the grid are unchanged:

1013 meshs (900 TRIA3, 110 SEG2, 3POI3)
507 nodes.
The groups of meshs correspond to:

\section*{GM30 <=> surface of the PIPE}

GM28 <=> section B (effort)
-
GM31 <=> not A1 (- R, 0, 0)
\(\cdot\)
GM27 <=> section A (embedding)
GM29 <=> SYMMETRY
Handbook of Validation
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Titrate:
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Date:

\author{
J. LAVERNE, F. LEBOUVIER
}

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\subsection*{3.3 Orders \\ Aster \\ This paragraph describes the algorithm used in the command file and presents the orders Code_Aster used. \\ Initialization of certain variables: \\ Radius of curvature: \(\mathbf{R c}=\mathbf{0 . 3}\) \\ Criterion of convergence: crit \(=2.0 \mathrm{E}+09\) (forced of Von Mises targets)}

One enters the loop python whose criterion of stop relates to the constraint max of Von Settings. As long as the criterion is not respected, the following instructions are carried out:

Rebuilding of the grid:
one
redefines
Rc in the geometrical file .geo of GMSH one launches gmsh via python to generate the file of grid .msh

Reading of grid (PRE_GMSH) and generation of grid (LIRE_MAILLAGE). One uses DEFI_GROUP to re-elect the groups of meshs according to the correspondence:
```


# GM30 <=> PIPE

# GM28 <=> EFOND

```
```


# GM31 <=> A1

```
```


# GM27 <=> ENCAST

```
\#

GM29
<=>
SYMMETRY
Definition of the finite elements used (AFFE_MODELE). The right pipes and the elbow are modelled by elements of hull (DKT).

Reorientations of the normals to the elements: one uses MODI_MAILLAGE to direct all elements in the same way, with a normal turned towards the interior.

Definition and assignment of material (DEFI_MATERIAU and AFFE_MATERIAU). mechanical characteristics are identical on all the structure.

Assignment of the characteristics of the elements hulls (AFFE_CARA_ELEM): thickness, vector \(V\) defining the reference mark of examination (key word ANGL_REP)

Definition of the boundary conditions and loading (AFFE_CHAR_MECA).
Piping is embedded in its base, on all the nodes located in the \(Y=0\) plan. Piping presents a symmetry plane \(\mathbf{Z}=\mathbf{0}\).

One calculates an effort distributed FY directed according to the axis \(Y\) and applied to the section B, (the effort
distributed is such as the resultant \(2 \mathrm{Pi}^{*}\) RMOY. FY = FTOT, FTOT being the total force that one wishes to apply). To apply the effort to the section B, one will use FORCE_ARETE.

Resolution of the linear elastic problem (MECA_STATIQUE).
Calculation of the stress field by elements to the nodes for each loading case (option "SIGM_ELNO_DEPL"). The constraints are calculated in the definite local reference mark
for each element using the vector \(V\) (preceding key word ANG_REP). To use NIVE_COUCHE to define the level of calculation in the thickness.

Calculation of the stress field equivalent by elements to the nodes calculated from stress field (option "EQUI_ELNO_SIGM").

Calculation of the preceding fields to the nodes (options "SIGM_NOEU_DEPL", Buckle python of optimization of the radius of curvature
"EQUI_NOEU_SIGM")

Impression of results (IMPR_RESU).
Determination of a table containing calculations of averages of the stress field equivalent to the nodes. ("POST_RELEVE_T")

Extraction of component VMIS of the preceding table via python.
Test of stop:
If VMIS is higher than CRIT, then one reiterates with \(\mathrm{Rc}=\mathrm{Rc}+\mathbf{0 . 2}\)
If not one leaves the loop python.
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Titrate:
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4
Results of modeling A
4.1 Values
tested
Rays of
Constraint max of Von
Iterations
curve
Settings
1
0.3 m
\(3.3315 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
2
0.5 m
\(2.7647 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
3
0.7 m
\(2.4256 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
4
0.9 m
\(2.1727 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)
5
1.1 m
\(1.9670 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\)

\section*{5}

Summary of the results
The optimal radius of curvature respecting the criterion: constraint max of Von Mises < to 2.0E +09 is of
1.1m. It was obtained after 5 iterations, and the maximum constraint of Von Mises found is of \(1.9670 \mathrm{E}+09 \mathrm{~N} / \mathrm{m}^{2}\).

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the plates and hulls
HT-66/05/005/A
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Version
5.0

Titrate:
SSLS501-Roll infinitely long subjected to two lines of load
Date:
01/10/01
Author (S):
P. MASSIN, F. LEBOUVIER Key
:
V3.03.501-A Page:
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Organization (S): EDF/MTI/MMN, DeltaCAD

\title{
Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ Document: V3.03.501
}

SSLS501-Roll infinitely long subjected
with two lines of load

\section*{Summary:}

This test represents a quasi-static calculation of an infinitely long cylinder subjected to two lines of load
diametrically opposite. It makes it possible to validate modeling finite elements COQUE_D_PLAN with catch in
count variation of curve between the under-surface and suction face (AFFE_CARA_ELEM: MODI_METRIQUE).

Displacements and the efforts obtained are compared with an analytical reference solution.
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V3.03 booklet: Linear statics of the plates and hulls
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLS501-Roll infinitely long subjected to two lines of load
Date:

\subsection*{1.1 Geometry}
\(y, v\)
, \(y\)

\section*{\(Q\)}
\(X\)
\(X, U\)
, X
Ray: \(R=1 \mathrm{~m}\)
Thickness: \(\boldsymbol{H}=0.1 \mathrm{~m}\)
, Z
R
\(Z, W\)

\section*{1.2}

Properties of material
The properties of material constituting the cylinder are:
\(E=105 \mathrm{~Pa}\)
Young modulus
\(=0\)
Poisson's ratio
1.3

Boundary conditions and loadings

CL: displacement following \(Z\) is null for the whole of the points of the cylinder
Force per unit of length: \(Q=1.5 \mathrm{~N} / \mathrm{m}\)
1.4 Conditionsinitial
Without object
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HI-75/01/010/A
Code_Aster \({ }^{\circledR}\)
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SSLS501-Roll infinitely long subjected to two lines of load
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Author (S):
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2
Reference solution
2.1
Method of calculation used for the reference solution
The reference solution obtained by KOITER-SANDERS [bib1] [bib2] is based on the theory ofdeep hulls.
2.2
Results of reference
The results of reference are:
Displacement following y to point a: 13.455
10-3 m
Displacement following \(X\) to point b: 12.255

Normal effort at point A
: 0.0
NR

Bending moment at point \(A\)
:
0.477 N.m

\section*{2.3 \\ Uncertainties on the solution}

Analytical solution

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

GEOFFROY P.: "Development and evaluation of a finite element for the non-linear analysis statics and dynamics of thin hulls '", Thesis of Doctor Engineer UTC, 1983.
[2]
BATOZ J.L. : "Non-linear Analysis of the elastic thin hulls of arbitrary form by curved triangular elements '", Thesis of science doctorate ( pH . D thesis), Department from civil engineering, Laval University, Quebec, Mars 1977.
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls

\section*{HI-75/01/010/A}

Code_Aster \({ }^{\circledR}\)
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5.0

\section*{Titrate:}

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\section*{:}

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

Grid of the arc of circle \(A B\) with 32 finite elements of type COQUE_D_PLAN
\(y, v\)
F
With
Conditions of symmetry:
32
- Not a: \(U==0\)

Z
- Not b: \(v==0\)

Z
B
\(X, U\)
Loading:
Z
- Not a: \(F=q / 2\)

Z

\section*{3.2}

Characteristics of the grid
A number of nodes: 17
A number of meshs and type: 8 SEG3

\subsection*{3.3 Functionalities}
tested

\author{
Orders Key word
}
factor Key word
AFFE_MODELE AFFE
"COQUE_D_PLAN"
AFFE_CARA_ELEM HULL
THICK
A_CIS: 0.833

\author{
MODI_METRIQUE: "YES" \\ AFFE_CHAR_MECA FORCE_NODALE \\ FY
}

\section*{Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HI-75/01/010/A}

Code_Aster \({ }^{\circledR}\)
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
Displacement following y to the point
13.455 E3 m
13.603 E3
1.099

With
Displacement following \(X\) to the point
12.255 E 3 m
12.360 E3
0.857

B

\author{
Normal effort at point A \\ 0.0 NR \\ -0.183 \\ ***** \\ Bending moment at point A \\ 0.477 N.m \\ -0.478 \\ 0.210
}

\subsection*{4.2 Parameters}
of execution
Version:
NEW 5.04.17

\section*{Machine:}

SGI-Origin2000 R12000

\section*{Obstruction memory:}

16 megabytes
Time CPU To use: 1.31 seconds
Handbook of Validation
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HI-75/01/010/A

Titrate:
SSLS501 - Roll infinitely long subjected to two lines of load
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\section*{5 \\ Summary of the results}

As a whole, the results obtained are very satisfactory:
the maximum change is \(1.1 \%\). on displacements,
the maximum change is \(0.21 \%\) at the bending time.
The value of the normal effort is far away from the reference solution. The result would be more precise if one
used a finer grid [bib1].
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLS502 - Orthotropic cylinder subjected to a line of load
Date:
19/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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SSLS502 - Orthotropic cylinder subjected to a line of load

\section*{Summary:}

This test represents quasi-static calculation, of a short orthotropic cylinder and an orthotropic long cylinder
subjected to a line of load. Their ends, the cylinders rest on rigid diaphragms. This case test makes it possible to validate modeling finite elements DST with meshs TRIA3 and QUAD4, a material
homogeneous orthotropic.

Displacements and the efforts obtained are compared with a reference solution experimental like to an analytical solution.

\author{
Handbook of Validation
}

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\section*{Titrate:}

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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}
\(Q\)
\(Y\)
\(Y\)
\(Z\)
\(H\)
Diaphragm
rigid
\(X\)
\(\boldsymbol{R}\)
Steel wire
\(L\)
Plastic
(ac
has rylic)
\(y\)
\(L=0.560 \mathrm{~m}\)
\(H=0.0061 \mathrm{~m}\)
roll short:
\(R=0.13595 \mathrm{~m}\)
\(Q=2357.143 \mathrm{~N} / \mathrm{m}\)
\(L=2.465 \mathrm{~m}\)
\(H=0.0061 \mathrm{~m}\)
roll long:
\(R=0.13595 \mathrm{~m}\)
\(Q=896.552 \mathrm{~N} / \mathrm{m}\)

\section*{1.2 \\ Properties of material}

The material constituting the cylinder is homogeneous orthotropic. The axes of orthotropism correspond with the curvilinear directions \(X\) and \(Y\).
[H
] = [hH; [H
\(=0\)
H
= h3 H /12
membrane inflection] []
membrane
]
; [inflection]
[]
\(H 11=3.0644 \times 109 \mathrm{~N} / \mathrm{m}^{2} ; H 12=1.1048 \times 109 \mathrm{~N} / \mathrm{m}^{2} ; H 13=0\)
\(H 22=18.597 \times 109 \mathrm{~N} / \mathrm{m}^{2}\); H23= 0
; H33 \(=1.250 \times 109 \mathrm{~N} / \mathrm{m}^{2}\)

\section*{1.3}

Boundary conditions and loadings
- CL: The ends of the cylinder rest on rigid diaphragms
- Modelings \(A\) and b: Forces per unit of length: \(Q=-2357.143 \mathrm{~N} / \mathrm{m}\)
- Modelings \(C\) and D: Force per unit of length: \(Q=-896.552\). \(\mathrm{N} / \mathrm{m}\)

\subsection*{1.4 Conditions}
initial
Without object

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\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution
We will use for this test two reference solutions, one experimental, resulting from work from Schwaighofer and Microys [bib2], the other drawn from work of Batoz in theory of the deep hulls [bib1].

\section*{2.2}

Results of reference
The results of reference are as follows:
Roll (A and B) short
Batoz [bib1]
Experiment [bib2]
Displacement \(W\) at the point \(F\)
0.35104 m 0.6

104 m
Displacement \(W\) at the point \(C\)
0.7103 m
0.6103 m

Displacement W at the point \(D\)
0.25104 m
0.1103 m

Constraint xx at the point \(F\)
0.35 MPa
0.325 MPa
Constraint yy at the point F
0.50 MPa
0.60 MPa
Roll long (C and D)
Batoz [bib1]
Experiment [bib2]
Displacement \(W\) at the point \(F\)
1.32103 m
1.35103 m
Displacement W at the point C
2.45103 m
2.46103 m
Displacement W at the point \(D\)
0.35103 m
0.51103 m
Constraint xx at the point F
1.68 MPa
1.9 MPa
Constraint yy at the point F
1.8 MPa
1.55 MPa
2.3
Uncertainties on the solution\(\sim 5 \%\) with regard to the solution of Batoz, undoubtedly much more - \(\mathbf{- 3 0 \%}\) - for the solutionexperimental.
2.4 References
bibliographical
[1]BATOZ J.L., DHATT G.: Modeling of the structures by finite elements, Flight 3, Hulls,HERMES.[2]SCHWAIGHOFER J., MICROYS H.F. : Orthotropic Cylindrical shells under line load, Newspaperof applied Mechanics, June 1979, Flight 46.statics and dynamics of thin hulls, Thesis of Doctor Engineer, University ofTechnology of Compiegne, 27/04/83.

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Z}

DST modeling (one models a half rolls)
Y
C
Z
- 8 elements in the circumferential direction
\(y\)
- 12 elements in the longitudinal direction

B
Z
F
R, \(W\)
\(X\)
- Boundary conditions: Side AB:
\(U=W=y=0\)
- Conditions of symmetry: Sides \(A D\) and BC: \(U=\)

E
\(y=z=0\)
\(X\)

\section*{Side cd.:}
\(v=x=z=0\)
D
- Force per unit of length side BC: \(q / 2=1178.5715 \mathrm{~N} / \mathrm{m}\)

With
L/2
3.2

Characteristics of the grid
A number of nodes: 224
A number of meshs and type: 192 QUAD4

\subsection*{3.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE
AFFE
"DST"
DEFI_MATERIAU
ELAS_COQUE
MEMB_L
MEMB_LT
MEMB_T
MEMB_G_LT
FLEX_L
FLEX_LT
FLEX_T
FLEX_G_LT
CISA_L
CISA_T
AFFE_CARA_ELEM
HULL
THICK
ANGL_REP
AFFE_CHAR_MECA

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Reference Reference
Aster \%
differences
numerical
expériementale
[bib1]
[bib2]
Displacement \(W\) at the point \(F\)
\(0.35 \times 104 m 0.6\)
104m 0.373
104m 6.703 [bib1]
37.757 [bib2]

Displacement W at the point C
0.7103 m
0.6103 m
\(0.721 \times 103\)

\author{
3.033 [bib1] \\ 20.205 [bib2] \\ Displacement W at the point D \\ 0.25104 m \\ 0.1103 m \\ 0.369 10-4 \\ 47.689 [bib1] \\ -63.078 [bib2]
}

Constraint SIXX at the point F
0.350 MPa
0.325 MPa
0.480 MPa
37.339 [bib1]
47.904 [bib2]

Constraint SIYY at the point F
0.500 MPa
0.600 MPa
0.490 MPa
-1.901 [bib1]
-18.259 [bib2]

\section*{4.2}

Value of normal displacement \(W\) along \(C D\)
EDF
Mechanical department and Digital Models
Electricity
\(W\) according to the angle along CD
from France
0.6

ROLL SHORT ORTHOTROPIC
DKT 8 X 12 QUAD4
0.4
0.2
0.0
mm)

T (
L acemen
```

é
p
D
-0.2
W normal Batoz reference
W normal
W normal experimental
-0.4
-0.6
-0.8
0
20
4 0
6 0
80
1 0 0
1 2 0
1 4 0
1 6 0
1 8 0
angle (deg)
agraf 19/04/2001 (c) EDF/DER 1992-1999

```

One can note that beyond the variations observed on the points tested C, F, D, normal displacement calculated along CD is close to the solution in theory "deep hulls" adopted by Batoz
[bib1]. One can charge the errors relating to the points \(F\) and \(D\) to the low value of the displacement (of the order of 105 m).

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\section*{4.3}

Value of the constraints along CD

\section*{EDF}

\section*{Mechanical department and Digital Models}

\author{
Electricity
}

Sigma \(X X\) along \(C D\)
from France
2
ROLL SHORT ORTHOTROPIC
DKT 8 X 12 QUAD4
1
0
-1
has
)
\(P\)
-2
M
(
your
\(T\)
\(\boldsymbol{R}\)
has
in
\(N\)
sigma XX
Co
-3
Experimental ref. Schwaighofer/Microys
Ref. Batoz
-4
-5
-6
-7
0
20
40

\section*{EDF}

\section*{Mechanical department and Digital Models}

Electricity
sigma YY along CD
from France
3
ROLL SHORT ORTHOTROPIC
DKT 8 X 12 QUAD4
2
1
0
has
)
\(P\)
-1
M
(
your
\(T\)
R
has
in
\(N\)
sigma YY
Co
-2
Experimental ref. Schwaighofer/Microys
Ref. Batoz
-3
-4
-5
-6
0

20
40
60
80
100
120
140
160
180
angle (deg)
agraf 05/06/2001 (c) EDF/DER 1992-1999

\section*{Handbook of Validation}

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\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}
5.0

\section*{Titrate:}

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One can note that the constraints calculated along CD are overall in agreement with the solution in theory "deep hulls" adopted by Batoz [bib1].

\subsection*{4.4 Remarks}
- The values of coefficients CISA_L and CISA_T are not available. Like the structure is thin \((h / R=0.045)\), one supposes that the effects of transverse shearing are negligible, we thus imposed CISA_L=CISA_T=1010.
- Displacement W normal (figure of [§4.2]) is expressed in the local cylindrical reference mark (R, Z), it
acts of normal displacement to the element of hull. Displacement W tested with [§4.1] is as for
he expressed in the total reference mark (displacement following Z).

\section*{Handbook of Validation}

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\section*{5 Modeling}

B
5.1

Characteristics of modeling

\section*{Z}

DST modeling (one models a half rolls)
Y
C
Z
- 8 elements in the circumferential direction
\(y\)
- 12 elements in the longitudinal direction

B
Z
F
R, W
X
- Boundary conditions: Side AB:
\(U=W=y=0\)
- Conditions of symmetry: Sides AD and BC: \(U=\)

E
\[
y=z=0
\]
\(X\)

Side cd.:
\(v=x=z=0\)
D
- Force per unit of length side BC: \(q / 2=1178.5715 \mathrm{~N} / \mathrm{m}\)

With
L/2

\author{
5.2 \\ Characteristics of the grid \\ A number of nodes: 224 \\ A number of meshs and type: 384 TRIA3
}

\subsection*{5.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE
AFFE
"DST"
DEFI_MATERIAU
ELAS_COQUE
MEMB_L
MEMB_LT
MEMB_T
MEMB_G_LT
FLEX_L
FLEX_LT
FLEX_T
FLEX_G_LT
CISA_L
CISA_T
AFFE_CARA_ELEM
HULL
THICK

\title{
ANGL_REP \\ AFFE_CHAR_MECA \\ FORCE_ARETE \\ FZ \\ \\ Handbook of Validation \\ \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
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6
Results of modeling B
6.1 Values
tested
Identification Reference
Reference
Aster \%
differences
[bib1]
[bib2]
Displacement \(W\) at the point \(F\)
0.35 10-4m 0.6
\(104 m 0.383\)
10-4
9.571 [bib1]
36.084 [bib2]

Displacement \(W\) at the point \(C\)
0.7 10-3m 0.6

\section*{103 m}
-7.138 10-4
1.985 [bib1]
18.982 [bib2]

Displacement W at the point D
0.25 10-4m 0.1

103 m
0.350 10-4
40.368 [bib1]
64.908 [bib2]

Constraint SIXX at the point F
0.350 MPa
0.325 MPa
0.470 MPa
34.348 [bib1]
44.682 [bib2]

Constraint SIYY at the point F
0.500 MPa
0.600 MPa
0.400 MPa
19.929 [bib1]
33.274 [bib2]

\subsection*{6.2 Remarks}
- The values of coefficients CISA_L and CISA_T are not available. Like the structure is thin \((h / R=0.045)\), one supposes that the effects of transverse shearing are negligible, we thus imposed CISA_L=CISA_T=1010.
- Displacement W normal is expressed in the local cylindrical reference mark ( \(R, Z\) ), it acts of normal displacement with the element of hull.

\section*{6.3}

Value of normal displacement along CD
The results obtained with a grid TRIA3 are very close to those obtained by the grid QUAD4.
Handbook of Validation

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Code_Aster \({ }^{\circledR}\)
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\section*{6.4 \\ Value of the constraints along CD}

\section*{EDF}

Mechanical department and Digital Models
Electricity
Sigma XX along CD
from France
2
ROLL SHORT ORTHOTROPIC
DKT 8 X 12 TRIA3
1
0
-1
has
)
P
-2
M
(
your
\(T\)
R
has
in
\(N\)
sigma XX
Co

Experimental ref. Schwaighofer/Microys
Ref. Batoz
-4
-5
-6
-7
0
20
40
60
80
100
120
140
160
180
angle (deg)
agraf 05/06/2001 (c) EDF/DER 1992-1999

\section*{Handbook of Validation}

V3.03 booklet: Linear statics of the hulls and the plates
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
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Date:
19/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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EDF
Mechanical department and Digital Models
Electricity
sigma YY along \(C D\)
from France
3

\section*{ROLL SHORT ORTHOTROPIC}

DKT 8 X 12 TRIA3
2
1
0
has
)
\(P\)
-1
M
(
your
\(T\)
R
has
in
\(N\)
sigma YY
Co
-2
Experimental ref. Schwaighofer/Microys
Ref. Batoz
-3
-4
-5
-6
0
20
40
60
80
100
120
140
160
180
angle (deg)
agraf 05/06/2001 (c) EDF/DER 1992-1999

The profiles of the constraints obtained by modeling B with TRIA3 are as a whole near to the solutions of Batoz.

\author{
Handbook of Validation
}

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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
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\section*{7 Modeling}

C

\section*{7.1 \\ Characteristics of modeling}

Z
DST modeling (one models a half rolls)

\section*{Y}

C
Z
- 8 elements in the circumferential direction
y
- 12 elements in the longitudinal direction

B
Z
\(F\)
\(R, W\)
X
- Boundary conditions: Side AB:
\(U=W=y=0\)
- Conditions of symmetry: Sides AD and BC: \(U=\)

E
\(y=z=0\)

\section*{X}

Side cd.:
\[
v=x=z=0
\]
\(D\)
- Force per unit of length side BC: \(q / 2=448.276 \mathrm{~N} / \mathrm{m}\)

With
L/2

\section*{7.2}

Characteristics of the grid
A number of nodes: 224
A number of meshs and type: 384 TRIA3

\subsection*{7.3 Functionalities}
tested

\author{
Orders Key word \\ factor \\ Key word
}

\author{
AFFE_MODELE \\ AFFE \\ "DST" \\ DEFI_MATERIAU \\ ELAS_COQUE \\ MEMB_L \\ MEMB_LT \\ MEMB_T \\ MEMB_G_LT \\ FLEX_L \\ FLEX_LT \\ FLEX_T \\ \(F L E X \_G \_L T\) \\ CISA_L \\ CISA_T \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ ANGL_REP
}

\section*{AFFE_CHAR_MECA}

FORCE_ARETE
FZ
8
Results of modeling \(C\)

\subsection*{8.1 Values}
tested
Identification Reference
Reference
Aster \%
difference
numerical
experimental
[bib1]
[bib2]
Displacement W at the point F
1.325103 m
1.35103 m
1.327103 m
0.154 [bibl]
1.701 [bib2]

Displacement \(W\) at the point \(C\)
2.45103 m
2.46103 m
2.379103 m
2.881 [bibl]
3.275 [bib2]

Displacement W at the point \(D\)
0.51103 m
0.35103 m
0.529103 m
3.859 [bib1]
51.337 [bib2]

Constraint SIXX at the point \(F\) 1.68 MPa
```

1.9 MPa
1.643 MPa
2.155 [bibl]
13.484 [bib2]
Constraint SIYY at the point F
1.8 MPa
1.55 MPa
1.782 MPa
0.986 [bibl]
14.984 [bib2]
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HT-66/02/001/A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

SSLS502 - Orthotropic cylinder subjected to a line of load
Date:
19/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\subsection*{8.2 Remarks}
- The value of coefficients CISA_L and CISA_T are not available. As the structure is thin \((h / R=0.045)\), that the effects of transverse shearing are negligible, us are supposed thus imposed CISA_L=CISA_T=1010.
- Displacement W normal is expressed in the local cylindrical reference mark \((R, Z)\), it acts of normal displacement with the element of hull.

\section*{8.3 \\ Value of normal displacement along CD \\ EDF \\ Mechanical department and Digital Models \\ Electricity}
\(W\) according to the angle along \(C D\)
from France
1.5

ROLL LONG ORTHOTROPIC
DKT 8 X 12 TRIA3
1.0
0.5
0.0
mm)

T(
-0.5
L
acemen
W normal
é
\(p\)
D
W normal Batoz reference
\(W\) normal experimental
-1.0
-1.5
-2.0
-2.5
0
20
40
60
80
100
120
140
160
180
angle (deg)
agraf 19/04/2001 (c) EDF/DER 1992-1999

One can note that beyond the variation observed on the experimental value at the point D, displacement normal calculated along \(C D\) is very close to the solution in adopted theory "deep hulls" by Batoz [bibl].

Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
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Date:
19/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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\section*{8.4 \\ Value of the constraints along CD}

EDF
Mechanical department and Digital Models
Electricity
Sigma XX along CD
from France
3
ROLL LONG ORTHOTROPIC
DKT 12 X 8 TRIA3
2
1
0
has
)
\(P\)
M
(
your
-1
\(T\)
R
has
in
\(N\)
Experimental ref. Schwaighofer/Microys

\section*{Co}

Ref. Batoz
sigma XX

\author{
EDF \\ Mechanical department and Digital Models Electricity \\ sigma YY along CD \\ from France \\ 3 \\ ROLL LONG ORTHOTROPIC \\ DKT 12 X 8 TRIA3 \\ 2 \\ 1 \\ 0 \\ has \\ ) \\ \(P\) \\ M \\ ( \\ your \\ -1 \\ \(T\) \\ R \\ has \\ in
}
```

N
Experimental ref. Schwaighofer/Microys
Co
Batoz theory "deep hulls"
sigma YY
-2
-3
-4
-5
O
20
4 0
6 0
80
100
120
140
1 6 0
1 8 0
angle (deg)
agraf 05/06/2001 (c) EDF/DER 1992-1999

```
The profiles of the constraints calculated by the code are overall in agreement with work of Batoz.
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Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:
SSLS502 - Orthotropic cylinder subjected to a line of load
Date:
19/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key:V3.03.502-A Page:15/18

\section*{9 Modeling}

D

\section*{9.1 \\ Characteristics of modeling}

\section*{Z}

DST modeling (one models a half rolls)

\section*{Y}

C
Z
- 8 elements in the circumferential direction
\(y\)
- 12 elements in the longitudinal direction
```

B

```

Z
\(F\)
\(R, W\)
\(X\)
- Boundary conditions: Side AB:
\(U=W=y=0\)
- Conditions of symmetry: Sides AD and BC: \(U=\)

E
\(y=z=0\)
X

Side cd.:
\(v=x=z=0\)
\(D\)
- Force per unit of length side BC: \(q / 2=448.276 \mathrm{~N} / \mathrm{m}\)

With
L/2

\section*{9.2}

Characteristics of the grid
A number of nodes: 224
A number of meshs and type: 192 QUAD4

\subsection*{9.3 Functionalities}
tested

\author{
Orders Key word factor \\ Key word \\ AFFE_MODELE \\ AFFE \\ "DST" \\ DEFI_MATERIAU \\ ELAS_COQUE \\ MEMB_L \\ MEMB_LT \\ MEMB_T \\ \(M E M B_{-} G_{-} L T\) \\ FLEX_L \\ FLEX_LT \\ FLEX_T \\ FLEX_G_LT \\ CISA_L \\ CISA_T \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ ANGL_REP \\ AFFE_CHAR_MECA \\ FORCE_ARETE \\ FZ \\ 10 Results of modeling \(D\) \\ \subsection*{10.1 Values} \\ tested \\ Identification Reference \\ Reference \\ Aster \% \\ difference \\ [bib1] \\ [bib2] \\ Displacement \(W\) at the point \(F\) \\ 1.325103 m \\ 1.35103 m \\ 1.329103 m \\ 0.365 [bib1]
}
1.494 [bib2]

Displacement W at the point C
2.45103 m
2.46103 m
2.369103 m
3.274 [bib1]
3.667 [bib2]

Displacement W at the point D
0.51103 m
0.35103 m
0.528103 m
3.634 [bib1]
51.009 [bib2]

\author{
Constraint SIXX at the point F \\ 1.68 MPa \\ 1.9 MPa \\ 1.79 MPa \\ 6.616 [bib1] \\ 5.729 [bib2] \\ Constraint SIYY at the point F \\ 1.8 MPa \\ 1.55 MPa \\ 1.84 MPa \\ 2.465 [bib1] \\ 18.991 [bib2] \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the hulls and the plates \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLS502-Orthotropic cylinder subjected to a line of load
Date:
19/09/02

\author{
Author (S): \\ P. MASSIN, F. LEBOUVIER Key \\ : \\ V3.03.502-A Page: \\ 16/18
}

\subsection*{10.2 Remarks}
- The value of coefficients CISA_L and CISA_T are not available. As the structure is thin \((h / R=0.045)\), that the effects of transverse shearing are negligible, us are supposed thus imposed CISA_L=CISA_T=1010.
- Displacement W normal is expressed in the local cylindrical reference mark ( \(R, Z\) ), it acts of normal displacement with the element of hull. Displacement \(W\) tested is that of the total reference mark
(displacement following Z).

\subsection*{10.3 Value of displacement along CD}

\section*{EDF}

Mechanical department and Digital Models
Electricity
\(W\) according to the angle along CD
from France
1.5

ROLL LONG ORTHOTROPIC
DKT 8 X 12 QUAD4
1.0
0.5
0.0
mm)

T(
-0.5
L acemen
W normal Batoz reference
é
p
D
W normal
W normal experimental
-1.0

One can note that beyond the variation observed on the experimental value at the point \(D\), displacement
normal calculated along CD is very close to the solution in adopted theory "deep hulls" by Batoz [bib1].
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HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLS502 - Orthotropic cylinder subjected to a line of load
Date:
19/09/02
Author (S):
P. MASSIN, F. LEBOUVIER Key

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10.4 Values of the constraints along CD

EDF
Mechanical department and Digital Models
```

Electricity
Sigma XX along CD
from France
3
ROLL LONG ORTHOTROPIC
DKT 12 X 8 QUAD4
2
1
0
has
)
P
M
(
your
-1
T
R
has
in
N
sigma XX
Co

```
Experimental ref. Schwaighofer/Microys
Ref. Batoz
-2
-3
-4
-5
0
20
40
60
80
100
120
140
160
180
angle (deg)
agraf 05/06/2001 (c) EDF/DER 1992-1999

\section*{EDF}

\section*{Mechanical department and Digital Models}

\section*{Electricity}
sigma YY along CD
from France
3
ROLL LONG ORTHOTROPIC

\section*{DKT 12 X 8 QUAD4}

2
1
0
has
)
\(\boldsymbol{P}\)
M
(
your
-1
\(T\)
\(\boldsymbol{R}\)
has
in
\(N\)
sigma YY
Co
Experimental ref. Schwaighofer/Microys
Batoz theory "deep hulls"
-2
-3
-4
-5
0
20
40
60
80
100
120
140
160
180
angle (deg)
agraf 05/06/2001 (c) EDF/DER 1992-1999

Code_Aster \({ }^{\circledR}\)
Version
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\section*{11 Summary of the results}

The results are as a whole satisfactory. The specific variations which appear at the points tested, in particular the point D, seem due to the experimental uncertainty, undoubtedly reinforced by an uncertainty as for the graphic taking away.
A contrario, the solutions suggested by Batoz in theory "deep hulls" are well checked by four modelings, with relative errors of less than 5\% for the long cylinder.

It appears that:
- modelings TRIA3 and QUAD4 are appreciably equivalent for this problem,
\(\cdot\) the relative errors are much weaker for the long cylinder (modelings C and D)
that for the cylinder runs (modelings \(A\) and \(B\) ): at the point \(F\), the error is reduced of a factor 10 compared to the reference solution of Batoz,
- the refinement of the grid does not minimize in a decisive way the relative variations, so much with TRIA3 that with the QUAD4.

It is thus noted that the results are degraded when the report/ratio length on the diameter decrease, indeed the geometrical effects become important with this type of modeling. It would be desirable to be able to carry out a calculation in finite elements of hulls in orthotropic medium, in order to
to better take into account the curve, the plates constituting a borderline case.
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V3.03 booklet: Linear statics of the hulls and the plates
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key
:
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Organization (S): EDF/AMA, DeltaCAD

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.503

SSLS503 - Plate laminated in inflection stacking antisymmetric simply supported

\section*{Summary:}

This test represents a quasi-static calculation of a laminated plate, in antisymmetric inflection stacking, simply supported, subjected to a pressure uniformly distributed.

4 modelings make it possible to validate:
- modelings finite elements DKT (QUAD4, TRIA3) and DST (QUAD4, TRIA3) in the case of one composite material ( \(\mathbf{3}\) different layers of orientation), - stresses shear transverse.

Displacements and the constraints obtained are compared with an analytical reference solution. Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
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Version
5.0

Titrate:
SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key

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1
Problem of reference

\subsection*{1.1 Geometry}

Z, W
\(y, v\)
B
C
\(=1.2 \mathrm{~m}\) have
\(H=0.012 \mathrm{~m}\)
p
Stacking
With
\(T\)
D
\(X, U\)
Z
\(0^{\circ}\)
\(90^{\circ}\)
has
\(0^{\circ}\)
\(L\)

\section*{1.2 \\ Properties of material}

The properties of material constituting the plate are as follows:

One-way (U):
\(E=4.1010 \mathrm{~Pa}\)
\(E=0.161010 \mathrm{~Pa}\)
(LX; Ty)
\(L\)
\(T\)
\(G=G=8.108 \mathrm{~Pa}\)
\(G=3.2 .108 \mathrm{~Pa}\)
\(l t\)
\(l z\)
\(t z\)
\(=0.25\)

\section*{\(l t\)}

Stacking:
- orientation:
[ 0 /90/0]
- natural:
[U/U/U]
- thickness:
[h/3/h/3/h/3]

\section*{1.3 \\ Boundary conditions and loadings}
- CL: the plate is simply supported on its contour
- Pression uniformly distributed: \(p=3000\) Pa

\subsection*{1.4 Conditions}
initial
Without object
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:

SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key

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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

Displacement: analytical solution obtained by decomposition in series of the form:
I X
jy
\(W=W \sin\)
sin
ij

Constraints: numerical solution [bib1], [bib2]

\section*{2.2}

Results of reference

The results of reference are as follows:
\(W(0,0,0)\)
0.01507 m

Displacement \(W\) in the center of plate (not \(A\) ),
\(\operatorname{SIXX}(0,0, h / 2)\)
2.4216 107 Pa Forced \(x x\) on the higher skin of the layer
\(3(z=h / 2)\) in the center of plate (not A),
SIYY (0, 0, h/6) layer 5.7810 106 Pa Forced yy on the higher skin of the layer with \(90^{\circ}\)
\(2(z=h / 6)\) in the center of plate (not \(A)\),
\(\operatorname{SIXY}(A / 2, A / 2, h / 2)\)
1.2825 106 Pa Forced xy at the point \(C\) on the higher skin of layer 3,
\(\operatorname{SIXZ}(A / 2,0,0)\)
2.3526105 Pa Forced \(x z\) at the point \(D\) on the average skin of

2 sleep ( \(z=0\) ),
\(\operatorname{SIYZ}(0, A / 2,0)\)
8.8950104 Pa Forced \(y z\) at the point \(B\) on the average skin of

2 sleep ( \(z=0\) ),

\section*{2.3}

\section*{Uncertainties on the solution}
- The reference solution is given for a number of terms in the series equal to 25 .
- The factor of correction of transverse shearing used is 5/6.
- With an important twinge \((a / h=100)\), the transverse level of shearing is weak and thus difficult to obtain with precision. There is then an uncertainty on the values of constraint ij calculated during the validation of test VPCS, the differences obtained by software on the components of shearing is about \(10 \%\).

\subsection*{2.4 References \\ bibliographical}

VPCS: Software package of composite structural analysis; Examples of validation. Review of composites and of advanced materials, Volume 5 - number except series 1995. Hermes edition.
[2]
PUTCHA, N.S. and REDDY, J.N. : With mixed shear flexible finite element for the analysis of laminated punts, computer meth. in applied mech. Eng. 44 (1984).
Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Z}

C
Modeling DKT (TRIA3)
- The plate is located in the plan \(Y=0.5\)
\(y\)
- Not A (0.4; 0.5; 0.25)

X
- Boundary conditions:

B
D
. Dimensioned BC: \(v=0\)
. Dimensioned CD: \(v=0\)
Y
- Conditions of symmetry: (local reference mark)

\section*{\(48^{\circ} 5\)}
. Dimensioned AB: \(U==0\)
y
With
\(X\)
. Dimensioned AD: \(v==0\)
\(X\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 49
A number of meshs and types: 72 TRIA3

\subsection*{3.3 Functionalities}
tested

\section*{Orders Key word}
factor
Key word
AFFE_MODELE
AFFE
"DKT"
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
MATER
ORIENTATION
AFFE_CARA_ELEM
HULL
THICK
ANGL_REP
AFFE_CHAR_MECA
FORCE_COQUE
NEAR
CALC_CHAM_ELEM
NUME_COUCHE

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key
:
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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested

\section*{Identification Reference Aster \%}
difference
\(v(0,0,0)\)
0.01507
0.01500
-0.492
\(\operatorname{SIXX}(0,0, h / 2)\)
2.4216107
2.43071070 .376
\(\operatorname{SIYY}(0,0, h / 6)\) layer with
5.7810107
\(5.7438107-0.644\)
\(90^{\circ}\)
\(\operatorname{SIXY}(A / 2, A / 2, h / 2)\)

\subsection*{4.2 Remarks}

The constraints are expressed in the reference mark of orthotropism defined by ANGL_REP (AFFE_CARA_ELEM), and by the normal of the element.

Components SIXX, SIYY and SIYZ are the average values of the two convergent meshs with points \(A\) and \(C\).

The variation obtained on SIXZ is due to the difference in modeling of transverse shearing: in reference, one uses a coefficient of transverse correction of shearing of 5/6. In Code_Aster, one calculates the distribution of shearings in the thickness, presumedly parabolic in each sleep.
The sign of SIXZ is opposed to that of the reference solution.

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\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key

\section*{5 Modeling \\ B}

\section*{5.1}

\section*{Characteristics of modeling}

\section*{Z}

Modeling DKT (QUAD4)

\section*{C}
- The plate is located in the plan \(Y=0.5\)
- Not A (0.4; 0.5; 0.25)
y
\(X\)
- Boundary conditions:

B
D
. Dimensioned BC: \(v=0\)
. Dimensioned CD: \(v=0\)
Y
- Conditions of symmetry: (local reference mark) \(48^{\circ} 5\)
. Dimensioned AB: \(U==0\)
y
With
\(X\)
. Dimensioned AD: \(v==0\)
X

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 49
A number of meshs and types: 36 QUAD4

\subsection*{5.3 Functionalities \\ tested}

\author{
Orders Key word \\ factor \\ Key word
}
```

AFFE_MODELE
AFFE
"DKT"
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
MATER
ORIENTATION
AFFE_CARA_ELEM
HULL
THICK
ANGL_REP
AFFE_CHAR_MECA
FORCE_COQUE
NEAR
CALC_CHAM_ELEM
NUME_COUCHE
NIVE_COUCHE
"SUP" "MOY"
OPTION
"SIGM_ELNO_DEPL"
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```
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5.0

Titrate:
SSLS503 - Plate laminated in antisymmetric inflection stacking
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:
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\section*{6}

Results of modeling B

\subsection*{6.1 Values}
tested

\section*{Identification Reference Aster \% difference}
\(v(0,0,0)\)
0.01507
0.01500
-0.431
\(\operatorname{SIXX}(0,0, h / 2)\)
2.4216107
2.43971070 .745
\(\operatorname{SIYY}(0,0, h / 6)\) layer with
5.7810106
\(5.7321106-0.845\)
\(90^{\circ}\)
\(\operatorname{SIXY}(A / 2, A / 2, h / 2)\)
1.2825106
1.2184 106-4.995
\(\operatorname{SIXZ}(A / 2,0,0)\)
-2.3526 105
-2.0112 10514.5
\(\operatorname{SIYZ}(0, A / 2,0)\)
8.8950104
8.60601043 .2

\subsection*{6.2 Remarks}

Components SIXX, SIYY and SIYZ are the average values of the two convergent meshs with points \(A\) and \(C\).

The variation obtained on SIXZ is due to the difference in modeling of transverse shearing: in reference, one uses a coefficient of transverse correction of shearing of 5/6. In Code_Aster, one calculates the distribution of shearings in the thickness, presumedly parabolic in each sleep.

The sign of SIXZ is opposed to that of the reference solution.

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SSLS503 - Plate laminated in antisymmetric inflection stacking
Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key

\section*{7 Modeling}

C

\section*{7.1 \\ Characteristics of modeling}

\section*{Z}

Modeling DST (TRIA3)
C
- The plate is located in the plan \(Y=0.5\)
- Not A (0.4; 0.5; 0.25)
y
X
- Boundary conditions:

B
D
. Dimensioned BC: \(v=0\)
. Dimensioned CD: \(v=0\)
Y
- Conditions of symmetry: (local reference mark)
\(48^{\circ} 5\)
. Dimensioned AB: \(U==0\)
y
With
X
. Dimensioned AD: \(v==0\)
\(X\)

\section*{7.2 \\ Characteristics of the grid}

A number of nodes: 49
A number of meshs and types: 72 TRIA3

\subsection*{7.3 Functionalities}
tested

\section*{Orders Key word}
factor
Key word
AFFE_MODELE
AFFE
"DST"
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
MATER
ORIENTATION
AFFE_CARA_ELEM
HULL
THICK
ANGL_REP
AFFE_CHAR_MECA
FORCE_COQUE
NEAR
CALC_CHAM_ELEM
NUME_COUCHE
NIVE_COUCHE
"SUP" "MOY"
OPTION
"SIGM_ELNO_DEPL"
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Code_Aster \({ }^{\circledR}\)

\author{
Version
}
5.0

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\section*{8 \\ Results of modeling C}

\subsection*{8.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
\(v(0,0,0)\)
0.01507
0.01517
0.655
\(\operatorname{SIXX}(0,0, h / 2)\)
2.4216107
2.13210712
\(\operatorname{SIYY}(0,0, h / 6)\) layer with \(90^{\circ}\)
5.7810106
6.96106
20.
\(\operatorname{SIXY}(\mathrm{A} / 2, A / 2, h / 2)\)
1.2825106
1.284106
0.1
\(\operatorname{SIXZ}(A / 2,0,0)\)
-2.3526 105
-1.5474103
34
\(\operatorname{SIYZ}(0, A / 2,0)\)
8.8950104

\subsection*{8.2 Remarks}

Components SIXX, SIYY and SIYZ are the average values of the two convergent meshs with points \(A\) and \(C\).

The variation obtained on SIXZ is due to the difference in modeling of transverse shearing: in reference, one uses a coefficient of transverse correction of shearing of 5/6. In Code_Aster, one calculates the distribution of shearings in the thickness, presumedly parabolic in each sleep.
The sign of SIXZ is opposed to that of the reference solution.
The other variations are probably due to the anisotropy of the triangular grid.
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5.0

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Date:
17/06/02
Author (S):
J.M. PROIX, P. MASSIN, F. LEBOUVIER Key

\section*{9 Modeling}

D

\section*{9.1}

\section*{Characteristics of modeling}

\section*{Z}

Modeling DST (QUAD4)
C

\title{
- The plate is located in the plan \(Y=0.5\)
}
- Not A (0.4; 0.5; 0.25)
y
X
- Boundary conditions:

B
D
. Dimensioned BC: \(v=0\)
. Dimensioned CD: \(v=0\)
Y
- Conditions of symmetry: (local reference mark) \(48^{\circ} 5\)
. Dimensioned AB: \(U==0\)
y
With
X
Dimensioned AD: \(v==0\)
\(X\)

\section*{9.2 \\ Characteristics of the grid}

A number of nodes: 49
A number of meshs and types: 36 QUAD4

\subsection*{9.3 Functionalities \\ tested}

\author{
Orders Key word \\ factor \\ Key word \\ AFFE_MODELE \\ AFFE \\ "DST" \\ DEFI_MATERIAU \\ ELAS_ORTH \\ DEFI_COQU_MULT \\ SLEEP \\ THICK \\ MATER \\ ORIENTATION
}
```

AFFE_CARA_ELEM
HULL
THICK
ANGL_REP
AFFE_CHAR_MECA
FORCE_COQUE
NEAR
CALC_CHAM_ELEM
NUME_COUCHE
NIVE_COUCHE
"SUP" "MOY"
OPTION
"SIGM_ELNO_DEPL"
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```
Code_Aster \({ }^{\circledR}\)

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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}
tested

Identification Reference Aster \% difference
\(v(0,0,0)\)
0.01507
0.01513

\subsection*{10.2 Remarks}

Components SIXX, SIYY and SIYZ are the average values of the two convergent meshs with points \(A\) and \(C\).

The variation obtained on SIXZ is due to the difference in modeling of transverse shearing: in reference, one uses a coefficient of transverse correction of shearing of 5/6. In Code_Aster, one calculates the distribution of shearings in the thickness, presumedly parabolic in each sleep.
The sign of SIXZ is opposed to that of the reference solution.
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\section*{Titrate:}

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\section*{11 Summary of the results}
- Déplacements: some is modeling used (DKT or DST) the results are satisfactory, the maximum error is lower than 0.7\%.
- Plane Contraintes: the results are more precise with modeling DKT, the error is lower than \(1 \%\) except for SIXY (QUAD4) where the error is \(5 \%\). For DST modeling the error is higher ( \(<8 \%\) ) with an important variation on SIXX (28\%) for mesh TRIA3.
- Transverse Cisaillement: some is modeling used (DKT or DST) the results obtained with the quadrangular grids are closer to the reference solution than those obtained with triangular grids. In the first case the error on the component SIXZ is lower than \(15 \%\), and the error on SIYZ is lower than \(3 \%\), while in the second case, the error on SIXZ is 35\% and that on SIYZ lies between 2\% and 24\%. Except worse precision of the triangular grids because of their anisotropy, the variation which remains with quadrangular grids is due to the difference in modeling of shearing transverse: in the reference, one uses a coefficient of transverse correction of shearing of 5/6. In Code_Aster, one calculates the distribution of shearings in the thickness, supposed parabolic in each layer.
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Titrate:
SSLS504 - Composite square plate made up of 3 layers, simply Date:

23/09/02
Author (S):
J.M. PROIX, F. LEBOUVIER Key
:
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Organization (S): EDF/AMA, DeltaCAD

Handbook of Validation
V3.03 booklet: Linear statics of the plates and hulls
Document: V3.03.504

SSLS504-Composite square plate made up of 3 layers, subjected to a loading
doubly sinusoidal

\section*{Summary:}

This test represents the quasi-static calculation of a composite square plate made up of 3 layers, simply
supported, subjected to a doubly sinusoidal loading. This case-test makes it possible to validate modeling
finite elements DST with meshs TRIA3 and QUAD4, a composite material multi-layer.
Displacements and the constraints obtained are compared with a numerical reference solution.

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}

Code_Aster \({ }^{\circledR}\)
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5.0

\section*{Titrate:}

SSLS504 - Composite square plate made up of 3 layers, simply Date:
23/09/02
Author (S):
J.M. PROIX, F. LEBOUVIER Key

\section*{:}

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1
Problem of reference

\subsection*{1.1 Geometry}
\(y, v\)
\(L=1 . m\)
\(H=0.1 \mathrm{~m}\)
Stacking
\(p=p o s i n(x / L) \sin (y / L)\)
\(Z, W\)
\(T\)
D
C
L
Z
\(0^{\circ}\)
\(90^{\circ}\)
\(0^{\circ}\)
With
B
\(X, U\)
L
H/4
H/2

The 3 layers have as a relative thickness: H/4, H/2, H/4

\section*{1.2}

Properties of material
The axes of orthotropism correspond to the curvilinear directions \(X\) and \(Y\).
\[
E=25 .
\]
\[
E=1 .(L X ; T y)
\]

L
\(T\)
\(\boldsymbol{G}=\boldsymbol{G}=0.5\)
\(G=0.2\)
lt
\(l z\)
\(t z\)
\(=0.25\)
\(l t\)

\section*{1.3}

Boundary conditions and loadings
- CL: displacement perpendicular to the plate, to its contour is null.
- Chargement: \(p=P o \sin (x / L) \sin (y / L)\) with \(P o=0.01\)

\subsection*{1.4 Conditions \\ initial \\ Without object \\ Handbook of Validation \\ V3.03 booklet: Linear statics of the plates and hulls \\ HT-66/02/001/A}

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SSLS504 - Composite square plate made up of 3 layers, simply Date:

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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is a numerical solution [bib3].

\section*{2.2 \\ Results of reference}

The numerical results of reference are as follows:
DST*
DST* (QUAD4)
Size
(TRIA3)
- Déplacement W at the point C (L/2, L/2,0)
-0.07323 -0.07417
- Contrainte xx at the point C (L/2, L/2, h/2) (layer 3)
-0.478-0.482
- Contrainte yy at the point C (L/2, L/2, h/4) (layer 2)
-0.339-0.4
- Contrainte \(x z\) at the point \(D(0, L / 2,0)(\) layer 2)
-0.0203 -0.0305
- Contrainte yz at the point B (L/2,0,0) (layer 2)
-0.0406-0.0204
* the reference solutions were obtained with a grid 6x6 [bib3].

\section*{2.3}

Uncertainties on the solution
< \(2 \%\)

\subsection*{2.4 References \\ bibliographical}
[1]
BATOZ J.L., DHATT G.: Modeling of the structures by finite elements, Flight 2, Beams and Plates, HERMES.
[2]
PAGANO N.J., Hatfield J.J. : "Elastic behaviour of multilayered bidirectional composite",
AIAA J., Flight 10, \({ }^{\circ} 7\), p. 931-933, 1972.
[3]
LARDEUR P.: Development and evaluation of two new finite elements of plates and composite hulls with influence of transverse shearing, Thesis of Doctorate
Engineer, University of Technology of Compiegne, 1990.
Handbook of Validation
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Version
5.0

\section*{Titrate:}

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\section*{3 Modeling}

\section*{With}

\section*{3.1}

Characteristics of modeling
\(y\)
\(y, v\)
C
Modeling DST (QUAD4)
D
- Boundary conditions:

\section*{Z, W}
. Dimensioned AB: \(W==0\)
\(y\)
. Dimensioned AD: \(W==0\)
X
- Conditions of symmetry:

Z
. Dimensioned BC: \(U==0\)
\(y\)
. Dimensioned CD: \(v==0\)
\(X\)
With
B
\(\boldsymbol{X}, \boldsymbol{U}\)
\(X\)

\section*{3.2}

Characteristics of the grid
A number of nodes: 49
A number of meshs and type: 36 QUAD4

\subsection*{3.3 Functionalities}
tested
Orders Key word
factor
Key word
AFFE_MODELE
AFFE
"DST"
DEFI_MATERIAU
ELAS_ORTH
DEFI_COQU_MULT
SLEEP
THICK
MATER
ORIENTATION
AFFE_CARA_ELEM
HULL
THICK

\author{
ANGL_REP \\ AFFE_CHAR_MECA_F \\ FORCE_COQUE \\ NEAR \\ CALC_CHAM_ELEM \\ NUME_COUCHE \\ NIVE_COUCHE \\ "SUP" "MOY" \\ OPTION \\ "SIGM_ELNO_DEPL"
}

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster Difference
(\%)
Displacement \(W\) at the point \(C(L / 2, L / 2,0)\)
-0.07417-0.07444
0.37

Constraint \(x x\) at the point \(C(L / 2, L / 2, h / 2)\)
-0.482-0.474
-1.7
Constraint yy at the point C (L/2, L/2, h/4)
-0.400-0.412
3
Constraint xz at the point \(D(0, L / 2,0)\)
-0.0305-0.03
-1.7
Constraint \(y z\) at the point \(B(L / 2,0,0)\)
-0.0204-0.021
2.8

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SSLS504 - Composite square plate made up of 3 layers, simply Date:
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Author (S):
J.M. PROIX, F. LEBOUVIER Key

\section*{:}

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\section*{5 Modeling}

B

\section*{5.1}

Characteristics of modeling

\section*{\(y\)}
\(y, v\)
D
C
Modeling DST (TRIA3)

\section*{- Boundary conditions:}

Z, W
. Side \(A B: W=y=0\)
. Side \(A D: W=x=0\)
Z
- Conditions of symmetry:
. Side BC: \(U=y=0\)
. Side CD: \(v=x=0\)
\(\boldsymbol{X}, \boldsymbol{U}\)
With
B
\(X\)

\section*{5.2}

Characteristics of the grid
A number of nodes: 49
A number of meshs and type: 72 TRIA3

\subsection*{5.3 Functionalities}
tested

\author{
Orders Key word factor \\ Key word \\ AFFE_MODELE \\ AFFE \\ "DST" \\ DEFI_MATERIAU \\ ELAS_ORTH \\ DEFI_COQU_MULT \\ SLEEP \\ THICK \\ MATER \\ ORIENTATION \\ AFFE_CARA_ELEM \\ HULL \\ THICK \\ ANGL_REP \\ AFFE_CHAR_MECA_F \\ FORCE_COQUE \\ NEAR \\ CALC_CHAM_ELEM \\ NUME_COUCHE \\ NIVE_COUCHE \\ "SUP" "MOY" \\ OPTION \\ "SIGM_ELNO_DEPL"
}

6
Results of modeling B
6.1 Values
tested

\section*{Identification Reference}

Aster Difference
(\%)
Displacement W at the point C (L/2, L/2,0)
-0.07323-0.07112
-2.9
Constraint xx at the point \(C(L / 2, L / 2, h / 2)\)
-0.478-0.4621-3.3
Constraint yy at the point C (L/2, L/2, h/4)

Constraint \(x z\) at the point \(D(0, L / 2,0)\)
-0.0203 -0.0217
7.3

Constraint \(y z\) at the point \(B(L / 2,0,0)\)
-0.0406-0.0435
7.3

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5.0

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\section*{7}

Summary of the results
- Déplacements: the result obtained with meshs QUAD4 is satisfactory (variation of 0.4\%). One observe a more important variation (3\%) for meshs TRIA3.
- Contraintes: the result obtained with meshs QUAD4 is satisfactory (maximum change of 3\%). One observes a more important variation (7\%) for meshs TRIA3.

This test thus makes it possible to validate the calculation of the composite plates under loading function of
geometry, as well in term of displacements of constraints.
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Code_Aster \({ }^{\circledR}\)
Version

Titrate:
SSLV04-Hollow roll in plane constraints

Date:
17/06/03
Author (S):
X. DESROCHES, Key P. HERMAN
:
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Organization (S): EDF-R \& D /AMA, CS IF

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal structures
V3.04.004 document

SSLV04-Hollow roll in plane constraints

\section*{Summary:}

This test is drawn from Guide VPCS (test SSLV04/89) and has as an aim a hollow roll charged in pressure
intern.

This three-dimensional problem is dealt with with various modelings:
- in 3D: 9 modelings (pentahedral, hexahedrons, tetrahedrons and pyramids, degrees 1 and 2),
- in 2D forced plane: 4 modelings (triangles and quadrangles degrees 1 and 2, quadrangles with 9 nodes),
- in axisymmetric 2D: 3 modelings (triangles and quadrangles degrees 1 and 2, quadrangles with 9 nodes).

The functionalities tested are:
- pressure distributed,
- basic effect (with fixed or variable pressure),
- imposed displacements,
- matrices of rigidity,
- deformations and constraints with the nodes,
- nodal reactions (modeling K).

There are 16 modelings.

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal structures
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV04-Hollow roll in plane constraints

\section*{Date:}

17/06/03
Author (S):
X. DESROCHES, Key P. HERMAN
:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}
```

y
R
E
F
C
45
D
P
With
B
Z
X
Internal ray
= 0.1 m has
External ray
B=0.2 m

```

\section*{Co-ordinates of the points:}

\section*{WITH B C}

\section*{D}

E F
\(X 0.1000 .200\)
\(0.1 \cos (22.5)\)
\(0.2 \cos (22.5)\)
\(2 / 2\)
2
\(y 0\).
0.
\(0.1 \sin (22.5)\)
\(0.2 \sin (22.5)\)
\(2 / 2\)
2
Z 0
0.
0.
0.
0.
0.

\section*{1.2}

Material properties
\[
\begin{aligned}
& E=2.105 \mathrm{MPa} \\
& =0.3
\end{aligned}
\]

1.3

Boundary conditions and loadings

Internal pressure:
\(P=60 \mathrm{MPa}\)
Pressure interns variable (modeling P only):
\(P\) varies linearly from 60 MPa with \(t=1 . s\) with 120 MPa with \(t=2 . s\)
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V3.04 booklet: Linear statics of the voluminal structures
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV04-Hollow roll in plane constraints

\section*{Date:}

17/06/03
Author (S):
X. DESROCHES, Key P. HERMAN
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution

In constraint planes (cylinder on free board at the ends)
\(z z=0\)
2

2
B
has
rr
\(=P\)
1-
b2-a2
R 2
2
2
B
has
\(=P\)
1+
b2-a2
R 2
\(R=0\)
\(P\)
a2
2
B
\(u r=\)
(1) \(+(1+) R\)

E b2-a2
\(\boldsymbol{R} 2\)
One obtains:
for
\(=0.1\)
\(=59.106\)
for
\(=0.2\)
\(=40.106\)
\(R\)
\(U\)
\(R\)
\(U\)
-
\(R\)
\(R\)
\(=60\).
-
\(=0\).
\(r r\)
\(r r\)
\(=100\).
\(=40\).
\(==0\).
\(==0\).
\(z z\)
\(R\)
\(z z\)
\(R\)

\section*{Passage in the system of Cartesian axes:}
```

2
2
xx
=rrcos+sin-2R sin cos
2
2
yy
=rr sin + cos + 2R sin cos
2
2
xy
=rr sin cos-sin cos-2R(cos-\operatorname{sin})

```

\section*{with:}
\(\cdot=0^{\circ}\) at points \(A\) and \(B\),
\(\cdot=22.5^{\circ}\) at the points \(C\) and \(D\),
\(\cdot=45^{\circ}\) at the points \(E\) and \(F\).

\section*{2.2}

Results of reference
Displacements ( \(U, v\) ) and forced ( \(x x, y y\)
, zz
, xy
) at the points \(A, B, C, D, E, F\).
2.3 References
bibliographical

\section*{[1]}

Guide VPCS. SSLV04/89
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J. COURBON. Resistance of the materials p 649

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Version
6.4

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Author (S):
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Elements 3D (PENTA6 and HEXA8)}

Grid obtained by extrusion starting from a grid 2D resembling the grid below (30
elements in the radial direction with déraffinement progressive and \(15+15\) elements in the direction circumferential).
F
Normally blocked face
\(y\)
E
\(45^{\circ}\)
With
B
X
Face with imposed pressure
Face blocked out of Dy

Along axis Z:
1 layer of elements
Total thickness:
0.01

Limiting conditions:
node \(F: u z=0\)
face AB blocked out of Dy
normally blocked face EF
pressure on face \(A E p=60\).

Names of the nodes:
With \(=\) N993
\(B=N 1443\)
\(C=N 1\)
D=N31
\(E=N 496\)
\(F=N 495\)

\section*{3.2}

Characteristics of the grid
A number of nodes: 1922
A number of meshs and types: 900 PENTA6, 450 HEXA8 and 90 QUAD4 (faces internal skin).

\subsection*{3.3 Functionalities}
tested

\section*{Orders}

\author{
AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO
}

FACE_IMPO
GROUP_MA
DNOR
PRES_REP

\section*{GROUP_MA}
"MECHANICAL" AFFE_MODELE "3D"
ALL

\section*{DEFI_MATERIAU ELAS}

CALC_NO "EPSI_NOEU_DEPL"
"SIGM_NOEU_DEPL"
POST_RELEVE "EXTRACTION"

\section*{MODI_REPERE DEFI_REPERE "USER"}

\author{
"CYLINDRICAL" \\ Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal structures \\ HT-66/03/008/A
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}

\section*{Localization Size}

\section*{Reference}

Aster \%
difference
With \(U\)
5.9

10-5 5.8950
10-5 0.08
\(v\)
0 .
eps
xx 60.
59.2225
1.30
yy
100. 100.4159 0.42
\(z z\)
0. 0.3093
0. -
1.0442 -
\(x x\)
-4.5 10-4
- 4.472 10-4 0.62
yy
5.9 10-4 5.904

10-4 0.08
xy
0 .
- 6.788 10-5 -

B U

4
10-5 3.9959
10-5 0.10
\(v\)
0 .
eps
\(x x 0\).
-1.7246
yy
40. 39.2451
1.89
\(z z\)
0. -
0.3761 -
xy
0. -
0.2659 -
\(x x\)
-0.6 10-4
- 6.692 10-5 11.54
yy
2. 10-4 1.994

10-4 0.31
xy
0 .
- 1.728 10-6 -

E U
4.17193

10-5 4.1708
10-5 0.03
\(v\)
4.17193 10-5 4.1708

10-5
0.03
\(x x 20\).
19.0824
4.59
yy
20. 21.1394
5.70
z.
0. 0.0870
xy
-80. -
79.88310 .15
\(x x\)
0.7 10-4 0.636

10-4 9.18
yy
0.7 10-4 0.769

10-4 9.92
\(x y\)
-5.2 10-4
- \(5.19210-40.15\)

F U
2.82843

10-5 2.8302
10-5 0.06
\(v\)
2.82843 10-5 2.8302

10-5 0.06
\(x x 20\).
18.9528
5.24
yy
20. 19.9104
0.45

\section*{zz}
0. 0.1198
\(x y\)
-20. -20.1809 0.90
\(x x\)
\(0.710-40.647\)
10-4 7.54
yy
0.7 10-4 0.709

10-4 1.35
\(x y\)
-1.3 10-4 -1.312
10-4 0.90

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\section*{5 Modeling}

B

\section*{5.1 \\ Characteristics of modeling}

\section*{Elements 3D (PENTA15 and HEXA20)}

Grid obtained by extrusion starting from the grid 2D below (modeling \(F\) )
\(y\)
B
Face blocked in \(d x\)
F
With
Normally blocked face

\section*{E}

Face with imposed pressure
\(45^{\circ}\)
X

\section*{Along axis Z: \\ 2 layers of elements \\ Total thickness: \\ 0.01 \\ Limiting conditions:}
node \(F=u z=0\)
face \(A B\) blocked in \(d x\)
normally blocked face EF
pressure on face \(A E\)
\(p=60\).

Names of the nodes:
WITH \(=\) NO2
B = NO361
\(C=\mathrm{NO} 121\)
\(D=N O 584\)
\(E=N O 155\)
\(F=N O 503\)

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 2115
A number of meshs and types: 400 PENTA15, 100 HEXA20 40 QUAD8 (faces skin interns)

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
AFFE_CHAR_MECA DDL_IMPO
}

GROUP_NO

FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "3D"
ALL
DEFI_MATERIAU ELAS

CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
"EPSI_ELNO_DEPL"
POST_RELEVE "EXTRACTION"

\author{
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:
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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested

\section*{Localization Size}

\section*{Reference}

Aster \%
difference
With \(U\)
O. eps
\(v\)
5.9 10-5 5.8944

10-5
-0.09
xx 100.
99.6056
-0.39
yy
- 60.
- 59.4473
-0.92
```

zz
0.0.0196
xy
0. 0.2481
xx
5.9 10-4 5.87
10-4 -
0 . 4 8
yy
-4.5 10-4
-4.47 10-4 -
0.74
xy
0. 1.61
10-6 -
B U
O. eps
v
4 10-5 3.9974
10-5 -
0 . 0 7
xx 40.
39.9711
0 . 0 7
yy
0. 0.0781
zz
0. 5.7992
10-3-

```
E U
4.17193
10-5 4.1680
10-5 -
0.09
\(v\)
4.17193 10-5 4.1680
10-5
- 0.09
\(x x 20\).
20.0515
0.26
yy
20. 20.0264
0.13
\(z z\)
0. - 0.0155
\(x y\)
80. 79.7918
0.7 10-4 0.702

10-4 0.34
```

yy
0.7 10-4 0.701
10-4 0.11
xy
5.2 10-4 5.19
10-4 -
0.26
F U
-2.82843
10-5
-2.82656 10-5 -
0,07
v
2.82843 10-5 2.82656

```
10-5 -
0.07
xx 20.
20.0099
0.05
yy
20. 19.9980
0.01
zz
0.
- 3.90 10-4 -
xy
20. 20.0122
0.06
\(x x\)
0.7 10-4 0.7005

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\section*{7 Modeling \\ C}

\section*{7.1}

Characteristics of modeling

\section*{Elements 3D (TETRA4)}
\(A B\) is on axis \(O X\)

Cutting:
21 equidistant nodes on segments \(A B, C D\) and \(E F\)

\section*{21 equidistant nodes on arcs \(A C E\) and \(B D F\)}

Along axis Z:
1 layer of elements
Total thickness:
0.01

Limiting conditions:
node \(F: u z=0\)
face AB blocked out of Dy
normally blocked face EF
pressure on face \(A E\)
\(p=60\).

Names of the nodes:
With \(=\) N165
\(B=N 4\)
\(C=N 209\)
\(D=N 82\)
\(E=N 244\)
\(F=N 1068\)

\section*{7.2}

\section*{Characteristics of the grid}

A number of nodes: 1115
A number of meshs and types: 3724 TETRA4 and 1760 TRIA3 (faces skin interns)

\subsection*{7.3 Functionalities}
tested

\section*{Orders}

\section*{AFFE_CHAR_MECA DDL_IMPO GROUP_NO}

FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "3D"
ALL
DEFI_MATERIAU ELAS

CALC_NO "EPSI_NOEU_DEPL"
"SIGM_NOEU_DEPL"

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\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested

\section*{Localization Size Reference}

Aster \% difference
With \(U\)
5.9

10-5 5.8901
10-5
0.17
\(v\)
0. eps
-
\(x x\)
-60.
- 57.2290
- 4.62
yy
100. 97.8711
2.13
\(z z\)
0. 0.0568
xy
0. -
2.6589 -
\(x x\)
-4.5 10-4
4
10-5 3.9878
10-5 -
0.30
\(v\)
O. eps
\(x x 0\).
1.5296
\(y y\)
40. 40.9839
2.46
0. -
0.1006 -
xy
0. -
0.8513 -
\(x x\)
- 0.6 10-4
- 6.897 10-4 14.95
yy

10-4 3.68
\(x y\)
0.
- 5.534 10-5 -

E \(U\)
4.17193

10-5 4.1655
10-5 -
0.15
\(v\)
4.17193 10-5 4.1655

10-5
- 0.15
xx 20.
17.9096
10.45
yy
20. 21.8929
9.46
\(z z\)
0. -
0.3679 -
xy
- 80.
- 77.6897
- 2.89
\(x x\)
0.7 10-4 0.573

10-4 -
18.20
yy
0.7 10-4 0.832

10-4 18.79
```

xy

- 5.2 10-4
-5.050
- 2.89
F U
2.82843
10-5 2.8251
10-5 -
0.12
v
2.82843 10-5 2.8251
10-5 -
0.12
xx 20.
18.4444
7.78
yy
20.19.8876
0.56
zz

0.     - 

0.3910
xy
-20.

- 20.1631
0.81
xx
0.7 10-4 0.630
10-4 -
10.05
yy
0.7 10-4 0.723
10-4 3.35
xy

```

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\section*{9 Modeling \\ D}
9.1

Characteristics of modeling

\section*{Element 3D (TETRA10)}
\(A B\) is on axis \(O X\)

Cutting:
11 equidistant nodes on segments \(A B, C D\) and \(E F\)
11 equidistant nodes on arcs \(A C E\) and BDF

Along axis Z:
1 layer of elements
Total thickness:
0.01

Limiting conditions:
node \(F: u z=0\)
face \(A B\) blocked out of Dy
normally blocked face EF
pressure on face \(A E\)
\(p=60\).

Names of the nodes:
With \(=\) N184
\(B=N 4\)
\(C=N 207\)
\(D=N 50\)
\(E=N 22\)
\(F=N 726\)

\section*{9.2}

\section*{Characteristics of the grid}

A number of nodes: 1395
A number of meshs and types: 652 TETRA10 and 480 TRIA6 (faces skin interns)

\author{
9.3 Functionalities \\ tested \\ Orders
}

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\section*{10 Results of modeling \(D\)}

\subsection*{10.1 Values}

\section*{Localization Size Reference}

Aster \%
difference
With \(\boldsymbol{U}\)
5.9

10-5 5.8974
10-5 -
0.04
\(v\)
0. eps
-
\(x x\)
- 60.
- 60.3816
0.64
\(y y\)
100. 99.1907
0.81
\(z z\)
0. -
0.9707

\section*{-}
\(x y\)
0. -
0.2979
\(x x\)
- 4.5 10-4
- 4.49 10-4 -
0.17
```

yy
5.9 10-4 5.88
10-4 -
0 . 3 4

```
```

xy
0.

- 1.94 10-6 -
B U
4
10-5 3.9989
10-5 -
0.03
v
0.eps
xx 0.
0.0388
yy
40.40.0725
0.18
zz

0.     - 

0.0046
xy
0.0.1634
xx

- 0.6 10-4
- 0.599 10-4 -
0.15
yy

2. 10-4 2.003
10-4 0.16
xy
0.1.062
10-6 -
E U
```
```

4 . 1 7 1 9 3
10-5 4.17021
10-5 0.04
v
4.17193 10-5 4.17021
10-5 0.04
xx 20.
19.1178
4 . 4 1
yy
20.19.6399
1.80
zz
0. -
1.0206

```
\(x y\)
- 80.
- 79.7804
- 0.27
\(x x\)
0.7 10-4 0.677
10-4 -
3.34
\(y y\)
0.7 10-4 0.711
10-4 1.50
\(x y\)
- 5.2 10-4
- 5.186 10-4 -
0.27
F \(\boldsymbol{U}\)
2.82843
10-5 2.82718
```

10-5 -

```
0.04
\(v\)
2.82843 10-5 2.82718
10-5 -
0.04
xx 20.
20.1903
0.95
\(y y\)
20. 19.9023
0.49
\(z z\)
0. -
0.0016
\(x y\)
- 20.
- 20.0570
0.28
\(x x\)
0.7 10-4 0.711

10-4 1.57
```

yy
0.7 10-4 0.692
10-4 -
1.10
xy

- 1.3 10-4
-1.304 10-4 0.28

```

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```

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\section*{11 Modeling}

E

\subsection*{11.1 Characteristics of modeling}

C_plan elements (TRIA3 + QUAD4)
Grid 2D resembling the grid below ( 30 elements in the radial direction with déraffinement progressive and 15+15 elements in the circumferential direction).
F
Normally blocked face
\(y\)
E
D
C
\(45^{\circ}\)
With
B
\(X\)
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB blocked out of Dy
normally blocked side EF
pressure on \(A E p=60\).
Names of the nodes:
With = N1
\(B=N 451\)
\(C=N 496\)
\(D=N 495\)
\(E=N 990\)
F \(=\boldsymbol{N} 989\)
11.2 Characteristics of the grid

A number of nodes: 961
A number of meshs and types: 900 TRIA3, 450 QUAD4

\subsection*{11.3 Functionalities}
tested
Orders

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "C_PLAN" ALL
DEFI_MATERIAU ELAS

CALC_NO "EPSI_NOEU_DEPL"

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12 Results of modeling \(E\)

\subsection*{12.1 Values}
tested

\section*{Localization Size Reference}

Aster \%
difference
With \(\boldsymbol{U}\)
5.9

10-5 5.8957
10-5 -
0.07
\(v\)
0.eps
\(x x\)
- 60.
- 59.3645
- 1.06
```

yy
100.100.2653 0.26

```
\(z z\)
0. 0. -
\(x y\)
0. -
1.0472 -
\(x x\)
- 4.5 10-4
- 4.472 10-4 -
0.62
\(y y\)
5.9 10-4 5.904
10-4 0.06
\(x y\)
0.
- 6.807 10-6 -
B U
4.
10-5 3.9965
10-5 -
0.09
\(v\)
0. eps
\(x x 0\).
-
1.4986
\(y y\)
40. 39.4415
-
1.40
\(Z 7\)
0. 0. -
```

xy
0. -
0.2658 -
xx
-0.6 10-4

- 0.667 10-5 11.09

```
```

yy
2. 10-4 1.995
10-4 -
0 . 2 7
xy
0.
-1.728 10-6 -
E U
4 . 1 7 1 9 3
10-5 4.17101
10-5 -
0 . 0 2
v
4.17193 10-5 4.17101

```
10-5
- 0.02
xx 20.
19.0706
-
4.65
\(y y\)
20. 21.1354
5.68
\(z z\)
0. 0. -
\(x y\)
- 80.
0.7 10-4 0.636
10.4 -
9.07
```

yy
0.7 10-4 0.771
10-4 10.10

```
\(x y\)
- 5.2 10-4
- 5.192 10-4 -
0.16

F U
2.82843

10-5 2.82996
10-5 0.05
\(v\)
2.82843 10-5 2.82996

10-5 0.05
xx 20.
18.9626
5.19
\(y y\)
20. 19.8483
0.76
\(z z\)
0. 0. -
\(x y\)
- 20.
- 20.2466
1.23
\(\boldsymbol{x} \boldsymbol{x}\)

\subsection*{0.7 10-4 0.650}

10-4 -
7.08

\section*{\(y y\)}
0.7 10-4 0.708

10-4 1.14
\(x y\)
- 1.3 10-4
- 1.316 10-4 1.23

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\section*{13 Modeling}

F

\subsection*{13.1 Characteristics of modeling}

\section*{C_plan elements (QUAD8 + TRIA6)}
y
B
D
Face blocked in \(d x\)
\(F\)
With
Normally blocked face
C
E
Face with imposed pressure
\(45^{\circ}\)
X

Limiting conditions:
side \(A B\) blocked in \(d x\)
normally blocked side EF
pressure on \(A E p=60\).

Names of the nodes:
With \(=N 2\)
\(B=N 361\)
\(C=N 121\)
\(D=N 584\)
\(E=N 155\)
\(F=N 503\)

\subsection*{13.2 Characteristics of the grid}

A number of nodes: 591
A number of meshs and types: 200 TRIA6, 50 QUAD8

\subsection*{13.3 Functionalities \\ tested}

Orders

\section*{AFFE_CHAR_MECA DDL_IMPO}

GROUP_NO

FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "C_PLAN"
ALL

DEFI_MATERIAU ELAS

CALC_CHAM_ELEM "SIGM_ELNO_DEPL"

POST_RELEVE "EXTRACTION"

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\section*{14 Results of modeling \(F\)}

\author{
14.1 Values
}
tested

\section*{Localization Size Reference}

Aster \%
difference
With \(\boldsymbol{U}\)
0.
0.
\(v\)
5.9

10-5 5.8945
10-5 -
0.09
xx 100.
99.6095
0.39
\(y y\)
- 59.4620
- 0.90

\section*{\(z z\)}
0. 0. -
```

xy
0. 0.2441

```
```

xx

```
5.9 10-4 5.872
10-4 -
0.47
\(y y\)
- 4.5 10-4
- 4.467 10-4 -
0.73
\(x y\)
0.1 .586
10-6 -
B \(U\)
0.eps
\(v\)
4
10-5 3.9974
10-5 -
0.07
xx 40.
39.9774
-
0.06
\(y y\)
0. 0.0786
\(z z\)
```

0.0. -

```
\begin{tabular}{l}
\(x y\) \\
0. \\
\hline
\end{tabular}
0.0181
\(x x\)
2. 10-4 1.998
10.4-
0.11
\(y y\)
- 0.6 10-4
- 0.596 10-4 -
0.67
\(x y\)
0.
- 1.176 10-7 -
E U
4.17193
10-5
- 4.16814 10-5 -
0.09
\(V\)
4.17193 10-5 4.16814
10-5
- 0.09
xx 20.
20.0024
0.01
\(y y\)
20. 20.0045
0.02
\(z z\)
0. 0. -
\(x y\)
80. 79.8164
```

0 . 2 3

```
\(x x\)
0.7 10-4 0.7001
10-4 0.01
\(y y\)
0.7 10-4 0.7002
10-4 0.03
\(x y\)
5.2 10-4 5.188
10-4 -
0.23
F
\(U\)
- 2.82843 10-5
- 2.82655 10-5 -
0.07
\(v\)
2.82843 10-5 2.82655
10-5 -
0.07
xx 20.
20.0083
0.04
\(y y\)
20. 19.9915
-
0.04
\(z z\)
0. 0. -

\section*{\(x y\)}
20. 20.0138
0.07
\(x x\)

\subsection*{0.7 10-4 0.7005}

10-4 0.08

\section*{\(y y\)}
0.7 10-4 0.6995
10.4
0.08
\(x y\)
- 1.3 10-4 1.3009

10-4 0.07

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\section*{15 Modeling}

G
15.1 Characteristics of modeling

C_plan (QUAD9)
F
Normally blocked face
E
D
C\(45^{\circ}\)With
B
\(X\)
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side \(A B\) blocked out of \(D y\)
normally blocked side EF
pressure on \(A E p=60\).
Names of the nodes:
With = N1

\[
B=N 347
\]

\[
C=N 21
\]

\[
D=N 432
\]

\[
E=N 39
\]

\[
F=N 229
\]
15.2 Characteristics of the grid
A number of nodes: 441
A number of meshs and types: 100 QUAD9
15.3 Functionalities
tested
Orders
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

\title{
DNOR
}

PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "C_PLAN"
ALL
DEFI_MATERIAU ELAS

\section*{CALC_CHAM_ELEM "SIGM_ELNO_DEPL"}

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\section*{16 Results of modeling \(G\)}

\subsection*{16.1 Values}
tested

\section*{Localization Size Reference}

Aster \%
difference
With \(\boldsymbol{U}\)
```

5 . 9
10-5 5.9000
10-5 0.00
v
0. eps
xx
-60.
-59.8354
-0.27
yy
100.99.8409
0 . 1 6
zz
0.0. -
xy
0. 0.0283
xx
-4.5 10-4

- 4.489 10-4 -0.24
yy
5.9 10-4 5.890
10-4 -
0 . 1 8
xy
-1.839 10-7 -
B U
4
10-5 3.9999
10-5-
0.001

```
```

0. eps
```
\(x x 0\).
-0.0189
\(y y\)
40. 40.0182
0.05
\(z z\)
0. 0. -
\(x y\)
0
- 3.6815 10-3 -
\(x x\)
- 0.6 10-4
- 0.601 10-4 0.20
\(y y\)
2. 10-4 2.001
10-4 0.06
\(x y\)
- 2.393 10-8 -
E U
4.17193
10-5 4.17195
10-5 0.00
\(v\)
4.17193 10-5 4.17195
10-5
0.00
xx 20.
19.9745
0.13
```

yy
20.20.0311
0 . 1 6
zz
0.0. -
xy

- 80. 

-79.8382
-0.20
xx
0.7 10-4 0.698
10-4 -
0 . 2 5
yy
0.7 10-4 0.702
10-40.28
xy

- 5.2 10-4
-5.189 10-4 -
0 . 2 0
F U
2.82843
10-5 2.82839
10-5 -
0.001
v
2.82843 10-5 2.82839
10-5-
0.001
xx 20.
19.9960
0 . 0 2
yy
20.20.0034
0 . 0 2

```
```

zz
0.0.-
xy
-20.
-20.0185
0.09
xx
0.7 10-4 0.6997
10-4 -
0.04
yy
0.7 10-4 0.7002
10-4 0.03
xy

- 1.3 10-4
- 1.301 10-4 0.09
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```

\section*{17 Modeling H}

\subsection*{17.1 Characteristics of modeling}

Elements axis (TRIA3 + QUAD4)

\section*{Center cylinder}

Node blocked out of Dy
\(y\)
E
F
C
\(0.01 m\)
D
X
With
B
Face with imposed pressure

\section*{Limiting conditions:}
node F blocked out of Dy
pressure on \(A E p=60\).

Names of the nodes:
With = N111
\(B=N 1\)
\(C=N 112\)
\(D=N 3\)
\(E=N 113\)
\(F=N 4\)
17.2 Characteristics of the grid

A number of nodes: 113
A number of meshs and types: 40 QUAD4, 80 TRIA3

\author{
17.3 Functionalities \\ tested \\ Orders \\ ```
AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FACE_IMPO \\ GROUP_MA \\ DNOR \\ PRES_REP \\ GROUP_MA \\ "MECHANICAL" AFFE_MODELE "AXIS" ALL \\ DEFI_MATERIAU ELAS
```

}
CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
CREA_RESU EVOL_ELAS DEPL
CALC_ELEM "SIGM_ELNO_DEPL"
INTE_MAIL_2D
POST_RCCM
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## 18 Results of modeling $\boldsymbol{H}$

### 18.1 Values

tested

## Localization Size Reference

Aster \% difference
With $U$
5.9

10-5 5.8992
10-5 -
0.01
$v$
0. -
-
$x x$

- 60. 
- 56.6060
- 5.66
$y y$

0. 1.0383-
$z z$
1. 101.2924
1.29
```
xy
0. -
1.1635
xx
-4.5 10-4
- 4.36 10-4 -
2.99
yy
-6.18 10-5 -
zz
5.9 10-4 5.898
10-4 -
0.03
xy
0.
- 1.06 10-6 -
B U
4
10-5 3.9997
10-5 -
0.01
v
0. -
xx 0.
0.8951
-
yy
0. -
0.4106
zz
40. 39.6001 -
```

```
xy
0. -
0 . 1 2 8 1
xx
-0.6 10-4
-0.632 10-4 5.43
```

$y y$
0.

- 6.011 10-5 -
$z z$

2. 10-4 1.999
10-4 -
0.02
$x y$
3. 

- 8.325 10-7 -
E U
5.9
10-5 5.8992
10-5 -
0.01
$v$

0.     - 

$x x$

- 60. 
- 56.6060
- 5.66
$y y$

0. 1.0383-
$z z$
1. 101.2924
1.29
```
xy
0.1.1635 -
xx
-4.5 10-4
- 4.365 10-4 -
2.99
yy
0.
- 6.184 10-5 -
zz
5.9 10-4 5.898
10-4 -
0.03
xy
0.1.063
10-6 -
FU
4
10-5 3.9997
10-5 -
0.01
v
0. -
xx 0.
-
0.4221
yy
0. -
0.2280
zz
40. 39.8015 -
0.50
```

```
xy
0. -
0.0020
-
xx
- 0.6 10-4
- 0.615 10-4 2.45
yy
0.
- 6.021 10-5 -
zz
2. 10-4 1.9998
10-4 -
0.01
xy
0.
- 1.280 10-8 -
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```


## 19 Modeling <br> I

19.1 Characteristics of modeling

Elements axis (TRIA6 + QUAD8)

## Center cylinder

Node blocked out of Dy
$y$
E
F
$0.01 m$
C
D
$X$
With
B
Face with imposed pressure

## Limiting conditions:

## Node F blocked out of Dy

pressure on $A E p=60$.

Names of the nodes:
With $=$ N8
$B=$ N174
$C=N 5$
$D=N 170$
$E=N 3$
$F=N 159$

### 19.2 Characteristics of the grid

A number of nodes: 175
A number of meshs and types: 20 QUAD8, 40 TRIA6

19.3 Functionalities<br>tested<br>Orders<br>AFFE_CHAR_MECA DDL_IMPO<br>GROUP_NO<br>\section*{FACE_IMPO}<br>GROUP_MA<br>\section*{DNOR}<br>PRES_REP<br>GROUP_MA<br>"MECHANICAL" AFFE_MODELE "AXIS" ALL<br>DEFI_MATERIAU ELAS

## CALC_CHAM_ELEM "SIGM_ELNO_DEPL"

## POST_RELEVE "EXTRACTION"

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20 Results of modeling I
20.1 Values
tested

Localization Size Reference
Aster \%
difference
With $U$
5.9

10-5 5.9000
10-5 0.00
$v$
0. -
$x x$

- 60. 
- 59.8976
- 0.17
$y y$

0.     - 

0.0024

## $z z$

100. 99.9089 -
0.09
$\boldsymbol{x y}$
101.     - 

0.0137
$x x$

- 4.5 10-4
- 4.493 10-4 -
0.14
$y y$

0. 

- 6.003 10-5 -
$z z$
5.9 10-4 5.894

10-4 -
0.10
xy

- 8.895 10-8 -

B U
4
10-5 4.0000
10-5 0.00
$v$
0. -
$x x 0$.
0.0308
-
$y y$
0. -
0.0020
$z z$
40. 39.9738 -
0.07
$x y$
0. 0.0131 -
$\boldsymbol{x} \boldsymbol{x}$

- 0.6 10-4
- 0.598 10-4 -
0.33

```
yy
0.
-6.002 10-5 -
```

$z z$
2. 10-4 1.998
10-4 -
0.09
$x y$
0. 8.495
10-8-
E U
5.9
10-5 5.9000
10-5 0.00
$v$
0. -
$x x$

- 60. 
- 59.8976
- 0.17
$y y$

0.     - 

0.0024
$z z$
100. 99.9089 -
0.09
$x y$
0. 0.0137 -
$\boldsymbol{x} \boldsymbol{x}$

- 4.5 10-4
- 4.493 10-4 -
0.14

```
yy
0.
- 6.003 10-5 -
zz
5.9 10-4 5.894
10-4 -
0 . 1 0
xy
0. 8.895
10-8 -
F U
4
10-5 4.0000
10-5 0.00
v
0. -
xx 0.
0 . 0 3 0 8
yy
0. -
0 . 0 0 2 0
zz
40.39.9738 -
0 . 0 7
xy
0. -
0 . 0 1 3 1
xx
- 0.6 10-4
-0.598 10-4 -
0 . 3 3
```

0. 

- 8.495 10-8 -


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## 21 Modeling <br> J <br> 21.1 Characteristics of modeling <br> Elements axis (QUAD9)

## Center cylinder

## Node blocked out of Dy

$y$
E
F
$0.01 m$
C
D
$X$
With
B
Face with imposed pressure

## Limiting conditions:

## node F blocked out of Dy

pressure on $A E p=60$.

Names of the nodes:
With = N196
$B=N 1$
$C=N 200$
$D=N 5$
$E=N 202$
$F=N 7$

### 21.2 Characteristics of the grid

A number of nodes: 205
A number of meshs and types: 40 QUAD9

### 21.3 Functionalities

tested

## Orders

AFFE_CHAR_MECA DDL_IMPO<br>GROUP_NO<br>FACE_IMPO<br>GROUP_MA<br>DNOR<br>PRES_REP<br>GROUP_MA<br>"MECHANICAL" AFFE_MODELE "AXIS" ALL<br>DEFI_MATERIAU ELAS

## CALC_CHAM_ELEM "SIGM_ELNO_DEPL"

## POST_RELEVE "EXTRACTION"

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22 Results of modeling J

```
22.1 Values
tested
Localization Size Reference
Aster %
difference
With U
5 . 9
10-5 5.9000
10-5 0.00
v
0.
xx
-60.
-59.8997
-0.17
yy
0. -
0 . 0 0 3 5
-
zz
100.99.9080 -
0 . 0 9
xy
0. -
0 . 0 1 4 1
xx
-4.5 10-4
- 4.494 10-4 -
0 . 1 4
yy
0.
-6.003 10-5 -
```

```
zz
5.9 10-4 5.894
10-4 -
0 . 1 0
xy
0.
-9.156 10-8 -
B U
4 .
10-5 4.0000
10-5 0.00
v
0.
xx 0.
0.0070
```

$y y$
0. -
0.0001
$z z$
40. 39.9936 -
0.02
$x y$
0. 0.0010 -
$x x$

- 0.6 10-4
- 0.5996 10-4 -
0.07
$y y$

0. 

$-6.000 \quad 10-5-$
2. 10-4 1.9996

10-4 -
0.02
$x y$
0.6 .748

10-9 -
$\boldsymbol{E} \boldsymbol{U}$
5.9

10-5 5.9000
10-5 0.00
$v$
0.
-
-
$x x$

- 60. 
- 59.8997
- 0.17
$y y$

0.     - 

0.0035
-
$z z$
100. 99.9080 -
0.09

```
xy
0.0.0141 -
xx
- \(4.510-4\)
- 4.494 10-4 -
0.14
\(y y\)
0.
- 6.003 10-5 -
```


## $z z$

5.9 10-4 5.894

10-4 -
0.10
$x y$
0.

- 9.156 10-8 -
$\boldsymbol{F} \boldsymbol{U}$

4. 

10-5 4.0000
10-5 0.00
$v$
0.
-
-
$x x 0.0 .0070$
$y y$
0. -
0.0001
$z z$
40. 39.9936 -
0.02

```
xy
0. -
0.0010
\(\boldsymbol{x} x\)
- 0.6 10-4
- 0.5996 10-4 -
0.07
\(y y\)
0.
- 6.000 10-5 -
```

2. 10-4 1.9996

10-4 -
0.02
$x y$
0.

- 6.748 10-9 -


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## 23 Modeling <br> K

### 23.1 Characteristics of modeling

## Elements 3D (PENTA6 and HEXA8)

## Grid obtained by extrusion starting from the grid 2D below (modeling E) <br> F <br> Normally blocked face <br> $y$ <br> E <br> G <br> $45^{\circ}$ <br> H <br> With <br> B <br> $X$ <br> Face with displacement <br> radial imposed <br> Face blocked out of Dy

Along axis Z:
2 layers of elements
Total thickness:
0.01

## Limiting conditions:

node $F: u z=0$
face $A B$ blocked out of $D y$
normally blocked face $E F$
face $A E$
radial displacement imposed on $5.9 E-5 ~ m$

Names of the nodes:
With $=$ No 1
$C=$ No36
$D=N o 166$
plan $Z=0.005$
$A 2=$ No 172
$C 2=\mathrm{No} 242$
D2 $=$ No5025
plan $Z=0.01$
A3 $=$ No173
C3 $=$ No243
D3 $=$ No503
Names of the nodes:
E = No41
H $=$ No9
$G=N o 38$
plan $Z=0.005$
E2 $=$ No252
H2 $=$ No 188
G2 $=$ No246
plan $Z=0.01$
E3 $=$ No253
H3 = No 189
G3 $=$ No247

### 23.2 Characteristics of the grid

A number of nodes: 513
A number of meshs and types: 400 PENTA6, 100 HEXA8 40 QUAD4 (faces skin interns)

### 23.3 Functionalities

tested

## Orders

# AFFE_CHAR_MECA DDL_IMPO 

GROUP_NO

FACE_IMPO<br>GROUP_MA<br>DNOR<br>"MECHANICAL" AFFE_MODELE "3D"<br>ALL<br>DEFI_MATERIAU ELAS

CALC_NO "REAC_NODA"

## MODI_MAILLAGE ORIE_PEAU_3D

### 23.4 Remarks

The loading is here in imposed displacement, contrary to other modelings. They are tested reactions.
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24 Results of modeling $K$
24.1 Values
tested
Localization Size Reference
Aster \%
difference
C Fx
1.0884 E3 1.0953 E3 0.64

Fy
4.5084 E4 4.5836 E4 1.67

C2 Fx
2.1768 E3 2.1571 E3
0.91

Fy
9.0170 E4 9.1304 E4 1.26

C3 Fx
1.0884 E3 1.0953

E3 0.64
Fy
4.5084 E4 4.5836 E4 1.67

H Fx
1.1636 E3 1.1709

E3 0.63
Fy
1.8429 E4 1.8527

E4 0.53
G Fx
1.0045 E3 1.0144

E3 0.99
Fy
6.1550 E4 6.2117

### 24.2 Remarks

One checks that the nodal forces of reactions are null in all the nodes, except on the nodes of surface $A E$ and surfaces EF and AB.

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Author (S):
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## 25 Modeling <br> L <br> 25.1 Characteristics of modeling

Elements 3D (PYRAM5)

## F <br> Normally blocked face

$y$

## Along axis Z:

each parallélipipède is cut out in 6 pyramids
Total thickness:
0.01

## Limiting conditions:

$$
\text { node } F: u z=0
$$

face AB blocked out of Dy
normally blocked face EF
pressure on face $\boldsymbol{A E}$
$p=60$.
Names of the nodes:
With $=$ N267
$B=N 142$
$E=N 29$
$F=N 1$

### 25.2 Characteristics of the grid

A number of nodes: 342
A number of meshs and types: 600 PYRAM5 620 QUAD4 (faces skin interns)

### 25.3 Functionalities

tested
Orders

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

```
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "3D"
ALL
```

DEFI_MATERIAU ELAS

CALC_CHAM_ELEM "SIGM_ELNO_DEPL"
"EPSI_ELNO_DEPL"
"SIEF_ELGA_DEPL"
"ENDO_ELNO_SIGM"
Handbook of Validation
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Code_Aster ${ }^{\circledR}$

# Version <br> 6.4 

## Titrate: <br> SSLV04-Hollow roll in plane constraints

Date:<br>17/06/03<br>Author (S):<br>X. DESROCHES, Key P. HERMAN<br>:<br>V3.04.004-E Page:<br>27/36

26 Results of modeling $L$

### 26.1 Values

tested
Identification Reference Aster \%
difference
U-node (A)
5.9 10-5 5.8873

10-5 -
0.21
v-node (A)
0 .
eps
$x x$ - node (A)

- 60. 
- 52.9567
- 11.74
yy - node (A)

100. 91.7830
8.22
$z z$ - node (A)
101.     - 

1.1206 -
xy - node (A)
xx-node (A)

- $4.510-4$
- 4.008 10-4 -
10.94
yy - node (A)
5.9 10-4 5.400
10-4 -
8.47
$x y-n o d e(A)$
0 .
- 2.933 10-5 -


## U-node (B)

4. 10-5 3.9936

10-5 -
0.16
v-node (B)
0.
eps
-
$x x-\operatorname{node}(B)$
0 .

- 0.7670
yy - node (B)

40. 39.5319
1.17
$z z-n o d e(B)$
O. -
0.3115 -
$x y-n o d e(B)$
41.     - 

1.5858 -
xx - node (B)

- 0.6 10-4
- 0.627 10-4 4.44

```
yy - node (B)
2. 10-4 1.993
10-4 -
0 . 3 6
xy - node (B)
0.
- 1.031 10-5 -
```


## U-node (E)

```
4.17193 10-5 4.16293
10-5
0.21
v-node (E)
4.17193 10-5 4.16293
10-5 -
0.21
\(\boldsymbol{x x}\) - node (E)
20.
19.3586
- 3.21
yy - node (E)
20. 31.5151
57.57
zz-node (E)
0. 2.5686
```

xy - node (E)

- 80 .
- 77.2309
- 3.46
xx - node (E)
0.7 10-4 0.457

10-4
34.76
yy - node (E)
0.7 10-4 1.247

10-4 78.12
xy - node (E)

## U-node (F)

2.82843 10-5 2.82393

10-5 -
0.16
v-node (F)
2.82843 10-5 2.82393

10-5 -
0.16
$x x-n o d e(F)$
20.
18.9523

- 5.24
yy - node (F)

20. 20.9510
4.75
zz-node (F)
21. 0.0035
$x y-n o d e(F)$

- 20. 
- 20.9897
4.95
xx - node (F)
0.7 10-4 0.633

10-4 -
9.60
yy-node (F)
0.7 10-4 0.763

10-4 8.96
xy-node (F)

- 1.3 10-4
- 1.364 10-4 4.95


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SSLV04-Hollow roll in plane constraints

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## 27 Modeling <br> M

### 27.1 Characteristics of modeling

Elements 3D (PYRAM13)

F<br>Normally blocked face



## Along axis Z:

each parallélipipède is cut out in 6 pyramids

## Total thickness:

0.01

## Limiting conditions:

$$
\text { node } F: u z=0
$$

face AB blocked out of Dy normally blocked face EF
pressure on face $\boldsymbol{A E}$
$p=60$.
Names of the nodes:
With $=$ N1403
$B=N 734$
$E=N 152$
$F=N 4$
27.2 Characteristics of the grid

A number of nodes: 1703
A number of meshs and types: 600 PYRAM13 620 QUAD8 (faces skin interns)

### 27.3 Functionalities

tested
Orders

MODI_MAILLAGE ORIE_PEAU_3D

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA

CALC_CHAM_ELEM "SIGM_ELNO_DEPL"

"EPSI_ELNO_DEPL"<br>"SIEF_ELGA_DEPL"<br>"ENDO_ELNO_SIGM"<br>Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal structures<br>HT-66/03/008/A

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28 Results of modeling M
28.1 Values
tested
Identification Reference Aster \%
difference
U-node (A)
5.9 10-5 5.8999

10-5
0.002

## v-node (A)

0 .
eps
$x x$ - node (A)

- 60. 
- 59.9880
- 0.02
yy - node (A)

100. 100.1277 0.13
$z z$ - node (A)
101. 0.0425
$x y$ - node (A)
102.     - 

0.0913 -
$x x$ - node (A)

- 4.5 10-4
- 4.502 10-4 0.04
yy - node (A)
5.9 10-4 5.906

10-4 0.09
xy - node (A)
0 .

- 5.934 10-7 -

U-node (B)
4. 10-5 4.0000

10-5 0.00
v-node (B)
0 .
eps
$x x$ - node (B)
0 .

- 0.0276
yy - node (B)

40. 40.0331
41. 0.0024
```
xy-node (B)
0. 0.0126
xx - node (B)
- 0.6 10-4
- 0.602 10-4 0.32
```

yy - node (B)
2. 10-4 2.002
10-4 0.10
$x y-n o d e(B)$
0. 8.177
10-8 -

## U-node (E)

4.17193 10-5 4.17183

10-5
0.002
v-node (E)
4.17193 10-5 4.17183

10-5 -
0.002
xx-node (E)
20.
19.9787
-0.11
yy - node $(\mathrm{E})$
20. 20.1612
0.81
zz-node (E)
0. 0.0425
$x y-n o d e(E)$

- 80 .
- 80.0580
0.7 10-4 0.696

10-4 -
0.59
yy-node $(E)$
0.7 10-4 0.708

10-4 1.11
xy-node (E)

- 5.2 10-4
- $5.20410-40.07$


## U-node (F)

2.82843 10-5 2.82844

10-5 0.00
v-node (F)
2.82843 10-5 2.82844

10-5 0.00
xx-node (F)
20.
20.0224
0.11
yy - node (F)
20. 19.9901
0.05
zz-node (F)
0. 0.0031
$x y-n o d e(F)$

- 20. 
- 19.9818
- 0.09
xx - node (F)
0.7 10-4 0.701

10-5 0.17
yy - node (F)
0.7 10-4 0.699

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SSLV04-Hollow roll in plane constraints

Date:
17/06/03
Author (S):
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29 Modeling
NR

### 29.1 Characteristics of modeling

## Elements 3D (PENTA15 and HEXA20)

Grid obtained by extrusion starting from a grid 2D resembling the grid below ( 8 elements in the radial direction, 4+4 elements in the circumferential direction) and duplicated for to have a complete section of the cylinder (on $360^{\circ}$ ).

## F

Normally blocked face
y
E
$45^{\circ}$

## With

B

## $X$

Face with imposed pressure
Face blocked out of Dy

## Along axis Z:

1 layer of elements
Total thickness:
0.01

Limiting conditions:
face AB blocked out of Dy normally blocked face EF
pressure on face $A E$
$p=60$.
basic effect on the sections $p=60$.

Names of the nodes:
With $=$ N5349
$B=N 6092$
$C=N 433$
$D=N 441$
$E=N 2180$
$F=N 1632$
29.2 Characteristics of the grid

A number of nodes: 8832
A number of meshs and types: 1024 PENTA15, 512 HEXA20, 1176 QUAD8 and 2048 TRIA6.

### 29.3 Functionalities

tested
Orders

# AFFE_CHAR_MECA DDL_IMPO <br> GROUP_NO 

FACE_IMPO<br>GROUP_MA<br>DNOR<br>PRES_REP<br>GROUP_MA<br>EFFE_FOND<br>"MECHANICAL" AFFE_MODELE "3D"<br>ALL<br>DEFI_MATERIAU ELAS

## CALC_NO "EPSI_NOEU_DEPL"

## "SIGM_NOEU_DEPL" <br> 29.4 Remarks

Contrary to preceding modelings, one takes into account the basic effect here applying to sections at the ends of the cylinder.
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## 30 Results of modeling NR

30.1 Values

tested

## Identification Reference Aster \%

 differenceU-node (A)
5.6 10-5 5.6000

10-5
0.00
v-node (A)
0 .
eps
xx - node (A)
-60.
-59.5972

- 0.67
yy - node (A)

100. 99.5187
0.48
$z z$ - node (A)
101. 19.9584
0.21
xy - node (A)
102. eps

## U-node (B)

3.4 10-5 3.4000

10-5
0.00
v-node (B)
0 .
eps

```
xx - node (B)
0.
2.0877 10-2
```

yy - node (B)
40. 39.9685
0.08
zz-node (B)
20. 19.9946
0.03
$x y-n o d e(B)$
0. eps
$x x-\operatorname{node}(E)$
20.
20.3287
1.64
yy - node (E)
20. 20.3287
1.64
zz-node (E)
20. 20.1739
0.87
xy - node (E)
-80. -79.9775
0.03

```
xx - node (F)
20.
20.0176
0.09
yy-node(F)
```


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## Code_Aster ${ }^{\circledR}$

Version
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Titrate:
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## Date:

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## 31 Modeling

O

### 31.1 Characteristics of modeling

C_plan elements (QUAD8 + TRIA6)
Grid 2D resembling the grid below (8 elements in the radial direction, 4+4 elements in the circumferential direction) and duplicated to have a complete section of the cylinder (on $360^{\circ}$ ).
B
D
Face blocked in dx
F
With
Normally blocked face

## C

E
Face with imposed pressure
X

## Limiting conditions:

side $A B$ blocked in dx
normally blocked side EF
pressure on $A E p=60$.

Names of the nodes:
With $=$ N249
$B=N 992$
$C=N 1667$
$D=N 1588$
$E=N 3776$
$F=N 3228$
31.2 Characteristics of the grid

A number of nodes: 3840
A number of meshs and types: 1026 TRIA6, 512 QUAD8

### 31.3 Functionalities

tested
Orders

```
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
"MECHANICAL" AFFE_MODELE "C_PLAN"
ALL
DEFI_MATERIAU ELAS
```

CALC_ELEM "SIEF_ELGA_DEPL"
CALC_NO "EPSI_NOEU_DEPL"

"SIGM_NOEU_DEPL"<br>\section*{Handbook of Validation}<br>V3.04 booklet: Linear statics of the voluminal structures HT-66/03/008/A

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## 32 Results of modeling $O$

### 32.1 Values

tested

## Identification Reference Aster \%

 differenceU-node (A)
5.9 10-5 5.8999

10-5

- 0.002
v-node (A)
0 .
eps
$x x$ - node (A)
-60.
-59.5340
- 0.78
yy - node (A)

100. 99.5453
0.45
zz-node (A)
101. 0 .
xy - node (A)
0.eps

## U-node (B)

4. 10-5 3.99996

10-5
0.00
v-node (B)
0 .
0 .
$x x$ - node (B)

```
0.
2.6874 10-2
```

yy - node (B)
40. 39.9716
0.07
$z z$ - node (B)
0. 0 .
$x y$ - node ( $B$ )
0. eps
U-node ( $E$ )
$4.1719310-54.17215$
10-5
0.005
v-node ( $E$ )
$4.1719310-54.17215$
10-5
0.005
$x x$ - node ( $E$ )
20.
20.2875
1.44
yy - node ( $E$ )
20. 20.2875
1.44
$z z-$ node ( $E$ )
0. 0 .
$x y$ - node ( $E$ )
-80. -79.9196
0.10
U-node (F)
2.82843 10-5 2.82841
10-5

- 0.001
$v$-node $(F)$
2.82843 10-5 2.82841
10-5- 0.001
$x x$ - node $(F)$

20. 

20.01670.08$y y$ - node (F)

$$
\text { 20. } 20.0167
$$

$$
0.08
$$

$$
z z-\operatorname{node}(F)
$$

$$
\text { 0. } 0 .
$$

$x y$ - node (F)-20. -19.9993

- 

0.004

## Handbook of Validation

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## 33 Modeling

$P$

### 33.1 Characteristics of modeling

## Elements 3D (PENTA15 and HEXA20) even grid that modeling NR

Grid obtained by extrusion starting from a grid 2D resembling the grid below ( 8 elements in the radial direction, 4+4 elements in the circumferential direction) and duplicated for to have a complete section of the cylinder (on $360^{\circ}$ ).
F
Normally blocked face
$y$
E
$45^{\circ}$
With
B
X
Face with imposed pressure
Face blocked out of Dy

Along axis Z:
1 layer of elements
Total thickness:
0.01

Limiting conditions:
normally blocked face EF
face $A B$ blocked out of $D y$
pressure on face $A E$
FP
basic effect on sections FP

With FP: linear pressure function of time being worth 60 . to $t=1 s$ and 120 . with $t=2 s$

Names of the nodes:
With $=$ N5349
$B=N 6092$
$C=N 433$
$D=N 441$
$E=N 2180$
$F=N 1632$

### 33.2 Characteristics of the grid

A number of nodes: 8832
A number of meshs and types: 1024 PENTA15, 512 HEXA20, 1176 QUAD8 and 2048 TRIA6.

### 33.3 Functionalities

tested

Orders

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO

```
FACE_IMPO
GROUP_MA
DNOR
\(A F F E \_C H A R \_M E C A \_F P R E S \_R E P\)
```


# CALC_NO "EPSI_NOEU_DEPL" 

"SIGM_NOEU_DEPL"<br>Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal structures<br>HT-66/03/008/A

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### 33.4 Remarks

Contrary to modeling NR, one tests here a basic pressure and effect variables in function time. A linear variation of the pressure involves a linear variation of the constraints.

34 Results of modeling $P$

### 34.1 Values

tested

Identification Reference Aster \% difference
U-node (A) with $t=1 . S$
5.6 10-5 5.6000

10-5
0.00
$v$-node (A) with $t=1 . S$
0 .
eps
$x x$ - node (A) with $t=1 . S$
-60.
-59.5414

- 0.76
yy - node (A) with $t=1 . S$

100. 99.6183-
0.38
$z z-$ node (A) with $t=1 . S$
101. 19.9976 -
0.01
$x y$ - node (A) with $t=1 . S$ 0.eps
$x x$ - node (A) with $t=2 . S$
-120.
-119.0829

- 0.76
$y y$ - node (A) with $t=2 . S$ 200. 199.2366 -
0.38
$z z$ - node (A) with $t=2 . S$

40. 39.9952 -
0.01
$x y$ - node (A) with $t=2 . S$ 0.eps

U-node (B) with $t=1$. $S$
3.4 10-5 3.4000

10-5
0.00
$v$-node (B) with $t=1 . S$
0 .
eps
$x x$ - node (B) with $t=1 . S$
0 .
2.6761 10-2
$y y$ - node (B) with $t=1 . S$
40. 39.9740 -
0.06
$z z-$ node (B) with $t=1 . S$
20. 19.9973-
0.01
$x y$ - node (B) with $t=1 . S$
0.eps
$x x-n o d e(B)$ with $t=2 . S$
0 .
5.3523 10-2
yy - node (B) with $t=2 . S$ 80. 79.9480 -
0.06
$z z$ - node (B) with $t=2 . S$ 40. 39.9946 -
0.01
$x y$ - node (B) with $t=2 . S$
0. eps
$x x-n o d e(E)$ with $t=1 . S$ 20.
20.3287
1.64
$\boldsymbol{y y}$ - node (E) with $t=1 . S$
20. 20.32871 .64
20. 20.17390 .87
$x y$ - node ( E ) with $t=1 . S$
-80. -79.9775-
0.03
$x x-n o d e(E)$ with $t=2 . S$ 40.
40.6575
1.64
$y y$ - node (E) with $t=2 . S$ 40. 40.65751 .64
$z z-$ node (E) with $t=2 . S$ 40. 40.34790 .87
$x y$ - node $(E)$ with $t=2 . S$ -160. -159.9550 -
0.03
$x x-n o d e(F)$ with $t=1 . S$ 20.
20.0176
0.09
$y \mathrm{y}$ - node (F) with $\mathrm{t}=1 . \mathrm{S}$
20. 20.01760 .09
$z z$ - node ( $F$ ) with $t=1 . S$
20. 20.00720 .04
$x y$ - node (F) with $t=1 . S$
-20. 20.00270 .01
$x x$ - node ( $F$ ) with $t=2 . S$
20.
40.0351
0.09
yy - node (F) with $t=2 . S$
20. 40.03510 .09
$z z$ - node ( $F$ ) with $t=2 . S$
0. 40.01440 .04
$x y$ - node (F) with $t=2 . S$
-20. 40.00540 .01

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Date:
17/06/03
Author (S):
X. DESROCHES, Key P. HERMAN

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## 35 Summary of the results

Summary of the errors max in \%

## 3D

Localization MOD A MOD B MOD C MOD D MOD L MOD M MOD NR MOD P
elem pe6, h 8
pe15, $h 20$
te4 te10 py5 py13
pe15, h20 pe15, h20
geom
$45^{\circ} 45^{\circ} 45^{\circ} 45^{\circ} 45^{\circ} 45^{\circ} 360^{\circ} 360^{\circ}$
$N b$
1922211511151395342170388328832
No
Dépl. With,
E 0.08
0.09
0.17
0.04

C_plan

Localization
MOD E
MOD F
MOD G
MOD $O$
Type of elements
tria3, quad4
tria6, quad8
quad9
tria6, quad8
Modelled geometry
$45^{\circ} 45^{\circ} 45^{\circ}$
$360^{\circ}$
A number of nodes

961
591
441
384
Displacements A,
E
0.07
0.09
0.00
0.01
$B, \boldsymbol{F}$
0.09
0.07
0.00
0.00

Constraints
4.65
0.39
0.27
1.44
$x x$
WITH, $E$

## Constraints

WITH, E
5.68
0.90
0.16
1.44
$y y$
$B, F$
1.40
0.04
0.05
0.08

## Constraints

WITH, $E$
Good
Good
Good
Good
$z z$
B, $\boldsymbol{F}$
Good
Good
Good
Good

## Constraints

WITH, E
0.16
0.23
0.20
0.10
$x y$
$B, F$
1.23
0.07
0.09
0.00

Axis

Localization

MOD H
MOD I
MOD J
Type of elements
tria3, quad4
tria6, quad8
quad9
A number of nodes

## 113

175
205
Displacements A,
E
0.01
0.00
0.00
$B, \boldsymbol{F}$
0.01
0.00
0.00

Constraints
5.66
0.17
0.17
$x x$
WITH, E
$B, F$
Good
Good
Good
Constraints
WITH, E
Good
Good
Good
$y y$
$\boldsymbol{B}, \boldsymbol{F}$
Good
Good
Good

## Constraints

WITH, E
1.29
0.09
0.09
$z z$
B, $\boldsymbol{F}$
1.00
0.07
0.02

Constraints
WITH, E
Good
Good
Good
$x y$
B, $F$
Good
Good
Good

- The results are more precise with elements of order 2.
- The problem is adapted more to an axisymmetric modeling. The results are better.
- The grids remain insufficient for the elements 3D of order 1: constraints and deformations modelings A, C, E and L (especially for modeling L in PYRAM5).
- The pyramids give results similar to the other elements 3D, with equivalent grid.
$\cdot$ Modelings NR and $P$ with basic effect and pressure constant or variables give of good results.
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## HT-66/03/008/A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV07 Stretching of a parallelepiped under its own weight
Date:
21/01/98
Author (S):
X. DESROCHES

Key:
V3.04.007-D Page:

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Organization (S): EDF/IMA/MMN
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.007
SSLV07-Stretching of a parallelepiped
under its own weight
Summary:
This static test 3D makes it possible to validate the following functionalities:

- loading in actual weight (gravity or internal force) and in uniform pressure,
- calculation of the potential energy of the structure,
- estimator of error in residue (modeling B).

It includes/understands 3 modelings. Its interest lies in the description of the effect of the Poisson's ratio (of contraction).

## Handbook of Validation

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HI-75/96/017 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV07 Stretching of a parallelepiped under its own weight
Date:
21/01/98
Author (S):

## X. DESROCHES

Key:
V3.04.007-D Page:
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1
Problem of reference
1.1 Geometry

Z
B
With
D
y

## $E$

$L$
B

## $X$

has
Height:
$L=3 m$
Width:
has $=1 \mathrm{~m}$
Thickness:
$B=1 \mathrm{~m}$
Co-ordinates of the points (in meters):
With
B
C
D
E
$X$
0.
0.
0.5
0.5

0 .
$y$
0.
0.
0.
0.
0.

Z
3.
0.
0.
3.
1.5
1.2

Material properties
$E=21011 \mathrm{MPa}$
$=0.3$
$=7.800 \mathrm{~kg} / \mathrm{m} 3$
1.3

Boundary conditions and loadings
Not a: $(U=v=W=0, X=y=Z=0)$
Actual weight following axis Z
Uniform constraint with traction for the higher face: $Z=g L=+229.554$. Pa

### 1.4 Conditions

## initial

Without object for the static analysis.
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2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card SSLV07/89 of the guide VPCS which presents method of calculation in the following way:
Displacements:
$G X Z$
Gy Z
G z2 G
G L2
$U$
= -
$v$
= -
W
$=$
$+$
$(x 2+y 2)-$
$E$

E
2nd
2nd
2nd

## Constraints:

$$
\begin{aligned}
& = \\
& = \\
& = \\
& = \\
& = \\
& = \\
& z z
\end{aligned}
$$

### 2.4 References

bibliographical
p. 279 with 282 (1961).

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Author (S):

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## 3 Modeling

## With

3.1

Characteristics of modeling
3D
Z
B
With
D
y
$L$
C
$X$
has
Cutting:
3 in height
2 in width and thickness
meshs hexa20
Limiting conditions:
on axis $A B$
DDL_IMPO: (GROUP_NO: ABsansA $D X=0 ., D Y=0$. )
in $A$ and $D$
(NODE: WITH $D X=0 ., D Y=0 ., D Z=0$. ), (NODE: $D D Y=0$. )
Names of the nodes:
Not $A=N 59$
Not $B=N 53$

Not $C=N 12$
Not $D=N 18$
Not $E=N 56$

## 3.2

Characteristics of the grid
A number of nodes: 111
A number of meshs and types: 12 HEXA20
Files:
Aster: yes
IDEAS: not
ALI-BABA: not

### 3.3 Functionalities

tested
Orders

## Keys

AFFE_MODELE
"MECHANICAL"
" $3 D$ "
ALL
[U4.22.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
GRAVITY
FORCE_FACE
GROUP_MA
CALC_CHAM_ELEM
OPTION
"SIGM_ELNO_DEPL"
[U4.61.01]
"EPOT_ELEM_DEPL"
POST_ELEM
ENER_POT
[U4.61.04]
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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:

## X. DESROCHES

Key:
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4
Results of modeling $A$
4.1 Values
tested
Localization
Type of value
Reference
Aster
\% difference
(m)

Not B
UB
0 .
< 1014

VB
0 .
< 1014

WB
1.721655106
1.7217106
< 0.1
Not C
CPU
0.
$=1014$

VC
0 .
< 1014

WC
1.707308106
1.7073106

### 4.2 Remarks

Modeling in HEXA20 is completely acceptable for this coarse grid.

## 4.3

## Contents of the file results

Displacements and constraints. Potential energy by element.

### 4.4 Parameters

## of execution

Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
4.26 seconds

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## 5 Modeling

B
5.1

Characteristics of modeling
3D
Cutting:
12 in height
8 in width and thickness
meshs HEXA8
Limiting conditions:
on axis $A B$
DDL_IMPO: (GROUP_NO: ABsansA $D X=0 ., D Y=0$. )
in $A$ and $D$
(NODE: WITH $D X=0 ., D Y=0 ., D Z=0$. ), (NODE: $D D Y=0$. )
Names of the nodes:
Not $A=N 533$
Not $B=$ N521

Not $C=N 989$
Not $D=$ N1001
Not $E=N 527$
5.2

Characteristics of the grid
A number of nodes: 1053
A number of meshs and types: 768 HEXA8

### 5.3 Functionalities

tested
Orders
Keys
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FORCE_INTERNE
GRAVITY
AFFE_CHAR_MECA_F
FORCE_FACE
GROUP_MA
MECA_STATIQUE
OPTION
"SIGM_ELNO_DEPL"
[U4.31.01]
"EPOT_ELEM_DEPL"
CALC_ELEM
OPTION
"ERRE_ELGA_NORE"
[U4.61.02]

### 5.4 Remarks

This modeling makes it possible to test the estimator of error in residue in $3 D$.
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Code_Aster ${ }^{\circledR}$
Version
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## Titrate:

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Date:
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Author (S):

## X. DESROCHES

Key:
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6
Results of modeling B
6.1 Values
tested
Localization
Type of value
Reference
Aster
\% difference
(m)

Not B
UB
0 .
< 1015

VB
0 .
1015

WB
1.721655106
1.7217106
< 0.1
Not C
CPU
0.
3.7081109

VC
0.

1015

WC
1.707308106
1.7095106
0.13

Not D
UD
1.721655107
1.6846107
2.2

VD
0.
< 1015
-
WD
1.434713108
1.2118108
15.5

Not E
EU
0.
< 1015

VE
0.
< 1015

WE
1.291241106
1.2917106
< 0.1
(Pa)
Not A
2.29554105
2.1739105
5.3
zz
HEX12
relative error
1.15
1.148
0.17

HEX600
relative error
1.30
0.07

### 6.2 Remarks

The grid remains insufficient for a modeling in HEXA8. The total relative error is weak (3\%) but $20 \%$ exceed on certain meshs.

## 6.3

## Contents of the file results

Displacements and constraints. Relative and absolute errors total by the estimator in residue. Errors by element. Potential energy by element.

### 6.4 Parameters

of execution
Version:
3.02.11

Machine:
CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
36.42 seconds

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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV07 Stretching of a parallelepiped under its own weight
Date:
21/01/98
Author (S):

## X. DESROCHES

Key:
V3.04.007-D Page:
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7 Modeling
C
7.1

Characteristics of modeling
3D
Cutting:
12 in height
8 in width and thickness
meshs hexa8
Limiting conditions:
on axis AB
DDL_IMPO: (GROUP_NO: ABsansA DX=0., DY=0. )
in A and D
(NODE: WITH DX=0., DY=0., DZ=0. ), (NODE: D DY=0. )
Names of the nodes:
Not A = N533
Not B = N521
Not C $=$ N989
Not D = N1001
Not $\mathrm{E}=\mathrm{N} 527$
7.2

Characteristics of the grid
A number of nodes: 1053
A number of meshs and types: 768 HEXA8
7.3 Functionalities
tested
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
AFFE_CHAR_MECA_F
FORCE_INTERNE
FORCE_FACE
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
OPTION
"SIGM_ELNO_DEPL"
[U4.61.01]

### 7.4 Remarks

This modeling makes it possible to test key word FORCE_INTERNE in AFFE_CHAR_MECA_F. Handbook of Validation
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HI-75/96/017 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
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Date:
21/01/98
Author (S):
X. DESROCHES

Key:
V3.04.007-D Page:
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8
Results of modeling C
8.1 Values
tested
Locali-

## Type of value

 ReferenceASTER
\% difference
sation
(m)

Not B
UB
0.
< 1015

VB
0.
< 1015
-
WB
1.721655106
1.7217106
< 0.1
Not C
CPU
0.
3.7081109

VC
0.
< 1015
WC
1.707308106
1.7095106
+0.13
Not D
UD
1.721655107
1.6846107
2.15

VD
0.
< 1015
WD
1.434713108
1.2118108
15.5
Not E
EU
0.
< 1015
VE
0.< 1015
WE
1.291241106
1.2917106
< 0.1
(Pa)
Not A
2.29554105
2.1739105
5.3
$z z$
8.2 Remarks
The grid remains insufficient for a modeling in HEXA8.
8.3
Contents of the file resultsDisplacements and constraints.
8.4 Parameters
of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
11.61 seconds
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Code_Aster ${ }^{\circledR}$
Version

## 4.0

Titrate:
SSLV07 Stretching of a parallelepiped under its own weight

## Date:

21/01/98
Author (S):
X. DESROCHES

Key:
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9
Summary of the results

## Type of value

Reference
Aster
Aster
(m)

Hexa20
Hexa8
(A)
(B)

UB
0.

VB
0.

WB
1.721655106
< 0.1\%
< $0.1 \%$
CPU
0.

X
VC
0.

WC
1.707308106
< 0.1\%
$0.1 \%$
UD
1.721655107
< $0.1 \%$
-2.2\%
VD
0.

WD
1.434713108
-0.2\%
-15.5\%
EU
0.

VE
0.

WE
1.291241106
< $0.1 \%$
$<0.1 \%$
(Pa)
With
2.29554105
< $0.1 \%$
$-5.3 \%$
zz
E
1.14777105
< $0.1 \%$
< $0.1 \%$
zz
Modeling:
With (HEXA20 cutting: 3 in $\mathrm{Z}, 2$ in X and Y )
B (HEXA8 cutting: 12 in $\mathrm{Z}, 8$ in X and Y )
Modeling in HEXA8 would require a grid much finer.
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## Code_Aster ${ }^{\circledR}$

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3
Titrate:
SSLV100 Hollow roll in plane deformations
Date:
24/08/99
Author (S):

## X. DESROCHES

Key:
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Organization (S): EDF/IMA/MMN

## Handbook of Validation

## V3.04 booklet: Linear statics of the voluminal structures

Document: V3.04.100
SSLV100 - Hollow roll in plane deformations

## Summary:

This test makes it possible to validate the elements of plane deformation on the following functionalities:

- pressure distributed,
- matrix of rigidity,
- imposed displacements:
- by
ddl,
by face of element.
It includes/understands 4 modelings.
The 3 first correspond to elements of the different type (linear and quadratic).
The last validates the displacements imposed by face (blocking of the normal component).
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Code_Aster ${ }^{\circledR}$
Version
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Titrate:
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Date:
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Author (S):


## X. DESROCHES

Key:
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1
Problem of reference
1.1 Geometry
y
R
E
F
C

$$
\mathrm{B}=0.2 \mathrm{~m}
$$

Co-ordinates of the points:
With

B
C
D

## 1.3

## Boundary conditions and loadings

Internal pressure: $\mathrm{P}=60 \mathrm{MPa}$
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Version
3
Titrate:
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Date:
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Author (S):

## X. DESROCHES

Key:
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2
Reference solution
2.1

Method of calculation used for the reference solution
Analytical
2
has
=
$z z$
$2 P b 2-a 2$
2
2
has
B
=
1 -
$r r$
Pb2-a2R
2
has
$B$
$=P$
$1+$
b2-a2
$R$
2
$=$
$R$
0
$P$
$a 2$
$b 2$
$U$
$=$
$(1+1-2+$
$R$
$)($
$)$
$R$
$E b 2-a 2$
2
2

One obtains:

$$
\text { for } R=01
$$

$$
U=5.7210
$$

$$
\text { for } R=0.2
$$

$$
U=3.6410
$$

$R$
$R$
$=60$.

$$
=100
$$

$$
=40 .
$$

$$
=12
$$

$$
=12
$$

$R$ (
$\sin$ )
with:
$=0^{\circ}$ at points A and B ,
$=22.5^{\circ}$ at the points C and D,
$=45^{\circ}$ at the points E and F .
2.2

Results of reference
Displacements ( $\mathrm{U}, \mathrm{v}$ ) and forced ( $x x, y y$
, zz
, xy
) at the points A, B, C, D, E, F.

### 2.3 References

bibliographical
[1]
Y.C. FUNG. Fundations of solid mechanics. Prentice-hall, Inc. Englewood Cliffs. NJ. 1965
p. 243 to 245.

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Author (S):
X. DESROCHES

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3 Modeling
With
3.1

Characteristics of modeling: D-plan (QUAD4 + TRIA3)

## F

Normally blocked face

## y <br> D

Face with imposed pressure
E
C
$45^{\circ}$
X
With
B
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO: (GROUP_NO: bordAB
DY: 0.)
side EF
FACE_IMPO: (GROUP_MA: faceEF
DNOR: 0. )
pressure on face AE
PRES_REP: (GROUP_MA: faceAE
NEAR: 60.)
Names of the nodes:
With $=$ N23
B = N1
C $=$ N391
D = N369
$\mathrm{E}=\mathrm{N} 451$
$\mathrm{F}=751$

## 3.2

## Characteristics of the grid

A number of nodes: 759
A number of meshs and types: 704 TRIA3, 352 QUAD4

### 3.3 Functionalities

tested
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA

## DNOR

PRES_REP
GROUP_MA
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
POST_RELEVE
CHAM_GD
"EXTRACTION"
[U4.74.03]
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## Code_Aster ${ }^{\circledR}$

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Key:
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4
Results of modeling A
4.1 Values
tested
Place
Size
Reference
Aster
\% difference

## tolerance

With
U
5.72105
5.7155105
0.08

102
v
0 .
eps
$x x$
-60.
56.3770
6.04
yy
100.
96.3917
3.61
$z z$
12.
12.0044
0.04
xy
0.
0.9563

C
U
5.28459105
5.2832105
0.03

102
v
2.18895105
2.1777105
0.51
$x x$
36.56854
33.5312
8.31
yy
76.56854
76.9335
0.48
$z z$
12.
13.0207
8.51
xy
56.56854
53.7445
4.99

E
U
4.04465105
4.0400105
0.11

102
v
4.04465105
4.0400105
0.11
$x x$
20.
23.4926
17.46
yy
20.
25.4141
27.07
zz
12.
14.6720
22.27
xy
-80.
78.3081
2.11

B
U
3.640105
3.6405105
0.01

102
0. eps
$x x$
0 .
0.4064
yy
40.
39.8759
0.31
$z z$
12.
11.8408
1.33
xy
0 .
0.4447

D
U
3.36292105
3.3603105
0.08

102
V
1.39297105
1.3945105
0.11
$x x$
5.85786
5.2229
10.84
yy
34.14214
33.8961
0.72
$z z$
12.
11.7357
2.20
xy
14.14214
20.1797
0.90

### 4.2 Remarks

The increase in the error, when one passes from AB to CD then EF , is ascribable with the grid (density in elements QUAD4 lower than that in TRIA3).

### 4.3 Parameters

## of execution

Version: 3.03.32
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
8.54 seconds

## 5 Modeling

B

## Handbook of Validation

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## Code_Aster ®

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## X. DESROCHES

Key:
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5.1

Characteristics of modeling: D-plan (QUAD8 + TRIA6)
y
B
D
Face blocked in dx
F
With
Normally blocked face
C
Face with imposed pressure
E
$45^{\circ}$
X
Limiting conditions:
side AB
DDL_IMPO: (GROUP_NO: bordAB
DY: 0.)
side EF
FACE_IMPO: (GROUP_MA: faceEF
DNOR: 0.)
pressure on AE
PRES_REP: (GROUP_MA: faceAE
NEAR: 60.)
Names of the nodes:
With $=\mathrm{N} 2$
B = N48

## Characteristics of the grid

A number of nodes: 729
A number of meshs and types: 192 TRIA6, 96 QUAD8

### 5.3 Functionalities

## tested

Orders

## Keys

AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
POST_RELEVE
CHAM_GD
"EXTRACTION"
[U4.74.03]
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## Code_Aster ©

Version
3

## Titrate:

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## X. DESROCHES

Key:
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6
Results of modeling B
6.1 Values
tested
Place
Size
Reference
Aster
\% difference
tolerance
With
U
0.
eps
102
v
5.72105
5.7155105
0.04
xx
100.
99.7100
0.29
yy
-60.
59.7725
0.38
$z z$
12.
11.9813
0.16
xy
0 .

| 0.2643 |
| :--- |
| - |
| C |
| U |
| 2.18895105 |
| 2.1881105 |
| 0.04 |
| 102 |
| v |
| 5.28459105 |
| 5.2826105 |
| 0.04 |
| $x x$ |
| 76.56854 |
| 76.7005 |
| 0.17 |
| $y y$ |
| 36.56854 |
| 36.4500 |
| 0.32 |
| $z z$ |
| 12. |
| 12.0751 |
| 0.63 |
| $x y$ |
| 56.56854 |
| 56.2844 |
| 0.50 |
| E |
| U |
| 4.04465105 |
| 4.0432105 |
| 0.04 |
| 102 |
| v |
| 4.04465105 |
| 4.0432105 |
| 0.04 |
| $x x$ |
| 20. |
| 20.0083 |
| 4.104 |
| $y y$ |

20. 

19.9988
6.105
$z z$
12.
12.0021
2.104
xy
80.
79.8176
0.23

B
U
0.
eps
102
v
3.640105
3.6390105
0.03
$x x$
40.
39.9924
0.02
yy
0 .
0.001338
$z z$
12.
11.9973
0.02
xy
0 .
0.04083

D
U
1.39297105
1.3926105
0.03

102

```
V
3.36292 105
3.3619105
0.03
xx
34.14214
34.1361
0.02
yy
5.85786
5 . 8 9 4 8
0 . 6 3
zz
12.
12.0093
0.08
xy
14.14214
14.1596
0.12
F
U
2.57387105
2.5731 105
0.03
102
v
2.57387105
2.5731 105
0.03
xx
20.
20.0000
3.106
yy
20.
19.9996
2.105
zz
12.
11.9999
7.106
xy
```


## 20.

19.9975
1.104

### 6.2 Remarks

The evolution of the error induced by the grid according to $\mathrm{AB}, \mathrm{CD}$ or EF , is clearly attenuated by report/ ratio
with modeling A.

### 6.3 Parameters

of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
6.11 seconds

7 Modeling

## C

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
3
Titrate:
SSLV100 Hollow roll in plane deformations
Date:
24/08/99
Author (S):

## X. DESROCHES

Key:
V3.04.100-D Page:
8/12
7.1

Characteristics of modeling: D-plan (QUAD9)
F
Normally blocked face
y
D
E
C
$45^{\circ}$
X
With
B
Face with imposed pressure
Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO: (GROUP_NO: bordAB DY: 0. )
side EF
FACE_IMPO: (GROUP_MA: faceEF DNOR: 0. )
pressure on AE
PRES_REP: (GROUP_MA: faceAE NEAR: 60. )
Names of the nodes:
With $=$ N1
B = N47
C $=$ N351
D = N374
E = N569
$\mathrm{F}=\mathrm{N} 423$
7.2

Characteristics of the grid
A number of nodes: 725

A number of meshs and types: 168 QUAD9
7.3 Functionalities
tested
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS
[U4.23.01]
CALC_CHAM_ELEM
"SIGM_ELNO_DEPL"
[U4.61.01]
POST_RELEVE
CHAM_GD
"EXTRACTION"
[U4.74.03]
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
3
Titrate:
SSLV100 Hollow roll in plane deformations
Date:
24/08/99
Author (S):

## X. DESROCHES

Key:

```
V3.04.100-D Page:
9/12
8
Results of modeling C
8.1 Values
tested
Place
Size
Reference
Aster
% difference
tolerance
With
U
5 . 7 2 1 0 5
5 . 7 1 7 3 1 0 5
0 . 0 5
102
v
0.
eps
xx
-60.
56.8334
0 . 2 7
yy
100.
99.84
0 . 1 6
Zz
12.
12.0023
0.02
xy
0.
0.00272
C
U
5.28459105
5 . 2 8 2 1 1 0 5
0 . 0 5
```

-80.
79.837
0.2
B
U
3.640105
3.6386105
0.04
102
v
0 .
eps
$x x$
0.
2.7103
yy
40.
40.0011
4. 104
$z z$
12.
11.9995
4. 104
xy
0.
4.8104
D
3.36292105
3.3617105
0.04
102
v
1.39297105
1.3924105
0.04
xx
5.85786
5.8557
34.14214
34.143
2.5105
$z z$
12.
11.9996
3. 105
xy
14.14214
14.1435

1. 104

F
U
2.57387105
2.5729105
0.04

102
v
2.57387105
2.5729105
0.04
$x x$
20.
19.999
3. 105
yy
20.
20.0002

1. 105
$z z$
2. 

11.9999
9. 106
xy
-20.
20.0025
0.01

### 8.2 Remarks

The evolution of the error induced by the grid according to $\mathrm{AB}, \mathrm{CD}$ or EF , is clearly attenuated by report/ ratio
with modeling A.

```
8.3 Parameters
of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5.67 seconds
9 Modeling
D
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A
```


## Code_Aster ®

Version
3
Titrate:
SSLV100 Hollow roll in plane deformations
Date:
24/08/99
Author (S):
X. DESROCHES

Key:
V3.04.100-D Page:
10/12
9.1

Characteristics of modeling: D-plan (QUAD4 + TRIA3)
F
Normally blocked face
y
E
D
G
Face with displacement
normal imposed
C
H
$45^{\circ}$
X
With

## B

Face blocked out of Dy
Limiting conditions:
side AB
DDL_IMPO: (GROUP_NO: bordAB DY: 0. )
side EF
FACE_IMPO: (GROUP_MA: faceEF DNOR: 0. )
on AE
normal displacement imposed on $5.72 \mathrm{E}-5 \mathrm{~m}$
FACE_IMPO: (GROUP_MA: faceAE DNOR: -5.72 E-5)
Names of the nodes:
With $=$ N23
B = N1
C $=$ N391
D = N369
$\mathrm{E}=\mathrm{N} 451$
$\mathrm{F}=\mathrm{N} 751$
H = N92
$\mathrm{G}=\mathrm{N} 447$

## 9.2

Characteristics of the grid
A number of nodes: 759
A number of meshs and types: 704 TRIA3, 352 QUAD4

### 9.3 Functionalities

tested
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
FACE_IMPO
GROUP_MA
DNOR
PRES_REP
GROUP_MA
AFFE_MODELE
"MECHANICAL"
"D_PLAN"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS

POST_RELEVE

CHAM_GD
"EXTRACTION"
[U4.74.03]
CALC_NO
"REAC_NODA"
[U4.61.03]
10
Results of modeling D
10.1 Values
tested
Localization
Size

## Reference

Aster
\% difference
C
Fx
0.1360
0.14069
3.45

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
3
Titrate:
SSLV100 Hollow roll in plane deformations
Date:
24/08/99
Author (S):
X. DESROCHES

Key:
V3.04.100-D Page:
11/12
Fy
0.056

### 10.2 Remarks

One checks that the nodal forces of reaction are null in all the nodes, except on the nodes of line $A E, E F$ and $A B$.

### 10.3 Parameters

of execution
Version: 2.03.04
Machine: CRAY C90
System:
UNICOS 6.0
Obstruction memory:
6 megawords
Time CPU To use:
10.17 seconds

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
3
Titrate:
SSLV100 Hollow roll in plane deformations
Date:

Author (S):
X. DESROCHES

Key:
V3.04.100-D Page:
12/12
11
Summary of the results
D_plan
modeling
Summary of the errors
With
B
C
max in \%
Displacements
WITH, B
0.08
0.04
0.05

C, D
0.51
0.04
0.05

E, F
0.11
0.04
0.05

Constraints
WITH, B
6.04
0.29
0.27

C, D
10.84
0.17
0.32
xx
E, F
17.46
4.104
2.104

## Constraints

WITH, B
3.61
0.38
0.16

C, D
0.72
0.63
0.14
yy
E, F
27.07
2.105
5.5.104

Constraints
WITH, B
1.33
0.16
0.02

C, D
8.51
0.63
0.02
$z z$
E, F
22.27
2.104
2.104

Constraints
WITH, B

C, D
4.99
0.50
0.2
xy
E, F

## 0.2

These 3 modelings appreciably have the same number of nodes; results obtained with elements of order 1 (modeling A in Tria3 and Quad4) are definitely less precise, in particular on internal wall.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98
Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
1/8
Organization (S): EDF/IMA/MMN, SAMTECH
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.104
SSLV104 - Beam in rotation

## Summary:

This test makes it possible to validate the linear elastic design of a slim beam subjected to a rotation of one of its
ends. Three modelings are tested: elements 3D (HEXA20) and elements of COQUE_3D (QUAD9 and TRIA7). That tests the inertias of rotation, without taking account of the elementary terms of stiffening centrifuge (cf [V3.04.105]).
The reference solution is analytical (1D). The results coincide perfectly with the reference solution.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98

Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
2/8
1
Problem of reference

### 1.1 Geometry

Directed slim beam carried in space by the axis of directing vector $(1,1,1)$.
0.5 m

Z
0.02 m

Y
Embedded face
X
Square section of surface: 4.0104 m 2
Length of the beam: 0.5 m
1.2

Material properties
$\mathrm{E}=2.1011 \mathrm{~Pa}$
$=0$
$=7800 \mathrm{~kg} / \mathrm{m} 3$
A_CIS $=0.8333$ (factor of correction of transverse shearing equal to $5 / 6$ for a theory of the type
Thin Reissner hull)
1.3

## Boundary conditions and loadings

Free beam fixed in rotation around an axis perpendicular to its greater dimension and
passing by the center of the embedded face.
Component of the vector rotation: $(1,0,1)$.
Number of revolutions: $=3000 \mathrm{rd} / \mathrm{s}$.
The important value number of revolutions does not have anything physics.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98
Author (S):

## J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
3/8
2

## Reference solution

## 2.1

Method of calculation used for the reference solution
In the local reference mark of the beam:
$2 U x$
$+2 \mathrm{x}=0$ with $U()=$
0
0
2
X
$X$
E
$U x(L)=(L)=$
0
$X$
$x x$
By integrating the preceding differential equation one obtains, in the reference mark of the beam:
2
x3
$U(X)=$
X L2 -
$U=U 2=$
$X$
0
2nd
$y$
3
Displacements of all points of the beam are thus written in the total reference mark:
2
R 3
$U$
=
2 -
$X(X, Y, Z)$
R L
2 3rd

```
3
2
R 3
U
=
2-
y(X,Y,Z)
RL
2 3rd
3
2
R3
U
=
2-
Z(X,Y,Z)
RL
2 3rd
3
front
R
EC. =
X2+Y2+Z2
2.2
```

Results of reference
Values of three displacements in the center of the section furthest away from the axis of rotation.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98
Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
4/8
3 Modeling

## With

## 3.1 <br> Characteristics of modeling

Elements 3D (HEXA20)
Regulated grid including:
4 elements on the section

50 elements over the length

200 elements on the whole
0.5 m

Z
0.02 m

Y
Embedded face
X

## 3.2 <br> Characteristics of the grid

A number of nodes: 1521
A number of meshs and types: 200 HEXA20

### 3.3 Functionalities

tested

Orders

## Keys

AFFE_MODELE
AFFE
PHENOMENON
"MECHANICAL"
[U4.22.01]
MODELING
"3D"
AFFE_CHAR_MECA
DDL_IMPO
DX, DY, DZ
[U4.25.01]
ROTATION
CALC_MATR_ELEM
OPTION

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98
Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
5/8
4
Results of modeling A
4.1 Values
tested
Identification

## Reference

Aster
\% difference
DX in L
8.44103
8.44103
0.05

DY in L
8.44103
8.44103
0.04

DZ in L
8.44103
8.441030.04
4.2 Parameters
of execution
Version: 4.00.02
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
6 seconds
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98
Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
6/8
5 Modeling
B
5.1

Characteristics of modeling
Elements of hull MEC3QU9H
0.02

Grid including:
2 elements according to the dispatcher

4 elements according to the length
8
elements
with
total
0.5 m

Z
y
X
Embedded face
5.2

Characteristics of the grid
A number of nodes: 45
A number of meshs and types: 8 QUAD9

### 5.3 Functionalities

tested
Orders

## Keys

AFFE_MODELE
AFFE
PHENOMENON

### 6.1 Values

Identification
Reference
Aster
\% difference
DX in L
8.44103
8.44 E3
0.04
DY in L
8.44103
8.44 E30.04DZ in L8.44103
8.44 E3
0.04
6.2 Parameters
of execution
Version: 4.00.14

Machine: CRAY C90<br>Obstruction memory:<br>16 MW<br>Time CPU To use:<br>4.3 seconds<br>Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal structures<br>HI-75/98/040 - Ind A

Code_Aster ®
Version
4.0

Titrate:
SSLV104 Beam in rotation
Date:
07/12/98
Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
7/8
7 Modeling
C
7.1

Characteristics of modeling
Elements of hull MEC3TR7H
0.02

Grid including:
4 elements according to the dispatcher

8 elements according to the length
64
elements
with
total
0.5 m

Z
y
X
Embedded face
7.2

Characteristics of the grid

A number of nodes: 217
A number of meshs and types: 64 TRIA7
7.3 Functionalities
tested
Orders
Keys
AFFE_MODELE
AFFE
PHENOMENON
"MECHANICAL"
[U4.22.01]
MODELING
"3D"
AFFE_CHAR_MECA
DDL_IMPO
DX, DY, DZ
[U4.25.01]
ROTATION
CALC_MATR_ELEM
OPTION
"RIGI_MECA"
[U4.41.01]
CALC_VECT_ELEM
OPTION
"CHAR_MECA"
[U4.41.02]
RESO_LDLT
MATR_FACT
[U4.51.02]
8
Results of modeling C
8.1 Values
tested
Identification

## Reference

Aster
\% difference
DX in L
8.44103
8.44 E3
0.08

DY in L
8.44103

### 8.44 E 3

0.02

DZ in L
8.44103
8.44 E3
0.03

### 8.2 Parameters

of execution
Version: 4.00.14
Machine: CRAY C90
Obstruction memory:
16 MW
Time CPU To use:
7.6 seconds

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV104 Beam in rotation

## Date:

07/12/98
Author (S):
J.M. PROIX, P. MASSIN, A. LAULUSA

Key:
V3.04.104-B Page:
8/8
9

## Summary of the results

The coincidence of the results with the analytical solution makes it possible to validate the loading due
to the forces
of inertia of rotation.
Modeling COQUE_3D with MEC3QU9H gives the solution with very few elements.
One will refer to test SSLV105 [V3.04.105] to evaluate the effect of the stiffening centrifuges for the element 3D, HEXA20.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/98/040 - Ind A

## Code_Aster ®

## Version

4.0

Titrate:
SSLV105 Raidissement centrifuges of a beam in rotation
Date:
22/01/98
Author (S):

## J.M. PROIX

Key:
V3.04.105-A Page:
1/6
Organization (S): EDF/IMA/MMN
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures

## Document: V3.04.105

SSLV105 - Stiffening centrifuges of a beam

## in rotation

## Summary:

Test of Mechanics of the structures in linear static analysis.
The geometry is that of a slim beam subjected to a rotation around one of its ends. Only one modeling: elements 3D (HEXA20). One tests here the inertias of rotation (like test SSLV104) with taking into account of the centrifugal stiffening.
The reference solution (analytical) takes into account the term of additional rigidity due to rotation.
results are identical to the reference solution.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV105 Raidissement centrifuges of a beam in rotation
Date:
22/01/98
Author (S):
J.M. PROIX

Key:
V3.04.105-A Page:
2/6
1
Problem of reference
1.1 Geometry

The structure is made up of a directed slim beam carried in space by the axis of vector director $(1,1,1)$.
0.5 m

Z
Y
0.02 m

X
Embedded face
Square section of surface: 4.0104 m 2
Length of the beam: 0.5 m
1.2

Material properties
$\mathrm{E}=2.1011 \mathrm{~Pa}$
$=0$
$=7800 \mathrm{~kg} / \mathrm{m} 3$

## 1.3

## Boundary conditions and loadings

Free beam fixed in rotation around an axis perpendicular to its greater dimension and passing by the center of the embedded face.
Co-ordinates of the vector rotation: $(1,0,1)$.
Number of revolutions: $=3000 \mathrm{rd} / \mathrm{s}$.
The important value number of revolutions does not have anything physics.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV105 Raidissement centrifuges of a beam in rotation
Date:
22/01/98
Author (S):

## J.M. PROIX

Key:
V3.04.105-A Page:
3/6
2
Reference solution
2.1

Method of calculation used for the reference solution
In local reference mark of the beam: the equation relating to $U x$ displacement (without neglecting

## lengthening) is:

2Ux
$+2 X+U=0$
2
(
$X$ )
X
E
With the boundary conditions $U()=$
X 0
0
$U x(L)=(L)=0$
$X$
$x x$
2
One poses: =
E
By integrating the preceding differential equation one obtains, in the reference mark of the beam:
(
$\sin X)$
$U(X)=\cos ($

- X
$U=U=$
$X$
y
2
0
L)

The displacement of any points of the beam is thus written in the total reference mark:
$1 \sin R$
$U$
=
$X(X, Y, Z)$
()
$3 \cos$
$R$

$$
X 2+Y 2+Z 2
$$

$$
2.2
$$

## Results of reference

Values of three displacements in the center of the section furthest away from the axis of rotation.
2.3

Uncertainty on the solution
Without object (analytical solution).
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ®

Version
4.0

Titrate:

## SSLV105 Raidissement centrifuges of a beam in rotation

Date:
22/01/98
Author (S):
J.M. PROIX

Key:
V3.04.105-A Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling
Elements 3D (HEXA20)
Regulated grid including:
4 elements on the section
50
elements
on
length
200
elements
with
total
0.5 m

Z
Y
0.02 m

X
Embedded face

## 3.2

Characteristics of the grid
A number of nodes: 1521
A number of meshs and types: 200 HEXA20
3.3 Functionalities
tested

## Orders

## Keys

AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
ROTATION
CALC_MATR_ELEM

RIGI_ROTA<br>[U4.41.01]<br>Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal structures<br>HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV105 Raidissement centrifuges of a beam in rotation
Date:
22/01/98
Author (S):
J.M. PROIX

Key:
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Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference
DX in L
8.75103
8.75103

0
DY in L
8.75103
8.75103

0
DZ in L
8.75103
8.75103

0
4.2 Parameters
of execution
Version: 3.5.27
Machine: CRAY C90
Obstruction memory:

## 8 MW

Time CPU To use:
7 seconds
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HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

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Date:
22/01/98
Author (S):

## J.M. PROIX

Key:
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Summary of the results
Correct operation of option RIGI_ROTA. To note the increase in axial displacement by report/ratio with the case without stiffening (SSLV104 [V3.04.104]).
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Date:
23/09/02
Author (S):
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Organization (S): EDF/AMA

# Handbook of Validation <br> V3.04 booklet: Linear statics of the voluminal structures <br> Document: V3.04.109 

## SSLV109-Full cylinder in nonuniform pressure mode 1

## Summary:

This test validates all the elements of Fourier (triangles and quadrangles of degrees 1 and 2) in elasticity.
functionalities are as follows:

- variable pressure in space,
- imposed displacements,
- matrices of rigidity Fourier mode 1,
- forced with the nodes Fourier mode 1,
- recombination of Fourier on displacements and constraints (modeling A),
- transverse isotropic material (modeling F).

The test has a quadratic analytical solution in displacements.
The interest of the test lies in:

- the comparison between solution calculated and analytical solution on the various finite elements, $\cdot$ the comparison between the results and Code PERMAS on elements TRIA6 (modeling A).

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## 1

Problem of reference

### 1.1 Geometry

$L=12$
With
C
$p=R$
Z
$R=1$
R
B
D

The modelled field is ACDB (plane $=0$ ).

## 1.2

Material properties
$E=72 \mathrm{~N} / \mathrm{m} 2$
$=0.3$

## 1.3

## Boundary conditions and loadings

```
ur ()
With
= uz ()
With
\(=u q()\)
With
\(=0\)
uz (A)
B
\(=0\)
\(R\)
\(p=p\)
cos
R
with \(p=1\). and \(R=1\) applied in \(Z=12\)
```


### 1.4 Conditions initial

Without object for the static analysis.
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## 2 <br> Reference solution

## 2.1 <br> Method of calculation used for the reference solution

M
p
2
$U$
$=$
,
cos with
$+$
2
$R(R Z$
)
(ur Z)
(ur Z)
Z
R
2EI
$2 E R$
p
$U$
,
$=$
cos with
= -

Z (R Z
$v(R Z)$
$v(R Z)$
$r z$
2EI

Displacements are thus written here:

$$
\begin{aligned}
& U R, Z)= \\
& + \\
& v(R, Z)=- \\
& ( \\
& W R, Z)=
\end{aligned}
$$

$z z(R, Z)=-R$

## 2.2 <br> Results of reference

$U, v, W, z z$
in
$R=0 ., 0.5,1$.
$Z=0 ., 6 ., 12$.
$U, U, U$
R
Z
in $R=0 ., Z=6 ., 12 .,=45^{\circ}$

## 2.3 <br> Uncertainty on the solution

Analytical solution.

### 2.4 References <br> bibliographical

## [1]

PERMAS-HS. Axisymmetric Continued with arbitrary loads. Stuttgart 1985. INTES publication $n^{\circ} 224$ pp 42-49.

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Number of the nodes:
With $=N 1$
$B=N 3$
$C=N 13$

$$
\begin{aligned}
& D=N 15 \\
& E=N 7 \\
& F=N 8 \\
& G=N 9
\end{aligned}
$$

blocked node
With
E
C
face
blocked
F
$y(Z)$
in $Z$
B
G
D
$X(R)$

Limiting conditions:
DDL_IMPO:
(NODE: WITH DX $=0$.
$D Y=0$.
$D Z=0$.)

Pressure on the face CD: PRES_REP (GROUP_MA: Boils NEAR: p)
p being defined by AFFE_CHAR_MECA_F by $p(X)=X$

## 3.2 <br> Characteristics of the grid

A number of nodes: 15
A number of meshs and types: 4 TRIA6, 1SEG3 on segment CD

3.3 Functionalities<br>tested<br>Orders

AFFE_MODELE<br>MECHANICS<br>"AXIS_FOURIER"<br>ALL<br>DEFI_FONCTION<br>NOM_PARA<br>" $X$ "<br>AFFE_CHAR_MECA_F<br>DDL_IMPO<br>NODE<br>PRES_REP<br>GROUP_MA<br>CALC_MATR_ELEM<br>"RIGI_MECA"<br>MODE_FOURIER<br>CALC_CHAM_ELEM<br>"SIGM_ELNO_DEPL"

### 3.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM (by defect, it is regarded as being worth 0).
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## 4 <br> Results of modeling $A$

### 4.1 Values

tested
Node Size
Reference
Aster \%
difference

```
B
U
2.0833 10-3 2.0835
10-3 9.10-3
v
0.
DDL_IMPO
W
-2.0833 10-3
-2.0834 10-3 3.10-3
zz
-1. -1.000001
6.10-4
E
U 0.25
0.250001
5.10-4
v
O.
-4.10-6
W
0.25
0 . 2 5 0 0 0 0
2.10-4
zz
0.
-1.10-4
F
U 0.250521
0.250522
4.10-4
v
-0.04166 -0.041668 3.10-3
W
0.0249479
0.0249479
-6.10-3
```

```
zz
-0.5 -0.50005 1.10-2
G
U 0.252083
0.252084
3.10-4
v
-0.083333
-0.083333-7.10-3
W
0.2479170.247916
1.10-4
zz
-1. -1.
9.10-3
C
U 1. 1.0006
6.10-3
v
0.
-5.10-5
W
1.
1.00006
6.10-3
zz
0.
-1.1 10-3
D
U 1.00208
1.00215
7.10-3
v
-0.16666 -0.166691 1.5 10-2
W
0.99791 0.997981
```

-1. -0.999 0.1

### 4.2 Remarks

The analytical solution is found with a precision $<0.02 \%$ for displacements and $<0.1 \%$ for the constraints.

With a numerical formula of integration at 6 points of GAUSS (instead of 3) to calculate the stiffness, one would find the relation at 1010 close (like PERMAS).
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## 5 Modeling

B

## 5.1

Characteristics of modeling

Number of the nodes:
With $=N 1$
$B=N 3$
$C=N 13$

```
D = N15
E=N7
F=N8
G=N9
blocked node
C
With
E
y(Z)
face
blocked
F
in Z
B
G
D
X(R)
```

Limiting conditions:
DDL_IMPO:
(NODE: WITH DX $=0$.
$D Y=0$.
$D Z=0$.
face $A B$
(GROUP_NO: $A B D Y=0$.
Pressure on the face CD: PRES_REP (GROUP_MA: Boils NEAR: p)
$p$ being defined by $A F F E \_C H A R_{-} M E C A \_F$ by $p(X)=X$

## 5.2 <br> Characteristics of the grid

A number of nodes: 15
A number of meshs and types: 2 QUAD8, 1 SEG3 on segment CD

### 5.3 Functionalities

tested

## Orders

AFFE_MODELE MECHANICS "AXIS_FOURIER" ALL<br>DEFI_FONCTION<br>NOM_PARA<br>" $X$ "<br>AFFE_CHAR_MECA_F<br>DDL_IMPO<br>NODE<br>PRES_REP<br>GROUP_MA<br>CALC_MATR_ELEM<br>"RIGI_MECA"<br>MODE_FOURIER<br>CALC_CHAM_ELEM<br>"SIGM_ELNO_DEPL"<br>MODE_FOURIER

### 5.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM (by defect, it is regarded as being worth 0).

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## 6 <br> Results of modeling B

### 6.1 Values

tested

## Node Size Reference

Aster \%
difference

```
B
U
2.0833 10-3 2.0833
10-3 10.10-11
v
0.
DDL_IMPO
W
-2.0833 10-3
-2.0833 10-3
10.10-11
zz
-1. -1 10.10-11
E
U0.25
0.25
10.10-11
v
O.
-3.9 10-14
W
```

```
F
U 0.250521
0.250521
10.10-11
```

$v$
$-0.04166-0.0416610 .10-11$
W
0.0249479
0.0249479
10.10-11
zz
$-0.5-0.510 .10-11$
$G$
$U 0.252083$
0.252083
10.10-11
$v$
-0.08333-0.08333 10.10-11
W
0.2479170 .247917
10.10-11
$z z$
$-1 .-1.10 .10-10$
$C$
U 1.1.10.1011
$v$
0.
-4.3.10-14
W
1.

## D

U 1.00208
1.00208
10.10-11
$v$
-0.16666-0.16666 10.10-11
W
0.997910 .99791
10.10-11

## $z z$

-1. -1. 10.10-11

### 6.2 Remarks

The analytical solution is found with 10 or 11 significant figures.

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## 7 Modeling <br> C <br> 7.1 <br> Characteristics of modeling

## Number of the nodes:

With $=$ N1
$B=N 3$
$C=N 13$
$D=N 15$
$E=N 7$
$F=N 8$
$G=N 9$
blocked node
With
E
C
face
$y(Z)$
blocked
F
in Z
B
G
D
$X(R)$

Limiting conditions:
DDL_IMPO:
(NODE: WITH DX $=0$.
$D Y=0$.
$D Z=0$.)
face $A B$
(GROUP_NO: $A B D Y=0$.
Pressure on the face CD: PRES_REP (GROUP_MA: Boils NEAR: p)
$p$ being defined by $A F F E \_C H A R_{-} M E C A \_F$ by $p(X)=X$

## 7.2 <br> Characteristics of the grid

A number of nodes: 15
A number of meshs and types: 2 QUAD9, 1 SEG3 on segment CD

### 7.3 Functionalities

tested

## Orders

AFFE_MODELE<br>MECHANICS<br>"AXIS_FOURIER"<br>ALL<br>DEFI_FONCTION<br>NOM_PARA<br>" $X$ "<br>AFFE_CHAR_MECA_F<br>DDL_IMPO<br>NODE<br>PRES_REP<br>GROUP_MA<br>CALC_MATR_ELEM<br>"RIGI_MECA"<br>MODE_FOURIER<br>CALC_CHAM_ELEM<br>"SIGM_ELNO_DEPL"<br>MODE_FOURIER

### 7.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM (by defect, it is regarded as being worth 0).

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## 8

Results of modeling $C$

### 8.1 Values

tested
Node Size
Reference
Aster \%
difference

## B

U
2.0833 10-3 2.0833
10-3 10.10-11
$v$
0 .
DDL_IMPO
W
-2.0833 10-3
10.10-11

## $z z$

$$
-1 .-1 \text { 10.10-10 }
$$

## E

U 0.250 .2510 .1011
$v$
0 .
-3.9 10-14
W
0.25
0.25
10.10-11
$z z$
0 .
$-4.10-12$

U 0.250521
0.250521
10.10-11
$v$
-0.04166
-0.04166
10.10-11

W
0.0249479
0.0249479
10.10-11
$z z$
-0.5-0.5 10.10-11
G
U 0.252083
0.252083
10.10-11
$v$
$-1 .-1.10 .10-11$
$C$
U 1 .
1.
10.10-11

0 .
6.10-12

## D

U 1.00208
1.00208
10.10-11
$v$
-0.16666
-0.16666
10.10-11

W
0.99791
0.99791
10.10-11
$z z$
-1. -1. 10.10-11

### 8.2 Remarks

The analytical solution is found with 10 or 11 significant figures.

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## 9 Modeling

## D

## 9.1

## Characteristics of modeling

Number of the nodes:
With = N1
B = N1129
$C=$ N1369
$D=N 2169$
$E=N 141$
$F=N 705$
$G=N 1269$
blocked node
With
$E$
$C$
face
$y(Z)$
blocked
$F$
in Z
$B$
$G$
$D$
$X(R)$

Limiting conditions:

DDL_IMPO:
(NODE: WITH DX $=0$.
$D Y=0$.
$D Z=0$.
face $A B$
(GROUP_NO: $A B D Y=0$.)
Pressure on the face CD: PRES_REP (GROUP_MA: Boils NEAR: p)
$p$ being defined by $A F F E \_C H A R \_M E C A \_F$ by $p(X)=X$

## 9.2

Characteristics of the grid
A number of nodes: 2169
A number of meshs and types: 1920 QUAD4, 8 SEG 2 on segment CD

### 9.3 Functionalities

tested

Orders

## AFFE_MODELE

MECHANICS

DEFI_FONCTION<br>NOM_PARA<br>" $X$ "<br>AFFE_CHAR_MECA_F<br>DDL_IMPO<br>NODE<br>PRES_REP<br>GROUP_MA<br>CALC_MATR_ELEM<br>"RIGI_MECA"<br>MODE_FOURIER<br>CALC_CHAM_ELEM<br>"SIGM_ELNO_DEPL"<br>MODE_FOURIER

### 9.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM (by defect, it is regarded as being worth 0).
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10.1 Values

tested

Node Size<br>Reference<br>Aster \%<br>difference

$B$
$U$
2.0833
$10-3$
10.0919
10-3
0.41
$v$
0.

DDL_IMPO

## W

- 2.0833 10-3
- 2.0674 10-3
0.76
$z z$
-1. -1.00974
0.97


## E

U 0.250 .24980 .07
$v$
0.
-2.7.10-10
W
0.25
0.24980 .07
$z z$
0. 0.0090
0.90

```
F
U 0.250521
```

0.250347
0.07
$v$
-0.04166
-0.04164
0.07
W
0.0249479
0.02493170 .06
$z z$
-0.5-0.51005
2.01
G
U 0.252083
0.251911
0.07
$v$
-0.083333
-0.083273
0.07
W
0.247917
0.2477520 .07
$z z$
-1. -1.0103
1.03
C
U 1.
0.99927
0.07
$v$
0.
-2.3.10-7
W
1.
0.999280 .07
0. 0.0088

D
U 1.00208
1.001357
0.07
$v$
-0.16666
-0.16653
0.08

W
0.99791
0.9972080 .06
$z z$
-1. -1.0027
0.27

### 10.2 Remarks

To obtain a precision of about $1 \%$ on the constraints, it is necessary to model structure very finely (8 elements radially and 240 axially).

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## 11 Modeling

E

### 11.1 Characteristics of modeling

Number of the nodes:
With $=N 1$
B $=$ N2421
$E=N 121$
$F=N 1331$
$G=N 2541$
blocked node
With
E
$y(Z)$
face
blocked
F
in Z
B
G
$X(R)$

Limiting conditions:

DDL_IMPO:
(NODE: WITH DX $=0$.
$D Y=0$.
$D Z=0$.)
face $A B$
(GROUP_NO: $A B D Y=0$.
Pressure on face EG: PRES_REP (GROUP_MA: Boils NEAR: p)
p being defined by AFFE_CHAR_MECA_F by $p(X)=X$

### 11.2 Characteristics of the grid

A number of nodes: 2541
A number of meshs and types: 4800 TRIA3, 20 SEG2 on segment EG

### 11.3 Functionalities

tested

## Orders

AFFE_MODELE<br>MECHANICS<br>"AXIS_FOURIER"<br>ALL<br>DEFI_FONCTION<br>NOM_PARA<br>" $X$ "<br>AFFE_CHAR_MECA_F<br>DDL_IMPO<br>NODE<br>PRES_REP<br>GROUP_MA<br>CALC_MATR_ELEM<br>"RIGI_MECA"

### 11.4 Remarks

To decrease the number of nodes, one modelled the structure for $y 6$.
The precision on the results is nevertheless less than for elements QUAD4.
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## 12 Results of modeling $E$

### 12.1 Values

tested

## Node Size

Reference
Aster \%
difference

## B <br> $U$

### 2.0833 10-3 2.0228

10-3
2.90
$v$
0.

DDL_IMPO
W
-2.0833 10-3
-2.1143 10-3
1.49
$z z$
-1. -1.0078
0.78

E
U 0.250 .249490 .20
$v$
0.
-2.4 10-5

W
0.25
0.249500 .20
$z z$
0.
-5.3 10-2

```
F
U 0.250521
0 . 2 5 0 0 3 0 . 1 9
v
-0.04166 -0.04161
0 . 1 3
W
0.2494790.24900 0.19
```

$z z$
-0.5-0.49738
0.52

### 12.2 Remarks

The precision on displacements is lower than 3\%, that on the constraints lower than $\mathbf{2 \%}$.
On this example, the TRIA3 converge definitely less quickly than the QUAD4 towards the solution exact.

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## 13 Modeling

F

### 13.1 Characteristics of modeling

Number of the nodes:
With $=N 1$
$B=N 3$
$C=N 13$

$D=N 15$
$E=N 7$
$G=N 9$
Number of the nodes:
With = N1
$B=N 3$
$C=N 13$
$D=N 15$
$E=N 7$
$G=N 9$

## blocked node

With
N4
E
N12
C
$y$ (Z)
face
blocked N2
N14
in Z
B
G
N12
D
$X(R)$

## Limiting conditions:

DDL_IMPO:
(NODE: WITH DX $=0$.
$D Y=0$.
$D Z=0$.
face $A B$
(GROUP_NO: $A B D Y=0$.
Pressure on the face CD: PRES_REP (GROUP_MA: Boils NEAR: p)
13.2 Characteristics of the grid

A number of nodes: 15
A number of meshs and types: 2 QUAD8, 1SEG3 on segment CD

13.3 Functionalities<br>tested<br>Orders

AFFE_MODELE<br>MECHANICS<br>"AXIS_FOURIER"<br>ALL<br>DEFI_MATERIAU<br>ELAS_ORTH<br>DEFI_FONCTION<br>NOM_PARA<br>" $X$ "<br>AFFE_CHAR_MECA_F<br>DDL_IMPO<br>NODE<br>PRES_REP<br>GROUP_MA<br>CALC_MATR_ELEM<br>"RIGI_MECA"<br>MODE_FOURIER<br>CALC_CHAM_ELEM<br>"SIGM_ELNO_DEPL"<br>MODE_FOURIER

### 13.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM (by defect, it is regarded as being worth 0).
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SSLV109-Full cylinder in pressure nonuniform mode 1

## Date:

23/09/02
Author (S):
X. DESROCHES Key
:

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14 Results of modeling $F$

### 14.1 Values

tested

Node Size<br>Reference<br>Aster \%<br>difference<br>\section*{N2}<br>U 2.6041666<br>2.6041666<br>1.10-9<br>W<br>-2.6041666-2.6041666<br>1.10-9<br>With<br>$z z$

```
0.
-1.10-13
B
zz
-1. -1. 3.10-10
N4
U 0.0625 0.0625 4.1010
W
0 . 0 6 2 5
0 . 0 6 2 5
4.10-10
E U
0.25
0 . 2 5
3.10-10
W
0 . 2 5
0 . 2 5
3.10-10
zz
0.
-1. 10-14
Gv
-0.083333-0.083333
2.10-10
zz
-1. -1. 2.10-11
N10 U
0.5625 0.5625 2.10-10
W
0.5625 0.5625 2.10-10
N12v
-0.125 -0.125 1.10-10
CU
1. 1. 1.10-10
W
1. 1. 1.10-10
```

$z z$
0.
7.10-14

N14 v
-0.083333-0.083333 9.10-11
D v
-0.166666-0.166666 9.10-11
$z z$
-1. -1. 1.10-11

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Titrate:
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15 Summary of the results
The elements of order 2 give the analytical solution.
The elements of order 1 converge slowly towards the solution and require very fine grids.
Times calculations remain however reasonable.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
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8.2

# Titrate: <br> SSLV110-Elliptic crack in an infinite medium 

Date:<br>15/02/06<br>Author (S):<br>E. GALENNE, X. DESROCHES Key<br>V3.04.110-C Page:<br>1/14<br>Organization (S): EDF-R \& D /AMA

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.110

SSLV110-Elliptic crack in an infinite medium

## Summary:

It is about a test in statics for a three-dimensional problem. This test makes it possible to calculate the rate of refund
of energy total and local on the bottom of crack by the method.

The rays of the crowns of integration are variable along the crack, and the rate of refund of energy room is calculated according to 2 different methods (LEGENDRE and LAGRANGE).

The interest of the test is the validation of the method in 3D and the following points:
comparison between the results and an analytical solution,
stability of the results according to the crowns of integration, comparison between 2 methods different for calculation from G local,

2 cases of equivalent loadings (pressure distributed and voluminal loading).
This test contains 4 different modelings.
The $3 r d$ modeling tests the derivative of $G$ compared to the parameters material and loading.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
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Code_Aster ${ }^{\circledR}$
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8.2

Titrate:
SSLV110-Elliptic crack in an infinite medium

## Date:

15/02/06
Author (S):
E. GALENNE, X. DESROCHES Key
:
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## 1

Problem of reference

### 1.1 Geometry

It is about an elliptic crack plunged in a presumedly infinite medium. Only one eighth is modelled
of a parallelepiped:

```
y
P
Z
X
120 mm
mm
m
0
1250 m
25 mm
725 mm
```

0: melts of elliptic crack

### 1.2 Properties

materials
$E=210.000 . \mathrm{MPa}$
$=0.3$
1.3

Boundary conditions and loadings
Symmetry compared to the 3 principal plans:
$U x=0$. in the plan $X=0$.
$U Y=0$. in the plan $Y=0$.
$U Z=0$. in the plan $Z=0$. out of the crack
The conditions of loadings are is:
$P=1$ MPa in the plan $Z=1250 \mathrm{~mm}$ (modelings $A$ and $B)$
that is to say:
$F Z=8.104 \mathrm{~N} / \mathrm{mm} 3$ on all the elements of volume (loading are equivalent to the precedent) (modelings $C$ and D).
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## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

The reference solution is an analytical solution resulting from SIH [bib1] and [bib2].

It is noted that the angle indicates here the parametric angle of the point $M$ (angle compared to axis OX of projected $M$ on the circle of radius $b$ ) and not the polar co-ordinate of this point.

## B

E(K)
a2

## has

2
/2
1/2
$E(K)=(1-k 22$
$\sin ) D$
0

Here: $=\mathbf{2 5} \mathbf{m m ~ B}=\mathbf{6} \mathbf{~ m m}$ have, therefore $K=0,9707728$
The values of the elliptic integrals $E(K)$ are tabulées in [bib3], according to asin (K) which is worth here $\mathbf{7 6 , 1 1}$. One finds then: $E(K)=1,0672$.
14
$\boldsymbol{B}^{2}$
From where the factor of intensity of the constraints in MPa.mm: $K()=$ 0680

4
$\sin ^{2}+$
$\cos ^{2}$
${ }^{2}$ has
$\left(-^{2}\right)$

Then, starting from the formula of Irwin (plane deformation): $G()=$

## KI() 2

$\boldsymbol{E}$

The total rate of refund of Gref energy is calculated by integration of $G$ (): Gref $=5,76.10-3$ J/mm.

Derived from G (modeling C):
For the derivative of $G$ compared to the Young modulus $E$, one can write:
$G$

G
$\boldsymbol{G}=$
(with =
,
302 4) thus
= -
$\boldsymbol{E}$
$\boldsymbol{E}$
$\boldsymbol{E}$
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In addition, while varying the Fz loading, one finds:

### 2.2 Bibliography

[1]
G.C. SIH: Mathematical Theories of Brittle Fractures - FRACTURE, flight II - Academic Press 1968
[2]
M.K. KASSIN and G.C. SIH: Three-dimensional stress distribution around year elliptical ace under arbitrary loadings J. Appl. Mech., 88, 601-611, 1966.
[3]
H. TADA, P.PARIS, G. IRWIN: The Stress Analysis of Cracks Handbook - Third Edition International ASM - 2000
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SSLV110-Elliptic crack in an infinite medium

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Author (S):

# E. GALENNE, X. DESROCHES Key 

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## 3 Modeling

With

# 3.1 <br> Characteristics of modeling <br> With $=$ N01099 ( $S=0$. ) <br> $B=\operatorname{N01259}(S=26.68)$ <br> $C=\operatorname{N01179(S=17.8;~=/4)~}$ 

C1
C2
C3
With
C
Z
Y
B
$X$

Loading: Unit pressure distributed on the face of the block opposed to the plan of the lip:
$P=1 . M P a$ in the plan $Z=1250 . \mathrm{mm}$.

## 3.2

Characteristics of the grid
A number of nodes: 1716
A number of meshs and types: 304 PENTA15 and 123 HEXA20

### 3.3 Functionalities

tested
Orders

DEFI_FOND_FISS LEVRE_SUP
GROUP_MA ALL

CALC_THETA FOND_3D

THETA_3D

CALC_G_THETA_T RESULT
TOUT_ORDRE

## NUME_ORDRE

LIST_ORDRE

CALC_G_LOCAL_T
"THETA_LEGENDRE"
DEGREE $=7$
"G_LEGENDRE"
$R_{-} I N F_{-} F O / R_{-} S U P_{-} F O$

AFFE_CHAR_MECA FORCE_FACE

### 3.4 Remarks

The degree of the polynomials of LEGENDRE used to calculate $G(S)$ is 7 (maximum value Aster).
For the 3 crowns of integration, rays $R_{-} I N F$ and $R_{-} S U P$ vary linearly along the bottom of fissure.

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures

## 4 <br> Results of modeling $A$

### 4.1 Values <br> tested

The values tested are:
the total rate of refund of energy $G$,
the rate of refund of energy room $G$ in all the nodes of the bottom of crack.
The grid includes/understands only one of the lips of the crack, it is thus necessary to use key word "SYME_CHAR"
automatically to multiply by 2 in calculation Aster the rate of refund of energy calculated by virtual extension of the single lip.

In the same way, $G$ total calculated here corresponds to the quarter of $G$ of reference defined previously, only one eighth of parallelepiped being represented.

## Identification Reference

Aster \%
difference

## G Crown Cl

1.44 10-3 1.410

10-3
-2.1
G Crown C2
1.44 10-3 1.451

10-3
0.8

G Crown C3
1.44 10-3 1.424

10-3
-1.1

G (A) crowns C1 7.171
10-5
6.829 10-5
-4.8
$G(A)$ crowns $C 27.171$
10-5
7.239 10-5
0.95

G (A) crowns C3 7.171
10-5
6.864 10-5
-4.3
$G(B)$ crowns C1 1.721
10-5
1.48 10-5
-13.8
$G(B)$ crowns $C 21.721$
10-5
1.57 10-5
-8.7
$G(B)$ crowns C3 1.721
10-5
1.90 10-5

G(C) crown C1 5.215
10-5
4.992 10-5
-4.3
G (C) crown C2 5.215
10-5
5.124 10-5
-1.7
G (C) crown C3 5.215
10-5
5.013 10-5
-3.9

### 4.2 Notice

The results are rather stable between the crowns safe at the point $B$ where the variation of $G(S)$ is more large and results far away from the reference solution. One can explain this variation by the grid of poor quality.

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures
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## Code_Aster ${ }^{\circledR}$

Version
8.2

## Titrate:

SSLV110 - Elliptic crack in an infinite medium

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15/02/06
Author (S):
E. GALENNE, X. DESROCHES Key

## 5 Modeling <br> B

## 5.1

Characteristics of modeling
With $=$ N01099 $(S=0$.
$B=\operatorname{N01259}(S=26.68)$
$C=\operatorname{N01179(S=17.8)}$
C1
C2
C3
With
C
Z
Y
B
X

Loading: Unit pressure distributed on the face of the block opposed to the plan of the lip:
$P=1$ MPa in the plan $Z=1250$ Misters.

## 5.2

## Characteristics of the grid

A number of nodes: 1716
A number of meshs and types: 304 PENTA15 and 123 HEXA20

### 5.3 Functionalities

tested
Orders

DEFI_FOND_FISS LEVRE_SUP
GROUP_MA

CALC_THETA FOND_3D

THETA_3D

CALC_G_THETA_T

CALC_G_LOCAL_T"THETA_LAGRANGE"
"G_LEGENDRE"
DEGREE $=4$

R_INF_FO/R_SUP_FO
$A F F E \_C H A R \_M E C A$ FORCE_FACE

### 5.4 Remarks

"THETA_LAGRANGE": the field is discretized starting from the functions of forms of the nodes of the bottom
of crack, but $G(S)$ is always discretized starting from the polynomials of LEGENDRE.
The degree of the polynomials of LEGENDRE used to calculate $G(S)$ is 4 [R7.02.01].
For the 3 crowns of integration, rays R_INF and R_SUP vary linearly along the bottom of fissure.
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## 6

Results of modeling B

### 6.1 Values

tested
The values tested are:
the total rate of refund of energy $G$,
the rate of refund of energy room $G$ in all the nodes of the bottom of crack.
The grid includes/understands only one of the lips of the crack, it is thus necessary to use key word "SYME_CHAR"
automatically to multiply by 2 in calculation Aster the rate of refund of energy calculated by virtual extension of the single lip.

In the same way, $G$ total calculated here corresponds to the quarter of $G$ of reference defined previously, only one eighth of parallelepiped being represented.

## Identification Reference

Aster \%
difference

## G Crown C1

1.44 10-3 1.410

10-3
-2.1
G Crown C2
1.44 10-3 1.451

### 6.2 Notice

The results are better than in modeling $A$ at the point B, but the disparity between crowns remains strong.

## Handbook of Validation

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Code_Aster ${ }^{\circledR}$
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Titrate:
SSLV110-Elliptic crack in an infinite medium

## Date:

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Author (S):
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## 7 Modeling

C

## 7.1

Characteristics of modeling
With $=$ N01099 $(S=0$.
$B=\operatorname{N01259}(S=26.68)$
$C=\operatorname{N01179}(S=17.8)$
C1
C2
C3
With
C
$Z$
$\boldsymbol{Y}$
$X$

Loading: Voluminal force FZ equivalent to a unit pressure on the face of the block opposed to plan of the lip:

FORCE_INTERNE: $F Z=8.104 \mathrm{~N} / \mathrm{mm} 3$ on all the elements of volume .
7.2

Characteristics of the grid
A number of nodes: 1716
A number of meshs and types: 304 PENTA15 and 123 HEXA20

### 7.3 Functionalities

tested
Orders

DEFI_FOND_FISS LEVRE_SUP
GROUP_MA
CALC_THETA FOND_3D

THETA_3D

CALC_G_THETA_T SENSITIVITY

CALC_G_LOCAL_T
"THETA_LEGENDRE"
DEGREE $=7$
"G_LEGENDRE"

# AFFE_CHAR_MECA FORCE_INTERNE 

### 7.4 Remarks

The degree of the polynomials of LEGENDRE used to calculate $\boldsymbol{G}(S)$ is 7 (maximum value Aster).
For the 3 crowns of integration, rays R_INF and R_SUP vary linearly along the bottom of fissure.
Handbook of Validation
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SSLV110-Elliptic crack in an infinite medium

Date:
15/02/06
Author (S):
E. GALENNE, X. DESROCHES Key

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## 8

Results of modeling $C$

### 8.1 Values

tested
The values tested are:
the total rate of refund of energy $G$,
the rate of refund of energy room $G$ in all the nodes of the bottom of crack,
the derivative of $\boldsymbol{G}$ compared to $\boldsymbol{E}$ and to the loading in voluminal force $\boldsymbol{F z}$.
The grid includes/understands only one of the lips of the crack, it is thus necessary to use key word "SYME_CHAR"
automatically to multiply by 2 in calculation Aster the rate of refund of energy calculated by virtual extension of the single lip.

In the same way, G total calculated here corresponds to the quarter of G of reference defined previously, only one eighth of parallelepiped being represented.

Identification Reference
Aster \%
difference

## G Crown C1

1.44 10-3 1.437

10-3
-0.2
G Crown C2
1.44 10-3 1.479

10-3
2.7

G Crown C3
1.44 10-3 1.450

10-3
0.7

G (A) crowns C1 7.171
10-5 6.962
10-5-2.9
G (A) crowns C2 7.171
10-5 7.379
10-5 +2.9
G (A) crowns C3 7.171

# G (B) crowns C1 1.721 

10-5 1.509
10-5-12.2
G (B) crowns C2 1.721
10-5 1.598
10-5 -7.1
G (B) crowns C3 1.721
10-5 1.629
10-5-5.2

G (C) crown C1 5.215
10-5 5.085
10-5-2.5
G (C) crown C2 5.215
10-5 5.219
10-5 0.1
G (C) crown C3 5.215
10-5 5.107
10-5-2.1

DG/dE crowns C1
-6.8610 10-9 -6.842
10-9
-0.2
DG/dE crowns C2
-6.8610 10-9 -7.041
10-9
2.7

DG/dE crowns C3
-6.8610 10-9 -6.907
10-9
0.7

DG/dFz crowns C1 3.5993 .592
-0.1
DG/dFz crowns C2 3.5993 .697
2.7

DG/dFz crowns C3 3.5993 .629
0.9

### 8.2 Notice

The results are rather stable between the crowns. One always notes worse results with node $B$.
The errors on the derivative of $G$ are comparable with those on $G$.

## Handbook of Validation

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Titrate:
SSLV110-Elliptic crack in an infinite medium

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Author (S):
E. GALENNE, X. DESROCHES Key

$$
:
$$

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## 9 Modeling <br> D

9.1

Characteristics of modeling
With $=$ N01099 $(S=0$.
$B=\operatorname{N01259}(S=26.68)$

C
Z

B
$X$

Loading: Voluminal force Fz equivalent to a unit pressure distributed on the face of the block opposed to the plan of the lip:

FORCE_INTERNE: $F Z=8.104 \mathrm{~N} / \mathrm{mm} 3$ on all the elements of volume .

## 9.2

Characteristics of the grid
A number of nodes: 1716
A number of meshs and types: 304 PENTA15 and 123 HEXA20
9.3 Functionalities
tested
Orders

DEFI_FOND_FISS LEVRE_SUP
GROUP_MA
CALC_THETA FOND_3D

THETA_3D

CALC_G_THETA_T

CALC_G_LOCAL_T"THETA_LAGRANGE"

# R_INF_FO/R_SUP_FO 

## AFFE_CHAR_MECA FORCE_INTERNE

### 9.4 Remarks

"THETA_LAGRANGE": the field is discretized starting from the functions of forms of the nodes of the bottom
of crack, but $\boldsymbol{G}(S)$ is always discretized starting from the polynomials of LEGENDRE.
The degree of the polynomials of LEGENDRE used to calculate $\boldsymbol{G}(S)$ is 7.
For the 3 crowns of integration, rays $R_{-} I N F$ and $R_{-} S U P$ are supposed to vary linearly on
bottom of crack.

## Handbook of Validation

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Date:
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Author (S):
E. GALENNE, X. DESROCHES Key

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## 10 Results of modeling $D$

### 10.1 Values

tested

The values tested are:
the total rate of refund of energy $G$,
the rate of refund of energy room $G$ in all the nodes of the bottom of crack.
The grid includes/understands only one of the lips of the crack, it is thus necessary to use key word "SYME_CHAR"
automatically to multiply by 2 in calculation Aster the rate of refund of energy calculated by virtual extension of the single lip.

In the same way, $G$ total calculated here corresponds to the quarter of $G$ of reference defined previously, only one eighth of parallelepiped being represented.

Identification Reference
Aster \%
difference

## G Crown C1

1.44 10-3 1.437

10-3
-0.2
G Crown C2
1.44 10-3 1.479

10-3
2.7

G Crown C3
1.44 10-3 1.450

10-3
0.7

G (A) crowns C1 7.171

G (A) crowns C2 7.171
10-5 7.597
10-5 5.9
G (A) crowns C3 7.171
10-5 7.575
10-5 5.7

G (B) crowns C1 1.721
10-5 1.636
10-5 -4.9
G (B) crowns C2 1.721
10-5 1.992
10-5-1.7
G (B) crowns C3 1.721
10-5 1.734
10-5 0.7

G (C) crown C1 5.215
10-5 5.071
10-5-2.7
G (C) crown C2 5.215
10-5 5.192
10-5 0.4
G (C) crown C3 5.215
10-5 5.108
10-5-2.1

### 10.2 Notice

The results are better than in modeling $\boldsymbol{C}$ at the point $\boldsymbol{B}$.

## Handbook of Validation

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11 Summary of the results
Calculation of G local:

2 methods (LEGENDRE and LAGRANGE) give the same results appreciably (less than 5\% of error compared to the analytical solution) except at the point $\boldsymbol{B}$ (not end ellipse on the large axis) where the Lagrange method is most precise,
loading case: the values obtained with the voluminal loading are slightly higher than those obtained with imposed constraints (including for the values of G). The differences are tiny and due to numerical integrations different on the term from volume and the term of edge.

Calculation of derived from $G$ :
the errors on the derivative of $G$ are weak and comparable with those on $G$.
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SSLV110-Elliptic crack in an infinite medium

Date:<br>15/02/06<br>Author (S):<br>E. GALENNE, X. DESROCHES Key<br>:<br>V3.04.110-C Page:<br>14/14

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Version
4.0

Titrate:
SSLV111 Estimator of error on a perforated plate
Date:
26/01/98
Author (S):

## X. DESROCHES

Key:
V3.04.111-A Page:
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Organization (S): EDF/IMA/MMN
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.111
SSLV111 - Estimator of error on a plate
perforated in linear elasticity
Summary:
This test validates and compares the 2 versions of the estimator of error of Zhu-Zienkiewicz (version 1 of 1987, noted
ZZ1, and version 2 of 1992, noted ZZ2) applied to the system of linear elasticity, in statics.
It comprises 5 modelings in plane constraints, corresponding each one to a type of finite element (TRIA3,
QUAD4, TRIA6, QUAD8, QUAD9).
The analytical solution is known and makes it possible to compare the errors estimated with the exact error.
The interest of the test resides:

- in the comparison enters the constraints smoothed with ZZ1 (continuous total smoothing) and ZZ2
(smoothing
room with patchs of elements),
- in the comparison of the estimators between them,
- in the qualitative and quantitative analysis of the results (relative errors total and local).

The test highlights the good behavior of ZZ2 on all the types of elements and the bad results of
ZZ1 on quadratic elements when the solution does not present a strong singularity, which is the case. Handbook of Validation
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Version
4.0

Titrate:
SSLV111 Estimator of error on a perforated plate
Date:
26/01/98
Author (S):
X. DESROCHES

Key:
V3.04.111-A Page:
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1
Problem of reference

### 1.1 Geometry

$y$
$=1.0$
$=1.0$
B
C
X
With
D
E
has
()
y
B
C
With
$R$
$X$
E
D
1.0
3.0
(H)
1.2

Material properties
$\mathrm{E}=1.000 \mathrm{MPa}$
$=0.3$.
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1.3

Boundary conditions and loadings
On AB, $U$
=
0
On ED, $U=$
y
0
F
$=(X=4)$
$X$
$x x$
On CD tractions $F=(X=4)$
y
$x y$
$F=$
$=$
$X$
$x y($
$4)$.
On BC tractions $F=$
=
$y$
$y y(y$

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2
Reference solution

## 2.1

Method of calculation used for the reference solution
One considers a portion of an infinite plate with a circular central hole, subjected to a loading
\&
\&
unit one-way in the direction $O X=1 E E$
$X$
$X$.
The analytical solution of this problem is [bib1]:
= -
$1 \cos ($
3 A
2
yy
) $\cos ($
4 )
$\cos$
4 )
r2
2
$-2 r 4$
2
4
has
= -
$1 \sin ($
3 A
2
$+$
$x y$
) $\sin ($
$4)$
$\sin ($
4 )
r2
2
$+2 r 4$
where:
$A$ is the ray of the hole,
$(R$,$) polar co-ordinates.$
2.2

Uncertainty on the solution
Analytical solution.

### 2.3 References <br> bibliographical

[1]
Zhu-Zienkiewicz: The superconvergent patch recovery and a posteriori error estimates -
1 leaves: the technical recovery (Int. J. for Num. Methods in Engineering vol. 33, p. 1355 (May 1992)).
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
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3 Modeling
With

## 3.1

Characteristics of modeling

## 3.2 <br> Characteristics of the grid

A number of nodes: 357 .
A number of meshs and types: 640 TRIA3.

### 3.3 Functionalities

tested

## Orders

Keys
MECA_STATIQUE
OPTION

# "SIEF_ELGA_DEPL" 

[U4.31.01]
CALC_ELEM
OPTION
"ERRE_ELEM_NOZ1"
[U4.61.02]
OPTION
"ERRE_ELEM_NOZ2"
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## 4

Results of modeling A

### 4.1 Values

tested
Identification

## Reference

Aster
\% difference

## tolerance

To $x x$ ZZ1
3.
2.823
5.91
0.1
xx ZZ2
3.
2.884
3.85
0.1
yy ZZ1
0.
0.261
0.3
yy ZZ2
0 .
0.207
0.3
xy ZZ1
0.
7.4103
0.1
xy ZZ2
0.
6.1102
0.1

P $x x$ ZZ1
1.15625
1.152
0.37
0.1
xx ZZ2
1.15625
1.145
0.98
0.1
yy ZZ1
0.15625
0.150
3.81
0.1
yy ZZ2
0.15625
0.145
7.00
0.1
xy ZZ1
0.125
0.117

### 6.11

0.1
xy ZZ2
0.125
0.124
0.68
0.1

Net M1 eabs ZZ1
1.33104
1.103
eabs ZZ2
8.13105
1.103
erel ZZ1
6.63\%
0.1
erel ZZ2
4.05\%
0.1
eabs ZZ1
0.445102
0.424102
4.76
eabs ZZ2
0.445102
0.451102
$+1.31$
erel ZZ1
3.44\%
$3.28 \%$
erel ZZ2
3.44\%
$3.49 \%$
ZZ1
0.952

ZZ2
1.013

### 4.2 Remarks

= estimated error is an indication of effectivity of the estimator.
exact error

## Contents of the files results:

- absolute errors and relative total by the 2 methods, - maximum and minimal values of the constraints and the errors,
- lists of the meshs where the relative error is higher than $10 \%$.


### 4.3 Parameters <br> of execution <br> Version: 3.02.11 <br> Machine: CRAY C90

System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
19.0 seconds

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## Code_Aster ®

Version
4.0

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SSLV111 Estimator of error on a perforated plate
Date:
26/01/98
Author (S):

## X. DESROCHES

## Key:

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5 Modeling
B
5.1

Characteristics of modeling
5.2

Characteristics of the grid
A number of nodes: 357 .
A number of meshs and types: 320 QUAD4.
5.3 Functionalities
tested

## Orders

## Keys

MECA_STATIQUE
OPTION
"SIEF_ELGA_DEPL"
[U4.31.01]

CALC_ELEM<br>\section*{OPTION}<br>"ERRE_ELEM_NOZ1"<br>[U4.61.02]<br>OPTION<br>"ERRE_ELEM_NOZ2"<br>Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal structures<br>HI-75/96/017 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SSLV111 Estimator of error on a perforated plate
Date:
26/01/98
Author (S):
X. DESROCHES

Key:
V3.04.111-A Page:
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6
Results of modeling $B$
6.1 Values
tested
Identification

## Reference

Aster
\% difference
tolerance
To $x x$ ZZ1
3.
3.017
0.57
0.1
xx ZZ2
3.
2.971

- 0.95
0.1
yy ZZ1
0 .

```
0 . 1 7
```

0.3
yy ZZ2
0.
0.136
0.3
xy ZZ1
0 .
2.8103
0.1
xy ZZ2
0 .
1.04102
0.1
P $x x$ ZZ1
1.15625
1.168
0.98
0.1
$x x$ ZZ2
1.15625
1.153
0.26
0.1
yy ZZ1
0.15625
0.158
1.28
0.1
yy ZZ2
0.15625
0.152
2.83
0.1
xy ZZ1
0.125
0.121
2.99
0.1
xy ZZ2
0.125
0.124
0.94
0.1

Net M1 eabs ZZ1
1.57104
1.103
eabs ZZ2
2.40104
1.103
erel ZZ1
5.79\%
0.1
erel ZZ2
8.83\%
0.1
eabs ZZ1
0.320102
0.294102
8.1
eabs ZZ2
0.320102
0.307102
4.2
erel ZZ1
2.48\%
2.28\%
erel ZZ2
2.48\%
2.37\%

ZZ1
0.919

ZZ2
0.958

### 6.2 Remarks

$=$ estimated error is an indication of effectivity of the estimator.
exact error

## Contents of the files results:

- absolute errors and relative total by the 2 methods,
- maximum and minimal values of the constraints and the errors,
- lists of the meshs where the relative error is higher than $10 \%$.


### 6.3 Parameters

## of execution

Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
13.4 seconds

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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
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Date:
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Author (S):
X. DESROCHES

Key:
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7 Modeling
C
7.1

Characteristics of modeling
7.2

Characteristics of the grid
A number of nodes: 357 .
A number of meshs and types: 160 TRIA6.
7.3 Functionalities
tested
Orders
Keys
MECA_STATIQUE
OPTION
"SIEF_ELGA_DEPL"
[U4.31.01]
CALC_ELEM
OPTION

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## Code_Aster ©

Version
4.0

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Author (S):

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## Key:

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8
Results of modeling C
8.1 Values
tested
Identification
Reference
Aster
\% difference
tolerance
To $x x \mathrm{ZZ1}$
3.
2.975
0.83
0.1
xx ZZ2
3.
2.957
1.43
0.1
yy ZZ1
0.
6.86102
0.
7.52102
0.3
xy ZZ1
0.
3.12102
0.3
xy ZZ2
0.
0.155
0.3

P $x x$ ZZ1
1.15625
1.166
0.85
0.1
xx ZZ2
1.15625
1.153
0.25
0.1
yy ZZ1
0.15625
0.167
6.92
0.1
yy ZZ2
0.15625
0.153
1.87
0.1
xy ZZ1
0.125
0.127
1.52
0.1
xy ZZ2
0.125
0.124
0.58
0.1

Net M1 eabs ZZ1
1.81104
1.103
eabs ZZ2
2.92104
1.103
erel ZZ1
4.69\%
0.1
erel ZZ2
7.56\%
0.1
eabs ZZ1
0.152102
0.123102

19
eabs ZZ2
0.152102
0.167102
+9.9
erel ZZ1
1.17\%
$0.95 \%$
erel ZZ2
1.17\%
$1.29 \%$
ZZ1
0.810

ZZ2
1.099

### 8.2 Remarks

= estimated error is an indication of effectivity of the estimator.
exact error

## Contents of the files results:

- absolute errors and relative total by the 2 methods,
$\cdot$ maximum and minimal values of the constraints and the errors, - lists of the meshs where the relative error is higher than $10 \%$.


### 8.3 Parameters

## of execution

Version: 3.02.11

```
Machine: CRAY C90
System:
UNICOS }8.
Obstruction memory:
8 megawords
Time CPU To use:
10.4 seconds
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```


## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
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Date:
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Key:
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9 Modeling
D
9.1

Characteristics of modeling
9.2

Characteristics of the grid
A number of nodes: 277.
A number of meshs and types: 80 QUAD8.
9.3 Functionalities
tested
Orders
Keys
MECA_STATIQUE
OPTION
"SIEF_ELGA_DEPL"
[U4.31.01]
CALC_ELEM
OPTION
"ERRE_ELEM_NOZ1"
[U4.61.02]

## OPTION

"ERRE_ELEM_NOZ2"
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Key:
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10
Results of modeling $\mathbf{D}$
10.1 Values
tested
Identification
Reference
Aster
\% difference
tolerance
To $x x$ ZZ1
3.
3.063
2.11
0.1
xx ZZ2
3.
3.037
1.24
0.1
yy ZZ1
0 .
0.101
0.3
yy ZZ2
0.
2.47102
0.3
xy ZZ1
0 .
5.8103
0.3
xy ZZ2
0 .
2.41102
0.3

P $x x$ ZZ1
1.15625
1.170
1.19
0.1
xx ZZ2
1.15625
1.153
0.29
0.1
yy ZZ1
0.15625
0.162
3.54
0.1
yy ZZ2
0.15625
0.153
1.87
0.1
xy ZZ1
0.125
0.124
1.09
0.1
xy ZZ2
0.125
0.124
0.84
0.1

Net M1 eabs ZZ1
6.1105
1.103
eabs ZZ2
2.1104

### 1.103

erel ZZ1
1.45\%
0.1
erel ZZ2
$5.01 \%$
0.1
eabs ZZ1
9.01104
2.90104
+67.9 (!
eabs ZZ2
9.01104
8.88104
1.5
erel ZZ1
$0.697 \%$
0.22\%
erel ZZ2
0.697\%
0.687\%

ZZ1
0.321

ZZ2
0.985

### 10.2 Remarks

$=$ estimated error is an indication of effectivity of the estimator
exact error

## Contents of the files results:

- absolute errors and relative total by the 2 methods,
$\cdot$ maximum and minimal values of the constraints and the errors,
- lists of the meshs where the relative error is higher than $10 \%$.


### 10.3 Parameters

of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
8.6 seconds

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11 Modeling
E

### 11.1 Characteristics of modeling

### 11.2 Characteristics of the grid

A number of nodes: 357 .
A number of meshs and types: 80 QUAD9.

### 11.3 Functionalities

tested
Orders
Keys
MECA_STATIQUE
OPTION
"SIEF_ELGA_DEPL"
[U4.31.01]
CALC_ELEM
OPTION
"ERRE_ELEM_NOZ1"
[U4.61.02]
OPTION
"ERRE_ELEM_NOZ2"
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12
Results of modeling E
12.1 Values
tested
Identification

## Reference

Aster
\% difference
tolerance
To $x x$ ZZ1
3.
3.070
2.33
0.1
xx ZZ2
3.
3.004
0.14
0.1
yy ZZ1
0 .
0.113
0.3
yy ZZ2
0 .
0.04
0.3
xy ZZ1
0 .
1.4103
0.1
xy ZZ2
0.15625
0.162
3.44
0.1
yy ZZ2
0.15625
0.153
2.11
0.1
xy ZZ1
0.125
0.124
1.06
0.1
xy ZZ2
0.125
0.124
0.94
0.1

Net M1 eabs ZZ1
6.1105
1.103
eabs ZZ2
2.1104
1.103
erel ZZ1
1.45\%
0.1

### 12.2 Remarks

$=$ estimated error is an indication of effectivity of the estimator exact error

## Contents of the files results:

- absolute errors and relative total by the 2 methods,
- maximum and minimal values of the constraints and the errors, - lists of the meshs where the relative error is higher than $10 \%$.


### 12.3 Parameters

of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
8.8 seconds

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## Code_Aster ${ }^{\circledR}$

## Version <br> 4.0

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Author (S):
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Key:
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Summary of the results
TRIA3
QUAD4
TRIA6
QUAD8
QUAD9
E
exact
3.44\%
2.48\%
$1.17 \%$
$0.697 \%$
$0.695 \%$
rel
ZZ1
3.28\%
2.28\%
$0.95 \%$
0.22\%
$0.21 \%$
ZZ2
3.49\%
2.37\%
1.29\%
$0.687 \%$
0.66\%

ZZ1
0.952
0.919
0.810

The constraints with the nodes, as a whole, are approximated better with ZZ 2 , especially for the elements ex
of order 2 . If one makes tighten $H$ towards 0 , the rates of convergence with H of

- are higher by
method ZZ2 for all the types of elements to those of method ZZ1 (* is the smoothed constraint).
Estimator ZZ1 is not reliable for the elements of order 2, the nodal constraints remain correct. One can to check in this particular case that 0 when $H 0$, which shows that continuous total smoothing proves insufficient to estimate the error in the case of a solution without singularities (case of this test).
ZZ 2 is on the other hand reliable and asymptotically exact ( 1 when $H 0$ ).
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Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SSLV112 - Calculation of G local by a Lagrangian method
Date:
17/02/02
Author (S):
G. DEBRUYNE, C. Key DURAND
:
V3.04.112-B Page:
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Organization (S): EDF/AMA

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.112

SSLV112-Calculation of G by the method
Lagrangian for a circular crack

## Summary

It is about a test in statics for a three-dimensional problem. This test allows the calculation of the rate of refund of
energy room by the Lagrangian method of propagation for an initial crack quasi-circular plunged in a presumedly infinite medium. One transforms it into circular crack of more important ray.

The interest of the test is to study the validity of the calculation of the rate of refund of energy room
after extension of
fissure. It is also to be able to calculate the rate of refund of energy starting from a grid fixed on one fissure variable geometry (in elasticity). Methods of calculation of G_LOCAL, THETA_LAGRANGE and of
THETA_LEGENDRE are used.
The test includes/understands two modelings.

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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLV112 - Calculation of G local by a Lagrangian method
Date:
17/02/02
Author (S):
G. DEBRUYNE, C. Key DURAND

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1
Problem of reference
1.1 Geometry

Z
$=1$.
G
H
I
F
O
B
E
Y
With
C

Large initial axis: $O A=35 \mathrm{~mm}$
Small initial axis: $O B=33.95 \mathrm{~mm}$
SupX = Face OEGH
Sup $Y=$ Face $O C I H$
Supfissz: Face ABEDC
mailpress: Face IFGH

## 1.2 <br> Material properties <br> Young modulus: $E=2.105 \mathrm{MPa}$ <br> Poisson's ratio: $=0.3$ <br> 1.3 <br> Boundary conditions and loadings

Face OEGH: $u x=0$
Face OCIH: $u y=0$
Face ABEDC: $u z=0$
Face IFGH: uniform constraint of traction $Z=1 \mathrm{MPa}$
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SSLV112 - Calculation of G local by a Lagrangian method

## Date:

17/02/02
Author (S):
G. DEBRUYNE, C. Key DURAND

## :

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## 2

Reference solution

## 2.1

Method of calculation used for the reference solution
has

For a circular crack of ray has in an infinite medium, the rate of refund of energy $G$ is equal to (1
4
$h$
G $=$
42
has

E

## Numerical application:

Initially, the crack is not strictly circular $(O A=35 \mathrm{~mm}, O B=33.95 \mathrm{~mm})$.
One transforms it into circular crack of ray has $=42 \mathrm{~mm}$ without touching with the grid, (it is the goal of this method) but while forming on the modules of the field theta in each node of the bottom. One has then
NR
in any point $G=$
$m m$
2.2
Results of reference
Values of G local in bottom of crack.
The solutions given in the "handbook" of SIH give the value of KI divided by by report/ratio
with the traditional definition [bib1].

### 2.3 References <br> bibliographical

## [1]

Solution of Sneddon (1946) in G.C. SIH: Handbook of stress-intensity factors Institute of Fracture and Solid Mechanics - Lehigh University Bethlehem, Pennsylvannie Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLV112 - Calculation of G local by a Lagrangian method
Date:
17/02/02
Author (S):
G. DEBRUYNE, C. Key DURAND
:
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3
Modeling a: method THETA_LAGRANGE

## 3.1 <br> Characteristics of modeling

## Z <br> $\boldsymbol{Y}$ <br> $X$

3.2<br>Characteristics of the grid<br>A number of nodes: 1754<br>3.3 Functionalities<br>tested<br>Orders<br>MECHANICAL AFFE_MODELE<br>3D<br>ALL<br>CALC_MATR_ELEM OPTION<br>"RIGI_MECA_LAGR"

A number of meshs and types: 304 PENTA 15 and 131 HEXA 20

CALC_G_LOCAL_T"THETA_LAGRANGE"

## PROPAGATION: 1

## DEGREE: 4

### 3.4 Notice

The initial crack is not circular $(O A=35 \mathrm{~mm}, O B=33.95 \mathrm{~mm})$ but the transformation Lagrangian makes it circular thanks to the field theta of module different from 1 in each node from melts of crack $(O A=O B=42 \mathrm{~mm}$ in the final configuration $)$.

The degree of the polynomials of LEGENDRE used to calculate $\boldsymbol{G}(S)$ is 4. Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLV112 - Calculation of G local by a Lagrangian method
Date:
17/02/02
Author (S):
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## 4

Results of modeling $A$

### 4.1 Values

tested
The number between brackets indicates the nth position of the node on the bottom
Identification Reference
Aster \%
difference
G local Node A (1)
1.2165 10-4 1.2406

10-4
1.98

G local Node (5)
1.2165 10-4 1.1268

10-4
7.96

G local Node (10)
1.2165 10-4 1.1406

10-4
6.65

G local Node (15)

1.2165 10-4 1.1892<br>10-4<br>2.30<br>G local Node (20)<br>1.2165 10-4 1.2013<br>10-4<br>1.26<br>G local Node (25)<br>1.2165 10-4 1.1825<br>10-4<br>2.88<br>G local Node B (33)<br>1.2165 10-4 1.3042<br>10-4<br>7.21

### 4.2 Notice

In calculation Aster, G local corresponds to the virtual extension of only one lip of the crack Gréf
NR
(half-crown), the value obtained is thus to compare with
= 12165

2
mm

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## 5 <br> Modeling b: method THETA_LEGENDRE

## 5.1 <br> Characteristics of modeling

Z
Y
$X$

## 5.2

Characteristics of the grid
A number of nodes: 1754
A number of meshs and types: 304 PENTA 15 and 131 HEXA 20

### 5.3 Functionalities

tested
Orders

## MECHANICAL AFFE_MODELE

3D
ALL
CALC_MATR_ELEM OPTION
"RIGI_MECA_LAGR"

# CALC_G_LOCAL_T "THETA_LEGENDRE" 

PROPAGATION: 1

DEGREE: 4

### 5.4 Notice

The initial crack is not circular $(O A=35 \mathrm{~mm}, O B=33.95 \mathrm{~mm})$ but the transformation Lagrangian makes it circular thanks to the field theta of module different from 1 in each node from melts of crack ( $O A=O B=42 \mathrm{~mm}$ in the final configuration) .

The degree of the polynomials of LEGENDRE used to calculate $G(S)$ is 4 .
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## 6 <br> Results of modeling B

### 6.1 Values

tested
The number between brackets indicates the nth position of the node on the bottom

## Identification Reference

Aster \%
difference
G local Node A (1)
1.2165 10-4 1.1455

10-4
6.20

G local Node (5)
1.2165 10-4 1.1258

10-4
8.06

G local Node (10)
1.2165 10-4 1.1476

10-4
6.00

G local Node (15)
1.2165 10-4 1.1797

10-4
3.12

G local Node (20)
1.2165 10-4 1.1974

10-4
1.60

G local Node (25)
1.2165 10-4 1.1960

10-4
1.71

G local Node B (33)
1.2165 10-4 1.1929

10-4
1.98

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## Code_Aster ${ }^{\circledR}$

Version
5.0

## Titrate:

SSLV112 - Calculation of G local by a Lagrangian method
Date:
17/02/02
Author (S):

G. DEBRUYNE, C. Key DURAND

## 7 <br> Summaries of the results

Calculation of G local:

2 methods (THETA_LEGENDRE and THETA_LAGRANGE) give the same ones appreciably results ( $8 \%$ of error to the maximum compared to the analytical solution),
the precision of the results is average because the extension of the crack is approximately 1.2, which is close to the maximum of extension reasonable for this method for a crack 3D,
method LEGENDRE is less expensive in time CPU.
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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
Date:
26/01/98
Author (S):

## X. DESROCHES

Key:
V3.04.113-A Page:

# Organization (S): EDF/IMA/MMN 

## Handbook of Validation

## V3.04 booklet: Linear statics of the voluminal structures

## V3.04.113 document

## SSLV113 - Estimator of error on a cylinder

## hollow Bi-materials

## Summary:

This test validates the estimator of error in pure residue, applied to linear elasticity $2 D$, in statics. One is considered
hollow roll made up of two materials and subjected to internal and external pressures.
2 modelings are axisymmetric, on quadrangles with 8 nodes.
The interest of the test lies in the comparison between the exact and calculated constraints, on the one hand, the error
estimated and the exact error, in addition. This test also makes it possible to show the validity of the estimator in residue
on a structure bimatériau, contrary to the estimator of Zhu-Zienkiewicz which is not applicable on structures presenting of discontinuities in the stress field (here with the interface material).
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HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
Date:
26/01/98
Author (S):

## X. DESROCHES

Key:
V3.04.113-A Page:
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1
Problem of reference
1.1 Geometry

Z
With
D
C
1
chechmate. 1

## chechmate. 2

p
$Q$
With
$E$
B
0 .
1.

3/2
2.
$R$
1.2

## Material properties

chechmate. 1:
$E=2$.
$=03$
chechmate. 2:
$E=1$.
$=03$

## 1.3

Boundary conditions and loadings
On $A B, U Z=0$.
on $c d ., U Z=0.91333=A$.
Pressure interns on $A D, p=1$.
External pressure on $B C, Q=2$.
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V3.04 booklet: Linear statics of the voluminal structures
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## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
Date:
26/01/98
Author (S):

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2
Reference solution
2.1
Method of calculation used for the reference solution
$\mu$
$I$
$E$
$=$
$I$
$($
$21+)$
I
E
=
I
(1-
2 ) ( $1+$ )
has
$=-0.98097$
B
= - 1.11741
1
1
calculated numerical data
has
$=-1.34405$
B
= -
2
2
0.30048 starting from the equations of Navier
For the material " $I$ ", one $a$ :
B
U
= R has
I
$+$
$R$
I
$U$

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V3.04 booklet: Linear statics of the voluminal structures

## HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
Date:
26/01/98
Author (S):

## X. DESROCHES

Key:
V3.04.113-A Page:
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3 Modeling

## With

## 3.1

## Characteristics of modeling

M1
M2
M3
M4
With
E
B

## 3.2

## Characteristics of the grid

A number of nodes: 23.
A number of meshs and types: 4 QUAD8.

### 3.3 Functionalities

tested
Orders

## Keys

CALC_ELEM
OPTION
"SIRE_ELNO_DEPL"
[U4.61.02]
OPTION
"ERRE_ELGA_NORE"
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
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## X. DESROCHES

Key:
V3.04.113-A Page:
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4
Results of modeling $A$
4.1 Values
tested
Identification
Reference
Aster
\% difference
tolerance
With
1.00003
1.06833
6.83
7.0
rr
4.43821
4.46731
0.66
2.0
0.19518
0.16596
14.9
15.0
$z z$
E
2.37\%
5.0
rel

## E chechmate. 1

1.95508
1.97893
1.22
2.0
rr
3.48316
3.49330
0.29
2.0
0.19518
0.18498
5.22
6.0
$z z$
E
1.05\%
5.0
rel
E chechmate. 2
1.95508
1.98398
1.48
2.0
rr
2.16049
2.13394
1.23
2.0
0.32135
0.32204
0.22
2.0
$z z$
E
0.152\%
5.0
rel
B
1.99999

### 4.2 Remarks

Grid being coarse (4 elements according to Gold), certain constraints close to the axis of axisymetry are badly approximated. The jump of and zz to the interface of 2 materials are on the other hand well detected.

### 4.3 Parameters

of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
4.7 seconds
4.4 Remarks

Relative error considered total $=1.40 \%$.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:

SSLV113 Estimator of error on a hollow roll Bi-materials
Date:
26/01/98
Author (S):

## X. DESROCHES

Key:
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## 5 Modeling

B
5.1

## Characteristics of modeling

M1
M10
M20
M11
With
E
B
5.2

## Characteristics of the grid

A number of nodes:
A number of meshs and types: 20 QUAD8.

### 5.3 Functionalities

tested
Orders
Keys
CALC_ELEM
OPTION
"SIRE_ELNO_DEPL"
[U4.61.02]
OPTION
"ERRE_ELGA_NORE"
Handbook of Validation
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HI-75/96/017 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
Date:

26/01/98<br>Author (S):<br>\section*{X. DESROCHES}

Key:
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6
Results of modeling B
6.1 Values
tested
Identification
Reference
Aster
\% difference
tolerance
With
1.00003
1.00351
0.35
0.5
rr
4.43821
4.43970
0.034
0.05
0.19518
0.19369
0.76
0.8
zz
E
0.57\%
0.6
rel
E chechmate. 1
1.95508
1.95583
0.039
0.05
rr
3.48316
0.009
0.01
0.19518
0.19486
0.16
0.2
$z z$
E
$0.14 \%$
0.2
rel
E chechmate. 2
1.95508
1.96166
0.34
0.5
$r r$
2.16049
2.15403
0.299
0.5
0.32135
0.32138
0.009
0.01
zz
$E$
$0.027 \%$
0.03
rel
B
1.99999
2.00003
0.002
0.01
$r r$

### 2.11555

2.11558
0.001

# 6.2 Parameters 

of execution
Version: 3.02.11
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 megawords
Time CPU To use:
5.3 seconds
6.3 Notice

Relative error considered total $=0.24 \%$.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV113 Estimator of error on a hollow roll Bi-materials
Date:
26/01/98
Author (S):

## X. DESROCHES

Key:
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7
Summary of the results
The estimator of error in residue "ERRE_ELGA_NORE' gives good results on the problems in bimaterials.

## Note:

The estimator of error of Zhu-Zienkiewicz does not give correct results. Indeed, with the interface it detects a strong error because it carries out a continuous smoothing of the constraints whereas it
exist a jump for $z z$ and.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV114 Movements of solid body 2D and 3D
Date:
26/01/98
Author (S):

## J. PELLET

Key:
V3.04.114-A Page:
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Organization (S): EDF/IMA/MMN

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.114
SSLV114 - Movements of solid body 2D and 3D
Summary:

This test validates (in 2D and 3D) key word LIAISON_SOLIDE of order AFFE_CHAR_MECA [U4.25.01].
This key word is used to rigidify an expressing whole of nodes by linear relations that displacements "rigidified" nodes are dependent between them by the equation:
(
$\mathbf{U} M)=($
$\mathbf{U}$
)
With + ()
WITH AMNDT.
This equation is valid only in small displacements.
The test problem the 2D and the 3D as well as the particular cases:

- geometrically confused nodes (2D and 3D),
- aligned nodes (in 3D).

Handbook of Validation
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## Code_Aster ®

Version
4.0

Titrate:
SSLV114 Movements of solid body 2D and 3D
Date:
26/01/98
Author (S):

## J. PELLET

Key:
V3.04.114-A Page:
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1
Problem of reference
1.1 Geometry

Problem 2D
3D problem

2 E1, E2, E3
I
F
NR
Q1, Q2, Q3
K
With
B
L
1
2
3
y
G
1
P
H
X

## 1.2

## Material properties

$\mathrm{E}=0$
= 0
The finite elements present in this problem are only used to define the ddls carried by the nodes. Their rigidity must be null.
1.3

## Boundary conditions and loadings

In this problem, one defines "solid" groups of nodes:

- in 2D:

WITH, B, C, D
E1, E2, E3

- in 3D:

F, G, H, I, J, K, L, M
O, NR, P
Q1, Q2, Q3
For each one of these groups of nodes, one imposes partial displacements so that them "solid" move while respecting:
2.

## translation: (

T
)
With $=($
T E)
$1=$
in 2 D :
3.
rotation: ()
WITH $=(E)$
$1=$
00
. 1
2.
translation: (
T F)
$=($
$\mathbf{T} N R)=($
T Q)
$1=3$
4.
in 3D:
0.

001
rotation: $(F)=(N R)=(Q) 1=0$.
002
0.

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## Code_Aster ® ${ }^{\circledR}$

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Titrate:
SSLV114 Movements of solid body 2D and 3D
Date:
26/01/98
Author (S):

## J. PELLET

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Selected displacements forced to lead to required displacements "solid" are:
2D
$\mathrm{DX}(\mathrm{A})=2$.
$\mathrm{DX}(\mathrm{E} 1)=2$.
$D Y(A)=3$.
$D Y(E 1)=3$.
DY $(\mathrm{B})=3.001$
$(+\mathrm{DRZ}(\mathrm{E} 1)=0.001$ for modeling B)
3D
$\mathrm{DX}(\mathrm{F}, \mathrm{NR}, \mathrm{Q} 1)=2$.
$D Y(F, N R, Q 1)=3$.
$\mathrm{DZ}(\mathrm{F}, \mathrm{NR}, \mathrm{Q} 1)=4$.
$\mathrm{DY}(\mathrm{J}, \mathrm{O})=2.002$
DY $(\mathrm{J}, \mathrm{O})=2.999$
DX $(\mathrm{I})=1.997$
$+\mathrm{DRZ}(\mathrm{NR})=0.003$
for modeling B
$\operatorname{DRX}(\mathrm{Q} 1)=0.001$
$\operatorname{DRY}(\mathrm{Q} 1)=0.002$
DRZ $(\mathrm{Q} 1)=0.003$
2
Reference solution

## 2.1

Method of calculation used for the reference solution
The movement of the "solids" being imposed, the reference solution (in displacement) is it imposed movement.

## The reference solution is thus exact (in small rotations).

2.2

## Results of reference

1. 

999
2.

In 2D:
$\mathbf{U}(C)=$
U.E. 2) $=$
3.

001
3.

1999

1998

2

In 3D:
$\mathbf{U}(L)=3$.
002
$\mathbf{U} P)=3001$
3.
4.
2.3

Uncertainty on the solution
Exact solution.
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Code_Aster ${ }^{\circledR}$
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SSLV114 Movements of solid body 2D and 3D
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Author (S):
J. PELLET

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3 Modeling
With
3.1

Characteristics of modeling
The finite elements affected on the meshs of the grid are those of modelings D_PLAN and 3D. ddls carried by the nodes is thus:
DX, DY in 2D,
DX, DY, DZ in 3D

## 3.2

## Characteristics of the grid

A number of nodes: 21
A number of meshs and types: 1 QUAD4, 1 HEXA8, 8 SEG2

### 3.3 Functionalities

## tested

## Orders

## Keys

AFFE_CHAR_MECA
LIAISON_SOLIDE

| 4 |
| :--- |
| Results of modeling A |
| 4.1 Values |
| tested |
| Identification |
| Reference |
| Aster |
| \% difference |
| DX (C) |
| 1.999 |
| 1.999 |
| 2 1012 |
| DY (C) |
| 3.001 |
| 3.001 |
| 21012 |
| 2 DX (E2) |
| 2.000 |
| 2.000 |
| 21012 |
| DY (E2) |
| 3.000 |
| 3.000 |
| $2_{1} 1012$ |
| DX (L) |
| 1.999 |
| 1.999 |
| 21012 |
| $3 D$ DY (L) |
| 3.002 |
| 3.002 |
| 21012 |
| DZ (L) |
| 3.999 |
| 3.999 |
| 21012 |
| DX (P) |
| 1.998 |
| 1.998 |
| 21012 |
| DY (P) |
| 3.001 |
| 3.001 |

DZ (P)
4.000
4.000
${ }^{2} 1012$
DX (Q3)
2.000
2.000
${ }^{2} 1012$
DY (Q3)
3.000
3.000
${ }^{2} 1012$
DZ (Q3)
4.000
4.000
${ }^{2} 1012$
4.2 Parameters
of execution
Version: 3.04
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
5 seconds
Handbook of Validation
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## Code_Aster © ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV114 Movements of solid body 2D and 3D
Date:
26/01/98
Author (S):
J. PELLET

Key:
V3.04.114-A Page:
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5 Modeling
B

## 5.1

## Characteristics of modeling

The finite elements affected on the meshs of the grid are those of modelings D_PLAN, COQUE_C_PLAN, 3D and POU_D_E.

- the nodes 2D carry the ddls DX, DY
(+ DRZ for B and E1),
- the nodes 3D carry ddls DX, DY, DZ
(+DRX, DRY, DRZ for I, NR and Q1).
5.2

Characteristics of the grid
A number of nodes: 21
A number of meshs and types: 1 QUAD4, 1 HEXA8, 10 SEG2
5.3 Functionalities
tested
Orders
Keys
AFFE_CHAR_MECA
LIAISON_SOLIDE
[U4.25.01]
6
Results of modeling B
6.1 Values
tested
Identification

## Reference

Aster
\% difference
DX (C)
1.999
1.999
${ }^{2} 1012$
DY (C)
3.001
3.001
${ }^{2} 1012$
2D DX (E2)
2.000
2.000
${ }^{2} 1012$
DY (E2)
3.000
3.000
${ }^{2} 1012$

## DX (L)

1.999
1.999
${ }^{2} 1012$
3D DY (L)
3.002
3.002
${ }^{2} 1012$
DZ (L)
3.999
3.999
${ }^{2} 1012$
DX (P)
1.998
1.998
${ }^{2} 1012$
DY (P)
3.001
3.001
${ }^{2} 1012$
DZ (P)
4.000
4.000

21012
DX (Q3)
2.000
2.000

21012
DY (Q3)
3.000
3.000

21012
DZ (Q3)
4.000
4.000
${ }^{2} 1012$
6.2 Parameters
of execution
Version: 3.04
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:

## 8 MW

Time CPU To use:
5 seconds
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## Code_Aster ®

Version
4.0

Titrate:
SSLV114 Movements of solid body 2D and 3D
Date:
26/01/98
Author (S):

## J. PELLET

Key:
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7
Summary of the results
The results are excellent ( ${ }^{2} 1012$ ).
Handbook of Validation
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Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

SSLV115 - Concrete element prestressed in compression and gravity
Date:
23/09/02
Author (S):
J. Mr. PROIX, P. BADEL

Key: V3.04.115-A Page: 1/4
Organization (S): EDF/AMA

Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal systems<br>Document: V3.04.115

SSLV115 - Concrete element prestressed in compression and gravity

## Summary:

This test allows a simple checking of calculations of gravity for the concrete elements with cables of prestressed, in linear mechanics of the structures static.

The concrete element is voluminal, and the elements of cable of prestressing are elements of bar.
Only one modeling is used: it makes it possible to test the application of gravity on elements of bar, for two directions of gravity.

The values tested are the resultants of the reactions on the supports, equal to the total weight of modeling.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLV115 - Concrete element prestressed in compression and gravity
Date:

# 23/09/02 

Author (S):
J. Mr. PROIX, P. BADEL

Key: V3.04.115-A Page: 2/4

## 1 <br> Problem of reference

### 1.1 Geometry

A right-angled parallelepiped modelling the concrete, and a line included in this volume modelling it cable of prestressed:

The cable is parallel to axis $X$. Its intersection with the plan ( $O y z$ ) is defined by the item (1. , 0.3).

## 1.2

Material properties
$E=2.11011$ Pa for the cable, and $E=3.1010$ Pa for the concrete.
Rho $=2104 \mathrm{~kg} / \mathrm{m} 3$ for the cable, and the concrete, Rho $=3 \mathrm{~kg} / \mathrm{m} 3$ (nonphysical values intended for to make dominating the weight of the cable)

## 1.3

Boundary conditions and loadings
$D Y=0$ as in point 1001, $D Z=0$ for all the nodes of the face $z=0$, and $D X=0$ for all the nodes of face $x=0$.

1 only loading is applied: gravity, with $g=10 \mathrm{~m} / \mathrm{s}^{2}$, successively in direction $Z$ then $X$.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLV115 - Concrete element prestressed in compression and gravity
Date:
23/09/02
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J. Mr. PROIX, P. BADEL

Key: V3.04.115-A Page: 3/4

## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

- Analytical Solution:

The resultant of the efforts (equal to the total weight) is worth:

# weight of the concrete: $P b=V^{*} R h o b * g$ 

weight of the cable: $P c=A * L^{*} R h o c * g$
in the direction where gravity is applied.

## 2.2 <br> Results of reference

- Résultante from the efforts: $R=132 \mathrm{~N}$


## 2.3 <br> Uncertainty on the solution

Analytical solution.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLV115 - Concrete element prestressed in compression and gravity
Date:
23/09/02
Author (S):
J. Mr. PROIX, P. BADEL

Key: V3.04.115-A Page: 4/4

## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The volume of concrete is modelled by only one element. The cable of prestressing is modelled by 4 elements BARS.

3.2<br>Characteristics of the grid<br>4 meshs SEG2, a mesh HEAX8

### 3.3 Functionalities <br> tested

Orders

AFFE_CARA_ELEM<br>BAR<br>$D E F I \_C A B L E \_B P$<br>AFFE_CHAR_MECA<br>GRAVITY<br>CALC_NO<br>REAC_NODA<br>STAT_NON_LINE

# 4 <br> Results of modeling $A$ 

### 4.1 Values

tested

## Loading case

Identification
Reference
Aster \%
difference
gravity according to $Z$
Rz
132
132
0
gravity according to $X$
X-ray
132
132
0

## 5 <br> Summary of the results

This test, very simple, makes it possible simultaneously to check the correct operation of gravity in elements of bar of prestressing, which is checked by the perfect coincidence of the results with analytical solution. It was introduced following the discovery $D$ `an anomaly on gravity into bars, and makes it possible to validate the correction.

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal systems
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV120 Stretching of an orthotropic parallelepiped
Date:
26/01/98
Author (S):

## G. DEBRUYNE

## Key:

V3.04.120-C Page:
1/6
Organization (S): EDF/IMA/MMN

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures

## Document: V3.04.120

## SSLV120-Stretching of a parallelepiped

 orthotropic under its own weight
## Summary:

This test of mechanics of the structures allows the evaluation of displacements and the constraints of one parallelepiped becoming deformed under its own weight. The material is elastic linear orthotropic. modeling is three-dimensional. The model is similar to test VPCS SSLV07 (but in this case the material is isotropic) and with test SSLV121 (in this case the material is isotropic transverse).
The variations of the results obtained by Aster range between 0,00 and 0,5\% of the calculated reference analytically.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV120 Stretching of an orthotropic parallelepiped
Date:
26/01/98
Author (S):

## G. DEBRUYNE

Key:
V3.04.120-C Page:
2/6
1
Problem of reference

### 1.1 Geometry

Z
X

B
With
$D$
y
E
L
B
X
C
has
Height: $L=3 \mathrm{~m}$
Width: has $=1 \mathrm{~m}$
Thickness: $B=1 \mathrm{~m}$
Co-ordinates of the points (in meters):
With
B
C
D
E
X
$X$
0.
0.
0.5
0.5

0 .
0.
y
0 .
0.
0.
0.
0.
0.5

Z
3.
0.

0 .
3.

## Material properties

YOUNG moduli in directions $X, y$ and $Z$ :
$E \_L=5.1011 P a, E_{-} T=5.1011 P a, E_{-} N=2.1011 \mathrm{~Pa}$.
Poisson's ratios in the xy plans, $x z$ and $y z$ :
$\_L T=0.1, \_L N=0.3, \_T N=0.1$.
Moduli of rigidity in the xy plans, $x z$ and $y z$ :
$G_{-} L T=7.692311010 \mathrm{~Pa}, G_{-} L N=7.692311010 \mathrm{~Pa}$,
$G_{-} T N=7.692311010 \mathrm{~Pa}$.
Density: $=7800 \mathrm{~kg} / \mathrm{m} 3$.

## 1.3

## Boundary conditions and loadings

Not a: $(U=v=W=0, X=y=Z=0)$
Actual weight following axis $Z$
Uniform constraint with traction for the higher face:
$Z=g L=+229.554 . P a$
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV120 Stretching of an orthotropic parallelepiped
Date:
26/01/98
Author (S):

## G. DEBRUYNE

Key:
V3.04.120-C Page:
3/6
2

## Reference solution

## 2.1

## Method of calculation used for the reference solution

The reference solution results from that given in card SSLV07/89 of guide VPCS (in considering in more one orthotropic elastic matrix). The analytical expression of the solution is following:
Displacements:
_ $x z G X Z$

```
_ yzG yZ
G z2 G
G L2
U = -
v= -
W= -
+
(_xzx2 +_yzy2) -
E_Z
E_Z
2E_Z
2E_Z
2E_Z
Constraints:
=
=
=
=
=
=
zz
G Z
xx yy xy yzzx 0
Z
XU
With
Of
D
L/2
E
L
B
C
X
It
WwB
B'
2.2
```

Results of reference
Displacement of the points B, C, D, E and X.
Constraints $z z$ of $A$ and $E$
2.3

## Uncertainty on the solution

Exact analytical results.

### 2.4 References

## bibliographical

[1]
TIMOSHENKO (S.P) Theory of elasticity - Paris - Polytechnic Bookshop CH. Béranger, p. 279 with 282 (1961)
[2]
S.W. TSAI, H.T. HAHN - Composite introduction to materials. Technomic Publishing Company (1980).

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV120 Stretching of an orthotropic parallelepiped
Date:
26/01/98
Author (S):
G. DEBRUYNE

Key:
V3.04.120-C Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling
3D meshs hexa20
Z
B
With
$D$
$y$
$L$
C
$X$
has
Cutting:
3 in height
2 in width and thickness

## Limiting conditions:

on axis $A B$
DDL_IMPO: (GROUP_NO: ABsansA $D X=0 ., D Y=0$. )
in $A$ and $D$
(NODE: WITH DX=0., $D Y=0 ., D Z=0$. )
(NODE: D DY=0.)
Names of the nodes:
With $=$ N59
$B=N 53$
$C=$ N12
$D=$ N18
$E=N 56$
$X=N 70$
3.2

Characteristics of the grid
A number of nodes: 111
A number of meshs and types: 12 HEXA20

### 3.3 Functionalities

tested
Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
GRAVITY
FORCE_FACE
GROUP_MA
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS_ORTH
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:

SSLV120 Stretching of an orthotropic parallelepiped
Date:
26/01/98
Author (S):
G. DEBRUYNE

Key:
V3.04.120-C Page:
5/6
4
Results of modeling $A$
4.1 Values
tested
Identification
Reference
Aster
\% difference
UB
0 .
1022
VB
0.

1022

WB
1.721655106
1.721674106
0.01

CPU
0.
= 1014
VC
0 .
= 1019

WC
1.707308106
1.707326106
0.01

UD
1.721655107
1.721652107
0.
$=1023$
WD
1.434713108
1.432400108
0.2

EU
0.
= 1022
VE
0 .
= 1022
WE
1.291241106
1.291260106
0.01
(Pa)
$z z$ (A)
2.29554105
2.2956105
< 0.01
$z z(E)$
1.14777105
1.14777105
< 0.01
$z z(X)$
2.29554105
2.29549105
< 0.01
UX
0.

1020

VX
5.738850108
5.738740108
0.01

WX
4.782375109
4.759220109

## 0.5

### 4.2 Remarks

Modeling in HEXA20 is completely acceptable for this coarse grid.

### 4.3 Parameters

of execution
Version: 3.04.00
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:
8 MW
Time CPU To use:
4.99 seconds

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV120 Stretching of an orthotropic parallelepiped
Date:
26/01/98
Author (S):

## G. DEBRUYNE

Key:
V3.04.120-C Page:
6/6
5

## Summary of the results

The results concerning displacements and the constraints are very close to the solution analytical with adopted modeling ( $<0.2 \%$ for displacements, $<0.5 \%$ for the constraints).
The elastic coefficients in the 3 directions of orthotropism were selected so as to obtain them same values of displacements at the points $B, C, D$ and $E$ that those calculated for a material isotropic (test SSLV007) or isotropic transverse (test SSLV121). Numerically, these values are very close relations of those of these tests at the points considered (of about a 10-6) the difference resulting from
method of construction of the matrices of rigidity in the various cases. As in point $X$, these values differ but correspond well to the reference solution.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

## Version

4.0

Titrate:
SSLV121 Stretching of a transverse isotropic parallelepiped
Date:
26/01/98
Author (S):
G. DEBRUYNE

Key:
V3.04.121-C Page:
1/6
Organization (S): EDF/IMA/MMN

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.121
SSLV121-Stretching of an isotropic parallelepiped
transverse under its own weight
Summary:
This test of mechanics of the structures allows the evaluation of displacements and the constraints of one parallelepiped becoming deformed under its own weight. The material is elastic linear isotropic transverse.
modeling is three-dimensional. The model is similar to test VPCS SSLV07 (but in this case the material is isotropic) and with test SSLV120 (in this case the material is orthotropic.).
The variations of the results obtained by Aster range between 0,00 and 0,2\% of the calculated reference analytically.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV121 Stretching of a transverse isotropic parallelepiped
Date:
26/01/98
Author (S):
G. DEBRUYNE

Key:
V3.04.121-C Page:
2/6
1

## Problem of reference

### 1.1 Geometry

Z
$X$

## 3.

3. 

1.2

## Material properties

YOUNG moduli in the xy plan and direction Z :
$E \_L=5.1011 \mathrm{~Pa}, E_{-} N=2.1011 \mathrm{~Pa}$.
Poisson's ratios relating to the xy plan and direction Z :
$\_L T=0.1, ~ \_L N=0.3$.
Modulus of rigidity relating to direction Z :
$G_{-} L N=7.692311010 \mathrm{~Pa}$.
Density: $=7800 \mathrm{~kg} / \mathrm{m} 3$.

## 1.3

Boundary conditions and loadings
Not $a:(U=v=W=0, X=y=Z=0)$
Actual weight following axis $Z$
Uniform constraint with traction for the higher face:
$Z=g L=+229.554 . P a$
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV121 Stretching of a transverse isotropic parallelepiped
Date:
26/01/98
Author (S):
G. DEBRUYNE

Key:
V3.04.121-C Page:
3/6
2
Reference solution
2.1

## Method of calculation used for the reference solution

The reference solution results from that given in card SSLV07/89 of guide VPCS (in considering in more one transverse isotropic elastic matrix). The analytical expression of the solution is as follows:
Displacements:
Z G X Z

```
_ ZGyZ
Gz2 G
G L2
U = -
v= -
W=-
+
(_zx2 +_zy2) -
E_Z
E_Z
2E_Z
2E_Z
2E_Z
Constraints:
=
=
=
=
=
=
zz
G Z
xx yy xy yzzx0
Z
XU
With
Of
D
L/2
E
L
B
C
X
It
WwB
B'
2.2
```

Results of reference
Displacement of the points B, C, D, E and X.
Constraints $z z$ of $A$ and $E$
2.3

## Uncertainty on the solution

Exact analytical results.

### 2.4 References

bibliographical
[1]
TIMOSHENKO (S.P) Theory of elasticity - Paris - Polytechnic Bookshop CH. Béranger, p. 279 with 282 (1961)
[2]
S.W. TSAI, H.T. HAHN - Composite introduction to materials. Technomic Publishing Company (1980).

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
SSLV121 Stretching of a transverse isotropic parallelepiped
Date:
26/01/98
Author (S):

## G. DEBRUYNE

Key:
V3.04.121-C Page:
4/6
3 Modeling
With
3.1

Characteristics of modeling
3D
Z
B
With
D
y
L
X
C
has
Cutting:
3 in height
2 in width and thickness
meshs hexa20
Limiting conditions:
on axis AB
DDL_IMPO: (GROUP_NO: ABsansA DX=0., DY=0. )
in A and D
(NODE: WITH DX=0., DY=0., DZ=0. )
(NODE: D DY=0.)
Names of the nodes:
With = N59
B = N53
C $=$ N12
D = N18
E = N56
X = N70

## 3.2 <br> Characteristics of the grid

A number of nodes: 111
A number of meshs and types: 12 HEXA20

### 3.3 Functionalities

## tested

Orders
Keys
AFFE_CHAR_MECA
DDL_IMPO
GROUP_NO
[U4.25.01]
GRAVITY
FORCE_FACE
GROUP_MA
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"MECHANICAL"
"3D"
ALL
[U4.22.01]
DEFI_MATERIAU
ELAS_ISTR_FO
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
SSLV121 Stretching of a transverse isotropic parallelepiped
Date:
26/01/98
Author (S):
G. DEBRUYNE

Key:
V3.04.121-C Page:
5/6
4
Results of modeling A

### 4.1 Values

## tested

## Identification

## Reference

Aster
\% difference
UB
0.

1022

VB
0.

1022

## WB

1.721655106
1.721672106
0.01

CPU
0 .
$=1014$
VC
0 .
= 1019

WC
1.707308106
1.707325106
0.01

UD
1.721655107
1.721649107
0.01

VD
0.
$=1023$
WD
1.434713108
1.432587108
0.2

EU
0.
= 1022
VE
0.
$=1023$
WE
1.291241106
1.291259106
0.01
(Pa)
$z z$ (A)
2.29554105
2.2956105
< 0.01
$z z$ (E)
1.14777105
1.14777105
< 0.01
$z z$ (X)
2.29554105
2.29549105
< 0.01
UX
0.

1020

VX
1.721655107
1.721649107

WX
1.434712108
1.432587108
0.15

### 4.2 Remarks

Modeling in HEXA20 is completely acceptable for this coarse grid.

### 4.3 Parameters

of execution
Version: 3.04.00
Machine: CRAY C90
System:
UNICOS 8.0
Obstruction memory:

Time CPU To use:

### 4.25 seconds

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
SSLV121 Stretching of a transverse isotropic parallelepiped
Date:
26/01/98
Author (S):

## G. DEBRUYNE

Key:
V3.04.121-C Page:
6/6
5

## Summary of the results

The results concerning displacements and the constraints are very close to the solution analytical with adopted modeling ( $<0.2 \%$ for displacements, $<0.5 \%$ for the constraints).
The fact that there is only one component of the constraints ( zz ) in the problem only allows to test 2 elastic coefficients ( E Z Z and NU_Z).
Although these coefficients are constant, they were introduced in the form of functions to test functionality ELAS_GITR_FO.
The elastic coefficients in plan XY and direction Z were selected so as to obtain them same values of displacements at the points B, C, D and E that those calculated for a material isotropic (test SSLV07) or orthotropic (test SSLV120). Numerically, these values are very close those of these tests at the points considered (of about a 10-6) the difference resulting from the mode of construction of the matrices of stiffness in the various cases. As in point X , these values differ but correspond well to the reference solution.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HI-75/96/017 - Ind A

## Code_Aster ${ }^{\circledR}$

## Version

7.2

## Titrate:

SSLV130 - Hollow roll into incompressible

Date:
23/09/03
Author (S):
Key S. MICHEL-PONNELLE
V3.04.130-C Page:
1/14
Organization (S): EDF-R \& D /AMA

## Handbook of Validation

V3.04 booklet: Linear statics of the voluminal structures
Document: V3.04.130

SSLV130-Hollow roll into incompressible

## Summary:

This test makes it possible to validate the quasi-incompressible elements in statics for a threedimensional problem,
axisymmetric or two-dimensional (plane deformations). One considers a hollow roll subjected to a pressure
intern. The material has a Poisson's ratio equal to 0.4999 and one uses the elements quasi incompressible (modeling INCO). In all the cases of modeling, one carries out the test or not while

```
imposing
the condition of perfect incompressibility (DDL_IMPO and GONF=0)
```

Four modelings are carried out for this problem. Modelings $A$ and $B$ make it possible to test quasi-incompressible modeling 3D (3D_INCO), on the one hand with HEXA20 (A) and on the other hand with
TETRA10 (B). Modelings $C$ and $D$ are studies $2 D$ being based on mixed grids QUAD8 and TRIA6. Modeling $C$ is the study in plane deformations ( $D_{-}$PLAN_INCO), modeling $D$ is a study axisymmetric (AXIS_INCO).

This test is similar to test SSLV100.

The numerical results are satisfactory for all modelings. The fact of imposing explicitly condition of incompressibility influences only very little the results.

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
HT-66/03/008/A

Code_Aster ${ }^{\circledR}$
Version
7.2

Titrate:
SSLV130 - Hollow roll into incompressible

Date:
23/09/03
Author (S):
Key S. MICHEL-PONNELLE
:
V3.04.130-C Page:
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## 1

Problem of reference

### 1.1 Geometry

$y$
$\boldsymbol{R}$
E
F

```
C
45
D
P
With
B
Z
X
Internal ray
= 0.1 m has
External ray
B=0.2 m
```


## Co-ordinates of the points

## WITH B

E
F

## C

D
$X$ 0.1.0.2.0.1* $\cos (45) 0.2 * \cos (45) 0.1 * \cos (22.5) 0.2 * \cos (22.5)$
$y$
0 0.0.1*sin (45)
$0.1 * \sin (45)$
$0.1 * \sin (22.5) 0.1 * \sin (22.5)$
Z
00
0
0
0
0
1.2
Properties of material
$E=2.105 \mathrm{MPa}$
$=0.4999$

## 1.3

# Pressure interns $P=60 \mathrm{MPa}$. <br> Handbook of Validation <br> V3.04 booklet: Linear statics of the voluminal structures <br> HT-66/03/008/A 

Code_Aster ${ }^{\circledR}$
Version
7.2

Titrate:
SSLV130-Hollow roll into incompressible

## Date:

23/09/03
Author (S):
Key S. MICHEL-PONNELLE
:
V3.04.130-C Page:
3/14

## 2 <br> Reference solution

## 2.1 <br> Method of calculation

The general solution in displacement is as follows:

```
B-2
has)
```

$\boldsymbol{R}$
$U=\boldsymbol{U}=0$
Z
In deformations:
2
Pa
2
B
1
(+)
$r r$
1
(-2) -
E (2
B-2
has)
2
R
2
Pa
2
B
$=$
1
( +
1
(-2) +

B-2
has)

```
2
R
=
=0
R
ZZ
```

In constraints:
2
has
2
B
$=P$
$r r$
2
21 -
2
B-has
R
2
2
has
B
$=P$
1+
2
2
2
B-has

## R

$=2 P$
$z z$
2
$B-2$
has
= 0
R
One obtains for a perfectly incompressible cylinder (= 0.5):

```
in R=0 1
. : U = 6. 105
in R=02
.:
= 3.105
R
ur
= 6
-. 10-4
= 1
-5
. 10-4
rr
rr
= 6. 10-4
= }1
. 10-4
= 60
-.
=0.
rr
rr
```

The passage in the Cartesian system is done using the following relations:

```
= cos2+
2
xx
rr
sin-2 sin cos
R
= sin}2
2
yy
rr
cos+2 sin cos
R
= sin cos-
2
2
xy
rr
sin cos-2
R(cos-sin)
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```

Code_Aster ${ }^{\circledR}$
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## 2.2

Sizes and results of reference
One compares with the values of reference:
displacements ( $U, v$ ) at points $A$ and $F$,
the strains $(x x, y y, x y)$ and the stresses $(x x, y y, z z, x y)$ at points $A$ and $F$,
equivalent strains and stresses equivalent to point $A$.
Lastly, to test the passage of the sizes of the points of Gauss to the nodes for the nodes mediums, one also tests the nonnull strains and stresses in a node medium of the structure.

### 2.3 References <br> bibliographical

## [1]

Y.C. FUNG: Foundations of solid mechanics. Prenctice-hall, Inc. Englewood Cliffs.

NJ. 1965, p. 243-245
[2]
[V3.04.100] Hollow roll in plane deformations

## 3 Modeling

With
3.1

Characteristics of modeling
Grid with incompressible elements 3D of type HEXA20 only

Face blocked in dx
F
With
Normally blocked face
E
Face with imposed pressure
$45^{\circ}$
$X$

## Along axis Z:

total thickness $\boldsymbol{E}=0.01$
2 layers of elements
For the needs for examination in a node medium, one defines the node NOEUMI $=A+(0.0 . e / 4)$ where
the strains and the stresses are the same ones as in $A$.
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## Limiting conditions:

DDL_IMPO =
GROUP_NO = ' FACSUP'
0.

GROUP_NO
$={ }^{\prime}$ FACINF'

```
DZ=
0.
faces AEFD (z=0 and Z = 0.01)
```

GROUP_NO = ' FACEAB' $D X=$
0.
face $A B$
FACE_IMPO $=$ GROUP_MA $=$ ' FACEEF ${ }^{\prime}$
DNOR =
0.
face EF
$P R E S \_R E P=G R O U P \_M A=' F A C E A E '$
CLOSE $=$
60.
face $A E$

## 3.2

Characteristics of the grid
A number of nodes: 1501 nodes
A number of meshs: 240 HEXA20

### 3.3 Functionalities

tested
Orders

DEFI_GROUP CREA_GROUP_NO

CRIT_NOEUD<br>"SUMMIT"<br>AFFE_MODELE MODELING "3D_INCO"

GROUP_MA
DEFI_MATERIAU ELAS

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FACE_IMPO
GROUP_MA
PRES_REP
GROUP_MA

## STAT_NON_LINE COMP_INCR

RELATION ELAS
NEWTON
REAC_ITER
1
3.4

Sizes tested and results
For $A$ and $F$, one notes the result obtained
first column without imposing GONF $=0$
second column while imposing GONF $=0$

## Identification

Reference
Aster
\% difference
With $\boldsymbol{U}$
0. 3.29

10-22-1.39
10-21
$v$
6. 10-5
5.9998 10-5
5.9995 10-5-0.003-0.008

## $x x$

100. 99.92
99.92
-0.078
-0.078
$y y$
-60. -59.91

- 59.91
-0.155
-0.155
$z z$

20. 19.98
19.99
-0.076
-0.056
$x y$
21.     - 8.48

10-3-8.48
10-3
$x x$
6. 10-4
5.9948 10-4
5.9945 10-4 -0.086-0.091
$y y$
-6. 10-4
-5.9915 10-5
-5.9918 10-4 -0.141-0.136
xy
0. -6.36

10-8-6.36
10-8

INVA_2
6.9282 10-4
6.9211 10-4
6.9211 10-4 -0.102
-0.102
PRIN_1
-6. 10-4
-5.9936 10-4
-5.9939 10-4 -0.107
-0.102
PRIN_2
0.9 .38
10-16
-1.3394 10-15
PRIN_3
6. 10-4 5.9942
10-4
5.9939 10-4 -0.097
-0.102
VMIS
138.5641138 .1433138 .1433 -0.304
-0.304
TRESCA
160. 159.5142
159.5142
-0.304
-0.304
PRIN_1
-60. -59.7571

- 59.7571
-0.405
-0.405
PRIN_2

20. 19.9961
20.0001
-0.019
0.0007
PRIN_3
21. 99.7571
99.7571
-0.243
-0.243
VMIS_SG
138.5641138 .1433138 .1433 -0.304
-0.304
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## Identification

Reference
Aster
\% difference
$\boldsymbol{F} \boldsymbol{U}$
-2.1213 10-5 -2.1216
10-5-2.1212 10-5
0.014-0.006

## $v$

+2.1213 10-5 2.1216
10-5 2.1212
10-5
$0.014-0.006$
$x x$
20. 20.01
20.01
-0.104 0.04
yy
20. 19.99
19.99
-0.035-0.035
$z z$
20. 19.999
19.999
-0.027-0.007
$x y$
20. 20.02
20.02
0.0870 .087
0.1 .01

10-7 7.11
10-8
$y y$
0. -1.08

10-8-4.07
10-8
$x y$
$1.510-41.5012$
10-4 1.5012
10-4
0.080 .08

Checking of the passage to the node for the nodes mediums (only for the result obtained without to impose GONF = 0) - value on node NOEUMI:

## Identification

Reference
Aster
\% difference
$x x$
100. 99.92
-0.076
$y y$
-60. -59.92-0.127

## $z z$

20. 19.996
-0.076
$x x$
21. 10-4 5.9942

10-4-0.019
$y y$
-6. 10-4 -5.9936
10-4-0.107

### 3.5 Remarks

One obtains very good results since for all the examined sizes, the difference between solution obtained with the code and the analytical solution is lower than $0.2 \%$. It is seen that the

## variation enters

the solutions obtained by imposing or not the condition tr $=0$ is unimportant.
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## 4 Modeling

B

## 4.1

Characteristics of modeling
Grid with incompressible elements 3D of type TETRA10 only
F
Normally blocked face
$y$
D
E
C
$45^{\circ}$
With
B
$X$
Face with imposed pressure

Face blocked out of Dy
$A B$ is on axis $O X$ (contrary to modeling $A$ ).

## Cutting:

6 equidistant nodes on segments $A B, C D$ and $E F$, 5 equidistant nodes on arcs ACE and BDF.
Along axis Z:
total thickness $\boldsymbol{E}=0.01$
2 layers of elements
For the needs for examination, one defines the node $N O E U M I=A+(0.0 . e / 4)$ where deformations and the constraints are the same ones as in $A$.

## Limiting conditions:

DDL_IMPO =
GROUP_NO = ' FACSUP'
$D Z=$
0.

GROUP_NO
$={ }^{\prime}$ FACINF'
$D Z=$
0.
faces $\operatorname{AEFD}(z=0$ and $Z=0.01)$
$G R O U P \_N O={ }^{\prime} F A C E A B B^{\prime} D Y=$
0.
face $A B$
FACE_IMPO = GROUP_MA = ' FACEEF'
DNOR =
0.

## 4.2

Characteristics of the grid
A number of nodes: 703
A number of meshs: 356 TETRA10
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Code_Aster ${ }^{\circledR}$
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### 4.3 Functionalities

tested
Orders

DEFI_GROUP CREA_GROUP_NO
CRIT_NOEUD
"SUMMIT"
AFFE_MODELE MODELING "3D_INCO" GROUP_MA
DEFI_MATERIAU ELAS

AFFE_CHAR_MECA DDL_IMPO<br>GROUP_NO<br>FACE_IMPO<br>GROUP_MA<br>PRES_REP<br>GROUP_MA<br>STAT_NON_LINE COMP_INCR<br>RELATION<br>ELAS<br>NEWTON

## 4.4

Sizes tested and results
For the points $A$ and $F$, one notes the result obtained without imposing $G O N F=0$ (column 1) and while imposing
GONF $=0($ column 2$)$

## Identification

Reference
Aster
\% difference
With $U$
6. 10-5 6.011

10-5 6.011
10-5 0.19
0.18
$v$
0. 2.65

10-23-3.06
10-22 - -
xx
-60. -59.51
-61.1452
-0.82
1.90
yy
100. 100.95
101.345
0.95
1.34
$z z$
20. 20.42
19.813
2.08
0.93
xy
0. -2.104
-2.60

-     - 

$x x$
-6. 10-4 -6.02
+6. 10-4 6.01
10-4 6.01
10-4 0.20
0.19
xy
0. - 1.58

10
-5 -1.58
10-5 - -
INVA_2
6.9282 10-4
6.9509 10-4
6.9509 10-4
0.3280 .328

PRIN_1
-6. 10-4
-6.0241 10-4
-6.0244 10-4
0.4010 .406

PRIN_2
0. -2.85

10-6
-2.86 10-6
PRIN_3
6. 10-4 6.0153

10-4
6.0150 10-4
0.2550 .250

VMIS
138.5641137 .4852
137.4852
-0.779-0.779
TRESCA
160. $158.7541158 .7543-0.779-0.779$

PRIN_1
-60. -58.6561-58.6560-2.240-2.240
PRIN_2
20. 20.416720 .42092 .0832 .104

```
PRIN_3
100. 100.0980 100.0980 0.098 0.098
VMIS_SG
138.5641 137.4852
137.4852
-0.779 -0.779
```


## Identification

```
Reference
Aster
\% difference
F U
\(2.121310-52.1210\)
10-5
2.1206 10-5 -0.02-0.04
\(v\)
\(2.121310-52.1210\)
10-5
2.1206 10-5 -0.02-0.04
\(x x\)
20. 20.1320 .120 .6270 .60
yy
20. 19.9619 .98
-0.209-0.11
zz
20. 20.02520 .035
0.1270 .17
xy
\(-20 .-19.98-19.970 .105-0.13\)
\(x x\)
0.6 .85
10-7 6.55
10-7
yy
0. -5.68
10-5 -5.98
10-7
xy
\(2.121310-52.1210\)
10-5
```


# Code_Aster ${ }^{\circledR}$ 

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Checking of the passage to the node for the nodes mediums (only for the result obtained without to impose $G O N F=0$ ) - value on node NOEUMI:

## Identification

## Reference

Aster

\% difference
xx
$-60.58 .988$
-1.687
yy
100. 100.501
0.501
$z z$
20. 20.685
3.424
$x x$
-6. 10-4 -5.97503
10-4-0.416
yy
6. 10-4 5.98584

### 4.5 Remarks

The results obtained here are a little worse than in the case of modeling $A$, but discretization is coarser since there are approximately 2 times less nodes in this case-test.
results are satisfactory all the same since the variations are lower than $0.2 \%$ for displacements, lower than $0.5 \%$ for the deformations and lower than $1 \%$ for the constraints. One note again that there is no significant improvement of the result when one imposes explicitly $\mathrm{tr}=0$.
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## 5 Modeling

C

## 5.1

## Characteristics of modeling

Grid with incompressible elements 2D of type QUAD8 and TRIA6
y
B
Face blocked in $d x$
F
With

Normally blocked face

## E

Face with imposed pressure
$45^{\circ}$
X

## Limiting conditions:

DDL_IMPO =
GROUP_NO = 'GRNM11'
$D X=$
0 .
side $A B$
$F A C E \_I M P O=G R O U P \_M A=$ ' GRMA12'
DNOR =
0 .
dimensioned EF
PRES_REP $=$ GROUP_MA $=$ ' GRMA13'
CLOSE $=$
60.
face $A E$

## Name of the nodes:

With $=$ N2, $B=$ N361, $C=$ N121, $D=$ N584, $E=$ N155, $F=$ N503

## 5.2

## Characteristics of the grid

A number of nodes: 591
A number of meshs: 200 TRIAG, 50 QUAD8.

5.3 Functionalities<br>tested<br>\section*{Orders}<br>DEFI_GROUP CREA_GROUP_NO CRIT_NOEUD "SUMMIT"

# AFFE_MODELE MODELING 

"D_PLAN_INCO"
GROUP_MA
DEFI_MATERIAU ELAS

AFFE_CHAR_MECA DDL_IMPO<br>GROUP_NO<br>FACE_IMPO<br>GROUP_MA<br>PRES_REP<br>GROUP_MA<br>STAT_NON_LINE COMP_INCR<br>RELATION ELAS<br>NEWTON<br>REAC_ITER<br>1<br>Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal structures<br>HT-66/03/008/A

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## 5.4 <br> Sizes tested and results

For the points A and F, one notes the result obtained without imposing GONF $=0$ (column 1) and while

## imposing

GONF $=0($ column 2$)$.

## Identification

## Reference

Aster
\% difference
With $U$
0. 1.59

10-21
9.53 10-22
$v$
6. 105
5.9935 10-5
5.9932 10-5 -0.108
-0.113
xx
100. 99.9399 .93
-0.071
-0.071
yy
$-60 .-59.72-59.72$
-0.464
-0.464
zz
20. 20.0920 .10
0.470
0.490
xy
0. 0.224
0.224

-     - 

$x x$
6. 104
5.987 10-4
5.987 10-4 -0.212
-0.217
yy
6. 104
-5.986 10-4
-5.986 10-4 -0.237
-0.232
0. 1.68

10-6
1.68 10-6

INVA_2
6.9282 10-4
6.9127 10-4
6.9127 10-4 -0.224
-0.224
PRIN_1
-6. 10-4
-5.9858 10-4 -5.9861 10-4 -0.237
-0.232
PRIN_2
0. 0.0
0.0

PRIN_3
6. 10-4 5.9873

10-4
5.9870 10-4 -0.212
-0.217
VMIS
$138.5641138 .1093138 .1093-0.328$
-0.328
TRESCA
160. 159.4749
159.4749
-0.328
-0.328
PRIN_1
-60. -59.6335
-59.6335
-0.611
-0.611
PRIN_2
20. 20.0940
20.0980
0.470
0.490

PRIN_3
100. 99.8414

## Identification

Reference
Aster
\% difference
F U
2.1213105
-2.1195 10-5 -2.1191 10-5
-0.086
-0.106
$v$
+2.1213 105
2.1195 10-5
2.1191 10-5
-0.086
-0.106
$x x$
20. 20.03
20.03
0.153
0.153
yy
20. 19.97
19.97
-0.134
-0.134
$z z$
20. 19.99
19.997
-0.036
-0.016
xy
20. 20.01
20.01
0.051
0.051
$x x$

### 5.5 Remarks

As for modeling 3D, the results obtained are completely satisfactory.

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Author (S):

## Key S. MICHEL-PONNELLE

```
6 Modeling
D
```


## 6.1 <br> Characteristics of modeling

Incompressible elements axi (TRIA6 + QUAD8)
Center cylinder
Node blocked out of Dy
$y$
E
$F$
$0.01 m$
C
D
With
B
X
Face with imposed pressure

For the needs for examination, one defines the nodes:

NOEUMIA $=A+(0.0 .01 / 4)$ where the strains and stresses are the same one as in $A$

NOEUMIB $=B+(0.0 .01 / 4)$ where the strains and stresses are the same one as out of $B$

## Limiting conditions:

DDL_IMPO =
GROUP_NO = ' FACSUP'
$D Y=$
0 .
$y=0.1$
GROUP_NO
= ' FACINF'
$D Y=$
0 .
$y=0$

# PRES_REP $=G R O U P_{-} M A=' F A C E A E '$ 

CLOSE $=$
60.
face $A E$

## 6.2

## Characteristics of the grid

A number of nodes: 175.
A number of meshs and types: 20 QUAD8, 40 TRIA6.

6.3 Functionalities<br>tested<br>\section*{Orders}<br>DEFI_GROUP CREA_GROUP_NO<br>CRIT_NOEUD<br>"SUMMIT"<br>AFFE_MODELE MODELING<br>"AXIS_INCO"<br>GROUP_MA<br>DEFI_MATERIAU ELAS

AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FACE_IMPO
GROUP_MA
PRES_REP
GROUP_MA
STAT_NON_LINE COMP_INCR
RELATION
ELAS
NEWTON
REAC_ITER
1

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## 6.4 <br> Sizes tested and results

For A and F, one notes the results obtained without imposing GONF $=0$ (column 1) and while imposing $G O N F=0($ column 2).

## Identification

Reference
Aster
\% difference
With $U$
6. 105
6.0000 10-5 5.9996

10-5
$-0.002-0.007$
$v$
0. 9.93

10-24-8.93
10-23
$x x$
-60. -59.91
-59.91

```
-0.150-0.150
yy
20.19.99
19.997 -0.036 -0.016
zz
100. 99.91
99.91
-0.086 -0.086
xy
0. 3.73
10-9
```

3.73 10-9
$x x$
6. 104
-5.99 10-4 -5.99
10-4
$-0.180-0.123$
yy
0. -1.69
10-15-1.69
10-15
$x y$
0 .
2.80 10-14 2.80
10-14
INVA_2
6.9282 10-4
6.9201 10-4
6.9201 10-4
-0.117-0.117
PRIN_1
-6. 10-4
-5.9923 10-4
-5.9926 10-4
-0.128-0.123
PRIN_2
0. 6.45
10-16
6.40 10-16
6. 10-4 5.9937

10-4
5.9934 10-4
$-0.105-0.110$
VMIS
138.5641138 .4116
138.4116
-0.110-0.110
TRESCA
160. 159.8239 159.8239-0.110-0.110

PRIN_1
-60. -59.9101-59.9101-0.150-0.150
PRIN_2
20. 19.9928 19.9968-0.036-0.016

PRIN_3
100. $99.913899 .9138-0.086-0.086$

VMIS_SG
138.5641
138.4116
138.4116
$-0.110-0.110$

## Identification

## Reference

Aster
\% difference
F U
3 10-5 3.0004
10-5
2.9998 10-5
$0.014-0.006$
$v$
0. -4.96

10-22
$-5.6910-22$
$x x$
0. 2.59

10-2
2.58 10-2
$x x$

$$
-1.5 \text { 10-4 -1.498 }
$$

10-4-1.498

$$
10-4
$$

$$
-0.129-0.109
$$

$$
y y
$$

0. -3.20

$$
10-8-3.20
$$

$$
10-8
$$

Checking of the passage to the node for the nodes mediums (only for the result obtained without to impose $G O N F=0$ )

## NOEUMIA

## Identification

## Reference

Aster
\% difference
$x x$
-60. -59.91-0.150
yy
20. 19.99
-0.036
$z z$
100. 99.91
-0.086
$x x$
-6. 10-4
$-5.992310-4-0.128$
z.
6. 10-4
5.9937 10-4 -0.105

## Handbook of Validation

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## Code_Aster ${ }^{\circledR}$

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## NOEUMIB

## Identification

Reference
Aster
\% difference
$x x$
0. -1.39

10-2
yy
20. 19.9999
-0.002
zz
40. 39.9993
-0.002
xx

### 6.5 Remarks

The precision obtained is very good.

## 7 <br> Summary of the results

With a Poisson's ratio very close to 0.5 , one finds the results of the solution analytical incompressible with a weak difference. It is noticed that it is not necessary to impose explicitly the condition of incompressibility $\mathrm{tr}=0$ to obtain good results since the results are quasi-identical that one activates or not, condition $G O N F=0$ with DDL_IMPO.

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Date:
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Author (S):
C. Key DURAND

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Organization (S): EDF/MTI/MMN

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## Summary

This case test validates modelings relating to linear elasticity which implement materials orthotropic whose properties are known in a reference mark defined by the user different from the total reference mark.
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## 1 <br> Problem of reference

### 1.1 Geometry

The total reference mark is reference mark (A, X, Y, Z). In this reference mark the co-ordinates of the nodes are:

To (0. , 0. , 0.)
B (3. , 1. , 0.)
C (2., 3. , 0.)
D (3.1, - 1)
For the 2D, one will study the behavior of the triangle ABC whose material properties are defined in the total reference mark $(A, X, y)$ represented on the figure; this reference mark is turned of an angle of $30^{\circ}$ around
of $Z$ compared to the total reference mark.
For the 3D, one will study the behavior of the tetrahedron ABCD whose material properties are defined in a local reference mark $(A, X, y, Z)$ obtained by rotation of the total reference mark according to angles'
nautical $\left(=30^{\circ},=20^{\circ},=10^{\circ}\right)$.
This reference mark is not represented on the figure.

## 1.2

Properties of material
The materials used are orthotropic and isotropic transverse.
One adopts the convention of terminology used in ASTER, i.e the suffixes L, T and NR means Longitudinal, Transverse and Normal.

The units will not be specified.
$E L=$

```
5000 IN =
8000
=0.,
18
=0.,
15
= 0.11
LT
LN
TN
EL = 11000; AND = 5000; IN = 8000
= 0.,
18
=0.,
15
= 0. ,
11
LT
LT
TN
G=
10500 G=
7000 G=13000
LT
LT
TN
EL
EL
AND
(It is known that
X
```

```
=
X,
TL
LT
NL
LN
NT
TN
AND
IN
IN
that is to say =
0.396,
=
6 2
0 2 0
=06875
0
TL
NL
NT
For the transverse isotropy, one keeps the same values while knowing as:
AND = EL,
=
G
and
TL
LN
EL
\(L T=2(1\)
```

```
+
LT
```

It is pointed out that these coefficients are defined in a local reference mark $(A, L, T, N R)$ turned of $30^{\circ}$ in plan $(L, T)$ compared to the reference mark total for the 2 D and turned with the nautical angles $\left(30^{\circ}\right.$, $\left.20^{\circ}, 10^{\circ}\right)$
compared to the total reference mark for the 3D.
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## 1.3 <br> Boundary conditions and loadings

The boundary conditions are of Dirichlet type. One makes the assumption of a field of displacement linear in $X$ and $y$ so that the field of deformation is constant.

For the $2 D$ one takes
$d X=2 x+4 y$
$D y=4 x+3 y$
For the $3 D$ one takes
$d X=2 x+3 y+4 z$
$D y=3 x+5 y+6 z$
$d Z=4 x+6 y+7 z$
For the 2D, one thus will impose:
for node A
$d X=0, D y=0$
for the node $B$
$d X=10, D y=15$
for the node $C$
$d X=16, D y=17$
and for the 3D:
for node A
$d X=0, D y=0, d Z=0$
for the node $B$
$d X=9, D y=14, d Z=18$
for the node $C$
$d X=13, D y=21, d Z=26$
for the node $D$
$d X=5, D y=8, d Z=11$

## 2 <br> Reference solution

## 2.1 <br> Method of calculation

Calculation is analytical.
One used the formal calculation programme Mathématica to carry it out.

One exposes of it the principle only for the $3 D$.
It is known that the field of displacement is:
$d X=2 x+3 y+4 z$
$D y=3 x+5 y+6 z$
$d Z=4 x+6 y+7 z$

The field of deformations $G$ in the total reference mark is thus constant and equal to:

234
$G=356$
467
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That is to say $P$ the matrix of passage allowing to make pass a vector of the total reference mark ( $A, X$, $Y, Z)$ to locate local ( $A, L, N R, T)$.

That is to say
$T$
L the tensor of deformation in the local reference mark. One a:

$$
L=P
$$

. $P$
$G$

The tensor of Hooke H L is known in the local reference mark, that is to say L the tensor of the constraints in it
locate. One a:
$=H$.
$L$
L
$L$
The tensor is obtained
$T$
$G$ of the constraints in the total reference mark by:
$=P$.
G
L. $P$
2.2

## Results of reference

They are obtained by carrying out the operations described above with Mathematica.

## 2.3 <br> Uncertainties on the solution

Uncertainty is null because the solution is analytical.

### 2.4 References <br> bibliographical

For the description of the matrices of Hooke for materials isotropic transverse and orthotropic for plane modelings 3D, constraints and plane deformations, the selected reference was:
`Matrix of Hooke for orthotropic materials `. Report/ratio interns applications in Mechanics
$n^{\circ}$ 79-018 of Jean-Claude Masson CISI.
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## 3 Modeling

## With

## 3.1 <br> Characteristics of modeling

Following modelings are implemented:
$2 D$ :

- axisymmetric
- constraints
plane
- deformations
plane
$3 D$.
For each one of these modelings, one tests materials isotropic transverse and orthotropic.
Note:
has) The transverse isotropy is not tested for the plane constraints because this case corresponds to isotropy.
b) For the axisymmetric case the stress field depends on the point of calculation.

This point is selected at the point of integration of the triangle (i.e it is the centre of gravity of the triangle).
c) It is pointed out that the orthtropie in an unspecified reference mark is not available for modeling as a Fourier because there is then coupling of all the components of the tensor of constraints:
Implementation the current makes it possible to use only the symmetrical components to leave which one can find the antisymmetric components but so that it is possible, it is not necessary that the slips induce tensile stresses.

## 3.2 <br> Characteristics of the grid

For the $2 D$, there is an element triangle with 3 nodes $A B C$.
For the $3 D$, there is an element tetrahedron with 4 nodes $A B C D$.

### 3.3 Functionalities <br> tested

## Orders

Key word
DEFI_MATERIAU ELAS_ORTH

DEFI_MATERIAU ELAS_ISTR

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## 4

Result of modeling $A$

### 4.1 Values

tested

## Identification Reference

Aster \%
difference
Case of the transverse isotropy 3D
name of the result: Mest 1
field depl

```
Dy (c)
21
21
0
field epsielgadepl
```

Epxy 3
3
0
Epxz 4
4
0
Epyz 6
6
0
field sief.elga.depl
If $x x$
50461,97
50461,97
0
If yy
80136,037
80136,037
0
If zz
68682,137
68682,137
0
If $x y$
39559,096
39559,096
0
If xz
30622,542
30622,542
0
If $y z$
84027,579
84027,579

```
If xx
50461,971
50461,971
O
field emelelga Ep
1.23652.106 1.23652.106
```

Field emelelnoelga Ep 1.23652.106 1.23652.106

## Case of the orthotropism 3D

name of the result: Mest 2
field depl

```
Dy (c)
21
21
O
field epsielgadepl
```

Epxy 3
3
0
Epxz 4
4
0
Epyz 6
6
0
field siefelgadepl

```
If xx
23170,539
23170,539
O
If yy
78600,676
78600,676
O
If zz
78692,318
78692,318
O
If xy
86435,100
86435,100
O
If xz
16449,622
16449,622
O
If yz
125577,226
125577,226
O
field sigmelnodepl
if xx
2370,539
2370,539
O
field enelelga Ep
1.55286.106 1.55286.106 0
field enelelnoelga Ep
1.55286.106 1.55286.106 0
Case of the transverse isotropy in
```


## axisymmetric

name of the result: Mest 3

Dy (c)
17
17
0
field epsielgadepl

Exxy 4<br>4<br>0<br>field siefolgadepl

## If $x x$

42930,079
42930,079
0
If yy
52252,113
52252,113
0
If $x y$
37288, 135
37288,135
0
field enelelga Ep 4.15741.105 4.15741.105 0

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```
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field enelcluoelga Ep
4.15741.105 4.15741.105
0
Case of the orthotropism into axisymmetric
```

name of the result: Mest 4
field depl Dy (c)
17
17
0
field epsielgadepl

```
Epxy 4
4
O
field siefelgadepl
```

If $x x$

19438,248
19438,248
0
If yy
75231,714
75231,714
0
If $x y$

```
53867?974
53867?974
O
field siefelgaelga Ep
4,91317-105 4,91317-1050
field enelchroelga Ep
4.91317-105 4.91317-105 0
Case of the transverse isotropy in
```


## plane deformations

name of the result: Mest 5

field depl Dy (c)<br>17<br>17<br>0<br>field epsielgadepl Epxy<br>4<br>4<br>0<br>field siefelgadepl

If $x x$
31612,684
31612,684
0
If $y z$
40934,718
8
0
If $x y$
37288,135
37288,135
0
field sigmelnodepl
if $x x$

```
31612,684
31612,684
O
field enelelga Ep
2.42167.105 2.42167.1050
field enelelnoelga Ep
2.42167.105 2.42167.1050
Case of the orthotropism in deformations
```


## plane

name of the result: Mest6
field depl Dy (c)
17
17
0
field epsielgadepl Epxy
4
4
0
field siefelgadepl

If $x x$
9931,422
9931,422
0
If yy
68733,870
68733,870
0
If $x y$
51262,119
51262,119
0
field sigmelnodepl
if $x x$

```
9931,422
9931,422
O
field Epenelelga Ep
3.180807.105 3.180807.1050
field enelelnoelga
3.180807.105 3.180807.105 0
Case of the orthotropism in constraints
```


## plane

name of the result: Mest 7
field depl Dy (c)
17
17
0
field epsielgadepl Epxy
4
4
0
field siefelgadepl

If $x x$
7454,007
7454,007
0
field emelelga Eo
3.10347.105 3.10347.105 0
field emelelnoelga Ep
3.10347.105 3.10347.105 0

In the asymmetrical case, the values of the field of the deformations and field of the constraints are given to the point of integration of the triangle (i.e its centre of gravity) whose co-ordinates are:
$X=1.666667$
$Y=1.333334$
$Z=0$
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## 5 <br> Summary of the results

The results provided by Mathématica and Aster are identical for all modelings usable with materials isotropic transverse and orthotropic.
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Date:
15/02/06
Author (S):
E. GALENNE, J.M. PROIX Key

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Organization (S): EDF-R \& D /AMA

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SSLV134-Fissure circular in infinite medium

## Summary

This test allows, after obtaining the field of displacement by MECA_STATIQUE, the calculation of the rate of restitution of energy room for a circular crack plunged in a presumedly infinite medium.

For the first modeling, only a half space defined by the plan of the crack is represented. Bottom of crack is then a closed curve (a circle) and is defined as such in DEFI_FOND_FISS. The rate of restitution room and total is compared with the analytical solution of reference.

Three following modelings make it possible to calculate the stress intensity factors K1 and K3, in 3D and axisymmetric, calculated by POST_K1_K2_K3.

Modeling B tests K1 for a grid 3D,
Modeling C tests K1 for an axisymmetric grid,
Modeling $D$ tests the combination of K1 and K3 for a grid 3D.
Lastly, modeling E makes it possible to validate the calculation of the bilinear form of $G$ on the same problem, and
modeling $F$ to validate same calculation for $G$ local.

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## 1

Problem of reference

### 1.1 Geometry

## Z

$=1 . \mathrm{MPa}$
$y$
has
$X$
$=1 . M P a$

The crack is circular (penny shaped ace) of ray has, in the Oxy plan. So that the medium is regarded as infinite, the sizes characteristic of the solid mass are about 5 times higher with ray $A$.

## 1.2 <br> Material properties

Young modulus: $E=2.105 \mathrm{MPa}$
Poisson's ratio: $=0.3$
1.3

## Boundary conditions and loadings

Lower face
: uniform constraint of traction $\mathrm{Z}=1 . \mathrm{MPa}$
Higher face
: uniform constraint of traction $Z=1 . M P a$
According to modeling, one also has boundary conditions of symmetry and blocking of movements of rigid body.

In the modeling $D$ where only the quarter of the parallelepiped is represented, one uses conditions with the limits of antisymetry for the loading of torsion: they amount imposing null them tangential displacements with a face. The loading of torsion is introduced in the form of a force surface tangential (shearing distributed) applied to the lips of the crack.

```
Y
X
Upper lip: F = -
= +
X
and F
has
Y
has
Y
X
Lower lip: F = +
=-
and F
has
Y
has
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```

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## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

For a circular crack of ray in an infinite medium, subjected to a uniform traction according to the normal in the plan of the lips, the rate of refund of energy room $G(S)$ is independent of the curvilinear $X$-coordinate $S$ and is worth [bib1]:

```
(1-2
```

)
$G(S)=$
42
has

## E

then the coefficient of intensity of K1 constraint is given by the formula of Irwin:
(1-2
)
2 A
G (S) =
$K 2$ is $K=$

If this crack is subjected to a shearing distributed on the lips:

## = <br> Z

has
(what is equivalent to a torsion ad infinitum), then one is in pure mode 3 and the factor of intensity of constraints corresponding is worth:
4 A
(1+)
$K=$
=
2
3
thus by the formula of Irwin $G(S)$
K
3
E
3
In the presence of the two combined modes, one will have:

```
(1-2
)
(1+)
G(S)=
K 2 +
K2
E
I
E
3
```

The théta-method connects the rates of refund of energy total and local by the equation variational following:

G
() $=$
ref.
$G(S) . m(S) d s$
where $m(S)$ is the normal at the bottom of crack and is the field speed of a virtual propagation crack.

If one chooses for the normal unit field at the bottom of crack, one obtains, since $\boldsymbol{G}(\mathbf{S})$ is

```
G
()=G(S 2
).
has
réf
```

2.2
Results of reference
Numerical application (case with loading of traction only):
It is considered that the crack is circular of ray has $=2 . m$Handbook of ValidationV3.04 booklet: Linear statics of the voluminal structuresHT-62/06/005/A

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For the loading considered, one obtains then:
$G(S)=$
11.586 J/m2

Gréf $=145.060$
J/m
K3
=
1.5958E6 J/m2

Numerical application (case with loading of torsion only):
$G(S)=$
7.3565 J/m2

Gréf $=92.44$
J/m
K1
=
1.0638E6 J/m2

### 2.3 Reference <br> bibliographical

Solution of Sneddon (1946) in G.C. SIH: Handbook of stress-intensity factors Institute of Fracture and Solid Mechanics - Lehigh University Bethlehem, Pennsylvannie

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## 3

Modeling a: Melts of crack closed, calculation of $G$.

## 3.1 <br> Characteristics of modeling

The interest of this modeling is to represent the entirety of the bottom of crack which is a curve closed, without benefitting from symmetries of the problem.

Only the loading of traction is taken into account.

## 3.2

Characteristics of the grid
A number of nodes: 11114
A number of meshs and type: 2432 PENTA 15

### 3.3 Functionalities

tested

DEFI_FOND_FISS FOND_FERME

CALC_THETA

CALC_G_THETA_T SYME_CHAR=' SYME'

CALC_G_LOCAL_T SYME_CHAR=' SYME'

POST_K1_K2_K3 SYME_CHAR=' SYME'

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### 3.4 Notice

One uses key word SYME_CHAR in operators CALC_G_THETA_T and CALC_G_LOCAL_T for to multiply automatically by two the rate of refund of energy calculated on only one lip of
fissure.
In the same way in POST_K1_K2_K3, key word SYME_CHAR allows to calculate the factors of intensity
constraints and the rate of refund of energy G_IRWIN by interpolation of displacements of a single lip of the crack. The displacement of the nodes mediums of the edges of the elements concerning the bottom of crack to the quarter of these edges would allow to improve the precision of calculation.

## 4 <br> Results of modeling $A$

### 4.1 Values

tested

## Identification Reference

Aster \%
difference

## G total

145.6
146.2
0.4

G local Node A - G Lagrange
11.586
11.82
2.0

G local Node B-G Lagrange
11.586
11.56
-0.2
G local Node C - G Lagrange
11.586
11.83
2.1

G local Node D - G Lagrange
11.586
11.81
1.9

G local Node A - G Lagrange_no_no
11.586
11.71
1.0

## G local Node B - G Lagrange_no_no

11.586
11.60
0.2

G local Node C - G Lagrange_no_no
11.586
11.72
1.2

G local Node D - G Lagrange_no_no
11.586
11.70
1.0

G (POST_K1_K2_K3 Method 3) - Node A
11.586
10.45
9.8

G (POST_K1_K2_K3 Method 3) - Node B
11.586
10.49
9.4

G (POST_K1_K2_K3 Method 3) - Node C
11.586
10.45
9.8

G (POST_K1_K2_K3 Method 3) - Node D
11.586
10.52
9.2

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## 5 <br> Modeling b: Post_K1_K2_K3 in 3D

## 5.1 <br> Characteristics of modeling

This modeling makes it possible to test the calculation of K1 using POST_K1_K2_K3 (method of extrapolation of displacements on the lips of the crack). Parameter ABSC_CURV_MAXI of the operator is selected so as to retain 5 nodes on the segment of extrapolation (dmax $=0,35$ ).

Only the loading of traction is taken into account.

## 5.2 <br> Characteristics of the grid

A number of nodes: 6536
A number of meshs and type: 432 PENTA 15 and 987 HEXA 20
The nodes mediums of the edges of the elements touching the bottom of crack are moved with the quarter of these
edges, to obtain a better precision.

### 5.3 Functionalities

tested

## Orders

DEFI_FOND_FISS
CALC_G_LOCAL_T
CALC_THETA THETA_3D
CALC_G_THETA_T
POST_K1_K2_K3

### 5.4 Notice

One represents only the quarter of the complete three-dimensional block and thus the quarter of the crack. Thus, it
is necessary to divide the theoretical value of reference of the total rate of refund by 4:
G
$=145.60 / 4=36.40 \mathrm{~J} / \mathrm{m}$
glob
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6
Results of modeling B

### 6.1 Values

tested

### 6.1.1 Results <br> of

CALC_G_THETA_T and $C A L C_{-} G_{-} L O C A L_{-} T$
Identification Reference
Aster \%
difference
G local Node 49
11.59
11.74

1,30
G local Node 1710

G local Node 77
11.59
11.73

1,22
G total
36.40
36.82

1,16

### 6.1.2 Results

of
POST_K1_K2_K3

## Identification Method

Reference
Aster \%
difference
Kl_MIN
Node 77
method 1
16.0E+05
$15.9 E+05$
-0,04
Kl_MAX
Node 77
method 1
$16.0 E+05$
$16.1 E+05$
0,66
G_MIN
Node 77
method 1
11.6E00
11.6E00
-0,08
G_MAX
Node 77
method 1
11.6E00
11.7E00

1,32
Kl_MIN
Node 49
method 1
16.0E+05
$15.9 E+05$
-0,03
Kl_MAX
Node 49
method 1
16.0E+05
$16.1 E+05$
0,66
G_MIN
Node 49
method 1
11.6E00
11.6E00
-0,06
G_MAX
Node 49
method 1
11.6E00
11.7E00

1,32
K1_MIN
Node 1710 method 1
16.0E+05
$15.9 E+05$
-0,07
Kl_MAX
Node 1710 method 1
16.0E+05
$16.1 E+05$
0,61
G_MIN
Node 1710 method 1
11.6E00
11.6E00
-0,15
G_MAX
Node 1710 method 1
11.6E00
11.7E00

1,22
Kl_MIN
Node 77
method 2
16.0E+05
$15.3 E+05$
$-4,02$
Kl_MAX
Node 77
method 2
16.0E+05
$15.9 E+05$
-0,21
G_MIN
Node 77
method 2
11.6E00
10.7E00
-7,87
$G_{-} M A X$
Node 77
method 2
11.6E00
$11.5 E 00$
-0,42
Kl_MIN
Node 49
method 2
16.0E+05
$15.3 E+05$
-4,04
Kl_MAX
Node 49
method 2
16.0E+05
$15.9 E+05$
-0,40
G_MIN
Node 49
method 2
11.6E00
10.6E00

G_MAX
Node 49
method 2
11.6E00
11.5E00
-0,63
K1_MIN
Node 1710 method 2
16.0E+05
$15.3 E+05$
-4,09
K1_MAX
Node 1710 method 2
16.0E+05
$15.9 E+05$
-0,25
G_MIN
Node 1710 method 2
11.6E00
10.6E00
-8,08
G_MAX
Node 1710 method 2
11.6E00
$11.5 E 00$
-0,49
K1
Node 77
method 3
16.0E+05
$15.5 E+05$
-2,62
G
Node 77
method 3
11.6E00
11.0E00
-5,16
K1
Node 49
method 3
$16.0 E+05$
11.6E00
11.0E00
$-5,19$
K1
Node 1710 method 3
16.0E+05
$15.5 E+05$
-2,68
G
Node 1710 method 3
11.6E00
11.0E00
$-5,29$

Note:

Method 3 calculates a single value for each parameter (K1_MAX = K1_MIN in file result).

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## 7.1 <br> Characteristics of modeling

This modeling makes it possible to test the calculation of K1 using POST_K1_K2_K3 (method of extrapolation of displacements on the lips of the crack) into axisymmetric.

Only the loading of traction is retained in this modeling.
Since one is in axisymmetric modeling, the relation between the total rates of refund of energy and room is [R7.02.01]:

```
G
() G (S.A.
```

réf
).
$=$
that is to say here $G$
$=23.17 \mathrm{~J} / \mathrm{m}$
ref.

## 7.2

## Characteristics of the grid

A number of nodes: 1477
A number of meshs and type: 402 QUAD 8 and 60 SORTED 6
The nodes mediums of the edges of the elements touching the bottom of crack are moved with the quarter of these
edges, to obtain a better precision.

7.3 Functionalities<br>tested<br>Orders<br>DEFI_FOND_FISS<br>CALC_G_LOCAL_T<br>CALC_THETA THETA_3D

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## 8

Results of modeling $C$

### 8.1 Values

tested

## Identification

Method
Reference
Aster
\% Difference
G
CALC_G_THETA
232E01
236E01
1,662

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## 9

Modeling D: Post_K1_K2_K3 in 3D modes 1 and 3

## 9.1 <br> Characteristics of modeling

The boundary conditions following are successively applied:
traction: as for modeling B;
torsion.
This modeling makes it possible to test the calculation of K1 and K3 combined using POST_K1_K2_K3 (method of extrapolation of displacements on the lips of the crack).

The nodes mediums of the edges of the elements touching the bottom of crack are moved with the quarter of these
edges, to obtain a better precision.

## 9.2

Characteristics of the grid

A number of nodes: 6536
A number of meshs and type: 432 PENTA 15 and 987 HEXA 20
The nodes mediums of the edges of the elements touching the bottom of crack are moved with the quarter of these edges, to obtain a better precision.

### 9.3 Functionalities

tested

## Orders

DEFI_FOND_FISS
CALC_G_LOCAL_T
CALC_THETA THETA_3D
CALC_G_THETA_T
POST_K1_K2_K3
AFFE_CHAR_MECA FORCE_FACE
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### 9.4 Notice

The two loading cases (traction and torsion) are taken into account. It is thus necessary to cumulate the values of $G$
for the two loadings. Moreover, one represents only the quarter of the complete three-dimensional block and
thus the quarter of the crack, it is thus necessary to divide the theoretical value of reference of the rate of refund
total by 4.
Thus
$G(S)=(11.586+7.356)=18.943 \mathrm{~J} / \mathrm{m} 2$
$G=(145.06+92.44) / 4=59.37 \mathrm{~J} / \mathrm{m}$
Only traction contributes to K1, only torsion contributes to $K 3$.
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## 10 Results of modeling $D$

### 10.1 Values

tested

## Identification <br> Method <br> Localization <br> Reference <br> Aster

G_LOCAL
CALC_G_LOCAL Legendre
Node 49
1,8943E+01 1,9055E+01
0,59
G_LOCAL
CALC_G_LOCAL Legendre
Node 1710
$1,8943 E+011,9089 E+01$
0,772
G_LOCAL
CALC_G_LOCAL Legendre
Node 77
1,8943E+01 1,9074E+01
0,694
G
CALC_G_THETA
$5,937 E+015,9849 E+01$
0,567

K1_MAX
POST_K1_K1_K3 Method 1
Node 49
$1,5958 E+061,5988 E+06$
0,189
K1_MIN
POST_K1_K1_K3 Method 1
Node 49
1,5958E+06 1,5952E+06
-0,038
K1_MAX
POST_K1_K1_K3 Method 2
Node 49
1,5958E+06 1,5924E+06
-0,21
K1_MIN
POST_K1_K1_K3 Method 2
Node 49
$1,5958 E+06$ 1,5620E+06
-2,116
K1_MAX
POST_K1_K1_K3 Method 1
Node 1710
$1,5958 E+06$ 1,5990E+06
0,202
K1_MIN
POST_K1_K1_K3 Method 1
Node 1710
$1,5958 E+061,5953 E+06$
-0,029
K1_MAX
POST_K1_K1_K3 Method 2
Node 1710
$1,5958 E+061,5925 E+06$
-0,202
Kl_MIN
POST_K1_K1_K3 Method 2
Node 1710
$1,5958 E+061,5618 E+06$
-2,129
K1_MAX
POST_K1_K1_K3 Method 1
Node 77
$1,5958 E+061,5982 E+06$
0,155
K1_MIN
POST_K1_K1_K3 Method 1
Node 77
$1,5958 E+061,5945 E+06$
-0,077
K1_MAX
POST_K1_K1_K3 Method 2
Node 77
$1,5958 E+061,5918 E+06$
-0,249
K1_MIN
POST_K1_K1_K3 Method 2
Node 77
1,5958E+06 1,5610E+06
-2,176

## K3_MIN

POST_K1_K1_K3 Method 1
Node 49
$1,0638 E+061,0564 E+06$
-0,704
K3_MAX
POST_K1_K1_K3 Method 1
Node 49
$1,0638 E+061,0589 E+06$
-0,464
K3_MIN
POST_K1_K1_K3 Method 2
Node 49
$1,0638 E+06$ 9,4420E +06
-11,246
K3_MAX
POST_K1_K1_K3 Method 2
Node 49
1,0638E+06 1,0387E+06
-2,361
K3_MIN
POST_K1_K1_K3 Method 1
Node 1710
$1,0638 E+061,0564 E+06$
-0,703
K3_MAX
POST_K1_K1_K3 Method 1
Node 1710
$1,0638 E+061,0589 E+06$
-0,464
K3_MIN
POST_K1_K1_K3 Method 2
Node 1710
$1,0638 E+069,4421 E+05$
-11,245
K3_MAX
POST_K1_K1_K3 Method 2
Node 1710
$1,0638 E+061,0387 E+06$

K3_MIN
POST_K1_K1_K3 Method 1
Node 77
$1,0638 E+061,0563 E+06$
-0,708
K3_MAX
POST_K1_K1_K3 Method 1
Node 77
$1,0638 E+061,0589 E+06$
-0,468
K3_MIN
POST_K1_K1_K3 Method 2
Node 77
1,0638E+06 9,4413E+05
-11,253
K3_MAX
POST_K1_K1_K3 Method 2
Node 77
$1,0638 E+061,0387 E+06$
-2,366


```
Node 49
    1,8943E+01 1,8551E+01
    -2,069
    G_MIN
    POST_K1_K1_K3 Method 1
    Node 1710
    1,8943E+01 1,8868E+01
    -0,395
    G_MAX
    POST_Kl_Kl_K3 Method 1
    Node 1710
    1,8943E+01 1,8887E+01
    -0,296
    G_MIN
    POST_Kl_Kl_K3 Method 2
    Node 1710
    1,8943E+01 1,6893E+01
    -10,819
    G_MAX
    POST_Kl_Kl_K3 Method 2
    Node 1710
    1,8943E+01 1,8553E+01
    -2,058
    G_MIN
    POST_Kl_Kl_K3 Method 1
    Node }7
    1,8943E+01 1,8856E+01
    -0,457
    G_MAX
    POST_K1_K1_K3 Method 1
    Node }7
    1,8943E+01 1,8875E+01
    -0,358
    G_MIN
    POST_K1_K1_K3 Method 2
    Node }7
    1,8943E+01 1,6882E+01
    -10,881
    G_MAX
    POST_Kl_Kl_K3 Method 2
    Node }7
    1,8943E+01 1,8541E+01
    -2,12
```

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## 11 Modeling E: Calculation of the bilinear form of $G$

### 11.1 Characteristics of modeling

The grid is identical to that of preceding calculations, but only the eighth of the block is retained (Oxyz quadrant)

1) Loading 1: Idem modeling B.
2) Loading 2: Face $X=10$. : uniform constraint of traction $Z=1$,

Face $Z=10$. : uniform constraint of traction $X=1$ (shearing).
3) Loading 3: Loading $1+$ Loading 2.
4) Loading 4: Loading 2 Loading 1.

Four calculations are static are carried out respectively producing displacements $U, v, u+v$, and considering.
11.2 Characteristics of the grid

A number of nodes:
2774
A number of meshs and type: 392 HEXA20 and 216 PENTA15
11.3 Functionalities
tested
Orders

## DEFI_MATERIAU ELAS

DEFI_LIST_REEL
"MECHANICAL" AFFE_MODELE
"3D"
AFFE_MATERIAU ALL

AFFE_CHAR_MECA FORCE_FACE
GROUP_MA
PRES_REP
NEAR
STAT_NON_LINE

DEFI_FOND_FISS

CALC_THETA

CALC_G_THETA_T COMP_ELAS
"CALC_G"
CALC_G_THETA_T COMP_ELAS
"CALC_G_BILI"

### 11.4 Notice

One represents only the eighth of the complete three-dimensional block and thus the eighth of the crack.
Thus, it is necessary to divide the theoretical value of reference of the total rate of refund by 8.
The bilinear form $G(U, v)$ checks the following properties:
$\boldsymbol{G}(\boldsymbol{U}, \boldsymbol{U})=\boldsymbol{G}(\boldsymbol{U})($

```
form)
l
G(U+v)-G(U-v)
G(U,v)=
(
form)
2
4
from where
G(2u)-G(2
-v)
G(U-v,U+v)=
=G(U)-G(v)
(
form)
3
4
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```

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12 Results of modeling $E$

### 12.1 Values

tested

## Identification Reference

Aster \%
difference
G total: $\boldsymbol{G}(\boldsymbol{U})$
1.82 10+1
$1.827310+1$
0.4

G bilinear: $\boldsymbol{G}(\boldsymbol{U}, \boldsymbol{U})$
1.82 10+1 1.8273

10+1 0.4
G total: $\boldsymbol{G}(\boldsymbol{v})$

### 6.8612

G bilinear: $G(v, v)$
form. 1
6.8612
0.

G total: $\boldsymbol{G}(u+v)$
-
4.7526 10+1 -

G bilinear: $\boldsymbol{G}(u+v, u+v)$
form. 1
$4.752610+10$.
G total: $\boldsymbol{G}(\boldsymbol{U} V)$
-
2.7428

G bilinear: $\boldsymbol{G}(\boldsymbol{U V}, \boldsymbol{U} \boldsymbol{V})$
form. 1
2.7428
0.

G bilinear: $G(v, U)$
form. 2
$1.119510+10$.
G bilinear: $\boldsymbol{G}(\mathbf{U V}, u+v)$
form. 3
$1.141210+10$.
One indicates by $U$ displacement corresponding to loading 1, and v corresponding displacement with loading 2. Loadings 3 and 4 correspondent with displacements ( $u+v$ ) and (considering).

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## 13 Modeling F: Calculation of the bilinear form of G local

### 13.1 Characteristics of modeling

The grid is identical to that of modeling $E$.

1) Loading 1: Idem modeling B.
2) Loading 2: Face $X=10$. : uniform constraint of traction $Z=1$,

Face $Z=10$. : uniform constraint of traction $X=1$ (shearing).
3) Loading 3: Loading $1+$ Loading 2.
4) Loading 4: Loading 2 Loading 1.

Four calculations are static are carried out respectively producing displacements $U, v, u+v$, and considering.

### 13.2 Characteristics of the grid

A number of nodes:
2774
A number of meshs and type: 392 HEXA20 and 216 PENTA15

### 13.3 Functionalities

tested

## Orders

DEFI_GROUP CREA_GROUP_NO

DEFI_MATERIAU ELAS

DEFI_LIST_REEL
"MECHANICAL" AFFE_MODELE
"3D"
AFFE_MATERIAU ALL

AFFE_CHAR_MECA FORCE_FACE
GROUP_MA
PRES_REP
NEAR
STAT_NON_LINE

DEFI_FOND_FISS

CALC_G_LOCAL_T COMP_ELAS
"CALC_G"
CALC_G_LOCAL_T COMP_ELAS
"G_BILINEAIRE"

### 13.4 Notice

One represents only the eighth of the complete three-dimensional block and thus the eighth of the crack. Thus, it is necessary to divide the theoretical value of reference of the total rate of refund by 8.

The bilinear form $G(U, v)$ checks the following properties:
$G(U, U)=G(U)($

```
form)
I
G(U+v)-G(U-v)
G(U,v)=
(
form)
2
4
from where
G(2u)-G(2
-v)
G(U-v,U+v)=
=G(U)-G(v)
(
form)
3
4
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```

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## 14 Results of modeling $F$

### 14.1 Values

tested
Node Smoothing R_inf R_supIdentification
Reference Code_Aster \% variation
N2667 Lag-Lag0.11.0
G local: G (U)5.79
5.7460.748N2667 Lag-Lag
0.1
1.0
$\boldsymbol{G}$ bilinear: $\boldsymbol{G}(\boldsymbol{U}, \boldsymbol{U})$
5.79
5.746
0.748
N2667 Lag-Lag
0.1
1.0
G max: $\boldsymbol{G}(\boldsymbol{U}, \boldsymbol{U})$
$1.4946 E+01$
N2667 Leg-Leg
0.5
1.5
G local: $\boldsymbol{G}$ (v)2.1737
N2667 Leg-Leg0.5

$$
1.5
$$

$$
G \text { bilinear: } G(v, v)
$$

2.1737
N2773 Leg-Leg0.5

## 1.5

$G \max : G(v, v)$
6.0334

N2667 Lag-Lag
0.2
2.0

G total: $\boldsymbol{G}(\boldsymbol{u}+\boldsymbol{v})$
$1.5150 E+01$

N2667 Lag-Lag 0.2
2.0 G bilinear
: G (u+v,

- 1.5150E+01 -
$u+v$ )
N2667 Lag-Lag
0.2
2.0
$\boldsymbol{G} \max : G(u+v, u+v)$
$3.8910 E+01$
[Figure 14.1-a] below shows the values of G local in bottom of crack obtained from bilinear form of G. It is the first loading which is applied (simple traction), it requests it first mode of opening of the crack. The fields as $G$ are discretized according to the method of Lagrange. The rays inferior and superior delimiting the crown in which fields decrease linearly are respectively equal to 0.1 mm and 1 mm . The value of reference is equal to $5.79 \mathrm{~J} / \mathrm{m} 2$, cf modeling $A$.
$6,00 E+00$
5,00E+00
4,00E+00
I
Re
has
G_BILI_LOCAL
3,00E+00
biline
G_REF
2,00E+00
$\boldsymbol{G}_{-}$


## Curvilinear X-coordinate

## Appear 14.1-a: $G$ bilinear local according to the curvilinear $X$-coordinate

One indicates by $U$ displacement corresponding to loading 1, and $v$ corresponding displacement with loading 2. Loading 3 corresponds to displacement $(u+v)$.

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## 15 Summaries of the results

Objective triple of this test is reached:

It is a question of validating the definition of the closed funds of crack and installations consequent calculation of G local. One checks in particular the independence of G local with respect to the angle for an axisymmetric crack and a loading. One notes a variation of less than $2 \%$ on the whole of the bottom of crack by two methods "LAGRANCE" and "LAGRANGE_NO_NO".

Moreover, this test makes it possible to validate the order POST_K1_K2_K3 which makes it possible to calculate them
stress intensity factors by exploiting the jump of displacements on the lips of fissure. This method, less precise than G_THETA, makes it possible to obtain here (with a grid adapted: nodes mediums of the edges touching the bottom of crack moved with the quarter of these edges) of the values of K1 and K3 to less than $1 \%$ of the reference (for the method of extrapolation number 1). With the method of extrapolation number 2, the variation can go up to $8 \%$. Precision of the method of extrapolation number 3, tested in modeling B, lies between those of the two preceding methods. Method 3 is however interesting because it provides a single value of the stress intensity factors and not not a maximum value and a minimal value.

Lastly, one validates the calculation of the bilinear form of $G$.

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Titrate:
SSLV135 Endommagement by tiredness under alternate biaxial loading Dates:
02/11/05
Author (S):
J. Key ANGLES

Handbook of Validation<br>V3.04 booklet: Linear statics of the voluminal systems<br>V3.04.135 document

SSLV135 Damage by tiredness under alternate biaxial loading

## Summary:

One presents a test here having an analytical reference [bib1]. The geometry treated here is a cube without defect with which one carries out a linear elastic mechanical calculation followed calculation of the plan of shearing criticizes in each point of Gauss and in each node.

Each of four modelings tests a criterion:
modeling a: criterion MATAKE;
modeling $b$ : criterion $D A N G \_V A N \_M O D I \_A C$;
modeling D: criterion DANG_VAN_MODI_AV,
modeling E: criterion FATEMI_SOCIE.
The first two criteria are said "to plan of critical shearing", they are adapted to the loadings periodicals. The last two criteria can be qualified criteria "in plan of critical damage", they can be used when the loading is not periodical.
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## 1

Problem of reference

### 1.1 Geometry

has
Face 1
has $=10$
has =
m
10 m
Face 1
(above)
P1
P2
$P$
P5

P6
Face 4
(on the right-hand side)
Face 5
F
(behind)
Face 3
F
(side ga
G uche)
P3
P4
Face 6
F
(in front of)
(Dev.
$Y$
$X$
P7
P8
F
F this 2
(below)
Z

The cube has 10 mm on side.
1.2

Material properties
Young modulus: $E=200000 \mathrm{MPa}$
Poisson's ratio: $=0.3$
Ultimate constraint: =
0
850 MPa
$U$
Curve of Wöhler (alternate traction and compression controlled in constraint):
Half amplitude of constraint 138.0152 .0165 .0180 .0200 .0250 .0295 .0 (MPa)
A number of cycles

```
1.0E+6 0.5E+
0.2E+
0.1E+
0.05E+
0.02E+
12.0E+
6
6
6
6
6
4
Half amplitude of constraint 305.0 340.0 430.0 540.0 690.0 930.0 1210.0
(MPa)
A number of cycles
10.0E+
5.0E+
2.0E+
1.0E+
5.0E+2 2.0E+2 1.0E+2
4
3
3
3
Half amplitude of constraint 1590.0 2210.0 2900.0
(MPa)
A number of cycles
50.0
20.0
10.0
Table 1.2-1: Curve of Wöhler
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```

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## 1.3

Boundary conditions and loadings

Displacements according to axis $X$ of face 3 are blocked ( $D X=0.0$ ).
Displacements according to the axis $Y$ of face 2 are blocked ( $D Y=0.0$ ).
Displacements of the P3 point are blocked according to axis $Z(D Z=0.0)$.
We apply an alternate biaxial loading (traction and compression) according to axes $X$ and $Y$. $F x(T)$ represents the alternate efforts applied to face 4 according to axis $X$ and $F y(T)$ represents them alternate efforts applied to face 1 according to axis $Y$.

Loading for modelings A and b:
200N
20
Fx
FxT)
100 N
10
$T$
0
$1 s$
$2 s$
-100N
-200N
Fy (T)

Loading for modelings C and D:

### 1.4 Conditions <br> initial

Without object for a static analysis.

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## 2 <br> Reference solution

## 2.1

Method of calculation used for the reference solution
In the case of an alternate biaxial loading where the pressures applied are such as: =,
$X$
$y$
with $>1$ and $<0$, one show [bib1] that it half amplitude of maximum shearing
$2=($
$+$
, where
2 and
2 represent the half amplitudes of constraints
$X$
y) 4
$X$
$y$
applied according to axes $X$ and Y. Moreover, there are two critical plans in which shearing is maximum:

```
Y
n1
n1
n22
X
Z
```

LStwo pl
Stwo p years of cis
years
has
of cis illement
maximum men
max badly

## 2.2

Results of reference for modelings $\boldsymbol{A}$ and $B$
See the references [bib2] and [R7.04.01].
Half amplitude of maximum shearing:

```
2(MPa)
2(MPa)
2(MPa)
X
y
100200 150
```


## Note:

The half amplitude of maximum shearing is identical for the two critical plans.
Normal vectors in the two critical plans:

## 12

Component y
12
12
Component Z 0
0

Normal maximum constraints in the fields of the normals $n 1$ and N2:
NR
$(N)=50 M P a$
max
1
and NR
$(N)=50 \mathrm{MPa}$
max
2

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Hydrostatic pressure maximum, independent with respect to the plans of normals n1 and $N 2$ :
$P=$
33333
33
MPa.

Normal average constraints in the fields of the normals n1 and N2:
NR (N)

```
MPa
m
0
1=
and NR (N)
MPa
m
0
2 =
```

Normal maximum deformations in the fields of the normals n1 and N2:
4
max (
$n 1)=75$
,
10 and
4
max (
$N 2)=75$
,
10

Normal average deformations in the fields of the normals n1 and N2:
(N) 0
m
$1=$
and (N)
0
m
$2=$

## Criterion of MATAKE

(N)

I

+ NR has
(N) $B, I=$

12
I

+ NR has
()
I
, $I=$
12
2
max
T
where $F$ and $T$ represent, respectively, the limit of endurance in alternating bending and the limit
of endurance in alternate torsion. Here $\boldsymbol{F} \boldsymbol{T}$ is equal to 5

1. Consequently we have:
(N)
MPa
eq
300
$1=$
and
(N)
MPa
eq
300
2 =

Numbers of cycles to the rupture in the fields of the normals n1 and N2:

From the curve of Wöhler, cf [Table 1.2-1], and equivalent constraints within the meaning of MATAKE, we obtain:
$N b(N)=N b(N)=10946$
Cr
1
Cr
2
cycles.
Damage in the fields of the normals n1 and N2:
5
ENDO (
-
$n 1)=E N D O(N 2)=913565$
,
10.

Criterion of Dang Van adapted to the periodic loadings:
DANG_VAN_MODI_AC
(N)

I

+ has P B, $I=$
12
2
where has $=1$ and $B=2$.
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## Equivalent constraints within the meaning of DANG VAN in the fields of the normals n1 and N2:

I
C
$(N)=$

$$
\begin{aligned}
& \text { eq } \\
& I \\
& + \text { has } P, I=, \\
& 12 \\
& 2 \\
& T
\end{aligned}
$$

where $C$ and $T$ represent, respectively, the limit of endurance in alternate shearing and the limit of endurance in alternate traction and compression. Here C T is equal to 5

## 1. Consequently we have:

$(\boldsymbol{N})=275 \mathrm{MPa}$ and $(\boldsymbol{N})=275 \mathrm{MPa}$.

Numbers of cycles to the rupture in the fields of the normals n1 and N2
From the curve of Wöhler, cf [Table 1.2-1], and equivalent constraints within the meaning of DANG VAN, we obtain:
$N b(N)=N b(N)=14903$ cycles.

```
Cr
l
Cr
2
Damage in the fields of the normals \(\boldsymbol{n 1}\) and N2:
```

-5
$E N D O(N)=E N D O(N)=709959$

6
10 .
1
2
2.3

Results of reference for modelings $C$ and $D$
See the references [bib2] and [R7.04.01].

Half amplitude of constraint:

2 (MPa)
$2(M P a)$
$X$
$y$
100200

Criterion of MATAKE adapted to the nonperiodic loadings: DOMM_MAXI

For this criterion there are no analytical results.
Criterion of Dang Van adapted to the nonperiodic loadings:
DANG_VAN_MODI_AV

For this criterion there are no analytical results.
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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Modeling 3D: 125 quadratic elements of volume: HEXA8.
Grid of the cube made with GIBI 2000
Appear of the grid of the cube
Test criterion MATAKE.

## 3.2 <br> Characteristics of the grid

The grid of the cube was obtained starting from the version 2000 of maillor GIBI.
A number of nodes: 216
A number of meshs: 465
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### 3.3 Functionalities

tested

Orders Options

LIRE_MAILLAGE
DEFI_GROUP CREA_GROUP_NO
CREA_GROUP_MA
DEFI_FONCTION NOM_PARA
"SIGM"
Interpol
"LOG"
DEFI_FONCTION NOM_PARA
"INST"
DEFI_MATERIAU ELAS
TIRE
CISA_PLAN_CRIT
DEFI_LIST_REEL
"MECHANICAL" AFFE_MODELE
"3D"
AFFE_MATERIAU ALL
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FACE_IMPO
GROUP_MA
FORCE_FACE
GROUP_MA
STAT_NON_LINE
MECA_STATIQUE
CALC_FATIGUE TYPE_CALCUL
"FATIGUE_MULTI"
OPTION
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4
Results of modeling $A$
4.1 Values
tested

## Identification

Type of value

## Reference

Aster Variation
(\%)
Nodes: N1; N206;
(Moment: 3)
1.00000E+02
$1.00000 E+02$
0.0
xx
Net: M60,
Not Gauss: 3
"
(Moment: 3)
$2.00000 E+02$
$2.00000 E+02$
0.0
yy
(N) 2
1.500000E+02
$1.500000 E+02$
0.0

1
"
component $X$ of n1 7.071068E01 7.071068E01
0.0
"
component there of $\boldsymbol{n 1}$
7.071068E01 7.071068E01
0.0
"
component Z of n1
0.0 6.123234E17
0.0
"
NR
()
max $n 1$
1
0.0 4.235754E14
0.0
()
$\max n 1$
1.750000E04 1.750000E04
0.0
(N)
m
1
$0.01 .564995 E 19$
0.0
(N)
eq
1
$3.000000 E+023.000000 E+02$
0.0
"
(N)
Cr
Nb
1
1.094600E+04 1.094600E+04
0.0
,"
ENDO (N)
1
9.135647E05 9.135647E05
0.0
(N) 2
1.500000E+02 1.500000E+02 0.0
2

## component Z of N2

$0.06 .123234 E 17$
0.0
" $N R$
(
)
$\max \mathrm{N} 2$
$5.000000 E+015.000000 E+01$
0.0
" $N R(\boldsymbol{N})$
m
2
0.0 4.235754E14
0.0
"
)
$\max \mathrm{N} 2$
1.750000E04 1.750000E04
0.0
"
(N)
m
2
$0.01 .564995 E 19$
0.0
"
(N)
eq
2
$3.000000 E+023.000000 E+02$
0.0

Nodes: N1; N206;
( $N$ )
Cr
$N b$
2
$1.094600 E+041.094600 E+04$
0.0
Net: M60,
Not Gauss: 7
,,
ENDO (N)
2
$9.135647 E 059.135647 E 05$
0.0

The variations being lower than 1.0E-08 we put zero in the table above.

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## 5 Modeling

B

## 5.1 <br> Characteristics of modeling

Except the criterion of tiredness tested, modeling $B$ is identical to modeling $A$. Test criterion DANG_VAN_MODI_AC.

## 5.2 <br> Characteristics of the grid

Identical to modeling $A$.

### 5.3 Functionalities

tested
The functionalities tested are identical to modeling A. only the option CRITERION of order CALC_FATIGUE is different:

## Orders Options

CALC_FATIGUE CRITERION
"DANG_VAN_MODI_AC"

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## 6

Results of modeling B

### 6.1 Values

tested

## Identification

Type of value
Reference
Aster Variation
(\%)
Nodes: N1; N206;
(Moment: 3)
$1.00000 E+02$

Not Gauss: 3
"
(Moment: 3)
$2.00000 E+02$
2.00000E+02
0.0
yy
"
(N)

1
$1.500000 E+02$
$1.500000 E+02$
0.0
"
component $X$ of $\boldsymbol{n 1}$
7.071068E01 7.071068E01
0.0
"
component there of $\mathbf{n 1}$
7.071068E01 7.071068E01
0.0
"
component Z of n1
0.0 6.123234E17
0.0
"
NR
()
max $n 1$
5.000000E $+015.000000 E+01$
0.0
"
$N R(N)$
m
1
0.0 4.235754E14
0.0

## ()

$\max \boldsymbol{n 1}$
1.750000E04 1.750000E04
0.0
"
(N)

## m

1
$0.01 .564995 E 19$
0.0
,
(N)
$e q$
1
$2.750000 E+022.750000 E+02$
0.0
,
(N)

Cr
Nb
1
$1.490300 E+041.490300 E+04$
0.0
,
ENDO (N)
1
6.709959E05 6.709959E05
0.0
"
(N)

2
$1.500000 E+021.500000 E+020.0$
,
component $X$ of N2 7.071068E01 7.071068E01
0.0
"
N2 component there 7.071068E01 7.071068E01 0.0
component Z of N2
$0.06 .123234 E 17$
0.0

```
"
NR
```

(
)
$\max \mathrm{N} 2$
$5.000000 E+015.000000 E+01$
0.0
" $N R(N)$
m
2
0.0 4.235754E14
0.0
"
(
$\max \mathrm{N} 2$
1.750000E04 1.750000E04
0.0
(N)
m
2
$0.01 .564995 E 19$
0.0
,
(N)
eq
2
$2.750000 E+022.750000 E+02$
0.0
Nodes: N1; N206;
(N)
Cr
Nb
2
$1.490300 E+041.490300 E+04$
0.0

Net: M60,
Not Gauss: 7

ENDO (N)

## 2

6.709959E05 6.709959E05
0.0

The variations being lower than 1.0E-08 we put zero in the table above.
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## 7 Modeling <br> C

## 7.1

## Characteristics of modeling

Except the criterion of tiredness tested and the loading, cf [§ 1.3], modeling $C$ is identical to modeling A.
Test criterion DOMM_MAXI.

## 7.2

## Characteristics of the grid

Identical to modeling $A$.

### 7.3 Functionalities <br> tested

The functionalities tested are identical to modeling A except the following options: CRITERION is modified, METHOD is not used any more and PROJECTION is added:

Orders Options<br>CALC_FATIGUE CRITERION<br>"DOMM_MAXI"<br>POJECTION<br>"UN_AXE"<br>"DEUX_AXES"

## 8 <br> Results of modeling C

### 8.1 Values

tested

## Identification

Type of value
Reference
Aster Variation
(\%)
Nodes: N1; N206;
(Moment: 3)
$1.00000 E+02$
1.00000E+02
0.0
$x x$
Net: M60,
Not Gauss: 3
"
(Moment: 3)
2.00000E+02
$2.00000 E+02$
0.0
yy
"
component $X$ of N
$\overline{1}$
_ _ $3.746066 E 01$
3.907311E01
"
component there of $\boldsymbol{N}$

## Code_Aster ${ }^{\circledR}$

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## 9 Modeling <br> D

## 9.1 <br> Characteristics of modeling

Except the criterion of tiredness tested and the loading, cf [§ 1.3], modeling $D$ is identical to modeling $A$.

## 9.2

Characteristics of the grid
Identical to modeling $A$.

### 9.3 Functionalities <br> tested

The functionalities tested are identical to modeling A except the following options: CRITERION is modified, METHOD is not used any more and PROJECTION is added:

Orders Options<br>CALC_FATIGUE CRITERION<br>"DANG_VAN_MODI_AV"<br>POJECTION<br>"UN_AXE"<br>"DEUX_AXES"

## 10 Results of modeling D

### 10.1 Values

tested

## Identification

Type of value
Reference
Aster Variation
(\%)
Nodes: N1; N206;
(Moment: 3)
$1.00000 E+02$
$1.00000 E+02$
0.0
$x x$
Net: M60,
Not Gauss: 3
"
(Moment: 3)
" component $X$ of n1

$$
\text { _ _ } 7.071068 E 01
$$

-     - 

"
component there of n1
_ _ 7.071068 E01
--
9
component Z of n1
_ _ 6.123234E17

-     - 

"
ENDO (N)
1
_ _ $1.341992 E 04$

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11 Modeling
E
11.1 Characteristics of modeling

Except the criterion of tiredness tested and the loading, cf [§ 1.3], modeling E is identical to modeling $A$.
Test criterion FATEMI_SOCIE.
11.2 Characteristics of the grid

Identical to modeling $A$.

### 11.3 Functionalities

tested
The functionalities tested are identical to modeling A except the following options: CRITERION is modified, METHOD and MECA_STATIQUE are not used any more and PROJECTION is added:

Orders Options
CALC_FATIGUE CRITERION
"FATEMI_SOCIE"
POJECTION
"UN_AXE"
"DEUX_AXES"

12 Results of modeling $E$
12.1 Values
tested
Identification
Type of value
Reference
Aster Variation
(\%)
Nodes: N1; N206;
(Moment: 3)
$1.00000 E+02$
$1.00000 E+02$
0.0
$x x$
Net: M60,
Not Gauss: 3
"
(Moment: 3)

## $2.00000 E+02$

$2.00000 E+02$
0.0
$y y$
-8.00000E-04
-8.00000E-04
0.0
xx (Moment: 3)
"
1.15000E-03
1.15000E-03
0.0
yy (Urgent: 3)
"
-1.50000E-04
-1.50000E-04
0.0
zz (Urgent: 3)
"
component $X$ of n1
_ _ $\pm 4.383711$ E01

-     - 

"
component there of n1 8.987940E01
" component Z of n1
_ _ $6.123234 E 17$
--
, 9
ENDO (N)
1
_ _ $1.682346 E 01$

13 Summary of the results

The results obtained are in perfect agreement with the reference solution for modelings $A$ and B. modeling C, D and $\mathbf{E}$ do not have reference solutions associated with the criteria.
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12/05/04
Author (S):
X. DESROCHES, C. REZETTE Key
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Organization (S): EDF-R \& D /AMA, DeltaCAD

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Document: V3.04.139

SSLV139 - Buckling of a circular plate
subjected to a compressive force uniformly
distributed on its contour

## Summary:

This test represents a calculation of stability of a circular plate subjected to a compressive force uniformly distributed on its contour. We determine the critical load leading to buckling rubber band of Euler as well as the associated modal deformation.

This test validates modeling AXIS_FOURIER for linear buckling with meshs QUAD8 and TRIA6 (circumferential mode equal to zero), and modeling AXIS with meshs QUAD8.

The critical load obtained is compared with an analytical reference solution.

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1
Problem of reference

### 1.1 Geometry

$Z$<br>Y<br>Fcr<br>Y<br>$X$<br>$Z$.<br>X<br>With<br>B

Thickness of the plate: $H=0.0005 \mathrm{~m}$
Ray of the plate: $R=0.115 \mathrm{~m}$

## 1.2

Properties of material
The properties of material constituting the plate are:
11
$E=2.110 \mathrm{~Pa}$

Young modulus
$=3$
0

Poisson's ratio

## 1.3 <br> Boundary conditions and loadings

Boundary conditions:
on the contour of the plate (not B): displacement following $Y=0$ and rotation around $Z=0$
Loading: one applies a compressive force uniformly distributed F to external contour plate.

### 1.4 Conditions

 initialWithout object.
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## Code_Aster ${ }^{\circledR}$

Version
7.2

Titrate:
SSLV139-Buckling of a circular plate subjected to a force
Date:
12/05/04
Author (S):
X. DESROCHES, C. REZETTE Key

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2

## Reference solution

## 2.1 <br> Method of calculation

The value of the critical load is given in [bib1] by the following expression:
68

14
D
Fcr $=$
2
R
with: $D$ the bending stiffness of the plate (in N.m) defined by the following expression:

```
3
=
Eh
D
I
(
12
2
-)
```

This critical load is associated a circumferential mode equal to 0 .

## 2.2 <br> Sizes and results of reference

For the characteristics given, the critical load is worth:
Cr
$F=2668.315 \mathrm{~N} / \mathrm{m}$

## 2.3 <br> Uncertainties on the solution

Analytical solution

### 2.4 References bibliographical

## [1]

S.P. TIMOSHENKO, J.M. MANAGES: Theory of elastic stability, second edition, DUNOD (1966)

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## 3 Modeling

## With

3.1<br>Characteristics of modeling<br>Modeling AXIS_FOURIER (QUAD8): 3 degrees of freedom per node (DX, DY, DZ)

## $\underline{Y}$ <br> D <br> $C$ <br> $X$ <br> B <br> Z

With

Limiting conditions:
group meshs $A D: D X=0, D Z=0$
group meshs $B C: D Y=0, D Z=0$
Characteristics of the discretization

Sides $A B$ and CD: 460 elements

Sides $A D$ and $B C: 4$ elements

3.2<br>Characteristics of the grid<br>A number of nodes: 6449<br>A number of meshs: 1840 (QUAD8)

\author{
3.3 Functionalities <br> tested <br> Orders <br> ```
AFFE_MODELE <br> AFFE <br> MODELING = "AXIS_FOURIER" <br> AFFE_CHAR_MECA DDL_IMPO <br> GROUP_MA <br> FORCE_CONTOUR <br> FX <br> MACRO_ELAS_MULT <br> CAS_CHARGE <br> MODE_FOURIER $=0$ <br> CALC_MATR_ELEM <br> OPTION = "RIGI_MECA" <br> OPTION = "RIGI_GEOM" <br> MODE_FOURIER $=0$

```
}
3.4Sizes tested and results
Identification
Reference
Aster \%
Difference
Pressure criticizes ( \(n=0\) )
8.4935 105 Pa
8.4847 105 Pa
-0.104
Displacement DY with the node \(D\)
19.99985
10-1-0.001
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Version
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Titrate:
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Date:
12/05/04
Author (S):
X. DESROCHES, C. REZETTE Key:

\subsection*{3.5 Remarks}

The pressure criticizes Pcr of reference, used in the command file, was obtained with to leave the referred critical load in the paragraph [\$2.2]:

\section*{F}

2
\(P\)
Cr
\(=\)
\(=849350.94 \mathrm{~N} / \mathrm{m}\)
Cr
\(2 H\)

The standardization of the clean mode for largest of the components of translation imply a value of reference equal to 1 for displacement DY to node \(D\).

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\section*{4 Modeling \\ B}

\section*{4.1}

\section*{Characteristics of modeling}

Modeling AXIS (QUAD8): 2 degrees of freedom per node ( \(D X, D Y\) )
```

Y
D
C
X
With
B
Z

```

Limiting conditions:
group meshs \(A D: D X=0\)
group meshs \(B C: D Y=0\)
Characteristics of the discretization
Sides \(A B\) and CD: 460 elements

Sides \(A D\) and \(B C: 4\) elements

\section*{4.2 \\ Characteristics of the grid}

A number of nodes: 6449
A number of meshs: 1840 (QUAD8)

\subsection*{4.3 Functionalities \\ tested \\ Orders}
```

AFFE_MODELE
AFFE
$M O D E L I N G=$ "AXIS"
AFFE_CHAR_MECA DDL_IMPO

```

\section*{GROUP_MA}

FORCE_CONTOUR
FX
MACRO_ELAS_MULT CAS_CHARGE
CALC_MATR_ELEM
OPTION = "RIGI_MECA"

\section*{OPTION = "RIGI_GEOM"}

MODE_ITER_SIMULT
METHOD = "SORENSEN"

TYPE_RESU = "MODE_FLAMB"
CALC_FREQ
OPTION = "SMALLER"
NORM_MODE
= "TRAN NORMALIZES"

\section*{4.4}

Sizes tested and results

\section*{Identification}

Reference
Aster \%
Difference
Pressure criticizes ( \(n=0\) )
8.4935 105 Pa
8.4847105 Pa
-0.104
Displacement DY with the node \(D\)
19.99985

10-1-0.001

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
7.2

\section*{Titrate:}

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\subsection*{4.5 Remarks}

The pressure criticizes Pcr of reference, used in the command file, was obtained with to leave the referred critical load in the paragraph [\$2.2]:

\section*{F}

2
\(P\)
Cr
\(=849350.94 \mathrm{~N} / \mathrm{m}\)
Cr
\(2 H\)

The standardization of the clean mode for largest of the components of translation imply a value of reference equal to 1 for displacement DY to node \(D\).

\section*{Handbook of Validation}

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\section*{Code_Aster \({ }^{\circledR}\)}

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\section*{5 Modeling}

C

\section*{5.1 \\ Characteristics of modeling}

Modeling AXIS_FOURIER (TRIA6): 3 degrees of freedom per node ( \(D X, D Y, D Z\) )

\section*{\(\underline{Y}\) \\ \(D\) \\ C \\ X \\ With \\ B \\ Z}

Limiting conditions:
group meshs \(A D: D X=0, D Z=0\).
group meshs \(B C: D Y=0, D Z=0\).
Characteristic of the discretization
Sides \(A B\) and CD: 690 elements
Sides \(A D\) and BC: 6 elements

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 17964
A number of meshs: 8.280 (TRIA6)

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
AFFE_MODELE \\ AFFE \\ MODELING = "AXIS_FOURIER" \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_MA \\ FORCE_CONTOUR \\ FX \\ MACRO_ELAS_MULT \\ CAS_CHARGE \\ MODE_FOURIER \(=0\) \\ CALC_MATR_ELEM \\ OPTION = "RIGI_MECA"
}

OPTION = "RIGI_GEOM"

MODE_FOURIER \(=0\)
MODE_ITER_SIMULT
METHOD = "SORENSEN"
\(T Y P E \_R E S U=\) "MODE_FLAMB"
CALC_FREQ
OPTION = "SMALLER"
NORM_MODE
= "TRAN NORMALIZES"

\section*{5.4 \\ Sizes tested and results \\ Identification \\ Reference \\ Aster \% \\ Difference}

\title{
Pressure criticizes ( \(n=0\) )
}
8.4935 105 Pa
8.6943 105 Pa
2.364

Displacement DY with the node D
19.99986

10-1-0.001
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\subsection*{5.5 Remarks}

The pressure criticizes Pcr of reference, used in the command file, was obtained with to leave the referred critical load in the paragraph [\$2.2]:
```

F
2
P
Cr
=
= 849350.94N/m
Cr
2H

```

The standardization of the clean mode for largest of the components of translation
imply a value of reference equal to 1 for displacement DY to node \(D\).

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\section*{6 \\ Summary of the results}

The results obtained are very satisfactory for meshs QUAD8 independently of modeling used (AXIS or AXIS_FOURIER): uncertainties on the critical pressure \(0.104 \%\) do not exceed.

However, it will be noted that modeling AXIS_FOURIER is definitely less precise with meshs TRIA6 that with meshs QUAD8.

This test with licence to test and compare modelings AXIS and AXIS_FOURIER in buckling linear of Euler of a circular mean structure subjected to an external force of compression uniformly distributed on its contour.

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Code_Aster \({ }^{\circledR}\)
Version
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\section*{Titrate:}

SSLV140 - Calculation of effective modules by a method Python
Date:

\title{
T. KANIT, J.M. PROIX Key
}

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Organization (S): EDF-R \& D /AMA

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems
V3.04.140 document

SSLV140-Calculation of effective modules by one method Python

\section*{Summary:}

One presents a test here having an analytical reference. The treated geometry is a whole of two cubes having different elastic properties. The goal is to find the Young modulus of the mixture made up of these two cubes along two directions.
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Titrate:
SSLV140 - Calculation of effective modules by a method Python
Date:
06/05/04
Author (S):

\section*{T. KANIT, J.M. PROIX Key}

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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}

\author{
P1 \\ \(P 2\) \\ P9 \\ P5 \\ P6 \\ P11 \\ P3 \\ P4 \\ P10 \\ Y \\ P7 \\ P8 \\ P12 \\ X \\ Z
}

Following surfaces are defined:
- Face YZ1: containing the nodes P1, P3, P5 and P7.
- Face YZ2: containing the nodes P9, P10, P11 and P12.
- Face XY1: containing the nodes P1, P2, P9, P3, P4 and P10.
- Face XY2: containing the nodes P5, P6, P11, P7, P8 and P12.
- Face XZ1: containing the nodes P3, P4, P10, P7, P8 and P12.
and following elements:
- Elément M1: containing the nodes P1, P2, P3, P4, P5, P6, P7 and P8.
- Elément m2: containing the nodes P2, P9, P4, P10, P6, P11, P8 and P12.

\section*{1.2 \\ Material properties}

Two materials are used:
- Matériau MAT1 allotted to the M1 element:

Young modulus: \(\mathrm{El}=200000 \mathrm{MPa}\)
Poisson's ratio: \(1=0.3\)
- Matériau MAT2 allotted to the element m2:

Young modulus: \(E 2=100000 \mathrm{MPa}\)
Poisson's ratio: \(2=0.3\)
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1.3

Boundary conditions and loadings

\section*{The first calculation:}

It is a simple calculation of traction according to direction \(X\) :
- One imposes a linear elastic strain \(=1\) on surface YZ2.
\(x x\)
- Surface YZ1 does not move according to direction X.

\section*{The second calculation:}

It is a simple calculation of traction according to the direction \(Y\) :
- One imposes a linear elastic strain
\(=1\) on surface XZ2.
yy
- Surface XZ1 does not move according to direction \(Y\).

\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation}

According to the general theory of the homogenisation of composite materials [bib1], the Young moduli manpower EFF
\(E\) and EFF
E following directions \(X\) and \(Y\) of a mixture having the form given above, is
\(y y\)
given by the following formulas:

1
F
F
1
2
\(+\)
E EFF
E
E
\(x x\)
1
2
\(E E F F=F E+F E\)

\subsection*{2.2 References bibliographical}
[1]
Mr. BORNET, T. BRETHEAU and P. GILORMINI: Homogenisation in mechanics of materials (T1). Hermes Science Publications - 2001.

Handbook of Validation
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06/05/04
Author (S):
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\section*{3 Modeling}

With

\section*{3.1}

\section*{Characteristics of the grid}

A number of nodes: 12.
Modeling 3D: 2 quadratic elements of volume: HEXA8.

\subsection*{3.2 Functionalities}
tested
Orders
Options
CREA_CHAMP
TYPE_CHAM
"ELGA_SIEF_R"
OPERATION
"EXTR"
RESULT
NOM_CHAM
"SIEF_ELGA_DEPL"
CALC_CHAM_ELEM
MODEL
CHAM_MATER
OPTION
"COOR_ELGA"
EXTR_COMP

Python orders are inserted directly in the command file ASTER. These orders are used to write functions of postprocessing on the fields of results, like the averages, the trace of a tensor of deformations or constraints.... etc fields of results are recovered by order EXTR_COMP.

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{The first calculation:}

The Young modulus following direction \(X\) in this case is the average of the constraints:

\section*{The second calculation:}

The Young modulus following the direction \(Y\) in this case is the average of the constraints:

\title{
Identification Reference
}

Aster \%
difference
< >
133333134134
1.00
\(x x\)
<>
150000150000
0.00
yy

\section*{5}

Summary of the results

The results obtained are in perfect agreement with the reference solution.
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\section*{Titrate:}

SSLV301 - Cylindrical beam comforts under load linearly distributed Date

\section*{Summary:}

The goal of the test is to validate a load linearly distributed, starting from an analysis 2D with decomposition in
Fourier series of the load.
2 calculations here are carried out:
- a calculation with the first 2 modes (0 and 1), - a calculation with the first 10 modes.

\title{
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}

Code_Aster \({ }^{\circledR}\)
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Titrate:
SSLV301 - Cylindrical beam comforts under load linearly distributed Date
17/06/03
Author (S):
X. DESROCHES Key
:
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\section*{1}

Problem of reference
1.1 Geometry

C

\section*{Length}
: L
\(=0.240 \mathrm{~m}\)
Ray
: R
\(=0.006 \mathrm{~m}\)

\section*{1.2}

Material properties
\(E=2.1 \times 1011 \mathrm{~N} / \mathrm{m} 2\)
\(=0.3\)

\section*{1.3}

Boundary conditions and loadings
- Embedded Arête AB
- Charge varying linearly according to Z on generator BC, being worth:
\(Q=0\) out of \(C\) and \(Q=3000 \mathrm{~N} / \mathrm{m}\) out of \(B\)

\subsection*{1.4 Conditions}
initial
Without object for the static analysis.
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is obtained analytically [bib1].

\section*{2.2}

Results of reference
- Radial Déplacement of the point C: urc \(=1.552 \times 103 \mathrm{~m}\)
- Contraintes embedding at point b: \(z z(B)=169.8 \times 106\) Pa

\section*{2.3}

Uncertainty on the solution

\title{
Analytical solution.
}

\subsection*{2.4 Reference bibliographical}

\section*{[1]}
S. TIMOSHENKO: Resistance of materials, 1st part. Polytechnic bookshop CH. Béranger, Paris, 1947
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\section*{3 Modeling}

With

\section*{3.1}

Characteristics of modeling
AXIS_FOURIER, T6 mesh
Cutting: 80 elements according to the length
2 elements in the thickness

Z
D
C

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 805 \\ A number of meshs and types: 320 TRIA6 \\ \subsection*{3.3 Functionalities} \\ tested \\ Orders \\ "MECHANICAL"AFFE_MODELE "AXIS_FOURIER" \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ AFFE_CHAR_MECA_F FORCE_CONTOUR \\ GROUP_MA \\ COMB_FOURIER NOM_CHAM \\ "DEPL" \\ "SIGM_ELNO_DEPL" \\ CREA_CHAMP "EXTR"
}
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:

\section*{X. DESROCHES Key}

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Values provided for \(=0\).
Standard localization
of
value
Reference
Aster
\% difference
Calculation 1 (2 modes)

\author{
Not C \\ ur (m) \\ \(1.552 \times 103\) \\ \(1.54839 \times 1030.232\) \\ Not B \\ \(z z(P a)\) \\ 169.8 X 106 \\ 168.73 X 1060.63 \\ Calculation 2 (10 modes)
}
```

Not C
ur (m)
$1.552 \times 103$
$1.54839 \times 1030.232$
Not B
$z z$ (Pa)
169.8 X 106

```

\subsection*{4.2 Notice}

The values of the arrow of the beam and the constraint of embedding are obtained with precision with the first two modes only.

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\section*{:}

17/06/03
Author (S):
X. DESROCHES Key

\section*{:}

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\section*{5}

Summary of the results
The results resulting from calculation are in concord with the analytical solution.
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Code_Aster \({ }^{\circledR}\)
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6.4

Titrate:
SSLV303-Roll embedded under actual weight and pressure
Date
:
15/07/03

\author{
Author (S): \\ X. DESROCHES Key \\ V3.04.303-A Page: \\ 1/16 \\ Organization (S): EDF-R \& D /AMA
}

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
Document: V3.04.303

\section*{SSLV303 - Roll embedded under actual weight and pressure}

\section*{Summary:}

The goal of the test is to validate a load of gravity as well as a pressure, starting from an analysis 2D with
decomposition in Fourier series of the load.

Two modelings are adopted for this analysis; they differ from the key word used to define gravity:

> - modeling a: to validate the key word GRAVITY, - modeling b: to validate key word FORCE_INTERNE.

Gravity is calculated in mode 1, and the pressure applied is given in mode 1.

The two loading cases are combined and compared with a numerical calculation in modeling 3D (model \(C)\).

The fourth modeling \((D)\) is built with an aim of validating key word FORCE_INTERNE defined to leave of a function.
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Code_Aster \({ }^{\circledR}\)
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Titrate:
SSLV303 - Roll embedded under actual weight and pressure
Date

15/07/03
Author (S):
X. DESROCHES Key

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{Length}
: L
\(=0.240 \mathrm{~m}\)
Ray
: R
\(=0.006 \mathrm{~m}\)

\section*{1.2}

Material properties

\section*{\(E=2.1 X 1011 \mathrm{~N} / \mathrm{m} 2\)}
\(=0.3\)
\(=7800 \mathrm{~kg} / \mathrm{m} 3\)

\section*{1.3 \\ Boundary conditions and loadings}
- Sections AB, CD embedded
- Gravité according to \(\boldsymbol{R}\)
: \(G=9.81 \mathrm{~m} / \mathrm{s} 2\)
\(\cdot\) Pression given by: \(p=P o \cos , P o=10.000 \mathrm{~N} / \mathrm{m} 2\)

\author{
1.4 Conditions initial \\ Without object for the static analysis. \\ Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal systems \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV303-Roll embedded under actual weight and pressure
Date
\(:\)
1
\(A\)
\(X\)
\(:\)
1
3
2
15/07/03
Author (S):
X. DESROCHES Key

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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution \\ In the case of load of gravity alone: \\ The value of the field of radial displacement, according to Z, is given by:}
```

Q4
2
Z
L
ur=

```

In the case of load of pressure, one carries out a comparison with the results of modeling \(C\).
2.2

Results of reference
- Déplacement in the median section, ( ur \(E)=0.3566 \times 106 \mathrm{~m}\)
- Contraintes embedding at point b: \(z z=0.2496\) X 106 Pa

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution for gravity.

\subsection*{2.4 Reference \\ bibliographical}

\section*{[1]}
S. TIMOSHENKO: Resistance of materials, 1st part. Polytechnic bookshop CH. Béranger, Paris, 1947
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV303-Roll embedded under actual weight and pressure Date
:
15/07/03
Author (S):
X. DESROCHES Key
:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \(A\)}

AXIS_FOURIER, T6 mesh
With
E
D
B
F
C
Z
\(\boldsymbol{R}\)

\section*{Cutting:}

80 elements according to the length
2 elements according to the ray

\section*{Loadings:}

\section*{C1:}
vertical gravity (Ug field) (G 2)
C2:
pressure \((\) Up field \() P o=10.000 \mathrm{~N} / \mathrm{m} 2\)
Components of displacements: ur (radial), \(u z\) (axial), \(U\) (circumferential)
Names of the nodes:
With \(=\) N1
\(B=N 2\)
\(C=N 3\)
\(D=N 4\)
\(E=N 249\)
\(\boldsymbol{F}=\boldsymbol{N} 87\)

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: 805 \\ A number of meshs and types: 320 TRIA6 \\ Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal systems \\ HT-66/03/008/A}

Code_Aster \({ }^{\circledR}\)
Version
6.4

\section*{Titrate:}

SSLV303 - Roll embedded under actual weight and pressure
Date
:

\section*{Author (S):}

\section*{X. DESROCHES Key}
:

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\author{
3.3 Functionalities \\ tested \\ Orders \\ "MECHANICAL" AFFE_MODELE "AXIS_FOURIER" \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ GRAVITY \\ CALC_CHAM_ELEM OPTION \\ "SIGM_ELNO_DEPL" \\ MODE_FOURIER \\ AFFE_CHAR_MECA_F PRES_REP \\ GROUP_MA \\ COMB_CHAM_NO \\ COMB_FOURIER
}

\author{
COMB_CHAM_ELEM \\ COMB_FOURIER \\ MACRO_ELAS_MULT CHAR_MECA_GLOBAL \\ MODE_FOURIER \\ CAS_CHARGE \\ TYPE_MODE \\ CHAR_MECA \\ Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal systems \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:

\title{
SSLV303-Roll embedded under actual weight and pressure
} Date

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Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Values provided for \(=0\).

\author{
Standard localization \\ of \\ value \\ Reference \\ Aster \\ \% difference \\ Ug field
}
(for \(=0\). \()\)
Not E, \(\boldsymbol{F}\)
ur (m)
3.566 X 107
\(3.541 \times 107\)
-0.701
\(\boldsymbol{U}(\boldsymbol{m})\)
0.
3.94 10-14

Not B
\(z z\) (Pa)
2.496 X 105
\(2.598 \times 105+\)
Not \(\boldsymbol{F}\)
\(7.82 \times 106\)
\(7.70 \times 1061.5\)
Not Brr (Pa)\(1.63 \times 106\)
1.41 X 106
-13.4
\(5.51 \times 106\)
\(5.65 \times 106\)
2.7
\(z z(P a)\)
\(1.65 \times 106\)
1.89 X 106
14.7
(Pa)
\(\boldsymbol{U p}\) field \(+\boldsymbol{U g}\)
(for \(=0\).)
Not E
ur ( \(m\) )
7.46 X 106
7.358 X 1061.3
Not F
ur (m)
7.44 X 106
7.348 X 1061.2
Not B
rr (Pa)
1.56 X 106
\(1.34 \times 106\)
-13.7
\(5.25 \times 106\)
\(5.398 \times 106\)
2.8
\(z z(P a)\)
\(1.57 \times 106\)
1.80 X 106
15.0
(Pa)

\subsection*{4.2 Remarks}
- The values of reference for the pressure (Up field) are obtained in modeling \(C\), with to start from a grid 3D.
-For gravity, it should be stressed that the order of the components in GRAVITY is: R, Z (whereas in FORCE_INTERNE the order is R, Z,).

\section*{4.3}

Contents of the file results
Displacements, constraints.

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems

\section*{HT-66/03/008/A}

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV303 - Roll embedded under actual weight and pressure
Date

\section*{5 Modeling \\ B}

\section*{5.1}

Characteristics of modeling B

\section*{AXIS_FOURIER, T6 meshs}

With
E
D
B
F
C
Z
\(\boldsymbol{R}\)

\section*{Cutting:}

80 elements according to the length
2 elements according to the ray

\section*{Loadings:}

C1: vertical gravity (Ug field) in the form of voluminal density of forces \(\mathbf{G}=76.518 \mathrm{~Pa}\) C2: pressure (Up field)

Components of displacements: ur (radial), uz (axial), \(U\) (circumferential)
Names of the nodes:
With \(=N 1\)
\(B=N 2\)
\(C=N 3\)
\[
\begin{aligned}
& D=N 4 \\
& E=N 249 \\
& F=N 87
\end{aligned}
\]
5.2
Characteristics of the grid
A number of nodes: 805
A number of meshs and types: 320 TRIA6
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4
Titrate:
SSLV303-Roll embedded under actual weight and pressure
Date
: ..... 15/07/03
Author (S):
X. DESROCHES Key
:
V3.04.303-A Page:
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5.3 Functionalities
tested
Orders
"MECHANICAL"AFFE_MODELE "AXIS_FOURIER" ..... ALL
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
FORCE_INTERNE
CALC_CHAM_ELEM OPTION
MODE_FOURIER
AFFE_CHAR_MECA_F PRES_REP
GROUP_MA
COMB_CHAM_NO
COMB_FOURIER
COMB_CHAM_ELEM
COMB_FOURIER
MACRO_ELAS_MULT CHAR_MECA_GLOBAL MODE_FOURIER
CAS_CHARGE
TYPE_MODE
CHAR_MECA
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4
Titrate:
SSLV303-Roll embedded under actual weight and pressure
Date
: ..... 15/07/03
Author (S):
X. DESROCHES Key
:
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V3.04.303-A Page:
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Results of modeling B
6.1 Values
tested
Standard localization
of

\section*{value}

\section*{Reference}

Aster
\% difference
Ug field

\author{
(for = 0.) \\ Not E, F \\ ur (m) \\ 3.566 X 107 \\ \(3.541 \times 107\) \\ -0.70 \\ \(\boldsymbol{U}(\boldsymbol{m})\) \\ 0. \\ 0. \\ Not B \\ \(z z\) (Pa) \\ 2.496 X 105 \\ 2.60 \(\times 105+\) \\ 4.1 \\ Up field
}
\((\) for \(=0\).
Not E
ur (m)
\(7.82 \times 106\)
7.71 X 1061.4

Not F
\(7.82 \times 106\)
\(7.70 \times 1061.5\)
Not B
rr (Pa)
1.63 X 106
1.41 X 106
-13.4
\(5.51 \times 106\)
\(5.65 \times 106\)

\title{
\(5.25 \times 106\)
}
5.398 X 106
2.8
\(z z\) (Pa)
\(1.57 \times 106\)
1.80 X 106
15.0
(Pa)

\subsection*{6.2 Remarks}
- The values of reference for the pressure ( \(U p\) field) are obtained in modeling \(C\), with to start from a grid 3D.
- The results obtained are rigorously identical to those of modeling A with GRAVITY.
6.3Contents of the file results
Displacements, constraints.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4
Titrate:
SSLV303 - Roll embedded under actual weight and pressure
Date
: ..... 15/07/03
Author (S):
X. DESROCHES Key
:
V3.04.303-A Page:
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7 Modeling
C
7.1
Characteristics of modeling C
3D, H20 Meshs and P15
Z
F
WITH, E
\(y, v\)
B\(B, F\)
\(X\)
\(y\)\(\boldsymbol{X}, \boldsymbol{U}\)

Position of the points: With, \(B\) in the section \(Z=0\)
\(E, F\) in the median section \(Z=L / 2\)

\section*{Cutting:}

20 elements according to the length
2 elements according to the ray, 8 elements according to the circumference.
The loading being symmetrical, the half only of the cylinder is modelled.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV303-Roll embedded under actual weight and pressure
Date

\section*{:}

15/07/03
Author (S):

\section*{X. DESROCHES Key}

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Boundary conditions:
- section embedded ends ( \(U=v=W=0\) )
- conditions of symmetry in the \(x z\) plan: \(v=0\)
1)

Pressure on the circumference (Up field)
The surface of the cylinder is divided into 8 lines of elements according to the circumference (1 line elements represents a sector of \(/ 8\) radians.
The pressure being in "Cos", it is supposed to be uniform on each line. For any point of
the surface of angle, (ranging between 1 and 2, \(1=\left(\begin{array}{l}\text { 1 }\end{array}\right)\)
,
, 1 N 8, the value of
8
\(2=N 8\)
pressure assigned to the line of elements containing this point is taken equalizes with:
p0 (cos
2
\(1+\cos 2)\).
2)

Vertical gravity according to X (Ug field)
Names of the nodes:
With \(=\) N845
\(B=N 965\)
\(E=N 865\)
\(F=N 995\)

\section*{7.2}

Characteristics of the grid
A number of nodes: 1285
A number of meshs and types: 160 HEXA20, 80 PENTA15

\author{
7.3 Functionalities \\ tested \\ Orders \\ \section*{"MECHANICAL" AFFE_MODELE "AXIS_FOURIER"} \\ ALL \\ AFFE_CHAR_MECA DDL_IMPO \\ GROUP_NO \\ FORCE_INTERNE \\ GROUP_MA \\ GRAVITY \\ AFFE_CHAR_MECA_F PRES_REP
}

\section*{GROUP_MA}

\section*{CALC_CHAM_ELEM OPTION}

\author{
SIGM_ELNO_DEPL
}

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A

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SSLV303 - Roll embedded under actual weight and pressure
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X. DESROCHES Key

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\section*{8 \\ Results of modeling C}

\author{
8.1 Values
}
tested

\author{
Standard localization \\ of \\ value \\ Reference \\ Aster \\ \% difference \\ Up field
}

Not E
\(U(m)\)
\(7.82 \times 106\)
\(v\) (m)
0 .
10-21
Not \(F\)
\(U(m)\)
7.816 X 106
\(v\) (m)
0.

10-21
Not B
\(x x(P a)\)
\(1.63 \times 106\)
\(1.65 \times 106\)
yy (Pa)
5.51 X 106
\(z z(P a)\)
\(U p\) field \(+U g\)

Not \(E\)
\(U(m)\)
7.46 X 106
\(v\) (m)
0.

10-21
Not \(F\)
\(U(m)\)
7.44 X 106
\(v\) (m)
0.

10-21
Not B
\(x x(P a)\)
\(1.56 \times 106\)
1.57 X 106
yy (Pa)

\subsection*{8.2 Remarks}
- It does not have there values of reference for this modeling. The results are to be compared with those of modelings AXIS_FOURIER (A, B, D).
- At the point B (located in the symmetry plane), one a: \(r r=x x,=y y\)

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

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6.4

Titrate:
SSLV303-Roll embedded under actual weight and pressure Date

\section*{: \\ 15/07/03}

Author (S):
X. DESROCHES Key

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\section*{9 Modeling}

D

\section*{9.1 \\ Characteristics of modeling \(\boldsymbol{D}\)}

AXIS_FOURIER, T6 meshs
With
E
D
B
F

\section*{C}

Z

R

\section*{Cutting:}

80 elements according to the length
2 elements according to the ray

\section*{Loadings:}

\section*{C1:}
vertical gravity (Ug field)
C2:
pressure (Up field)
Components of displacements: ur (radial), uz (axial), \(U\) (circumferential)
Names of the nodes:
With \(=\) N1
\(B=N 2\)
\(C=N 3\)
\(D=N 4\)
\(E=N 249\)
\(\boldsymbol{F}=\boldsymbol{N} 87\)
9.2

Characteristics of the grid
A number of nodes: 805
A number of meshs and types: 320 TRIA6
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
Version
6.4

\section*{Titrate:}

SSLV303 - Roll embedded under actual weight and pressure Date
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:
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Author (S):
X. DESROCHES Key
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```

\subsection*{9.3 Functionalities}

```

Orders
"MECHANICAL"AFFE_MODELE "AXIS_FOURIER"
ALL
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
AFFE_CHAR_MECA_F FORCE_INTERNE
CALC_CHAM_ELEM OPTION
"SIGM_ELNO_DEPL"
MODE_FOURIER
AFFE_CHAR_MECA_F PRES_REP
GROUP_MA
COMB_CHAM_NO
COMB_FOURIER
COMB_CHAM_ELEM
COMB_FOURIER
MACRO_ELAS_MULT CHAR_MECA_GLOBAL
MODE_FOURIER
CAS_CHARGE
TYPE_MODE

```

\author{
CHAR_MECA \\ Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal systems \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.4

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Author (S):
X. DESROCHES Key
:
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10 Results of modeling \(D\)
10.1 Values
tested

Standard localization
of
value
Reference
Aster
\% difference
Ug field
(for \(=0\). \()\)
Not \(\boldsymbol{E}, \boldsymbol{F}\)
ur (m)
\(3.566 \times 107\)
\(3.535 \times 107\)
-0.84
0.
0.

Not B
\(z z\) (Pa)
\(2.496 \times 105\)
\(2.60 \times 105+\)

\section*{4.1}

Up field
(for \(=0\).)
Not E
ur (m)
\(7.82 \times 106\)
7.71 X 1061.4

Not F
\(7.82 \times 106\)
\(7.70 \times 1061.5\)
Not B
rr (Pa)
\(1.63 \times 106\)
1.41 X 106
-13.4
\(5.51 \times 106\)
\(5.65 \times 106\)
2.7
\(z z\) (Pa)
\(1.65 \times 106\)
\(1.89 \times 106\)
14.7
(Pa)

\subsection*{10.2 Remarks}
- The values of reference for the pressure (Up field) are obtained in modeling \(C\), with to start from a model 3D.
\(\cdot\) The results obtained are identical to those of modelings \(\boldsymbol{A}\) and \(B\).
10.3 Contents of the file results

Displacements, constraints.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A
Code_Aster \({ }^{\circledR}\)
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Titrate:
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Date
:
15/07/03
Author (S):

\section*{X. DESROCHES Key}
:

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11 Summary of the results
Maximum differences (in \%) between modelings AXIS_FOURIER and modeling 3D, observed at the points E, F,
\(B\) (in the plan \(=0^{\circ}\) ), on the combined loading cases.

\section*{Localization}

Variation
AXIS_FOURIER/3D
In (\%)
Displacements U:
NOT F
1.5
\(=U\) in \(3 D\)
\(=u r\) in \(A X I\)
Constraints \(z z\)
NOT B
2.8

Constraints xx (3D)

\section*{NOT B}
-14.1
\(=r r(A X I)\)
Constraints yy (3D)
NOT B
14.6
\(=(A X I)\)
- The results between modelings 3D on the one hand and AXIS_FOURIER on the other hand, are concordant with regard to displacements (variation of 1.5\%) and the bending stress \(z z\) (variation of 2.8\%).
\(\cdot A\) embedding, the relation \(x x=y y=0\) involves:
\(x x=y y=\)
\(1-z z\)
The relation of embedding is well checked at the point B, in modeling 3D.
- In addition, at the point B, one has moreover:
\(x x=r r\)
\(y y=\)
In modeling AXIS_FOURIER, the difference between the two constraints is approximately \(\mathbf{2 5 \%}\).
- The second calculation on model AXIS_FOURIER was carried out with a finer grid:

4 elements in the thickness instead of 2, denser grid in the vicinity of embedding AB (total 800 TRIA6).
The variation observed on the constraints \(r\) and at point \(A B\) remains: \(r r=1.51 \times 106,=2.08 \times 106\) (loading case combined).
The relation of embedding \(x x=y y\) thus is checked much better on the model \(3 D\), with one grid in the thickness however coarse.
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V3.04 booklet: Linear statics of the voluminal systems
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Code_Aster \({ }^{\circledR}\)
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6.4

Titrate:
SSLV306-Beam 3D in imposed displacements

Date

17/06/03
Author (S):

\section*{X. DESROCHES Key}

\section*{:}

V3.04.306-A Page:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems

\section*{Document: V3.04.306}

SSLV306-Beam 3D in imposed displacements

\section*{Summary:}

The purpose of the test is to validate the displacements imposed on faces (FACE_IMPO), their values being variable
in space. These values are imposed at the end of a beam 3D, modelling a bending strain.
Handbook of Validation
V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A

Code_Aster \({ }^{\circledR}\)
Version
6.4

Titrate:
SSLV306-Beam 3D in imposed displacements
Date
```

:
17/06/03

```

Author (S):
X. DESROCHES Key

V3.04.306-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Length}
: L
\(=2 \mathrm{~m}\)
Square section, on side
: has
\(=0.2 \mathrm{~m}\)
Moment of inertia
: I
\(=1.333 \times 104 \mathrm{~m} 4\)

\section*{1.2}

Material properties
\(E=2.1 X 1011 \mathrm{~Pa}\)
\(=0.3\)
1.3

Boundary conditions and loadings
- Encastrement of section ABCD
- Déplacement imposed on face EFGH:
- constant
vo in the direction \(y\), vo \(=0.952 \times 105 \mathrm{~m}\)
varying according to the position there point of the section, and being worth:
\(u \boldsymbol{u}=\boldsymbol{y} O, O=0.714 X 105\) radians

\subsection*{1.4 Conditions \\ initial}

Without object for the static analysis.
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V3.04 booklet: Linear statics of the voluminal systems

\section*{HT-66/03/008/A}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
6.4

\section*{Titrate:}

SSLV306-Beam 3D in imposed displacements

\section*{Date}
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17/06/03
Author (S):
X. DESROCHES Key
\[
:
\]

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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The displacements imposed equivalent on a force applied at the end of resultant:

\author{
\(F=\) \\ \(v=\) \\ \(N R\) \\ O \\ 100 \\ L3
}

O represents the rotation of section EFGH:
FL2
\(O=\)
2EI
The bending stress \(x x\) with embedding is worth then:
(
FL
\(A B C D\)
\(x x\)
\(= \pm\)
I/y
2.2

Results of reference
- Déplacement v of the points \(E, F, G, H\)
- Stresses bending xx at the points \(A, B, C, D\)

\author{
2.3 \\ Uncertainty on the solution \\ Analytical solution. \\ \section*{Handbook of Validation} \\ V3.04 booklet: Linear statics of the voluminal systems \\ HT-66/03/008/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.4

\title{
Titrate: \\ SSLV306-Beam 3D in imposed displacements
}

\section*{Date}

\section*{: \\ 17/06/03 \\ Author (S): \\ : \\ V3.04.306-A Page: \\ 4/6 \\ 3 Modeling \\ With}
X. DESROCHES Key

\section*{3.1 \\ Characteristics of modeling}

3D, H20 meshs
\(y, v\)
\(Z, W\)
D
C
B
H
G
E
F
\(X, U\)

Loading by displacements imposed on face EFGH:
-DY: 0.952 X 105
\(\cdot D X:\) function of there defined in 2 points: \(F(0)=0\)
F \((0,1)=-0.0714 E-5\)

\section*{Cutting:}
- 20 elements according to the length
\(\cdot 2\) elements according to the width and the thickness

\section*{3.2}

Characteristics of the grid
A number of nodes: 621
A number of meshs and types: 80 HEXA20

\subsection*{3.3 Functionalities}
tested

Orders
"MECHANICAL"AFFE_MODELE "3D"
ALL
AFFE_CHAR_MECA DDL_IMPO
GROUP_NO
AFFE_CHAR_MECA_F FACE_IMPO
GROUP_MA
CALC_CHAM_ELEM OPTION
"SIGM_ELNO_DEPL"

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems
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Version
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Titrate:
SSLV306-Beam 3D in imposed displacements
Date

17/06/03
Author (S):
X. DESROCHES Key

\section*{: \\ V3.04.306-A Page:}
```

4
Results of modeling A

```

\subsection*{4.1 Values}
tested

\author{
Standard localization \\ of \\ value \\ Reference \\ Aster \\ \% difference \\ Points E, F, G, H \\ \(v\) (m) \\ \(9.52 \times 106\) \\ 9.52 X 1060 \\ Points \(\boldsymbol{E}, \boldsymbol{F}\) \\ \(\boldsymbol{U}(\boldsymbol{m})\) \\ \(7.14 \times 107\) \\ 7.14 X 1070. \\ Points G, \(\boldsymbol{H}\) \\ \(\boldsymbol{U}(\boldsymbol{m})\) \\ \(7.14 \times 107\) \\ 7.14 X 1070.
}

Points \(A, B\)
\(x x\) (Pa)
\(1.5 \times 105\)
\(1.64 \times 1059.5\)
Points C, D
\(\boldsymbol{x x}\) (Pa)
\(1.5 \times 105\)
\(1.64 \times 1059.5\)

\section*{Handbook of Validation}

V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A

\section*{X. DESROCHES Key}

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\section*{5 \\ Summary of the results}

The functionality " displacements imposed function" provides the awaited results; values of stress bending are satisfactory, given that the dealt with problem is a problem of inflection.

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V3.04 booklet: Linear statics of the voluminal systems
HT-66/03/008/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

SSLV307 - Cylinder obliques under uniform axial loading
Date:
23/09/02
Author (S):

\section*{X. DESROCHES Key}

\title{
Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal structures \\ Document: V3.04.307
}

SSLV307-Cylinder obliques under load axial uniform

\section*{Summary:}

The purpose of the test is to validate the various types of linear relations, defined by key words LIAISON_DDL, LIAISON_OBLIQUE, LIAISON_GROUP.

It also makes it possible to test the option "symmetries cyclic" starting from the modeling of a sector of the cylinder.

The analysis is carried out in 3D.

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HT-66/02/001/A
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Version
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SSLV307-Cylinder obliques under uniform axial loading

\section*{Date:}

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Author (S):
X. DESROCHES Key

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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Average radius: \(\boldsymbol{R o}=1 \mathrm{~m}\)}

Thickness:
H \(=0.02 \mathrm{~m}\)
Height:
\(L=4 m\)

3
Cosine directors of the axis of the cylinder: (0.0, 0,5, )

2
Center local \(X\) parallel with the total axis \(X\).

\author{
1.2 \\ Material properties \\ \[
\begin{aligned}
E & =2.1 \times 1011 P a \\
v & =0.3
\end{aligned}
\]
}

\section*{1.3}

Boundary conditions and loadings

\section*{- Axial Déplacement no one at the low end \((W=0)\)}

For the other boundary conditions (linear relations), to see paragraph [§3].
- Uniform Axial loading per unit of length \(Q=10000 \mathrm{~N} / \mathrm{m}\), applied at the high end.

\subsection*{1.4 Conditions \\ initial}

Without object for the static analysis.
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}
- Radial Déplacement in local reference mark ( \(X, y, Z\) ):

12
2
\(q v R o\)
3
\(\boldsymbol{U}\)

\section*{2.2}

Results of reference
- Déplacement raidal: ur = \(7.14 \times 107\) m
- In the local plan ( \(X, Z\) ), yy = 1.25 X \(105 \mathrm{~Pa}, z z=3.75 \mathrm{X} 105 \mathrm{~Pa}\)

\section*{2.3}

Uncertainty on the solution
- Analytical Solution

\subsection*{2.4 References \\ bibliographical}
[1]
R.J. ROARK and W.C. YOUNG: Formulated for stress and strain, 5è edition. New York, Mc Graw-Hill, 1975

\author{
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}

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{Section of the cylinder}
\(y, v\)

\section*{Modeling:}
\(1 / 4\) of the cylinder following the circumference 2 zones:
zone 1 = left lower
(0
Z L/2)
zone \(2=\) left higher (L/2 Z L)

\section*{Cutting:}

20 elements according to the length 16 elements according to the circumference

\section*{2 elements in the thickness}

Co-ordinates of points ( \(\mathrm{R}, \mathrm{Z}\) )
WITH G BE
G1
F
A2
H
B2
E2 H1
F2
A3 I B3 E3 I1 F3
A'2
\(H^{\prime}\)
\(B^{\prime} 2 e^{\prime} 2 H^{\prime} 1 F^{\prime} 2\)
R IH R Re IH R Re IH R Re IH R Re IH R Re IH R Re
0. 0. 0. 90. 90. 90. 0. 0.0 .90 .90 .90 .0 .0 .0 .90 .90 .90.

Z 0. 0.0.0.0.0. L/2 L/2
L/2
L/2
L/2
\(L / 2 L L L L L L\)

IH = interior ray
Re \(=\) external ray
the points \(A 2, H, B 2, E 2, H 2, F 2\) are in the section \(Z=L / 2\) of zone 1 the points \(A^{\prime} 2, H^{\prime}, B^{\prime} 2, E^{\prime} 2, H^{\prime} 2, F^{\prime} 2\) are opposite respective in zone 2
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```

Boundary conditions:
- Conditions of support \(W=0\) at the base (section \(Z=0\).) introduced by key word LIAISON_OBLIQUE
- Conditions of symmetry \(v=0\). on face AB introduced by key word LIAISON_OBLIQUE
- Conditions of symmetry \(U=0\). on face EF introduced by key word LIAISON_OBLIQUE
- Identification of the nodes common to the 2 zones (section \(Z=L / 2)\) by key word LIAISON_GROUP.

Loading:
Density of surface charge \(p=q / h=500000 \mathrm{~N} / \mathrm{m} 2\), along the axis, is in total reference mark:
\(\boldsymbol{F x}=0\).
\(F y=p / 2\)
3
\(F z=p\)
2
Name of the nodes:
plan \(Z=0\).
\(W I T H=N R 1\)
\(B=N R 321\)
\(E=N R 1740 F=N R 1541 G=N R 1540\)
plan \(Z=2\)
A2 = NR 961
\(B 2=N R 993\)
plan \(\mathrm{Z}=2\)
\(A^{\prime} 2=N R 3361 B^{\prime} 2=N R 3364 E^{\prime} 2=N R 2159 F^{\prime} 2=N R 2155\)
\(H^{\prime}=N R 3360\)
\(H^{\prime} 1=\) NR 2156
(zone 2)
plan \(Z=4\)
\(A 3=\) NR \(3359 B 3=\) NR \(3355 I=\) NR 3356
\(E 3=N R 2151\) F3 \(=\) NR 2154
II = NR 2150

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: 4298 \\ A number of meshs and types: 160 HEXA20, 320 PENTA15}

\subsection*{3.3 Functionalities}
tested

\author{
Orders \\ AFFE-MODELE \\ "MECHANICAL" \\ "3D" \\ AFFE_CHAR_MECA \\ FORCE_CONTOUR \\ LIAISON_OBLIQUE \\ LIAISON_GROUP
}

\title{
AFFE_CHAR_MECA_F \\ LIAISON_DDL \\ CALC-CHAM-ELEM \\ OPTION \\ "SIGM_ENLO-DEPL" \\ \\ Handbook of Validation \\ \\ Handbook of Validation \\ V3.04 booklet: Linear statics of the voluminal structures \\ HT-66/02/001/A
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\section*{4}

Results of modeling \(A\)
4.1 Values
tested
Values of displacements \(U, V\), \(W\) read on file
Standard localization
of Reference
Aster
\% difference
value
Not \(G\)
\(\boldsymbol{U}(\boldsymbol{m})\)
7.143 X 107
7.13895 X 107
0.057

\section*{V}
(m)
0. 10-22

W
(m)
0. 10-23

Not H, \(\boldsymbol{H}^{\prime}\)
\(\boldsymbol{U}(\boldsymbol{m})\)
7.143 X 107
7.13859 X 107
0.062

Not I
U(m)
7.143 X 107
7.13739 X 107
0.078

Not G1
U(m)
0.

10-23

Points H1, H'1
\(\boldsymbol{U}(\boldsymbol{m})\)
0.

10-22

Values of displacements \(U, v\), ur in local reference mark calculated starting from \(U, V, W\)

Standard localization
of Reference
Aster
\% difference
value
Not \(G\)
ur (m)
7.143 X 107
7.13895 X 107
0.057
\(v\)
(m)

\section*{0. 10-22}

Not \(\boldsymbol{H}, \boldsymbol{H}^{\prime}\)
ur (m)
7.143 X 107
7.13859 X 107
0.062
\(v\)
(m)
0. 10-12

Not I
ur (m)
\(7.143 \times 107\)
7.13739 X 107
0.078
(m)
0. 10-12

Not A2, A'2
\(v(m)\)
0.

10-12

\section*{Points B2, B'2}

Not G1
\(\boldsymbol{U}(\boldsymbol{m})\)
0.

10-23
```

ur (m)
7.143 X }10
7.14676 X }10
-0.053
Points H1, H'1
U(m)
0.
10-22
ur (m)

```
7.143 X 107
7.14708 X 107
-0.057
Not II
\(\boldsymbol{U}(\boldsymbol{m})\)
0.

10-22
ur (m)
7.143 X 107
7.14796 X 107
-0.069
Points E2, E'2
\(\boldsymbol{U}(\boldsymbol{m})\)
0.

10-21

Points F2, F'2
\(\boldsymbol{U}(\boldsymbol{m})\)
0.

10-22
Points A, B, G

A2, B2, H
YY (Pa)
\(1.25 \times 105\)
\(1.2500 \times 105\)
0.
\(A^{\prime} 2, B^{\prime} \mathbf{2}, H^{\prime}\)
A3, B3, I
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Points A, B, G

A2, B2, H
ZZ (Pa)
3.75 X 105
\(3.700 \times 105\)
0.
\(A^{\prime} 2, B^{\prime} 2, H^{\prime}\)
A3, B3, I

\subsection*{4.2 Remarks}
- Radial displacement ur is obtained with a good precision.

3
- The conditions of symmetry on face \(A B(v=0\) locally, is

V \(05 \mathrm{~W}=0\) ) are checked
2
at the points \(A 2, A^{\prime} 2, G, B 2, B^{\prime} 2, H, H^{\prime}, I\) considered.
In the same way, the conditions of symmetry on face \(E F(U=U=0)\) are checked at the points \(E 2\), \(E^{\prime}\) 2,
F2, F'2, G1, H1, H'1, I1 considered.
Key word LIAISON_OBLIQUE is thus validated.
- The identification of the nodes common to the 2 zones by key word LIAISON_GROUP is also validated: displacements \(U, V, W\) are identical to the points \(A^{\prime} 2, B^{\prime} 2, H^{\prime}, E^{\prime} 2, F^{\prime} 2, H^{\prime} 1\) in comparison of displacements to opposite respective A2, B2, H, E2, F2, H1.
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SSLV311 - Murakami 9.39. Fissure in quarter of ellipse to the corner of a disc
Date:
15/02/06
Author (S):
E. GALENNE

Key: V3.04.311-A Page: 1/8
Organization (S): EDF-R \& D /AMA

Handbook of Validation
V3.04 booklet: Linear statics of the voluminal structures
V3.04.311 document

SSLV311 - Murakami 9.39. Fissure in quarter of ellipse with the corner of a thick disc in rotation

\section*{Summary:}

This test results from the validation independent of the version in breaking process.

\section*{Applicability:}

\author{
Linear breaking process
}

Type of analysis:
Statics

Type of behavior:
Isotropic linear rubber band
Type of model:
Three-dimensional
A number of modelings:
1
Objective:
Basic test into three-dimensional for isotropic elastic materials, in field limited in three directions, in the presence of a voluminal loading.

\section*{Explored parameters:}

\section*{-}

Fixed parameters:
A/t reports/ratios, b/a, R2/R1, t/R1
Precision of the results:
Average standard deviation of 3\% with the analytical reference solution

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Author (S):
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{B}

With

\section*{Internal ray:}

R1 \(=0,1 \mathrm{~m}\)
External ray:
\(R 2=0,6 \mathrm{~m}\)
Thickness:
\(T=0,2 \mathrm{~m}\)
Half large axis:
\(=0,05 \mathrm{~m}\) have
Small half centers:
\(B=0,0125 \mathrm{~m}\)

\section*{1.2}

Properties of material

\author{
Young modulus \\ \(E=2.105 \mathrm{MPa}\) \\ Poisson's ratio \(=0.3\) \\ Density \\ \(=7800 \mathrm{~kg} / \mathrm{m} 3\)
}

\section*{1.3}

Boundary conditions and loading
The model will be limited to the part of the thick disc located in the half space \(Y 0\), the plan of fissure vertical being a symmetry plane.

In the absence of nodes on the axis of revolution, a rigid mode will be blocked by a linear relation between degrees of freedom.
That is to say points:
With (R1,0, T) B
(- R1,0, T)
Blocking of the translation in \(X: U X(A)+U X(B)=0\)
Blocking of the translation in \(Y: U Y=0\) in plan \(X O Z\), except for the lips of the crack.
Blocking of the translation in \(Z: U Z(A)=0\)
Blocking of rotation around \(O X\) : ensured by the \(C . L\) of symmetry in plan XOZ
Blocking of rotation around OY: UZ \((B)=0\)
Blocking of rotation around OZ: ensured by the C.L of symmetry in plan XOZ
Loading: stationary angular velocity = 500 radians/second
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\section*{2}

Reference solution
2.1

Method of calculation used for the reference solution
Method of integral equation of border.

\section*{2.2}

Results of reference
\(3+\)
2
1 -
2
2
\(K=\)
\(\cdot \boldsymbol{R}+\)
R
\(B \boldsymbol{F}\) where the geometrical factor of correction is given,
I
2
1
I
4
\(3+\)
according to the parametric angle of the ellipse, with the figure below.

The selected a/t report/ratio corresponds to the higher curve (squares).

\section*{2.3}

Uncertainty on the solution
The maximum change enters the points marked and the curve being of \(2 \%\), the misreading on the curve
is lower than the announced maximum error (5\%).

\subsection*{2.4 References \\ bibliographical}
[1]
Y. MURAKAMI: Stress Intensity Factors Handbook, box 9.39, pages 786-791. The Society of Materials Science, Japan, Pergamon Press, 1987.

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

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\section*{3.2}

Characteristics of the grid
The initial grid consists of 8890 nodes and 2203 elements, including 1264 elements CU20 and 939 elements PR15.
After the conversion of the grid of quadratic to linear, the number of nodes is reduced to 2230. This conversion is made essential by the use of the operator DEFI_FISS_XFEM, who function for the moment that with linear elements.

\section*{3.3 \\ Functionalities tested}

Calculation of the factors of intensity of the constraints buildings, in all the nodes of the bottom of crack, by method THETA.

The factors of intensity of the constraints buildings are calculated on a crown of lower ray Rinf \(=0,00075 \mathrm{~m}\) and of ray higher Rsup \(=0,0025 \mathrm{Mr}\).

\author{
Orders
}

CREA_MAILLAGE QUAD_LINE

\title{
AFFE_CHAR_MECA FORCE_INTERNE
}

FORMULATE

\section*{DEFI_FISS_XFEM}
\(C A L C_{-} G_{-} L O C A L_{-} T\) " \(C A L C_{-} K_{-} G\) "

\section*{EXCIT}

\section*{Handbook of Validation}

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\section*{4}

Results of modeling A

\subsection*{4.1 Values}
tested

\section*{Identification}

Reference (Pa.m)
Aster (Pa.m)
\% difference
KI, \(=0\) degrees
5,657E+07 5,789E+07-2,33
KI, = 1,4 degrees
5,945E+07 5,360E+07 9,84
\(K I,=2,8\) degrees
6,292E+07 6,596E+07-4,84
KI, = 4,3 degrees
6,638E+07 6,606E+07 0,48
KI, \(=5,9\) degrees
6,984E+07 6,902E+07 1,18
KI, = 7,6 degrees
7,273E+07 7,289E+07-0,22
\(K I,=9,5\) degrees
7,562E+07 7,597E+07-0,47
KI, = 11,6 degrees
7,908E+07 8,053E+07-1,83
KI, = 14,4 degrees
8,197E+07 8,261E+07-0,78
\(K I,=16,9\) degrees
8,543E+07 8,695E+07-1,78
KI, = 20,5 degrees
8,889E+07 8,785E+07 1,17
KI, = 25,1 degrees
9,178E+07 9,190E+07-0,13
KI, = 31,1 degrees
9,466E+07 9,173E+07 3,09
KI, = 39,5 degrees
9,640E+07 9,562E+07 0,81
KI, = 51,5 degrees
9,755E+07 9,510E+07 2,51
KI, = 68,5 degrees
9,755E+07 9,824E+07-0,71
\(K I,=90\) degrees
9,640E+07 9,720E+07-0,83
The parametric angles of the values tested correspond to the position of the 17 points of the bottom of fissure. The figure below makes it possible to compare the result of calculation with the reference solution.
The average standard deviation is very satisfactory:
(K ref. - K Aster 2
I
I
) \(d s\)
Average standard deviation =
=
= 3,11 \%
(Kref 2
I
) ds
100
95
90
85
0.5)

Note:
The voluminal loading is introduced here using key word FORCE_INTERNE (order AFFE_CHAR_MECA) and of FORMULA. The results are equivalent if the key word is used ROTATION.
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\section*{5 \\ Summary of the results}

Within sight of the precision announced on the results of reference (5\%) and the average standard deviation
obtained (3,11\%), the results provided by Code_Aster are satisfactory.

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Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

Key:
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Organization (S): EDF/IMA/MMN
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V3.05 booklet: Linear statics of the assembled structures
Document: V3.05.100
SSLX100-Mix 3D - Hull - Beam in inflection
Summary:
This test makes it possible to validate for a linear elastic design:
- a mixture of various mechanical models: model 3D (element HEXA20), model of hull (element DKT) and model of beam (elements POU_D_E, or elements COQUE_C_PLAN), - of the linear relations between degrees of freedom.

The test is based on the elastic analytical solution of a beam in inflection, the reduced number of elements for
the various models leads to a poor solution, which is however improved clearly with employment of boundary conditions appropriate to the theory of the beams.
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Titrate:

SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

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1
Problem of reference
1.1 Geometry
\(y\)
hull thickness 1
beam of section
M2
rectangular
B
C2
has
O
C
D
M
\(X\)
M1
With
C1
\(F=-1\).
NR
M4
L1
L2
L3
Z
\(y\)
Z
B
has
beam of section
rectangular
\(L 1=L 2=L 3=10 \mathrm{~mm}\)
\(B=1 \mathrm{~mm}\)
\(=3 \mathrm{~mm}\) have

Material properties
E \(=200.000 \mathrm{MPa}\)
\(=0.3\)
\(=0.0\) make it possible to avoid the variation of orthogonal curve induced by the effect Poisson in plates, which causes a difference between the theories of beams and plates, out of average fibre.
1.3

Boundary conditions and loadings
- force \(F y=1\) (load 1) or couples \(C z=1\) (load 2)
- defined or applied to neutral fibre
- embedding of the section \(X=0\)
- continuity of displacements of translation on \(A B\)
- continuity of displacements of translation out of C
- equality of displacements of rotation around Z on C1-C2
- for the points M of the section (M1 m2 M4) displacements of translation \(U(M)\) depend
linearly displacement of rotation \(Z\) of the points \(P\) of \(A B\)
\(U(M)=Z(P) . y+d x(P)\)
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\section*{Key:}

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2
Reference solution
2.1

Method of calculation used for the reference solution
Analytical solution, isostatic structure.
The elastic arrow, elastic constraints and axial deformations and bending moment in all not \(X\)-coordinate \(X\) are given by:
- Charge \(n^{\circ} 1\) : force \(F y=1\)
\(M y(X)=F y . L(1-x / L)\)
(= E. Iz. uy " \((X)\) in elasticity)
\(u y(X)=\) Fy L. x2. \((3-x / L) /(6\). E.Iz)
(in elasticity)
(in elasticity)
\(x x(X, y)=F y . L(1-x / L) . y /(E . I z)\)
(in elasticity)
\(x x(X, y)=-F y . L(1-x / L) . y / I z\)
- Charge \(n^{\circ} 2\) 2: Cz=1 or rotation drz couples \(=C z . L /(E . I z)\)
\(M y(X)=C z\)
(= E. Iz. uy " (X) in elasticity)
uy \((X)=C z . x 2 . /(2\). E.Iz)
(in elasticity)
(in elasticity)
\(x x(X, y)=C z . y / I z\)
(in elasticity)
\(x x(X, y)=-C z, y /(E . I z)\)
with:
\(L=L 1+L 2+L 3=30 \mathrm{~mm}\)
\(I z=A . h 3 / 12=0.25 \mathrm{~mm} 4\)
\(d r z=0.0006\)
2.2

Results of reference
Axial arrows, constraints and deformations and bending moments in 4 points of the axis of the beam.
2.3

Uncertainty on the solution
Analytical solution.

\section*{Handbook of Validation}

V3.05 booklet: Linear statics of the assembled structures
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

Key:
V3.05.100-B Page:
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3 Modeling
With
3.1

Characteristics of modeling
Elements HEXA20, DKT and POU_D_E
N19
N18
N17
N25
N20
N16
N24
T4
N15
N13
N14
N12
N11
N26
T3
S1
S2
HE1
N10
N23
N23
N9
N28
N29
T2
N5

Not NR --> N1
Not A --> N4
Not C --> N23
Not D --> N29
Charge \({ }^{\circ} 1\) : force Fy
Total embedding on the section out of O

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes: 28
A number of meshs and types: 1 HEXA20, 4 TRIA3/DKT, 2 SEG2/POU_D_E

\subsection*{3.3 Functionalities}

\section*{tested}

Orders

\section*{Keys}

AFFE_MODELE
AFFE
MODELING
"3D"
[U4.22.01]
AFFE
MODELING
"DKT"
AFFE
MODELING
"POU_D_E"
AFFE_CARA_ELEM
HULL
[U4.24.01]
BEAM
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]

\author{
LIAISON_GROUP \\ LIAISON_DDL \\ FORCE_NODALE \\ MECA_STATIQUE \\ [U4.31.01] \\ Handbook of Validation \\ V3.05 booklet: Linear statics of the assembled structures \\ HI-75/98/040 - Ind A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)

\section*{J.P. LEFEBVRE, F. VOLDOIRE}

Key:
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4
Results of modeling A
4.1 Values
tested
- Charge n \({ }^{\circ}\) : force Fy

\section*{Identification}

\section*{Reference}

Aster
\% difference
one (N1 node)
0.
0.
0.
uM (N26 node)
0.0267
0.0231
13.5
uA (N4 node)
0.0267
0.0230
13.8
uC1 (N22 node)
0.0933
0.0855
8.4

CPU (N23 node)
0.0933
0.0856
8.3
uD (N29 node)
0.18
0.1686
6.4

MyN (N1 node)
-30.
MyM (N26 node)
-20.
MyA (N4 node)
-20.
MyC1 (N22 node)
-10.
MyC (N23 node)
-10.
MyD (N29 node)
0 .

\subsection*{4.2 Parameters}
of execution
Version: 4.00.02
Machine: CRAY C98
System:
UNICOS 9.0
Obstruction memory:
8 MW
Time CPU To use:
4 seconds
Handbook of Validation
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\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE
Key:
V3.05.100-B Page:
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5 Modeling
B
5.1
Characteristics of modeling
Elements HEXA20, DKT and POU_D_E
N19
N18
N17
N25
N20
N16
N24
N15
T4
N13
N14
N12
N11
N26
T3
S1
HE1
S2
N10
N23
N27
N9
N28
N29
T2
N5
T1
N7
N8
N6
N4
N1

Not NR --> N1
Not A --> N4
Not C --> N23
Not D --> N29
Charge \(n^{\circ} 1\) : force Fy
Embedding on the section out of O carried out by a connection 3D_POUTRE between the face N1 N13 N19 N7 and
a discrete element located on the origin.
Additional relation, compared to modeling A, between C1 C2 and C, introduced by LIAISON_ELEM: "COQ_POU".

\section*{5.2}

\section*{Characteristics of the grid}

A number of nodes: 29
A number of meshs and types: 1 HEXA20, 4 TRIA3/DKT, 2 SEG2/POU_D_E, 1 POI1/DIS_TR, 1 QUAD8, 2 SEG2/BORD_DKT

\subsection*{5.3 Functionalities}

\section*{tested}

\section*{Orders}

\section*{Keys}

AFFE_MODELE

\section*{AFFE}

MODELING
"3D"
[U4.22.01]
AFFE
MODELING
"DKT"

\section*{AFFE}

MODELING
"POU_D_E"
AFFE_CARA_ELEM
HULL
[U4.24.01]
BEAM
DISCRETE
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
LIAISON_GROU
```

LIAISON_ELEM
OPTION
"3D_POU"
FORCE_NODALE
"COQ_POU"
MECA_STATIQUE
OPTION
"SIEF_ELGA_DEPL"
[U4.31.01]
"EFGE_ELNO_DEPL"
POST_RELEVE_T
ACTION
OPERATION
"AVERAGE"
[U4.74.04]
RESULTANT
"DX" "DY" "DZ"
MOMENT
"DRX" "DRY" "DRZ"
Handbook of Validation
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```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

Key:
V3.05.100-B Page:
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6
Results of modeling \(B\)
6.1 Values
tested
- Charge n \({ }^{\circ}\) 1: force Fy

Identification
Reference
Aster

\section*{\% difference}
one (N1 node)
0.
3.81 E18
0.
uM (N26 node)
0.0267
0.0260
2.6
uA (N4 node)
0.0267
0.0259
2.9
uC1 (N22 node)
0.0933
0.0928
0.5

CPU (N23 node)
0.0933
0.0929
0.4
uD (N29 node)
0.18
0.1804
+0.2
Mz (not O)
-30.
-30.
0
Mz (not M)
-20.
-20.
0
Mz (point C)
-10.
-10.
0
Mz (N23 node)
-10.
-10.
0
Mz (N29 node)
0 .

\subsection*{6.2 Remarks}

The moments resulting at the points \(\mathrm{O}, \mathrm{M}\) and C are obtained by POST_RELEVE (moment resulting from
nodal forces on the edges A B and C 1 C 2 ).
6.3 Parameters
of execution
Version: 4.00.02
Machine: CRAY C98
System:
UNICOS 9.0
Obstruction memory:
8 MW
Time CPU To use:
4 seconds
Handbook of Validation
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\section*{Code_Aster ®}

Version
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

Key:
V3.05.100-B Page:
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7 Modeling
C

\section*{7.1}

Characteristics of modeling
Elements HEXA20, DKT and COQUE_C_PLAN
N19
N18
N17
N25
N20
N16

In N29: charge \(\mathrm{n}^{\circ}\) 1: force Fy , load \(\mathrm{n}^{\circ}\) 2: Cz or rotation drz couples
Displacements DY and DZ on section 0 null on average (order LIAISON_DDL).
Note:
As the width of the beam is \(\mathrm{a}=3 \mathrm{~mm}\), the Young modulus is multiplied by 3 in material of Coque_C_Plan.
7.2

Characteristics of the grid
A number of nodes: 29
A number of meshs and types: 1 HEXA20, 4 TRIA3/DKT, 1 SEG3/COQUE_C_PLAN

\subsection*{7.3 Functionalities}

\section*{tested}

\section*{Orders}

\section*{Keys}

AFFE_MODELE
AFFE
MODELING
"3D"
[U4.22.01]
AFFE
MODELING
"DKT"
AFFE
MODELING
"COQUE_C_PLAN"
AFFE_CARA_ELEM

\section*{HULL}
[U4.24.01]
BEAM
AFFE_CHAR_MECA
DDL_IMPO
[U4.25.01]
LIAISON_GROU
LIAISON_DDL
FORCE_NODALE
MECA_STATIQUE
[U4.31.01]
RECU_CHAMP
RESULT
NOM_CHAM
"DEPL"
[U4.62.01]
CALC_ELEM
RESULT
OPTION
"EFGE_ELNO_DEPL"
[U4.61.02]
"SIGM_ELNO_DEPL"
"EPSI_ELNO_DEPL"
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Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

Key:
V3.05.100-B Page:
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8
Results of modeling C
8.1 Values
tested
Identification
Reference
Aster (= 0.3)
\% difference
Charge 1: force Fy
one (N8 node)
0.
5.97 10-5
0.00
uM (N26 node)
0.0267
0.02528
5.30
uA (N4 node)
0.0267
0.02519
5.64
uC1 (N22 node)
0.0933
0.09136
2.08

CPU (N23 node)
0.0933
0.09148
1.95
uD (N29 node)
0.18
0.1766
-1.90
MyN (N8 node)
-30.
n.c.

MyM (N26 node)
-20.
n.c.

MyA (N4 node)
-20.
n.c.

MyC1 (N22 node)
-10.
n.c.

MyC (N23 node)
-10.
n.c.

MyD (N28 node)
-5.
-5.0012
0.025
xx (X, h/2) (N28 node)
-30.0000
-30.0007
0.025
xx (X, h/2) (N28 node)
5.0 10-5
5.0012 10-5
0.025

Charge 2: Cz couples
one ( N 1 node)
0.
2.28 10-6
0.00
uM (N26 node)
0.0010
0.001005
0.451
uA (N4 node)
0.0010
0.000998
-0.224
uC1 (N22 node)

\subsection*{8.2 Remarks}

The calculation of the efforts and the moment in the Coque_C_Plan element is carried out with the node N28 medium
so that the interpolation is correct; knowing that the Young modulus is triple so that the product E. Iz that is to say identical in all the model, the constraints are it too.

\subsection*{8.3 Parameters}
of execution
Version: 4.02.12
Machine: CRAY C98
System:
UNICOS 9.0
Obstruction memory:
8 MW
Time CPU To use:
16,2 seconds
Handbook of Validation
V3.05 booklet: Linear statics of the assembled structures
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\section*{Code_Aster \({ }^{\circledR}\)}

\section*{Version}
4.0

Titrate:
SSLX100 Mixes 3D - Hull - Beam in inflection
Date: 01/12/98
Author (S)
:
J.P. LEFEBVRE, F. VOLDOIRE

Key:
V3.05.100-B Page:
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\section*{Summary of the results}
- The grid is very coarse in elements 3D and plate. The test deserves a modeling more fine, since the results are influenced by the way of describing the conditions of embedding out of O . the modeling A led to an error of \(14 \%\) to the maximum, - however that with a good taking into account of these conditions, the solution is clearly better (modeling B led to an error of \(3 \%\) maximum).
- The comparisons of the constraints and efforts give good results (modeling B). For the element of hull 1D, the results are very good.

\section*{Handbook of Validation}

V3.05 booklet: Linear statics of the assembled structures
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Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

SSLX101 - Pipe right modelled in hulls and beams

\section*{Date:}

05/02/00
Author (S):

\section*{J.M. PROIX}

Key: V3.05.101-A Page: 1/8
Organization (S): EDF/IMA/MMN

\author{
Handbook of Validation \\ V3.05 booklet: Linear statics of the assembled structures \\ Document: V3.05.101
}

SSLX101-Pipe right modelled in hulls
and in beams

\section*{Summary:}

The purpose of this test is to validate the connection hull-beam. The pipe is embedded at an end, and subjected to 4
successive efforts (traction and 3 moments) on the other end. A half of the pipe is with a grid in hulls, the other is with a grid in beams. Embedding and the connection between the hull part and the beam are carried out by a connection hull-beam, allowing in particular to transmit to the hull only the torque efforts of beam type, without generating secondary stresses.

The reference solution is analytical (RDM). The variation with the numerical solution (from 3 to 5\%) is explained by
fact that the grid in hulls actually consists of element plans (facets). The geometry of the pipe is thus itself approximate. The solution obtained makes it possible to check that connection enters the elements of
hull and the element of beam is correct.
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Code_Aster \({ }^{\circledR}\)
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5.0

Titrate:

Date:
05/02/00
Author (S):
J.M. PROIX

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\section*{1 \\ Problem of reference}

\subsection*{1.1 Geometry}

Right pipe length 80m, modelled in hulls between 0 and 40m, and in beams between 40 and 80 m . External ray: 2m, thickness: 0.1m.
The axis of the pipe is in the Oxy plan, tilted of 30 degrees compared to \(O X\).

\section*{\(y\)}
x1
\(P 2\)
P1
\(X\)
\(Z\)
1.2

Material properties
\(E=2.1011 P a\)
\(=0.3\)

\section*{1.3}

Boundary conditions and loadings
Embedding "of beam type" in \(X=y=0\), realized by a connection hull-beam between the C1 edge of hull and a P1 point (located out of \(O\) ). It is this point which is blocked.

4 unit loading cases applied to the point P2 (80*cos30, 80*sin30, 0)
\(F x 1=1 N\) traction following the axis \(O x 1\), is \(F x=\cos 30 . F x\) and \(F y=\operatorname{sin30.Fx1}\) Torque \(M x 1=1 \mathrm{Nm}\) around \(\mathrm{Ox1}\), is \(M X=\cos 30 . M x 1\) and \(M y=\sin 30 . M x 1\)
Bending moment My1= 1Nm around Oy1 is \(M x=-\sin 30 . M y 1\) and \(M y=\cos 30 . M y 1\)

Bending moment around \(O Z\), is \(M z=1 \mathrm{Nm}\)
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Code_Aster \({ }^{\circledR}\)
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Author (S):
J.M. PROIX

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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
Analytical solution for each of the \(\mathbf{4}\) cases of loading:
In theory of beams, within the framework of the assumptions of Euler-Bernouilli, the solution of the problem of reference is that of a right beam subjected to efforts and moments at an end and embedded at the other end:

\section*{\(X\)}

Traction: \(U\)
```

E X
I
IGJ
1
M there X
Inflection around OY: =
. E
1
y
y
I
I.E.(internal excitation)
1
y1
M
Inflection around O
Z X
Z:Z =
.ez
EIz

```

\section*{2.2}

\section*{Results of reference}

\section*{Traction:}
```

L

```
L
ux
=
1 (P2)
Fx1 ESL
thus
ux (P2) = Fx
.cos()
30
1 ES
L
uy(P2) = Fx
sin ()
30
1 ES
```


## Torsion:

```
L
X
=
1 (P2)
Mx1 GJ
thus
L
X(P2)=
MX
cos()
30
IGJ
L
y(P2)=
MX
sin()
30
IGJ
Inflection around: OY
I
L
y=
My
1
1 I.E.(internal excitation) y1
thus X =
- y.si(
N
)
30
I
y=
y
co
)
```

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05/02/00
Author (S):

## J.M. PROIX

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## Inflection around: $O Z$

L
$Z=$
Mz EIz
L2
and
$u y=M z$

1
$2 E I z$
thus
$u x=$
uy $\sin ()$
30
1
$u y=u y \cos ()$
30
1
Note:
The use of the connection hulls beams for embedding and the loadings allows
to remain within the framework of the assumption of Euler-Bernouilli (cf [R3.03.06]). The analytical solution
the preceding one is thus well the reference solution of the problem.

## 2.3 <br> Uncertainty on the solution

# Analytical solution. <br> Handbook deValidation <br> V3.05 booklet: Linear statics of the assembled structures <br> HI-75/01/010/A 

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SSLX101 - Pipe right modelled in hulls and beams

Date:
05/02/00
Author (S):
J.M. PROIX

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

1 mesh POII (modeling DIS_TR), 1280 meshs QUAD4 (modeling DKT), 4 meshs SEG2
(POU_D_E). 32 meshs SEG2 on each edge of the hull.

## 3.2 <br> Characteristics of the grid

A number of nodes: 4416
A number of meshs and types: 1 POII, 4 SEG2, 1280 QUAD4

### 3.3 Functionalities

tested

Orders

## Keys

AFFE_MODELE
MODELING
DKT

```
[U4.22.01]
POU_DE
DIS_TR
AFFE_CARA_ELEM
HULL
[U4.24.01]
BEAM
```


## DISCRETE

AFFE_CHAR_MECA

```
4
```

4
Results of modeling A

```
Results of modeling A
```


### 4.1 Values

tested
Displacements and rotations at the P2 point (to be multiplied by 1.E-10 m).

## Loading case

Identification
Reference
Aster \%
difference
Traction DX
2.82732 .7942
1.2

Traction DY
1.63241 .6132
1.2

Torsion DRX
1.93195
1.87133 .1

Torsion DRY
1.1154

```
1.0804 3.1
Inflection Y
DZ
-68.64
-64.88
5 . 5
Inflection Y
DRX
-0.858
-0.827
3.7
Inflection Y
DRY
1.4861
1.4319
3.7
Inflection Z
DX
-34.32
-32.44
5.5
Inflection Z
DY
59.44
56.19
5.5
Inflection Z
DRZ
1.716
1.653
3.7
```


### 4.2 Parameters of execution

Version: 5.2

Machine: SGI/ORIGIN 2000
System:
Obstruction memory: 128 Mo

## Time CPU To use:

11 seconds

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Titrate:
SSLX101 - Pipe right modelled in hulls and beams
Date:
05/02/00
Author (S):

## J.M. PROIX

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## 5 Modeling <br> B

5.1

## Characteristics of modeling

The beam part is modelled by pipe sections. The hull part is modelled in DKT.

## 5.2 <br> Characteristics of the grid

A number of nodes: 4420
A number of meshs and types:

- 1 mesh POIl (modeling DIS_TR),
- 1344 meshs QUAD4 (modeling DKT), 32 meshs SEG2 on each edge of the hull . 4 meshs SEG3 (PIPE)


### 5.3 Functionalities <br> tested <br> Orders

## Keys

AFFE_MODELE
MODELING
DKT
[U4.22.01]
POU_DE
$D I S \_T R$

## AFFE_CARA_ELEM

HULL
[U4.24.01]
BEAM
DISCRETE
$A F F E \_C H A R \_M E C A$
LIAISON_ELEM
COQUE_TUYAU
[U4.25.01]
MACRO_ELAS_MULT
[U4.31.03]

6

## Results of modeling $A$

### 6.1 Values

tested
Displacements and rotations at the P2 point (to be multiplied by 1.E-10 m).
Loading case
Identification
Reference
Aster \%
difference
Traction DX
2.82732 .7942
1.2

Traction DY
1.63241 .6132
1.2

Torsion DRX
1.4325
3.7
Inflection Z
DX
-34.32-32.40

### 6.2 Parameters

 of executionVersion: 5.1

# Machine: SGI/ORIGIN 2000 

System:
Obstruction memory: 128 Mo
Time CPU To use:
15 seconds

## Handbook deValidation

V3.05 booklet: Linear statics of the assembled structures

## HI-75/01/010/A

## Code_Aster ${ }^{\circledR}$

Version
5.0

Titrate:
SSLX101 - Pipe right modelled in hulls and beams
Date:
05/02/00
Author (S):
J.M. PROIX

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## 7 <br> Summary of the results

The reference solution is analytical (RDM). The variation with the numerical solution (from 3 to 5\%) be explained by the fact that the grid in hulls actually consists of plane elements (facets). geometry of the pipe is thus itself approximate. The solution obtained makes it possible to check that it connection between the elements of hull and the element of beam is correct.

## Handbook deValidation

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HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
SSLX101 - Pipe right modelled in hulls and beams
Date:
05/02/00

## J.M. PROIX

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Handbook deValidation
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HI-75/01/010/A

Code_Aster ${ }^{\circledR}$
Version
5.4

Titrate:
SSLX102 - Piping bent in inflection

Date:
19/09/02
Author (S):
J. Mr. PROIX, P. MASSIN, C. Key DURAND

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## Handbook of Validation

V3.05 booklet: Linear statics of the assembled structures
V3.05.102 document

SSLX102 - Piping bent in inflection

## Summary:

This test validates the modeling of the phenomena of ovalization in pipings: an elbow, prolonged by right pipes is subjected to one bending moment. This one causes in the elbow an ovalization which propagate and diminishes in the right parts, and which modifies the rigidity of the whole of piping. For
to check the exactitude of the results, one tests the flexibility of the whole of piping (value of displacement with
the end for one imposed moment) and elements of hulls DKT.
The case test comprises six modelings:

- For modeling A, the elbow is with a grid in hulls (modeling DKT), modeling pipe is used for the right parts, the connection is ensured by a connection COQUE_TUYAU.
$\cdot$ For modelings B and C, modeling PIPE is assigned to the whole of piping:
TUYAU_3M for B and TUYAU_6M for C ( $M=$ modes of Fourier).
$\cdot$ In modeling D, piping is with a grid in hulls (elements COQUE_3D), the ends are with a grid in beams to apply the loadings.
- Modélisation E: Modeling PIPE (3 modes of Fourier) is assigned to the whole of piping.
- For modeling $F$, the elbow is with a grid in voluminal meshs, the right parts in elements

PIPE and the connection is ensured by a connection 3D_TUYAU.
Modeling A makes it possible to validate the good transmission of ovalization (mode 2) between the elements PIPE,
and validates connection COQUE_TUYAU, two modelings B and C make it possible to validate the elements PIPE
(with 3 and 6 modes of Fourier) in linear elasticity and last modeling makes it possible to validate the maid
transmission of ovalization (mode 2) between the elements PIPE, and validates the connection 3D_TUYAU.
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## 1

## Problem of reference

### 1.1 Geometry

## Piping bent in plan XY. The right parts have as a length $L=1200$ Misters.

The elbow has as a radius of curvature: $R c=305 \mathrm{~mm}$

## With <br> B <br> C <br> D <br> Mz

The tubular section has for average radius $R=105.5 \mathrm{~mm}$ and a thickness $E=8.18 \mathrm{~mm}$

## 1.2

Properties of materials
$E=200000 \mathrm{MPa}$
$=0.3$

## 1.3

Boundary conditions and loadings
Embedding in A
2 loading cases:

1) Moment MZ imposed in D: MZ=17000Nm (cross-bending).
2) Inflection except plan: My moment imposed in $D$
$M y=17000 \mathrm{Nm}$

### 1.4 Conditions

initial

Without object.

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## 2

Reference solution
2.1

Method of calculation used for the reference solution

Comparison with other numerical results obtained with grids 3D or hulls (in
tally in particular of the tripartite Card 3455 of the ECA, (see references), and with a calculation COQUE_3D
enough fine (modeling D).

## 2.2

Results of reference

For one moment applied MZ in D of 17000 Nm, displacement DY of the same point D is worth:

Type of calculation
Dy not D (mm)
Calculation ECA (hulls + Victus) 0.02
Calculation COQUE_3D
0.02012 (modeling $D$ )

We choose for reference the value $D y=0.02$ Misters.
For the inflection except plan, the value of reference (fine calculation COQUE_3D) is worth 1.5657 102 Misters.

## 2.3

Precision on the results of reference
Owing to the fact that the reference solution is numerical, one can evaluate the precision according to [§2.2] to 2\%.

### 2.4 References <br> bibliographical

## [1]

M.N. BERTON: "Elastoplastic Calculations of pipings with CASTEM 2000. Formulation VICTUS. Synthesis of card 3455". Note CEA/LDM 96/6036
[2]
J.M. PROIX, A. BEN HAJ YEDDER: "Project CACIP: study of a piping bent in inflection". Note EDF/DER HI-75/98/001/0

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The elbow is with a grid in hulls (meshs QUAD4, modeling DKT). All the right parts are with a grid in elements pipes (meshs SEG3, modeling PIPE).

## 3.2

Characteristics of the grid
320 meshs QUAD4

## 8 meshs SEG3

32 meshs SEG2 (edges of hulls).

3.3 Functionalities<br>tested<br>Orders<br>Key word factor<br>Single-ended spanner word<br>\section*{Argument}<br>AFFE_MODELE<br>AFFE<br>MODELING<br>"TUYAU_3M"<br>AFFE_CHAR_MECA<br>LIAISON_ELEM<br>OPTION<br>"COQUE_TUYAU"<br>MECA_STATIQUE

## 4

Results of modeling $A$

### 4.1 Values

tested

Loading case
Displacement of the point $D$
Reference
Aster \%
diff
$M z=17000 \mathrm{Nm}$
DY (mm)
0.02
0.01941
3.0
$M y=17000 \mathrm{Nm}$
DZ (mm)
-0.015657
-0.0157

## 0.3

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## 5 Modeling

B
5.1

Characteristics of modeling
The whole of piping is with a grid with bent elements PIPE, rights or.

## 5.2 <br> Characteristics of the grid <br> 86 meshs SEG3

### 5.3 Functionalities

tested

Orders

Key word factor
Single-ended spanner word
Argument
AFFE_MODELE

AFFE<br>MODELING<br>"TUYAU_3M"

6
Results of modeling B
6.1 Values
tested

## Loading case

Displacement of the point $D$
Reference
Aster \%
diff
$M z=17000 \mathrm{Nm}$
DY (mm)
0.02
0.0186
6.8
$M y=17000 \mathrm{Nm}$
DZ (mm)
-0.015657
-0.0154
1.9

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## 7 Modeling <br> C <br> 7.1 <br> Characteristics of modeling

All piping is modelled in TUYAU_6M (6 modes of Fourier).

## 7.2 <br> Characteristics of the grid

86 meshs SEG3

7.3 Functionalities<br>tested<br>Orders<br>Key word factor<br>Single-ended spanner word<br>Argument<br>AFFE_MODELE<br>AFFE<br>MODELING<br>"TUYAU_6M"

## 8 <br> Results of modeling $C$

### 8.1 Values

tested

## Loading case

Displacement of the point $D$
Reference
Aster \%
diff
$M z=17000 \mathrm{Nm}$
DY (mm)
0.02
0.0199
0.01
$M y=17000 \mathrm{Nm}$
DZ (mm)
-0.015657
-0.01598
2
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## 9 Modeling

D

## 9.1 <br> Characteristics of modeling

- Piping is with a grid in hulls (elements hulls 3D).
- The ends are with a grid in beams (POU_D_T) to be able to apply them easily
boundary conditions.
- This modeling constitutes a reference solution for modelings $A, B$ and $C$ in
private individual for the inflection except plan where one does not have results published.


## 9.2 <br> Characteristics of the grid

680 meshs QUAD8
2 meshs SEG2

### 9.3 Functionalities <br> tested

Orders
Key word factor
Single-ended spanner word

## Argument

CREA_MAILLAGE
MODI_MAILLE
OPTION
"QUAD8_9"
AFFE_MODELE
AFFE
MODELING
"COQUE_3D"
AFFE_CHAR_MECA
LIAISON_ELEM
OPTION
"COQUE_POU"

## 10 Results of modeling $D$

### 10.1 Values

tested

## Loading case

Displacement of the point $D$
Reference
Aster \%
diff
$M z=17000 \mathrm{Nm}$
DY (mm)
0.02
0.0192
0.6
$M y=17000 \mathrm{Nm}$
DZ (mm)
-0.015657
-0.015601
0.4

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11 Modeling
E

### 11.1 Characteristics of modeling

The whole of piping is with a grid with bent elements PIPE, rights or, resting on meshs with 4 nodes.

### 11.2 Characteristics of the grid

18 meshs SEG4 (10 in the elbow, 4 in each right part).

### 11.3 Functionalities

tested

## Orders

Key word factor
Single-ended spanner word
Argument
AFFE_MODELE
AFFE
MODELING
"TUYAU_3M"
CREA_MAILLAGE
MODI_MAILLE
OPTION
"SEG3_4"
DEFI_GROUP
CREA_GROUP_NO
OPTION
"NOEUD_ORDO"
AFFE_CARA_ELEM
ORIENTATION
CARA
"GENE_TUYAU"
STAT_NON_LINE
COMP_INCR
RELATION
"ELAS"

12 Results of modeling $E$
12.1 Values
tested
Loading case
Displacement of the point D Reference (MOD B)
Aster \%
diff
$M z=17000 \mathrm{Nm}$
DY (mm)
0.0186
0.018540.4
$M y=17000 \mathrm{Nm}$
DZ (mm)
-0.0154
-0.0153
0.2
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13 Modeling ..... F

### 13.1 Characteristics of modeling

The two right parts of piping are with a grid with bent elements PIPE, resting on meshs with 3 nodes. The elbow is with a grid with voluminal meshs HEXA20.

This modeling thus makes it possible to validate the good transmission of ovalization (mode 2) between
elements PIPE on the right parts and the elbow in 3D, and validates the connection 3D_TUYAU.

### 13.2 Characteristics of the grid

234 meshs SEG3.
512 meshs HEXA20.

### 13.3 Functionalities

tested
Orders
Key word factor
Single-ended spanner word

Argument

AFFE_MODELE
AFFE
MODELING
"TUYAU_3M"
AFFE
MODELING
`3D
AFFE_CARA_ELEM
ORIENTATION
CARA
"GENE_TUYAU"
MECA_STATIQUE

14 Results of modeling $F$

### 14.1 Values

tested

## Loading case

Displacement of the point $D$
Reference
Aster \%

```
diff
Mz=17000 Nm
DY (mm)
0 . 0 2 0 0
0 . 0 1 9 2 2
4 . 4
My = 17000 Nm
DZ (mm)
-0.015657
-0.01561
0 . 0
```


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```
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```

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15 Summary of the results
This test not having reference solutions analytical, but numerical, the variations noted (in on this side $2 \%$ except 1 value) can be regarded as reasonable.

More precisely, for modeling A (elbow with a grid in hulls DKT and right beams in PIPE) one can estimate that the solution obtained ( $2.7 \%$ of variation in cross-bending, and $0.4 \%$ in inflection except plan, compared to the reference: grid all in hulls of modeling D) makes it possible to validate the good operation of connection coque_tuyau.

For modeling B (elements PIPE, 3 modes of Fourier), the important variation on cross-bending
(6.8\%) is due to the fact that piping is relatively thin, therefore that ovalization in the made elbow to appear of the modes of Fourier of a nature higher than 3.

In fact, modeling C (PIPES, 6 modes of Fourier) is very close to the reference ( $0.01 \%$ in cross-bending, and $2 \%$ in inflection except plan). The element pipe is thus validated in elasticity for these
loadings, compared to a solution in hulls (modeling D).
Modeling E (elements PIPE with 4 nodes) gives results identical to modeling B, at a cost of weaker calculation due to a less fine grid.

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Organization (S): EDF/ERMEL/PEL

V3.06 booklet: Linear statics of the axisymmetric structures
Document: V3.06.100

SSLA100 - Infinite cylinder subjected to a field of voluminal and surface forces

## Summary:

This test of linear quasi-static mechanics makes it possible to validate the assignment of a loading of field of
forces, surface or voluminal.
The studied structure is cylindrical. The fields with the nodes of voluminal and surface density of forces are
read in a file with the Ideas format. For the voluminal loading, the field read varies quadratically in function of the distance to the axis; for the surface loading, the field read corresponds to an internal pressure.

Three modelings of the same problem are carried out:
modeling 3D;
axisymmetric modeling 2D;
modeling 2D plane deformations;
The reference solution is analytical.
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# Author (S): 

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$:$
$V$
2

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Problem of reference

### 1.1 Geometry

Z
$y$
$X$

Selected geometrical dimensions are as follows:

## height

$$
=0.5 \mathrm{~m} ;
$$

$$
\cdot
$$

interior ray
$=1 \mathrm{~m}$;
-
external ray = 1.2 Mr.

## 1.2

Properties of material
The cylinder consists of a homogeneous material which follows a law of elastic behavior linear:

$$
E=10 \mathrm{~Pa}
$$

$$
=1 \mathrm{Kg} / \mathrm{m3} ;
$$

$$
=0.3
$$

## 1.3

Boundary conditions and loadings (cf [Figure 1.3-a])

The voluminal force considered is radial, it varies in a quadratic way with the ray: $F V=. r^{2}$ with $=1 \mathrm{~N} / \mathrm{m} 3$.
The surface force considered is applied to the internal wall of the cylinder, perpendicular to wall (is equivalent to an internal pressure imposed on the cylinder): $F S(R=R i n t)=1 \mathrm{~N} / \mathrm{m}^{2}$.

The boundary conditions make it possible to be placed on the assumption of the plane deformations on one
section of the cylinder: vertical displacements blocked on the sections high and low of the cylinder.
Note:
For modeling 3D, the suppression of the clean modes is ensured by the conditions of plane 2D applied to the low section of the cylinder. This type of boundary conditions allows to obtain an axisymmetric in displacement, directly comparable solution with analytical solution.

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Modeling 3D:
FS
$U z=0$
FV
$U z=0$
Axisymmetric modeling 2D:
$U z=0$
FS
FV
$U z=0$

## Modeling plane 2D:

FV
$\boldsymbol{U Y}=\mathbf{0}$
FS
$U X=0$
$U X=0$
$\boldsymbol{U Y}=\mathbf{0}$

## Appear 1.3-a: Boundary conditions and loadings

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## 2 <br> Reference solution

## 2.1

Method of calculation used for the reference solution
The problem of linear static mechanics axisymmetric considered can be solved in manner analytical. One solves independently the response to the request forces voluminal and forces surface to summon them then.

Voluminal force quadratic $F V(R)=R^{2}$
One considers the equilibrium equations in cylindrical co-ordinates:
$\boldsymbol{R}$
$\boldsymbol{R}$
$r z$
$\boldsymbol{R}$
$F$
0
R
$\boldsymbol{R}$
$\boldsymbol{R}$
$\boldsymbol{R}$
$+2$
$+F=0$ which is simplified being given axial symmetry
R
Z
R
R
Z $\boldsymbol{R}$ $+$ $+$

$$
+2
$$

$$
+
$$

$$
=0
$$

| F |
| :---: |
| Z |
| $\boldsymbol{R}$ |
| Z |
| $\boldsymbol{R}$ |
| $\boldsymbol{R}$ |
| - |
| $\boldsymbol{R}$ |
| $\boldsymbol{R}$ |
| in: |
| + |
| + |
| $=$ |
| Fr 0 |
| $\boldsymbol{R}$ |
| $\boldsymbol{R}$ |

By using the law of behavior then the relations deformation-displacements, one leads to $u^{\prime}$
$\boldsymbol{U}$
the following differential equation: $U$
V
$+$
$=0$
$\boldsymbol{R}$
$\boldsymbol{R}^{2}$

E(1-)
(1+)(1-2)
The voluminal force applied is of the type: $f V=. r^{2}$
The solution of the differential equation is written then:
$-C$
1
$(+) 1$
$(-2) r 4$
$U=$
$1-$
$+C R$
$e ́ q$
$2.1-1$
$R$

15th 1
(-
2
2
)
The two constants of integrations c1 and c2 are given thanks to the boundary conditions:
(
int
$\boldsymbol{R}$
$=0$
(R)
ext. $=0$
4-321+R2R2(R3-R3)
C
int ext.
int
ext.
$1=$

```
1-
15
E
R2-R2
One obtains:
ext.
int
1
(+) 1
(-2) 4-3
R3-R3
C
=
R3-R2
int
ext.
2
)
E
ext.
1-15
int R2-R2
```

ext.
int
Surface force standard pressure FS (Rint) $=P$

The problem to be solved is of comparable nature, but with a voluminal force applied null: $f V=0$
$=0$.
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## - C

The solution in displacement [éq 2.1-1] is written then: $U=$ $1+C$, having to observe the conditions:

What gives:

## 2.1-2

E
R2-R2int R
ext.

## 2.2 <br> Results of reference

## Numerical application:

## height

$$
=0.5 \mathrm{~m} ;
$$

## interior ray

$$
=1 \mathrm{~m} ;
$$

external ray

$$
=1.4 \mathrm{~m} ;
$$

## $P$

$=1 \mathrm{~N} / \mathrm{m}^{2}$.
by injecting the numerical values in the solutions [éq 2.1-1] and [éq 2.1-2] one finds afterwards summation:

```
U(.
1)
0=.
0 52130982m
```


## 2.3 <br> Uncertainties on the solution

Null (analytical reference solution).<br>Handbook of Validation<br>V3.06 booklet: Linear statics of the axisymmetric structures HM-77/01/149/A

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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

The cylinder is modelled in voluminal elements 3D:
P2
P1

## 3.2 <br> Characteristics of the grid

The cylinder is represented by a regular grid of quadratic elements with 20 nodes containing:

8 elements;

96 nodes.
The grid contains 1 only element in the radial and vertical direction and 8 cuttings on circumference.

### 3.3 Functionalities <br> tested <br> Orders Key word <br> factor

Key word<br>LIRE_RESU NOM_CHAM<br>FVOL_3D<br>LIRE_RESU NOM_CHAM<br>FSUR_3D<br>AFFE_CHAR_MECA EVOL_CHAR<br>\section*{4<br><br>Results of modeling $A$}<br>\subsection*{4.1 Values}<br>tested<br>\section*{Identification Moments Reference}<br>Aster \%<br>difference<br>UX in Pl<br>1<br>0.52130982<br>0.52097<br>6.54 10-2 \%<br>$U X$ in $P 2$<br>1<br>0.44203108<br>0.44178<br>5.74 10-2 \%

### 4.2 Parameters <br> of execution

Version: 6.01.19
Machine: Origin 2000
Obstruction memory: 16 Mo
Time CPU To use: 2.64 seconds
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## 5 Modeling <br> B

## 5.1

## Characteristics of modeling

A longitudinal section of the cylinder is modelled in voluminal elements 2D, while considering the assumption of axisymetry.

## P1 <br> P2 <br> 5.2 <br> Characteristics of the grid

The cylinder is represented by a regular grid of quadratic elements with 8 nodes containing:

4 elements;
21 nodes.
The grid contains 2 cuttings in the radial direction and 2 cuttings in the vertical direction.

### 5.3 Functionalities

tested
Orders Key word
factor
Key word
LIRE_RESU NOM_CHAM
FVOL_2D

LIRE_RESU NOM_CHAM

FSUR_2D
AFFE_CHAR_MECA
EVOL_CHAR

6
Results of modeling B
6.1 Values
tested

## Identification Moments Reference

Aster \%
difference
$U X$ in Pl
1
0.52130982
0.52129
4.07 10-3 \%
$U X$ in $P 2$
1
0.44203108
0.44202
3.95 10-3 \%

### 6.2 Notice

Modeling more powerful than the 3D because 2 cuttings in the radial direction and not of discretization circonférencielle.

### 6.3 Parameters

of execution
Version: 6.01.19
Machine: Origin 2000
Obstruction memory: 16 Mo
Time CPU To use: 1.89 seconds
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## 7 Modeling

C

## 7.1 <br> Characteristics of modeling

A transverse section of the cylinder is modelled in voluminal elements 2D, while considering the assumption of the plane deformations.
P1
P2
7.2

Characteristics of the grid
The cylinder is represented by a regular grid of quadratic elements with 8 nodes containing:

## 8 elements;

40 nodes.
The grid contains 1 only cutting in the radial direction and 8 cuttings in the vertical direction (like the 3D).

### 7.3 Functionalities

tested
Orders Key word
factor

Key word
LIRE_RESU NOM_CHAM
FVOL_2D
LIRE_RESU NOM_CHAM
FSUR_2D
AFFE_CHAR_MECA
EVOL_CHAR

## 8

Results of modeling $C$

### 8.1 Values

tested
Identification Moments Reference
Aster \%
difference
UX in P1
1
0.52130982
0.52131
6.76 10-2 \%
$U X$ in $P 2$
1
0.44203108
0.44204
5.74 10-2 \%

### 8.2 Remarks

Modeling of performance very close to the 3D because same discretizations circonférencielle and radial.

8.3 Parameters<br>of execution

Version: 6.0
Machine: Origin 2000
Obstruction memory: 16 Mo
Time CPU To use: 1.89 seconds
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V3.06 booklet: Linear statics of the axisymmetric structures

Code_Aster ${ }^{\circledR}$
Version
6.0

Titrate:
SSLA100-Infinite cylinder subjected to a field of voluminal forces
Date:
30/11/01
Author (S):
B. RIOU Key
:
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## 9

Summary of the results
The results obtained by the code_Aster are very close to the analytical solution, in spite of very coarse grids.

Modelings 3D and 2D plane give further information very close because they present them same discretizations circonférencielle and radial. Axisymmetric modeling 2D is more powerful because it presents 2 cuttings in the radial direction and not of discretization circonférencielle.

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Code_Aster ${ }^{\circledR}$
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Date:
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Author (S):
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Version
8.1

Titrate:
SSLA103 - Calculation of the withdrawal of desiccation and the endogenous withdrawal
Date:
01/09/05
Author (S):
S. MICHEL-PONNELLE, J. EL GHARIB Key
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## Handbook of Validation

V3.06 booklet: Linear statics of the axisymmetric structures
Document: V3.06.103

SSLA103 - Calculation of the withdrawal of desiccation and of endogenous withdrawal on a cylinder

## Summary:

The purpose of this case-test is to validate the calculation of the withdrawal of desiccation and the endogenous withdrawal. It also tests
possibility of making depend the characteristics materials on the hydration and drying (in the case of it
model of Mazars). It is about a cylinder which undergoes a drying and a uniform hydration. The temperature
also vary.
The cylinder is modelled by four elements quadrangles with 8 nodes for modelings $A, C, E$ and $F$ and by
an element HEXA20 for modelings B and D. For modelings $A$ and $B$, the behavior is presumedly elastic, which makes it possible to validate the calculation of withdrawal at the same time with STAT_NON_LINE and with
MECA_STATIQUE. Modelings $C$ and $D$ make it possible to validate the calculation of withdrawal with the law of MAZARS
local and not-local (without activation of the damage). Modeling E validates the calculation of withdrawal with law ENDO_ISOT_BETON and modeling F coupling ENDO_ISOT_BETON/BETON_UMLV_FP

The results obtained by Code_Aster are identical to the analytical solution of reference.
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Version
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Titrate:
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01/09/05
Author (S):
S. MICHEL-PONNELLE, J. EL GHARIB Key

## :

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1
Problem of reference
1.1 Geometry

Cylindrical test-tube
Z
D
C
D
$a=1 \mathrm{~m}$
has
R
With
B
has

## 1.2

## Material properties

For modelings $A$ and B, the material is supposed to be elastic and the characteristics materials are constants to be able to validate calculation with MECA_STATIQUE,
For modelings $C$ and D, one uses the law of MAZARS and certain parameters depend on the hydration and of drying.
Modeling E makes it possible to test law ENDO_ISOT_BETON, and modeling F the coupling ENDO_ISOT_BETON/BETON_UMLV_FP, knowing that the parameters materials of the law BETON_UMLV_FP are selected so that one does not have creep and thus which one finds it behavior of law ENDO_ISOT_BETON. In both cases, the characteristics materials are constants.
Let us announce that being given the loading (dilation, hydration and free drying), any damage does not develop: one thus finds in all the cases, the elastic solution.

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Modeling A and isotropic b: Elasticity
Modeling C and D: MAZARS
$E=30000 \mathrm{MPa}$
$E=10000 \mathrm{MPa}$ for $\mathrm{C}=100 \mathrm{l} / \mathrm{m} 3$
$=0.2$
30000 MPa for $C=801 / m 3$
$=1.6610-5(\mathrm{l} / \mathrm{m} 3)-1$
$=0.25$ for $h=0$
0.15 for $h=1$
endo $=1.510-5$

```
= 1.0 10-5 ' C-1
= 1.66 10-5 (l/m3)-1
endo = 1.5 10-5
= 1.0 10-5 ' C-1
Ac=1.4
At=0.8 for C=100l/m3
0.6 for C= 80l/m3
Bc=2000
LT=10000 for h=0
11000 for h=1
d0 = 10-4
= 1.06
Modeling E: ENDO_ISOT_BETON
Modeling F:
E = 30000 MPa
ENDO_ISOT_BETON/BETON_UMLV_FP
=0.2
= 1.66 10-5 (l/m3)-1
```

See modeling E +

```
S
1 9
K = 10 MPa
endo = 1.5 10-5
R
= 1.0 10-5 ' C-1
S
1 9
K=10 MPa
I
T
D
1 9
K=10
y = 4.0 MPa,
MPa
R
C
y=53.4 MPa
```

$S$
19
= $10 \mathrm{MPa} . j$
E
R
$T=-1.0103 \mathrm{MPa}$
$S$
19
$=10 \mathrm{MPa} . j$
I
D
19
= $10 \mathrm{MPa} . j$
R
D
19
$=10 \mathrm{MPa} . j$
I

## 1.3

Boundary conditions and loadings
On side $A B: u z=0$
One varies uniformly on the structure:
$\cdot$ the temperature of $T=20^{\circ} \mathrm{C}$ at initial time until $t=120^{\circ} \mathrm{C}$ at final time

- water content of $100 \mathrm{l} / \mathrm{m} 3$ at initial time up to $80 \mathrm{l} / \mathrm{m} 3$ at final time
$\cdot$ the hydration varies from 0. at initial time with 1. at final time.
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## 2 <br> Reference solution

## 2.1

Method of calculation used for the reference solution
Being given the nature of the requests, the total deflection is only due to the withdrawal and to thermal dilation. Consequently, one a:

## HT

rd
Re
$=++=(T-T I-C-C I-H I$
ref.) $D$
(0
) D
D
with:

T, the temperature at time $T$
Tref, the temperature of reference
C0, water content initial (water content $\mathbf{H R}=100 \%$ ).
-
$C$, water content at time $T$
$H$, the degree of hydration at time $T$
, the dilation coefficient
, the coefficient of withdrawal of desiccation
, the endogenous coefficient of withdrawal
The elastic strain being null in this problem, the constraints are null, like the damage in the case of modelings with the law of MAZARS and ENDO_ISOT_BETON.

## 2.2 <br> Results of reference

One checks the value of the deformation after 3600 days, as well as the constraint. One also checks that the plastic deformation is null, as well as the damage for modelings concerned. The results are tested with STAT_NON_LINE like with MECA_STATIQUE (for modelings $A$ and $B$ )

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## Code_Aster ${ }^{\circledR}$

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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling

## Y

D3

## N2

N5

# The loading and the boundary conditions are modelled by: 

# $F A C E \_I M P O=-F\left(G R O U P_{-} M A=D 1, D Y=0.\right)$ 

TEMP_CALCULEE=TEMP1
SECH_CALCULEE=SECH1
HYDR_CALCULEE=HYDR1

## 3.2

Characteristics of the grid
A number of nodes:
21
A number of meshs and types: 4 QUAD8

### 3.3 Functionalities

tested
Orders

DEFI_MATERIAU ELAS_FO
ALPHA

## K_DESSIC

## B_ENDOGE

AFFE_MATERIAU AFFE<br>SECH_REF<br>AFFE_CHAR_MECA FACE_IMPO

TEMP_CALCULEE<br>SECH_CALCULEE<br>HYDR_CALCULEE<br>STAT_NON_LINE COMP_INCR<br>RELATION ELAS<br>CALC_ELEM OPTION "EPSP_ELNO"<br>MECA_STATIQUE

CALC_ELEM OPTION "SIGM_ELNO_DEPL"

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Author (S):

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## 3.4 <br> Results of modeling $A$

For calculation with STAT_NON_LINE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIEF_NOEU_ELGA are null as well as the plastic deformation (EPSP_NOEU).
For calculation with MECA_STATIQUE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIGM_NOEU_DEPL are null.

## Calculation STAT_NON_LINE

## Variables Moment Reference

Aster \%
difference
xx
36006.53

10-4 6.53
10-4 8.30
10-14
36006.53

10-4
6.53 10-4 -4.98

10-14
yy
$p$
$x x$
36000.

## Calculation MECA_STATIQUE

Variables Moment Reference<br>Aster \%<br>difference<br>$x x$<br>36006.53<br>10-4 6.53<br>10-4 1.83<br>10-13<br>36006.53<br>10-4 6.53<br>10-4 -3.15<br>10-13<br>yy<br>$x x$<br>36000.<br>1.66<br>10-9 -<br>36000.<br>-8.04<br>10-8 -

yy

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## 4 Modeling <br> B <br> 4.1 <br> Characteristics of modeling

face
C yz
face
C xz
Z
y
$F$
F it
Cxy
$X$

The loading and the boundary conditions are modelled by:
$F A C E \_I M P O=\left(\_F\left(G R O U P \_M A=" F A C E X Y ", D Z=0.\right)\right.$,
F (GROUP_MA

## 4.2 <br> Characteristics of the grid

A number of nodes:
20
A number of meshs and types: 1 HEXA20

### 4.3 Functionalities

tested

## Orders

DEFI_MATERIAU ELAS_FO
ALPHA

## K_DESSIC

B_ENDOGE

AFFE_MATERIAU AFFE
SECH_REF
AFFE_CHAR_MECA FACE_IMPO

TEMP_CALCULEE<br>SECH_CALCULEE<br>HYDR_CALCULEE<br>STAT_NON_LINE COMP_INCR

## MECA_STATIQUE

## CALC_ELEM OPTION "SIGM_ELNO_DEPL"

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## 4.4 <br> Results of modeling B

For calculation with STAT_NON_LINE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIEF_NOEU_ELGA are null as well as the plastic deformation (EPSP_NOEU).
For calculation with MECA_STATIQUE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIGM_NOEU_DEPL are null.

## Calculation STAT_NON_LINE

Variables Moment Reference<br>Aster \%<br>difference<br>$x x$<br>36006.53

## Calculation MECA_STATIQUE

## Variables Moment Reference

Aster \%
difference
$x x$
36006.53

10-4 6.53
10-4-6.64
10-14
36006.53

10-4 6.53

## Code_Aster ${ }^{\circledR}$

Version
8.1

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## 5 Modeling

C

## 5.1

## Characteristics of modeling

Y
D3
N2
N5
N7

```
N3
N6
N4
N8
N1
N17
D4
N15
D2
N10 N11
N16
N9
N14
N21
N13 N12
N2O
N19 N18
X
D1
```

The loading and the boundary conditions are modelled by:
$F A C E \_I M P O=\_F\left(G R O U P \_M A=D 1, D Y=0.\right)$
TEMP_CALCULEE=TEMPI
SECH_CALCULEE=SECH1
HYDR_CALCULEE=HYDR1

## 5.2 <br> Characteristics of the grid

A number of nodes:
21
A number of meshs and types: 4 QUAD8

### 5.3 Functionalities

tested
Orders

DEFI_MATERIAU ELAS_FO
ALPHA

## K_DESSIC

B_ENDOGE<br>MAZARS_FO

AFFE_MATERIAU AFFE<br>SECH_REF<br>AFFE_CHAR_MECA FACE_IMPO

TEMP_CALCULEE
SECH_CALCULEE
HYDR_CALCULEE
STAT_NON_LINE COMP_INCR
RELATION MAZARS
CALC_ELEM OPTION "EPSP_ELNO"
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Author (S):

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## 5.4 <br> Results of modeling $C$

For calculation with STAT_NON_LINE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIEF_NOEU_ELGA are
null as well as the plastic deformation (EPSP_NOEU) and the variable of damage (VARI_NOEU_ELGA, V1).

Variables Moment Reference<br>Aster \%<br>difference<br>xx<br>36006.53<br>10-4 6.53<br>10-4 -5.65<br>10-5<br>36006.53<br>10-4 6.53<br>10-4-3.58<br>10-5<br>yy<br>p<br>$x x$<br>36000.<br>0.619<br>p<br>36000.<br>0.032<br>yy<br>$x x$<br>36000.<br>-3.91<br>10-10 -<br>36000.<br>-2.05<br>10-10 -<br>yy<br>D<br>36000.<br>0.0

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## 6 Modeling <br> D

6.1

Characteristics of modeling
face
C yz
face
C xz
Z
y
$F$
F it
C xy
$X$

The loading and the boundary conditions are modelled by:
$F A C E \_I M P O=\left(\_F\left(G R O U P \_M A=" F A C E X Y ", D Z=0.\right)\right.$, _F (GROUP_MA
=
"FACEXZ",

```
DY=
0.),
F (GROUP_MA
=
"FACEYZ",
DX=
0.))
TEMP CALCULEE=TEMP1
SECH_CALCULEE=SECH1
HYDR_CALCULEE=HYDR1
```


## 6.2 <br> Characteristics of the grid

A number of nodes:
20
A number of meshs and types: 1 HEXA20

6.3 Functionalities<br>tested<br>Orders<br>DEFI_MATERIAU ELAS_FO<br>ALPHA<br>K_DESSIC<br>B_ENDOGE<br>MAZARS_FO

## NON_LOCAL

## AFFE_MODELE

AFFE_MATERIAU AFFE
SECH_REF

## Code_Aster ${ }^{\circledR}$

Version
8.1

Titrate:
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Date:
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Author (S):

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## 6.4 <br> Results of modeling $D$

For calculation with STAT_NON_LINE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIEF_NOEU_ELGA are null as well as the plastic deformation (EPSP_NOEU) and the variable of damage VARI_NOEU_ELGA, V1.

## Variables Moment Reference <br> Aster \% <br> difference <br> xx <br> 36006.53 <br> 10-4 6.53 <br> 10-4 1.16

## Titrate:

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## Author (S):

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## 7 Modeling <br> E <br> 7.1 <br> Characteristics of modeling

```
Y
```

D3
N2
N5
N7
N3
N6
N4
N8
N1
N17
D4
N15
D2
N10 N11
N16
N9
N14
N21
N13 N12
N20
N19 N18
X
D1

The loading and the boundary conditions are modelled by:
$F A C E \_I M P O=\_F\left(G R O U P \_M A=D 1, D Y=0.\right)$
TEMP_CALCULEE=TEMP1
$S E C H \_C A L C U L E E=S E C H 1$

## 7.2 <br> Characteristics of the grid

A number of nodes:
21
A number of meshs and types: 4 QUAD8

7.3 Functionalities<br>tested<br>Orders<br>DEFI_MATERIAU ELAS_FO<br>ALPHA<br>K_DESSIC<br>B_ENDOGE<br>BETON_ECR_LINE

AFFE_MATERIAU AFFE<br>SECH_REF<br>AFFE_CHAR_MECA FACE_IMPO<br>TEMP_CALCULEE<br>SECH_CALCULEE<br>HYDR_CALCULEE<br>STAT_NON_LINE COMP_INCR<br>RELATION<br>ENDO_ISOT_BETON<br>CALC_ELEM OPTION "EPSP_ELNO"

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## Code_Aster ${ }^{\circledR}$

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Titrate:
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Author (S):
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## 7.4 <br> Results of modeling E

For calculation with STAT_NON_LINE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIEF_NOEU_ELGA are null as well as the plastic deformation (EPSP_NOEU) and the variable of damage (VARI_NOEU_ELGA, V1).

## Variables Moment Reference <br> Aster \% difference <br> $x x$ <br> 36006.53 <br> 10-4 6.53 <br> 10-4 1.33 <br> 10-13 <br> 36006.53 <br> 10-4 6.53 <br> 10-4 4.98 <br> 10-14 <br> yy <br> $p$ <br> $x x$ <br> 36000. <br> 2.30 <br> 10-8

## $p$

36000. 

-3.30
10-10
$y y$
$x x$
36000.
-4.49
10-19 -
36000.
-1.78
10-19 -
yy
D
36000.
0.0

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Author (S):
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## 8 Modeling

F
8.1

## Characteristics of modeling

```
Y
D3
N2
N5
N7
N3
N6
N4
N8
N1
N17
D4
N15
D2
N10 N11
N16
N9
N14
N21
N13 N12
N2O
N19 N18
X
D1
```

The loading and the boundary conditions are modelled by:
$F A C E \_I M P O=\_F\left(G R O U P \_M A=D 1, D Y=0.\right)$
TEMP_CALCULEE=TEMP1
SECH_CALCULEE=SECH1
HYDR_CALCULEE=HYDR1

## 8.2 <br> Characteristics of the grid

A number of nodes:
21
A number of meshs and types: 4 QUAD8

### 8.3 Functionalities

## DEFI_MATERIAU ELAS_FO

K_DESSIC<br>B_ENDOGE<br>BETON_ECRO_LINE<br>\section*{BETON_UMLV_FP}<br>AFFE_MATERIAU AFFE<br>SECH_REF<br>AFFE_CHAR_MECA FACE_IMPO

TEMP_CALCULEE<br>SECH_CALCULEE<br>HYDR_CALCULEE<br>STAT_NON_LINE COMP_INCR<br>\section*{RELATION KIT_DDI}<br>CALC_ELEM OPTION "EPSP_ELNO"<br>Handbook of Validation<br>V3.06 booklet: Linear statics of the axisymmetric structures<br>HT-66/05/005/A

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Author (S):

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## 8.4 <br> Results of modeling $\boldsymbol{F}$

For calculation with STAT_NON_LINE, one tests the components of the tensor of the deformations EPSI_NOEU_DEPL after 3600 days. It is also checked that constraints SIEF_NOEU_ELGA are null as well as the plastic deformation (EPSP_NOEU) and the variable of damage (VARI_NOEU_ELGA, V1).

## Variables Moment Reference <br> Aster \% <br> difference <br> $x x$ <br> 36006.53 <br> 10-4 6.53 <br> 10-4 1.06 <br> 10-12 <br> 36006.53 <br> 10-4 6.53 <br> 10-4 8.97 <br> 10-13 <br> yy <br> $p$ <br> $x x$ <br> 36000.

## 9 <br> Summary of the results

The results obtained with Code_Aster are identical to the analytical solution. It was thus validated calculation of thermal dilation and the endogenous withdrawal and desiccation for the elastic model, that it is with STAT_NON_LINE or MECA_STATIQUE, like for the law of Mazars, local version or not-local, for law ENDO_ISOT_BETON and the case of the coupling BETON_UMLV_FP/ENDO_ISOT_BETON. Let us announce that modelings A and B also allow to validate the calculation of the withdrawals for laws VMIS_ISOT_TRAC and VMIS_ISOT_LINE which uses
even routine that ELAS.
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## Code_Aster ${ }^{\circledR}$

Version
8.2

Titrate:
DEMO005 - Thin cylinder under hydrostatic pressure

Date:
01/09/05
Author (S):
J. LAVERNE, F. LEBOUVIER Key

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Organization (S): EDF-R \& D /AMA, DeltaCAD

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DEMO005 - Thin cylinder under pressure hydrostatic

## Summary:

Case-test DEMO005 is a very simple example of use of Code_Aster. It makes it possible to illustrate them orders "impossible to circumvent" on the calculation of a tank.

The thin cylindrical tank is subjected to a variable pressure with the height corresponding to one hydrostatic pressure.

Since the geometry and the loading are axisymmetric, an axisymmetric modeling 2D is chosen to model the problem, the grid representing a vertical section of the cylinder.

One calculates displacements and the constraints in the vertical section of the cylinder. Handbook of Validation
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Titrate:
DEMO005-Thin cylinder under hydrostatic pressure

## Date:

01/09/05

## Author (S):

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1
Problem of reference

### 1.1 Geometry

$R=1 m$
${ }^{\prime}$
$X$
$P$
Z
$L=4 m$
Appear 1.1-a: Geometrical characteristics and loadings
Selected geometrical dimensions are as follows:

## height <br> $L=4 m$

## 1.2

Properties of material
The cylinder consists of a homogeneous material (steel) which follows an elastic law of behavior linear:

Young modulus:

$$
E=2.11011 \mathrm{~Pa}
$$

Poisson's ratio:

$$
=0.3
$$

## 1.3 <br> Boundary conditions and loadings

The pressure $P$ is applied to the internal wall of the cylinder. This pressure varies linearly: it $20.000 \mathrm{~N} / \mathrm{m}^{2}$ at the base of the cylinder and 0 in the high part is worth.

Following displacements at the base of the cylinder are null there, the cylinder can move radially.

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## 2 <br> Reference solution

## 2.1 <br> Method of calculation used for the reference solution

The reference solution is obtained numerically. It is thus only about one test of not regression.

## 2.2

Results of modeling
Urgent displacements Reference
DX in A
1
4.6814 10-6

DX out of B
1
4.6528 10-6

DX out of C
1
1.2022 10-6

DY out of C
1
-2.6396 10-6
DX in D
1
3.3102 10-9

DY in D
1
-2.8170 10-6

Displacements are expressed in meters.

## 2.3

Uncertainty on the solution

## Reference solution: not regression

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

The cylinder is modelled in elements axisymmetric 2D.

```
y
D
C
ldf
Discretization:
```

- 5 meshs according to $X$
- 20 meshs following y
charg
lfa
B
In support
$X$
Appear 3.1-a: Longitudinal section of the cylinder


## 3.2

Characteristics of the grid
The longitudinal section of the cylinder is represented by a regular grid containing:

136 elements (6 POI1, 30 SEG2, 100 QUAD4).

## 126 nodes.

Creations of groups of meshs for the application of the loading and the limiting conditions:

- support: to block vertical displacement
- ldf, lfa: to apply a variable pressure to the part cylinder interns.

Creations of groups of nodes:
$\cdot A, B, C, D:$ used to note displacements in these characteristic nodes.
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## 3.3

Command file Aster

BEGINNING: obligatory order to start

Reading of grid (PRE_GMSH) and generation of grid (LIRE_MAILLAGE).

Definition of groups of nodes and meshs for the assignment of the loading and of limiting conditions (DEFI_GROUP).

Definition of the finite elements used (AFFE_MODELE). One assigns to all the meshs grid modeling AXIS.

Definition and assignment of material (DEFI_MATERIAU and AFFE_MATERIAU).
The mechanical characteristics are identical on all the structure.

- Affectation of the loading: Definition of a function representing the pressure $P$ on the face intern of cylinder (DEFI_FONCTION), then assignment of the pressure on the internal edges cylinder represented by the groups of meshs LDF and LFA (AFFE_CHAR_MECA_F).
- Affectation of the boundary conditions (AFFE_CHAR_MECA).

Displacements following there are blocked at the base of the cylinder on the level of the group of meshs SUPPORT.

Resolution of the linear elastic problem (MECA_STATIQUE).
Calculation of displacements and the constraints at the points of Gauss of each element to be left displacements.

Calculation of the constraints to the nodes.
One calculates the stress field to the nodes of each element from displacements (CALC_ELEM).

Impression of results (IMPR_RESU).
One prints in form listing the displacement and the constraints of all the grid. One also prints the field of displacement to format GMSH, for a visualization results with GMSH.

Tests relating to the values of displacements to nodes A, B, C, D. (TEST_RESU)
These tests make it possible to check nonthe regression of the code.

END: obligatory order to close an execution.

### 3.4 Notice

For modeling "AXIS", the axis of revolution is always axis $Y$.
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## 4

Results of modeling $A$

### 4.1 Values

tested
Identification unit Reference Aster \%
difference
DX in $A$
m
4.6814 10-6 4.6814

10-6 0.
DX out of B
m
4.6528 10-6 4.6528

10-6 0.
DX out of C
m
1.2022 10-6 1.2022
m
-2.6396 10-6 -2.6396
10-6 0.

## DX in D

m
3.3102 10-9 3.3102

10-9 0.
DY in D
m
-2.8170 10-6 -2.8170
10-6 0.

5
Summary of the results
Following displacements $X$ are relatively important at the base of the cylinder, where the loading is highest. One can note that following displacements there of the high part of the cylinder are negative, which was foreseeable with the sight of the problem.

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Calculation of an elastic hyperstatic plane gantry
Date:
01/09/99
Author (S):
F. VOLDOIRE

Key:
V3.90.001-A Page:

Organization (S): EDF/IMA/MMN
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V3.90 booklet: Theoretical references of tests in linear statics
Document: V3.90.001
Calculation of an elastic hyperstatic plane gantry
Summary:
The goal of this note is to expose the method of calculation used to determine the reference solution of case-test SSLL 14, entitled: "Plane Gantry articulated in foot".
One uses the method of the forces (hyperstaticity 1), by taking account only of the energy of inflection: assumption
slim beams.
One considers four loading cases separately.
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1 ..... 4
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1.2 Requests ..... 5
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72.1 Reactions of support
$\qquad$
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$m$ M
8.5 Calculation of $D=$
1.22
I.E.(internal excitation)
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y
C
One considers the gantry opposite,
p
subjected to various loads.
has
C
C
1
2
Hyperstaticity of degree 1.
F
Hyperstatic unknown factor: $X$ :
1
F
2
moment out of $C$.
Top-load distributed $p$ on
H
C1 C.
Two forces F1, F2, and a couple
With
in $C 1$.
B
$X$

```
tg
=2a}=0,4\operatorname{cos
() - 1 = 1,16=1,077033
(
)
C
tg
"
=
2 A
(+ H)
1,2
GOES
VB
B
=
"
With
B
2cos
; sin
= B has
H
H
With
B
1
Isostatic requests under real load distributed p on
C C
1
1.1
Isostatic reactions of supports
p "
p}
"
H+H
```

```
=0;V+V
=
;
"V
=
With
B
With
B
B
2 cos
8 cos
Part CB is articulated and charged only at its ends:
H
B
BC=0
H=-V
```

$\operatorname{tg}$
$V$
$B$
$B$
$B$

From where isostatic reactions:
p"
$3 p^{\prime \prime}$
$p^{\prime \prime}$
$p^{\prime \prime}$
$H$
$=$
$\operatorname{tg} ; V$
$=$
;
H
= -
$\operatorname{tg} ; V$
$=$
With
With
B
B
"tg
"
=
B
$8 \cos$
(
$8 A+H)$

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### 1.2 Requests

## Beam

In C1
M
$N R$
$3 p^{\prime \prime}$
NR
= -
Iso
$y$
$8 \cos$
"
V
p
V
=
With
Viso
tg
$8 \cos$
0
$p^{\prime \prime}$
H
M
$=-$
$y$ tg
With
Iso
$8 \cos$
Beam C B
2
M
NR
$p^{\prime \prime}$
NR
= -
$+$
Iso
$8 \cos$
$y$
"
V
p
V
$=-$
tg

```
V
Iso
B
8 cos
0
p"
H
=
B
M
y
Iso
tg
8 cos
Beam C C
M
px
= -
cos-
sin}
V
NR
H
V
Iso
With
With
sin
cos
p
"
X
NR
= -
tg}+3\operatorname{tg}-8\operatorname{tg
```

| file://IZ//process/valid/p2150 |
| :---: |
| + |
| $p$ |
| $y$ |
| 8 |
| " |
| $p x$ |
| $V$ |
| $=H \sin -V \cos +$ |
| Iso |
| With |
| With |
| $\cos$ |
| cos |
| $p^{\prime \prime}$ |
| $=$ |
| $\left(\operatorname{tg} \operatorname{tg}-3+8 \mathrm{X} /{ }^{\prime \prime}\right)$ |
| GOES |
| 8 |
| 2 |
| H |
| $p x$ |
| With |
| $M$ |
| $=-$ |
| + VX-Hy |
| Iso |
| With |
| With |
| $2 \cos$ |
| X |

```
p
```

$x 23$ " $X$ " $y$ tg
(= 0 in $C$ )!

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Beam C C2
NR
NR
$=H \cos -V$
Iso
B
$B \sin$
$p$
= -
$(\operatorname{tg}+\operatorname{tg})$
$y$
$M$

8
+
$V$
$V$
$=$
Iso
$B$
$B$
$B$
co
$p^{\prime}$

```
= -
(tg .tg-) 1
8
M
=Hy+V("- X)
V
Iso
B
B
B
p
H
= -
(y\operatorname{tg}-("-X))(= 0 C
in
)!
B
8
X
cos
```


### 1.3 Diagrams

$B=$
$2 \cos$
+
+
$p$
$"$
-
$-H \operatorname{tg}$

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2
Requests under concentrated force $F 1$ (downwards)
2.1 Reactions
of support
F1
$H+H$
=
With
B
0 ;
C
$V+V$
$=F$
With
B
1
;
C
1
2
H
H
With
B
$\mathrm{AC}=\mathbf{0}=$

BC
V
V
With
$B$
V
V

With

```
B
With
B
H
H
With
B
```

From where:
1
1
1
1
H
=
$F \operatorname{tg}$
; $V$
1
=
F
;
H
1
$=-F \operatorname{tg}$
;
V
With
B
B
2
2
2
21
2.2 Requests
Beam
In Cl:
1
NR
=-F
Iso
1
2
1
tg
$V$
$=$
$F$
Iso
21
1
$M$
$=-F y \operatorname{tg}$
Iso

21
Beam C B
2
:

1
$N R$
$=-F$
Iso
1

2

1
$\operatorname{tg}$
V
$=-F$

Iso

21

## 2 <br> Beam C C2:

```
1
NR
```

$=-F$
1
$+$
Iso
( $\mathrm{tg} \cos \sin$ )
2
1
V
$=-F$
1
-
Iso
( $\mathrm{tg} \sin \cos$ )
2
1
M
$=-F$
11
1

-     - 

Iso
( $y \operatorname{tg}(X)$ )

```
2
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```

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### 2.3 Diagrams

(Fl downwards)
F1
$-H$ tg
2
$-F 1 h \mathrm{tg}$
$=-F l h$
1
2
$4(a+h)$

- F1/2
- F1/2

NR
V
M
Iso
Iso
Iso

## 3

Requests under the concentrated force $F 2$ (towards the left)

### 3.1 Reactions

of support

- $H a+H B=F 2$;

C
C
$+V B$ GOES $=0$;
1
C
" $V$
2
$B+H F 2$

```
=0
2
HB
```

```
BC=
V
V
0
With
V
B
B
With
B
H
H
With
B
```

From where:
H
H
H
H
H
$=F$
11 -
tg
;
V
;
$H$
F
tg
;
With
With
2
$B$
1
$B$
$=$
$=$
$=-2$

## Note:

$H$
$H$

## $H$

$2 \mathrm{a}+H$
=
1 -
$" \operatorname{tg}$
$($
$;$
$\operatorname{tg}$
$2 A+H)$
$=$
$($
$2 A+H)$
2
2
$H^{\prime \prime}$
$"-($
$4 A+a h)$
$\operatorname{tg} \sin -\cos$
$=-2$ (

$$
\begin{aligned}
& \bar{B} \text { has }+H)
\end{aligned}
$$

$\operatorname{tg} \cos \sin$ ..... 4 (

B has + H)

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3.2 Requests

Beam A C1:

NR
$=-F H /{ }^{\prime}$
Iso

2
$H$
V
$=F$
21 -
tg
Iso

```
H
M
=-Fy
2
1-
tg
Iso
```


## Beam C B

2
NR
= F H/"
Iso
2
H
V
$=F$
tg
2 "H
M
$=-F$
$y \operatorname{tg}$
Iso

$$
=F
$$

```
I'
H
H
M
= F
X
2
-1-tg
y
Iso
```


## Beam C C2:

```
H
NR
\(=F\)
2
+
Iso
\((\operatorname{tg} \cos \sin )\)
```

V
= F

```
2
Iso
( \(\mathrm{tg} \sin \cos\) )
"H
M
\(=F\)
2
Iso
\((y \operatorname{tg}(X))\)

\subsection*{3.3 Diagrams}
(
\(-H 2 \mathrm{a}+H\)
F2
\(+\)
2 A
\((+H)\)
- H
H2
F
F
2
2
2 (+ H has)
M
NR
Iso
Iso
Viso

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4

\section*{Requests under the concentrated couple (positive)}

\subsection*{4.1 Reactions}
of support
- \(H A+H B=0\);

V
C
WITH \(+V B\)
\[
=0 \text {; }
\]
\[
\begin{aligned}
& \text { " } \\
& C \\
& V \\
& 2 \\
& D
\end{aligned}
\]
\[
B+
\]
\[
=0
\]
\[
H
\]

\section*{B}
\(\mathbf{B C}=\)
0
V
V
V
With
```

B
B
With
B
H
H
With
B

```
From where:
H
\(=-\operatorname{tg} " ; V\)
\(="\)
With
With
H
\(=\operatorname{tg}{ }^{`} ; V\)
\(=-"\)
B
B
Note:
tg
1
"
(
\(2 A+H)\)
4.2 Requests
Beam A C1:
NR
\(=-"\)
Iso
V
\(=-\operatorname{tg} \times\)
Iso
M
=
\(y\)
\(\operatorname{tg}\) "
Iso
Beam C B
2
```

:
= "
Iso
V
= tg`
Iso
M
=
y
tg
Iso
Beam C C
1
:
NR
=
tg cos-
Iso
sin)
V
tg sin}
Iso
cos)
M
=
+tg-

```

\section*{Beam C C2:}
\(N R\)
```

tg cos+

```
Iso
(
\(\sin )\)
```

V
=
tg sin
Iso
(
cos)
"
M
=
tg -"-
Iso
(y
(X))

```

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\subsection*{4.3 Diagrams}
(positive)
(
\(-H+2 \mathrm{a}\)
2 (+ H has)
H
H
2 A
(+h)
2 (+H has)
- tg
- 1
/
"
"
"

NR
M
Iso
Viso
Iso
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5
Requests under moment \(X\) hyperstatic
5.1 Reactions
of support
\(X\)
\(X\)
\(+\)
\(H+H\)
\(=\)

With
B
0 ;
C
\(V+V\)
\(=\)
C
C2
With
B
0 ;
1
"V
\(=\)
B
0 ;
V
V
\(H\) (+ H has) \(-X\)
=
With
B
B
0
With
```

B
H
HB

```
With

From where reactions:
\(X\)
X
H
\[
=-
\]
;

V
\(=0\)
; H
=
;
V
\(=\)
With
0
+ H has
With
B
+ H has
B

\subsection*{5.2 Requests}

\section*{Beam A Cl:}

NR
=
\(X\)
0
\(X\)
V
=
+ H has
\(X\)
\(M\)
\(=\)
\(y\)
\(X\)
+ H has
Beam C B
2

NR

X
0
\(X\)
\(V\)
\(=\)
\(X\)
+ H has
\(X\)
\(M\)
\(=\)
\(y\)
\(X\)
+ H has
Beam C C
```

1

```
:
\(X\)
\(N R\)
cos
\(X\)
+ H has
\(X\)
V
= -
\(\sin\)
X
+ H has
```

X
X
M
=
y
=
+
X
(H X tg)

```
+ H has
\(+H\) has
Beam C C2:

\section*{X}
\(N R\)
=
cos
X
+ H has
\(X\)
\(V\)
\(=\)
\(\sin\)
X
+ H has
\(X\)
\(M\)
+ H has
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5.3 Diagrams

X
\(X\)
\(X \sin\)
\(X \cos\)
+ H has
\(+\)
+ H has
X H
X H
has
has
\(+H\)
\(+H\)
O
O
X
+ H has
NR
V
M
\(X\)
\(X\)
\(X\)
6
Requests under specific dummy loads out of \(C\)
In order to calculate displacement out of \(C\), using the Principle of Virtual work (cf.
paragraph [§8]), it is necessary to establish the diagrams of requests under the action of two "fictitious" forces \(F\) and \(G\) applied out of \(C\).

\subsection*{6.1 Reactions}
\(H+H\)
\(=-F\)
G
With
B
;
\(V+V\)
\(=-G\)
F
With
B
;
H
C
With
B
\(\mathbf{A C}=\mathbf{0}=\)
\(\mathbf{B C}\)
\(C\)
\(C\)
\(V\)
\(V\)
2
With
\(B\)
1
\(V\)
\(V\)

With
\(B\)
With
\(B\)
\(H\)
\(H\)
With
\(B\)

From where:
1
```

1
H
= -
+
;
= -

+ cot G
With
(FG tg)
GOES
(GF
2
2
1
1
H
= -
;
=
- cot G
B
(FG tg)
VB
(GF
)
2
2
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```

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6.2 Requests

Beam A Cl:
1
\(N\)
\(=\)
\((g+F \operatorname{cotg})\)
2
1
\(v=-(F+G \operatorname{tg})\)

2

1
m
=
\((F+\mathrm{g} \operatorname{tg}) y\)

2
Beam C B
2

1
\(N\)
\(=\)
(GFcotg)
\[
v=-(F-G \operatorname{tg})
\]
Beam C C
\((F+g \operatorname{tg}) \cos +(g+F \operatorname{cotg}) \sin\)
2
2
1
1
\(v=-(F+G \operatorname{tg}) \sin +(G+F \cot \mathrm{G}) \cos\)
2
2
1
1
m
\(=+(F+G \operatorname{tg}) y-(G+F \cot \mathrm{G}) X\)
2
2
Beam C C2:
```

1
N
= -
(F-Gtg) cos + (GF\operatorname{cotg})\operatorname{sin}
2
2
1
1
v=-(F-G tg) \operatorname{sin}-(G-F\operatorname{cot}\textrm{G})\operatorname{cos}
2
2
1
1
m
= -(F-G\operatorname{tg})y-(G-F\operatorname{cotG})("- X)

```

\section*{2}

2

\subsection*{6.3 Diagrams}

Here diagrams of requests under the action of the two "fictitious" forces \(F\) and \(G\). One considers here: \(F 0, G F\)
cotg.
G
\(+\)
F
- fh /2
fh
gh "
2
\(+\)
"
4
has
(+ H)
\(+g h\)
4 A
\((+H)\)
```

N
v
m
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```

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7

\section*{Determination of moment \(X\) hyperstatic}

One places oneself in elasticity; one considers only the energy of inflection, the beams being slim. The state
naturalness is supposed to be virgin (not prestressings nor of displacement of support).
The complementary potential is then:
(
2
2
M
+ M1 X
\(+1\)
Iso
)
(MR. MR. X
Iso
)
\(F^{*}(X)=\)
\(+\)
I.E.(internal excitation)
I.E.(internal excitation)

It is stationary with balance, from where:

\section*{M1 Miso}

M1
\(M\)
\(X\)
\(=\)
+
\(X\)
\(I s\)
\(S\)
I.E.(internal excitation)
pot
1
I.E.(internal excitation)
charp
2
I.E.(internal excitation)
pot
1
```

=
I.E.(internal excitation)

```
charp
2

The coefficient of flexibility is the sum of:

\section*{M2}
\(2 H H\)
2
I.E.(internal excitation)

3 I.E.(internal excitation)
has
H
```

=
+
+
2
(has
+H)
frames

```
that is to say:
3
2
2
2
H
(Bh3 \(+\mathrm{a}+a 3 h\) )
E
=
\(+\)
+ H) 2 has 3I
3I
1
2

\section*{Numerical application:}

In the example considered:
\(I=2 I\)
4
\(=50\)
, 10
m4
1
2

\section*{\(H=2 A=8 m ; "=20 m ; B=\)}

\section*{\(m\)}

One studies one after the other the various loadings to calculate the second members \(S\).
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\subsection*{7.1 Charge}

\section*{distributed}
\(p\) on C C

\section*{1}

The second member \(S\) due to \(p\) is:
MR. M
\(2 H\)
H
3
"
"
1
Iso
Pb H

2
pH B
```

=
I.E.(internal excitation)

```
3 I.E.(internal excitation)
has
2
1
1
\(+H(\)
\(8 A+H)\)
= \(E\)
24
posts
(+ H has) II
MR. M
pb2h
2
"
11
1
Iso
1 H
```

=
I.E.(internal excitation)

```
8 A
2
2
+ H) I.E.(internal excitation) 2
has
2
\(+H+6\)
\(+H\) has
\(=E\)
48
C C
(+ H has) I
2
2
"
\(+\)
MR. M
1
\(p\)
1
Iso
B
has
H
has
```

22
S
S (2a 3h)bh
H
S
ds
=
I.E.(internal excitation)
I.E.(internal excitation)
(
8A+H)20

```
\(B\) -
B
C C
2
2
\(+\)
1
2
\(p^{\prime \prime} B\)
2
2
\(=\)
\(H-a h-h a s\)
2
(2
)
\(E(+H\) has \() I\)
48

From where:
3
2
2
2
2
p"B4h
(
hb H
\(3+\) has)
( \(b h-a h-a\) )
\(S\)
=
\(+\)
\(+\)
\(E(+H\) has \() 2\)
\(96 I\)
I
I
1
2
2

\section*{Numerical application:}
\(I=2 \mathrm{I}\)
\(H=2 A\);
\(p=3.000 \mathrm{NR} / \mathrm{m}\)
1
2
(downwards)
2
p" bh2
13
\(S\)
=
4h +
B

NR m4
From where:
- moment out of \(C\) :

X
= 18.672.994

\section*{NR m}
- reaction in \(a\) :

B"
X
Pb" 8 - X
H
= \(p\)
\(=\)
With
\(8 A+H)\)
\(+H\) has
(+ H has)
H
=
With
5.17537

NR
3pb
V
\(=\)

\section*{With}

0
4
V
=
With

\subsection*{24.23324}

\section*{NR}

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7.2 Charge
specific
F1 out of \(C\)
The second member is obtained using:
MR. M
\(2 H\)
H
3

11
11
1
Iso
F H
1

2
FH
```

1
=
I.E.(internal excitation)

```
3 I.E.(internal excitation)
has
2
1
1
\(+H 4(+H\) has \()=E\)
12
posts
(+ H has) I1
MR. M
2b
F"
1
Iso
\(1 H\)
1
H
1
has
```

=
I.E.(internal excitation)
I.E.(internal excitation)
4 (has
2
2
+H)2

+ H has + 6

```
\(+H\) has
frames
2
FB" (
H H
1
\(3+\) has)
\(=E\) (
+ H) 2 has I
24
2
From where:
2
F"H 2h2
(
B H
3
1
+ has)
\(S\)
\(=\)
\(E\) (+ H has) 2
24
I
I

1
2

Numerical application:
\(I=2 \mathrm{I}\)
\(H=2 A ; F=20.000 N R\)
1
2
1
(downwards)
\(F\) "H2

From where
- moment out of \(C\) :

X
= 41.422.161

\section*{NR m}
- reaction in \(a\) :

1
\("\)
\(X\)
\(F\) "4
1
- X
\(H\)
+ H has
+ H has
(+ H has)
```

H
=
With
4.8814866

```
NR
1
V
\(=\)
F1-
With
0
2
V
\(=\)
With
10.0000
. NR
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7.3

Concentrated loading F2 in C1
The second member is obtained using:
MR. M
H
3
1
Iso
H
```

F
(
H 2a
2
+H)
2
FH(2a
2
+H)
=
=
I.E.(internal excitation)

```
3 I.E.(internal excitation)
has
2
1
1
\(+H 2(+H\) has \()\)
E
12
    WITH C
    (+ H has) II
1
2
MR. M
H
4
1
H
(- F H
2
2
```

FH
Iso
2
=
I.E.(internal excitation)

```
3 I.E.(internal excitation)
has
2
1
1
\(+H\)
\(2 A+H\) )
E
12
C B
(+ H has) I
2
1
MR. M
B
F H2
1
Iso
2
\((a+H) 1 H\)
1
has
```

=
I.E.(internal excitation)
I.E.(internal excitation)

```
\(2 A\)
2
2
\(+H)\)
2
6
\(C C\)
+ H has +
+ H has
1
2
\(F b h\)
2
2
2
\((h 3+7 \mathrm{ah}+2 \mathrm{a})\)
\(=E(\)
+ H) 2 has I
24
2
\(M R . M\)
\(B\)
2
2
1
\(I s o\)
\(-F H\)
2
1 H
1 A
2
\(-F b h(H\)
2
\(3+h a s)\)
2
\(=\)

2 A
2
2
2
+H) 2

6
24
C C
\(+H\) has +
\(+H\) has \(=E(+H\) has \() I 2\)
2
From where:
2
F ha 2h2
B
\(S\)
\(=\)
2
\(+\)
(H
\(3+h a s)\)
\(E(+H\) has \() 2\)
\(12 I\)
I
1
2
Numerical application:
\(I=2 \mathrm{I}\)
;
\(H=2 A ; F=10.000 N R\)
1
2
2
(towards the left)

2
F H2 has
\(S\)
=
2
(2h+7b)
E(+H has) \(2 I\)
12
1

\subsection*{19.497.025 55 NR m}

4
From where:
- moment out of \(C\) :

X
= 8.2844321

\section*{NR m}
- reaction in \(a\) :
\(2 \mathrm{a}+\mathrm{H}\)
\(X\)
F (has
2
\(+H / 2)-X\)
H
\(=F 2\)
\(=\)
(
\(2 A+H\) )
\(+H\) has
(+ H has)
H
=
With
5.976.297

NR
```

V
= FH
With
2
/"
V
=
With
4.000 00
NR
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```

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\subsection*{7.4 Couples}
specific
in Cl
The second member is obtained using:
MR. M
3

1
- 2 h

H
H

3 I.E.(internal excitation)

\section*{has}

2
1
1
\(+H(\)
\(2 A+H\) )
E
6
posts
(+ H has) II
MR. M
B
(H
1
Iso
\(+2 \mathrm{a}) 1\)
H
1
has
```

=
I.E.(internal excitation)
I.E.(internal excitation)
2 A
2
2
+H)2

```
```

6
CC
+H has +
+H has
1
2
(H+2a)(H
3+has)
=
B
E(
+H)2 has I
24
2
MR.M
B
1
Iso
-H 1H
1 A
2

- hb (H
3+has)
=
I.E.(internal excitation)
I.E.(internal excitation)
2A
2
2
2
+H)}

```
\(+H\) has \(=E(+H\) has \() I\)
2
2
From where:
2
- 2h3
(
\(a b H\)
\(3+\) has)
\(S\)
\(=\)
E(+H has) 212 I
I
1
2

\section*{Numerical application:}
```

I=2I
;
H=2 A; = -100 000 NR m

```
1
2
(direction hands clock)
2
\(S\)
\(=\)
3
2
[H-has (B h3+a)]
\(E(+H\) has \() I\)
6

\subsection*{11.571.281 93}

NR m
4
From where
- moment out of \(C\) :

X
\(=4.9167243\)

NR m
- reaction in \(a\) :

\section*{- \\ X}
- 2 - \(X\)

H
\(=\)
\(=\)
With
(
\(2 A+H)\)
\(+H\) has
(+ H has)
H
=
With
4.576.394

NR

V
\(=\)
With
"
V
=
With
5.000.000

NR

\author{
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7.5 Summary

CASE
Moment out of \(C\)
Reactions of \(A\) (NR)
(N.m)

Ha
GOES
\(p\) on C C
18672.994
5175.37
24233.240

1
F1 out of \(C\)
41422.161
4881.487
10000.000
\(F\)
8284.432
5976.297
4000.000

2 in \(C 1\)
out of \(C\)
4916.724
4576.394
5000.000

1
TOTAL

\section*{Notice}

Recall: in post \(A C 1\) : normal effort \(=-\) GOES, shearing action \(=h a\).

\section*{8}

\section*{Calculation of displacement out of \(C\)}

One considers also only the elastic energy of inflection (slim beams). By applying the Principle virtual Work on the structure subjected to the fictitious forces of the paragraph [§6], working in sought displacements, one calculates the numbers \(W\) and \(D\) depending linearly on \(F\) and \(G\) :
\[
m(M+X M 1
\]

\section*{+}
Iso
)
\(m\) (M
X M
Iso
1)
\(F U+G v\)
=
W
\(X d\)
+
\(=\)
+
+
\(C\)
,
\(F, G\)
C
C
)
I.E.(internal excitation)
I.E. (internal excitation)
1
pot
charp
2

\subsection*{8.1 Charge}

\section*{distributed}
\(p\) on C C
1
\(m\) M
2h
gh "
- pbh "

2
- gpbh3 2

II
Iso
I.E.(internal excitation)

3 I.E.(internal excitation)
(
4 A
2
1
1
\(+H)(\)
\(8 A+H)\)
E
\(+\)
96
posts
(H has) II
\(m\) M
2
- p" hb2

Iso
2
2
\((F(a+h)+\mathrm{g})(h a)\)
```

=
EI2
E
+
384
C C
(H has) I
1
2
M
B
pbh "
fh
G H
2
-pb2 " H2 "-2
+
Iso
(G
F (H has))

```
```

=

```
=
I.E.(internal excitation)
I.E.(internal excitation)
I.E.(internal excitation)
I.E.(internal excitation)
3
3
8A+H)2
8A+H)2
(
(
4A+H)=
4A+H)=
E
E
2
2
+
+
1 9 2
1 9 2
C C
C C
2
```

2

```

\section*{Numerical application:}
\(I=2 \mathrm{I}\)
\(H=2 A\);
\(p=3.000 \mathrm{NR} / \mathrm{m}\)
1
2
(downwards)
2
\[
G^{\prime \prime} 2 \mathrm{~h}+B
\]

\section*{fbh}

E (+ H has) \(2 I\)

2-2

192
1

2154065922

NR m
3
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8.2 Charge
specific
Fl out of \(C\)
\(m M\)
H
gh"
- F H

32
2
"
```

1
2
-F gh
Iso
=
1
=
I.E.(internal excitation)
3 I.E.(internal excitation)
(
4A
2
1
1
+H)(
4A+H)
E
+
48
posts
(H has) II
mM
B
gh"

- F H
2
2
"
"
1
2
- Fgbh
Iso

```

```

"
W
=
1
(H+2b)
E(+H has) 2I
48

```
1
315.100.365 0
. NR m
5

\subsection*{8.3 Charge} specific
F2 in C1
\(m M\)
H
F H
11
2
fh
gh
- fh
gh
Iso
\(-2 \mathrm{a}+H\)
\(+\)
\(H\)
\(=\)
1
1
\(+H)\)
(
) 2 ( 4 a
\(+H)+2\)
(
\(4 A+H)\)
posts
2
- F
3
\(2 H\)
\(=\)
2
"2
2
\((A g+F(a+h))\)
\(E(+H\) has \() I\)
24
1
\(m M\)
2
- F bh2
Iso
2
2
"2
2
\((A g+F(a+h))\)
I.E.(internal excitation)
2
E
```

+ 

24
charp
(H has) I2
From where:
2

- F H2
H
B
W
=
2
2
2
(Ag+2f (a+h)) +
E (+H has)
24
I
I
1
Numerical application:
I=2I
H=2A;F=10.000 NR
1
2
2
(towards the left)
2
3
2
+ 

=
2
FHH2b
G+9gh)

```
```

)
W
E (+ H has)I
48
1
-315100365
NR m
4
Handbook of Validation
V3.90 booklet: Theoretical references of tests in linear statics
HI-75/96/037 - Ind A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
4.0

Titrate:
Calculation of an elastic hyperstatic plane gantry
Date:
01/09/99
Author (S):
F. VOLDOIRE

Key:
V3.90.001-A Page:
22/24
8.4 Couples
specific
in Cl
\(m M\)
H
H
fh
G H
"
fh
G" H
Iso
```

+ 

I.E.(internal excitation)
3 I.E.(internal excitation)
(
2 A
1
1
+H)}
4A
+H)+-
+
2 (4a+h)
posts
2
h3" G
=
E(+H has) 2I
24
1
mM
B
fh
G" H
fh
G" H
Iso
-2a+H

```
=

\section*{I.E.(internal excitation)}

\section*{3 I.E.(internal excitation)}
```

2
4A+H)+

```
\(+\)
2 (4a+h)
charp
- 2
bh
=
2
" 2
2
\((A g+F(a+h))\)
\(E(+H\) has \() I\)
24
2
Numerical application:
\(I=2 \mathrm{I}\)
\(H=2 A ;=-100.000 \mathrm{Nm}\)
1
2
2
H2
W

2
( \(G\) (h-b) -9 fhb)
\(E\) (+ H has) I
24
1
- 266.666.667

NR m
3
\(m\) M

\subsection*{8.5 Calculation}
of
\(D=\)
1
I.E.(internal excitation)
\(m M\)
2h
\(G{ }^{\prime \prime} H\)
H
2
G" h3
1
\(=\)

\section*{\(=\) \\ I.E.(internal excitation)}

3 I.E.(internal excitation)
(
4 A
+ H) has + H
E
\(+\)
12
1
\(1 H\)
1 A
\(G H\)
2
GB
H H
\(3+h a s\) )
=
=
I.E.(internal excitation)
I.E.(internal excitation)
2
has
2
2
2
\(+H+6\)
\(+H\) has (
\(4 A+H\) )
\(E\)
\(+\)
24
frame
(H has) I2

From where (it is noted that \(D\) does not depend on \(F\) ):
2
G H
" 2 h 2
(
B H
```

3+has)
D
=
+
I
1
2

```

\section*{Numerical application:}
```

I=2I
;
H=2A

```
1
2
2
"H2
D
\(=\)
G
( \(2 \mathrm{~h}+7 \mathrm{~b}\) )
E (+H has) 2 I
24
1
4.8742564
-
\(m 4\)
Handbook of Validation
V3.90 booklet: Theoretical references of tests in linear statics
HI-75/96/037 - Ind A

\section*{Code_Aster ®}

Version
4.0

Titrate:
Calculation of an elastic hyperstatic plane gantry
Date:
01/09/99

\section*{Author (S):}

\section*{F. VOLDOIRE}

Key:
V3.90.001-A Page:
23/24
8.6

Summary of displacements \(C P U\) and \(v C\)
```

I
4
= 50
, 10
m4
1
E
= 210.000 MPa
CASE
X
XD
wV
pressure on C C
18672.994
91016960.3
184930109.4
1
F1 out of C
41422.161
201902233.4
315100365.0
F
8284.432
40380445.6
63020073.0
2 in C1
out of C
4 9 1 6 . 7 2 4
23965373.4
14775091.25
1
CASE
W
U
(m)

```
```

v
(m)
H
C
C
pressure on C C
83519999.94
0.0110476
0.012422374
1
Fl out of C
0.00
0.00
0.01497330
F
226872262.8
0.03000956
0.00299466
2 in Cl
out of C
206790328.5
0.0273532
0.001215646
1
Note:
2
D
=
GD
4
, with: D
= 4.8742564
m
E (+ H has) 2 I1
2
W
=
2
(GW+FW
V
H) to see higher

```
```

E (+ H has)I1

```
2
2
\(U\)
\(=\)
W
;
\(v\)
\(=\)
2
2
\((W+X d\)
C
\(H\)
C
V
)
\(E(+H\) has \() I\)
E (has
1
\(+H) I I\)
2
10
-4
\(=\).
NR m
E (+ H has)
13227513210
2
1
I

Handbook of Validation
V3.90 booklet: Theoretical references of tests in linear statics
HI-75/96/037 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
4.0

Titrate:
Calculation of an elastic hyperstatic plane gantry
Date:
01/09/99
Author (S):
F. VOLDOIRE

Key:
V3.90.001-A Page:
24/24
Comparison Aster - reference analytical (R.) CASE
Moment out of \(C\)
Reaction
Reaction \(V\)
Displacement
With
Displacement \(v C\)
(N.m)
\(H A(\mathbf{N R})\)
(NR)
CPU (m)
(m)
\(p\) on
R:
18672.994
5175.37
24233.24
0.0110476
0.012422374

C C
Aster:
18673.20
5175.36
24233.2
0.0110472
0.0124233

1
F
41422.161
4881.487
0.00000
0.01497330

1 out of \(C\)
R:
Aster:
41422.40
4881.47
10000.0
0.0000
0.0

F
R:
8284.432
5976.297
4000.00
0.03000956
0.00299466

2 in \(C 1\)
Aster:
8284.34
5976.31
4000.0
0.0300098
0.00299450
out of \(C\)
R:
4916.724
4576.394
5000.00
0.0273532
0.001215646

1
Aster:
4916.62
4576.38
5000.0
0.0273536
0.00121583

Foot-note:
Aster calculation was carried out by taking very slim elements, so that: S" \(2 \ll I\). Thus, the energy of inflection is prevalent. The values of Aster calculation result from case-test VPCS called SSLL14,
with the following data:
\(m 4\)
\(;\)
\(I\)
4
\(=\)
\(m 4\)
5010
2510
;
E
\(=210.000 \mathrm{MPa}\)
1
2
'
\(H=2 A=8 \mathrm{~m} ; "=20 \mathrm{~m} ; B=\)
11
. 6 ,
2
\(p=3.000 \mathrm{NR} / \mathrm{m}\) (downwards),
\(F=\)
\(N R\)
1
20000
(downwards),
\(F=\)
NR
2
10000
(towards the left),
\(=-100.000 \mathrm{Nm}\).

\author{
Handbook of Validation
}

V3.90 booklet: Theoretical references of tests in linear statics
HI-75/96/037 - Ind A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA01 - Infinite hollow roll in thermal balance

Date:
30/08/02
Author (S):
J. Key PELLET
.
V4.01.001-C Page:
1/10
Organization (S): EDF/AMA

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures Document: V4.01.001

TPLA01-Infinite hollow roll in balance thermics

\section*{Summary:}

Linear stationary thermics.

Axisymmetric model; 3 modelings.
Analytical solution.
Interest of the test:
- all axisymmetric elements: triangles and quadrangles, degrees 1 and 2,
- boundary conditions varied: exchange, imposed temperature, imposed flow,
- validation partial of the matrix of thermal "mass" because one makes "a false" transient.

The results are not affected by the distortion of the meshs \(h / l=40\).
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA01-Infinite hollow roll in thermal balance

Date:
30/08/02
Author (S):
J. Key PELLET
:
V4.01.001-C Page:
2/10

1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Z}

IH
Re
Interior ray
\(I H=0.300 \mathrm{~m}\)
External ray
\(R e=0.35 \mathrm{~m}\)
Not F
```

R=0.32m
R
Z
D
C
E
F
With
B
R
1.2
Material properties
= 1 W/m }\mp@subsup{}{}{\circ}\textrm{C
CP = 2. J/m3 ' C (voluminal heat)

```

\section*{1.3}
```

Boundary conditions and loadings

```
- [CD.] [AB]:
\(=0 \mathrm{~W} / \mathrm{m} 2\)
- [EA]:
\(T=T i=100^{\circ} \mathrm{C}\)
- [ED]:
= \(I=1729.9091 \mathrm{~W} / \mathrm{m} 2\) (returning flow)
- [CB]: exchange
\(H=H e=500 \mathrm{~W} / \mathrm{m} 2{ }^{\circ} \mathrm{C}\)
\(T=T e=17.03444{ }^{\circ} \mathrm{C}\)

\subsection*{1.4 Conditions}
initial
To make this stationary calculation, a transitory calculation is made for which the boundary conditions are
constants in time. This makes it possible to test elementary calculations of mass intervening in the first member as well as the second member.
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
TPLAO1 - Infinite hollow roll in thermal balance

\section*{Date:}

30/08/02
Author (S):
J. Key PELLET
:
V4.01.001-C Page:
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}
```

R
T(R)=Ti+Log
IH
T - T
T
E
I
I: temperature in skin"interns"
=
;
R
T
E
E: temperature in "external" skin
Log

```

R
I

\section*{2.2}

Results of reference
Temperatures and flow at the points \(A, B, D, F\).

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\author{
Handbook of Validation
}

V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

TPLA01 - Infinite hollow roll in thermal balance

\section*{Date:}

30/08/02
Author (S):
J. Key PELLET

V4.01.001-C Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ axis (TRIA3, QUAD4)}
\(y\)
\(L=2.5 \quad 10-3\)
\(D\)
\(C\)
\(X y\)
\(H=0.1\)
In 0.3.0.0 N1
B 0.35
0.0
\(N 41\)
\(E\)
\(G\)
\(D\)
\(0.300 .10 ~ N 43\)
\(E\)
0.300 .05 N 2
\(F 0.32\)
0.0
\(N 17\)
\(J\)
0
\(X\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes: 63.
A number of meshs and types: 40 TRIA3, 20 QUAD4

\author{
3.3 Functionalities \\ tested \\ Orders \\ AFFE_MODELE \\ THERMICS \\ AXIS \\ THER_LINEARE \\ TEMP_INIT \\ STATIONARY \\ EXCIT \\ CHARGE \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ FLUX_REP \\ EXCHANGE
}

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLAO1 - Infinite hollow roll in thermal balance

Date:
30/08/02
Author (S):

\section*{J. Key PELLET}

V4.01.001-C Page:
5/10

\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values \\ tested}

\section*{Identification Reference}

Aster \%
difference
T(A) 100100.0000 .00
T (B) 2020.0020 .01
\(T(F) 66.506\)
66.5070 .00
\(T(D) 10099.9330 .07\)
(A) 1729.911722 .620 .42
(B) 1482.781487 .98
0.35
(D) 1729.911723 .260 .38
(F) 1621.791628 .17
0.39
(F) 1621.791615 .500 .39

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLA01 - Infinite hollow roll in thermal balance

\section*{Date:}

30/08/02
Author (S):
J. Key PELLET

V4.01.001-C Page:
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\section*{5 Modeling \\ B \\ 5.1 \\ Characteristics of modeling \\ axis (TRIA6, QUAD8)}
\(y\)
\(L=2.5 \quad 10-3\)
\(D\)
\(C\)
\(X y\)

\(H=0.1\)
In 0.3.0.0 N180
B 0.35
0.0
N10
\(E\)
\(D\)
0.300 .10 N178
\(E\)
0.300 .05 N183
\(F 0.32\)
0.0
N112
0
\(X\)
With
\(F\)

F

\section*{5.2 \\ Characteristics of the grid}

A number of nodes: 185.
A number of meshs and types: 40 TRIA6, 20 QUAD8

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
AFFE_MODELE \\ THERMICS \\ AXIS \\ THER_LINEARE \\ TEMP_INIT \\ STATIONARY \\ EXCIT \\ CHARGE \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ FLUX_REP \\ EXCHANGE
}

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA01 - Infinite hollow roll in thermal balance

Date:
30/08/02
Author (S):
J. Key PELLET
:
V4.01.001-C Page:
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\section*{6 \\ Results of modeling B}

\subsection*{6.1 Values}
tested

\section*{Identification Reference}

\author{
Aster \%
}
difference
T(A) 100100.00
0.00

T (B) 2020.000 .00
\(T(F) 66.506\)
66.5060 .00
\(T(D) 100100.00\)
0.00
(A) 1729.911729 .89
0.00
(B) 1482.781482 .77
0.00
(D) 1729.911729 .88
0.00
(F) 1621.791621 .77
0.00
(F) 1621.791621 .78
0.00

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)

\author{
Version \\ 5.0
}

Titrate:
TPLA01 - Infinite hollow roll in thermal balance

Date:
30/08/02
Author (S):
J. Key PELLET
:
V4.01.001-C Page:
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\section*{7 Modeling}

C

\section*{7.1}

Characteristics of modeling
axis (TRIAG, QUAD9)
```

y
L=2.5 10-3
D
C
X y
H=0.1
In 0.3.0.0 N199
B 0.35
0.0
N10
E
D
0.30 0.10 N197
E
0.30 0.05 N203
F 0.32
0.0
N124
O

```
```

X
With
F
B

```

\section*{7.2}

\section*{Characteristics of the grid}

A number of nodes: 205.
A number of meshs and types: 40 TRIA6, 20 QUAD9

\subsection*{7.3 Functionalities}
tested

\author{
Orders
}

\author{
AFFE_MODELE \\ THERMICS \\ AXIS \\ THER_LINEARE \\ TEMP_INIT \\ STATIONARY \\ \section*{EXCIT} \\ CHARGE \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ \(F L U X \_R E P\) \\ EXCHANGE
}

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLAO1 - Infinite hollow roll in thermal balance

\section*{Date:}

30/08/02
Author (S):

\section*{J. Key PELLET}

\author{
. \\ V4.01.001-C Page:
}

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\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
T(A) 100100.00
0.00
\(T\) (B) 2020.000 .00
\(T(F) 66.506\)
66.5060 .00

T (D) 100100.00
0.00
(A) 1729.911729 .89
0.00
(B) 1482.781482 .77
0.00
(D) 1729.911729 .88
0.00
(F) 1621.791621 .77
0.00
(F) 1621.791621 .77
0.00

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures

Titrate:
TPLA01 - Infinite hollow roll in thermal balance

Date:
30/08/02
Author (S):

\section*{J. Key PELLET}

V4.01.001-C Page:
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\section*{9 \\ Summary of the results}

This problem is correctly solved:
- with the various types of elements whatever the degree of interpolation,
\(\cdot\) is not affected by the shape of the elements \(h / l=40\).
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
TPLA04 Release of power in a hollow roll
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.01.004-A Page:
1/6
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\title{
Handbook of Validation
}

V4.01 booklet: Stationary thermics of the axisymmetric structures
Document V4.01 004

TPLA04-Release of power in one
hollow roll

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It comprises an axisymmetric modeling 2D which tests the axisymmetric elements in thermics, them boundary conditions in imposed temperature and the boundary conditions of the heat source type.

This test aims to validate the taking into account of the heat source by comparing the results obtained with those provided by VPCS.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLA04 Release of power in a hollow roll
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.004-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Z}

Re
IH
Interior ray \(\mathbf{I H}=1 \mathrm{~m}\)
External ray \(\boldsymbol{R e}=2 \mathrm{~m}\)
Length L
\(\underset{R}{Q}\)
R

\section*{1.2}

Properties of material
\(=1.0 \mathrm{~W} / \mathrm{m}^{\circ} \mathrm{C}\)
Thermal conductivity

\section*{1.3 \\ Boundary conditions and loadings}
- Imposed Températures:

Interior surface: \(T i=T(R=I H)=20^{\circ} \mathrm{C}\),
External surface: \(\boldsymbol{T e}=\boldsymbol{T}(\boldsymbol{R}=\mathrm{Re})=20^{\circ} \mathrm{C}\),
\(\cdot\) Released Puissance uniform \(Q=100 \mathrm{~W} / \mathrm{m} 3\).
1.4 Conditionsinitial
Without object.
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0
Titrate:TPLA04 Release of power in a hollow roll
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

:
V4.01.004-A Page:
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card TPLA04/89 of guide VPCS.
- Température according to \(R\) :

\section*{\(\boldsymbol{R}\)}
\(T(R)=T+\)
```

(R2-R2
I
)
-(r2-R2)
I
E
I
4
R
I
ln
R

```

\section*{- Density flux according to R:}
```

dT

```
Q (R2-R2)
\((R)=-\)
E
I
= -
\(-2 r 2\)
Dr.
\(4 r\)
R
\(\ln (E)\)
\(\boldsymbol{R}\)

The cylinder is supposed infinitely long (l>>Re)

\section*{2.2}

Results of reference
Temperature and density flux for \(R=1.0,1.2\) and 1.5

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
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Version
5.0

Titrate:
TPLA04 Release of power in a hollow roll
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{3 Modeling}

\section*{With}

\section*{3.1 \\ Characteristics of modeling}

\section*{AXIS (QUAD9)}
```

y(Z)
0.1 m
D
C
m20
m17 m16
m11 m10
m1
X(R)
With
E
F
B
IH = 1.m
1.2m
1.5m
Re=2.m
Limiting conditions:
Not
R
Z
Node
With
1.0
0.0
N1
-dimensioned AD,BC T = 20 ' C
D
1.0
0 . 1
N3

- all
Q = 100 W/m3
E
1.2

```
```

0.0
N25

- dimensioned AB, CD = 0. W/m2
F
1.5
0 . 0
N61
B
2.0
0 . 0
N121
C
2.0
0 . 1
N123

```
3.2
Characteristics of the grid
123
A number of meshs and types: 20 QUAD9
3.3 Functionalities
tested
Orders

\author{
AFFE_MODELE THERMICS \\ AXIS \\ ALL \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ SOURCE \\ THER_LINEAIRE \\ EXCIT
}

\author{
RECU_CHAMP \\ NUME_ORDRE \\ CALC_CHAM_ELEM \\ FLUX_ELNO_TEMP
}

\subsection*{3.4 Remarks}

Voluminal heat CP does not intervene in this test, but must obligatorily be declared. One \(C P=2.0 \mathrm{~J} / \mathrm{m} 3{ }^{\circ} \mathrm{C}\) takes.

The condition limits \(=0\). is implicit on the free edges.

\author{
Handbook of Validation
}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
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TPLA04 Release of power in a hollow roll
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20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference

\author{
tolerance \\ Temperature ( \({ }^{\circ} \mathrm{C}\) )
}

Node n1 (a: \(R=1.0\) )
20.00
20.0000
\(0.000 \%\) *
1\%
Node n25 (E: R = 1.2)
28.73
28.7276
-0.008\%
1\%
Node n61 (F: R = 1.5)
32.62
32.6222
0.007\%

1\%

Density flux (W/m \({ }^{2}\) )

Net m20 n1 ( \(R=1.0\) )
-58.20
-58.1592
-0.070\%
1\%
Net m17n25 ( \(R=1.2\) )
Net m10 n61 ( \(R=1.5\) )
2.87
2.8782
0.285\%
1\%

\subsection*{4.2 Parameters \\ of execution}

Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
64 Mo
Time CPU To use: 1.98 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA04 Release of power in a hollow roll

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{5}

Summary of the results
The results obtained are very satisfactory. The maximum change is \(\mathbf{- 0 . 0 0 8 \%}\) in temperature and of \(0.316 \%\) in flow.

This test made it possible to test the taking into account of a source term within meshs QUAD9 with one
modeling AXIS (AFFE_CHAR_THER associated with the key word SOURCE).

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA05 Bars cylindrical with density flux

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.005-A Page:
1/8
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures Document V4.01 005

TPLA05 - Cylindrical bar with density flux

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It includes/understands two modelings, the first which tests the voluminal elements, the second, the elements 2 D axisymmetric.

Boundary conditions in imposed temperature and of density flux are taken into account. The results resulting from this case test are compared with those provided by VPCS.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
TPLA05 Bars cylindrical with density flux

Date:

\section*{20/09/02}

\section*{Author (S):}
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

V4.01.005-A Page:
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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

R

Z

O

L
```

$R=0.01 m$ (ray of the cylinder)
$L=1 m$

```

\section*{1.2}

Properties of material

\section*{\(=33.33 \mathrm{~W} / \mathrm{m}^{\circ} \mathrm{C}\)}

Thermal conductivity

\section*{1.3 \\ Boundary conditions and loadings}
- Imposed Températures,
- \(\boldsymbol{T}=0^{\circ} \mathrm{C}\) in \(\mathrm{Z}=0\).,
- \(T=500^{\circ} \mathrm{C}\) in \(\mathrm{Z}=1\),
- Constant Density flux on cylindrical surface: \(=200\) W/m2 (outgoing flow).

\subsection*{1.4 Conditions \\ initial}

Without object.

\title{
Handbook of Validation \\ V4.01 booklet: Stationary thermics of the axisymmetric structures \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA05 Bars cylindrical with density flux

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.005-A Page:
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2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that given in card TPLA05/89 of guide VPCS

\section*{R \\ Z \\ O \\ L}

\section*{Z}
- Température according to Z: \(T(Z)=\) \(Z(Z-L)+T\),
R
1 L
\(\cdot T(z=0)=0 T(z=L)=T 1\).

The cylinder is supposed infinitely long ( \(L \gg r\) )
The temperature minimum is of \(-4.17^{\circ} \mathrm{C}\) in \(\mathrm{Z}=0.083 \mathrm{~m}\)

\section*{2.2}

Results of reference
Temperature in \(Z=0 ., 0.1, \ldots, 0.8,0.9,1.0\)

\section*{2.3}

Uncertainty on the solution
< \(1 \%\)
Approximate analytical solution (approximation: \(\boldsymbol{T}=\) cte, for any \(\mathbf{R}\) )

\subsection*{2.4 References \\ bibliographical}
[1]
Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA05 Bars cylindrical with density flux

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{3 Modeling \\ With}

\section*{3.1 \\ Characteristics of modeling}

\section*{3D (PENTA15, HEXA20)}
\(X(R)\)
Limiting conditions:
\(=30^{\circ}\)
- faces \(A B C D, A B E F=0\)
- face DFEC
\(=-200 \mathrm{w} / \mathrm{m}^{2}\)
- face AFD
\(T=0^{\circ} \mathrm{C}\)
D
C
- face NOZZLE
\(T=500^{\circ} \mathrm{C}\)
F

E
Cutting:
Z
- 80 elements
according to Z
- 2 elements according to

\section*{With}
- 2 elements according to \(X\)
y
B

\section*{3.2 \\ Characteristics of the grid}

A number of nodes:
1937
A number of meshs and types: 160 PENTA15, 160 HEXA20 (and 160 QUAD8)

\subsection*{3.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE \\ AFFE \\ THERMICS \\ \(3 D\) \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ FLUX_REP \\ THER_LINEAIRE \\ EXCIT \\ CHARGE \\ RECU_CHAMP \\ NUME_ORDRE
}

\subsection*{3.4 Remarks}

Voluminal heat CP does not intervene in this test, but must be declared for Code_Aster. One
\(C P=1.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}\) takes.
The condition limits \(=0\). is implicit on the free edges.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLA05 Bars cylindrical with density flux

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.005-A Page:
5/8

\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )
\(Z=0.0 R=.0\) (n1: With)
```

0 . 0 0
0 . 0 0 0
0.0000*
.00001
R=.01 (n17: D)
0 . 0 0
0 . 0 0 0
0.0000*
.00001
Z=0.1 R=.0(n193)
-4.00
-3.991
-0.234%
1%
R=.01 (n209)
-4.00
-4.021
0.517% 1%
Z=0.2 R=.0(n385)
4 . 0 0
4 . 0 0 5
0.123%
1%
R=.01 (n401)
4 . 0 0
3.975
-0.627%
1%
Z=0.3 R=.0(n577)
24.00
2 4 . 0 0 2
0.007%
1%
R=.01 (n593)
24.00
2 3 . 9 7 2
-0.118%
1%
Z=0.4 R = . 0(n769)
56.00
56.000
0.000
1%

```
```

R=.01 (n785)
56.00
5 5 . 9 7 0
-0.054%
1%
Z=0.5 R = .0(n961)
100.00
99.999
-0.001%
1%
R=.01 (n977)
100.00
9 9 . 9 6 9
-0.031%
1%
Z = 0.6 R = .0 (n1153)
156.00
156.000
0.000%
1%
R=.01 (n1169)
1 5 6 . 0 0
155.970
-0.019%
1%
Z = 0.7 R = .0 (n1345)
2 2 4 . 0 0
224.002
0.001%
1%
R=.01 (n1361)
224.00
223.972
-0.013%
1%
Z=0.8 R = .0 (n1537)
304.00
304.005
0.002%
1%
R=.01 (n1553)
304.00
303.975

```
\[
R=.01(n 1745)
\]
\[
396.00
\]
\[
395.979
\]
\[
-0.005 \%
\]
\[
1 \%
\]
\[
Z=1.0 R=.0(n 1921: B)
\]
\[
500.00
\]
\[
500.000
\]
\[
0.0000^{*}
\]
\[
.00001
\]
\[
R=.01 \text { (n1937: C) }
\]
\[
500.00
\]
500.000
0.0000*
. 00001
(*: Imposed temperature)

\subsection*{4.2 Parameters \\ of execution}

Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 3.21 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\title{
Titrate: \\ TPLA05 Bars cylindrical with density flux
}

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

V4.01.005-A Page:
6/8

\section*{5 Modeling}

B

\section*{5.1 \\ Characteristics of modeling}

\section*{AXIS (TRIA6)}

\section*{Limiting conditions:}
\(\boldsymbol{X}(\boldsymbol{R})\)
- dimensioned CD
\(=-200 . \mathrm{W} / \mathrm{m}^{2}\)
- dimensioned \(A D\)
\(T=\)
\(0^{\circ} \mathrm{C}\)
D
C
- dimensioned BC
\(T=\)
\(500^{\circ} \mathrm{C}\)
Cutting:
\(y\) (Z)
- 80 elements
according to \(y\)
With
B
- 2 elements according to \(X\)

\section*{5.2}

Characteristics of the grid
A number of nodes:
805
A number of meshs and types: 320 TRIA6 (and 80 SEG3)

\subsection*{5.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE \\ AFFE \\ THERMICS \\ AXIS \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ FLUX_REP \\ THER_LINEAIRE \\ EXCIT \\ CHARGE \\ RECU_CHAMP \\ NUME_ORDRE
}

\subsection*{5.4 Remarks}

Voluminal heat CP does not intervene in this test, but must be declared for Code_Aster. One \(C P=1.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}\) takes.

The condition limits \(=0\). is implicit on the free edges.
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

Titrate:
TPLA05 Bars cylindrical with density flux

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

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\section*{6}

Results of modeling \(A\)
6.1 Values
tested
Identification Reference
Aster \%
difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )
\(Z=0.0 R=.0\) (n1: With)
0.00
0.000
0.0000*

00001
\(R=.01\) (n5: D)
0.00
0.000
0.0000*
```

00001
Z = 0.1 R = .0 (n81)
-4.00
-3.990
-0.257%
1%
R=.01 (n85)
-4.00
-4.021
0.513% 1%
Z=0.2 R=.0(n161)
4 . 0 0
4 . 0 0 6
0.152%
1%
R=.01 (n165)
4 . 0 0
3.976
-0.618%
1%
Z=0.3 R=.0(n241)
24.00
2 4 . 0 0 3
0.013%
1%
R=.01 (n245)
24.00
2 3 . 9 7 2
-0.116%
1%
Z=0.4 R = .0(n321)
56.00
56.001
0.002
1%
R=.01 (n325)
56.00
5 5 . 9 7 0
-0.053%
1%
Z=0.5 R = .0(n401)
100.00
100.001

```
```

0.001%
1%
R=.01 (n405)
100.00
99.970
-0.030%
1%
Z=0.6 R=.0(n481)
156.00
156.001
0.001%
1%
R=.01(n485)
156.00
155.970
-0.019%
1%
Z=0.7 R=.0(n561)
224.00
224.003
0.001%
1%
R=.01 (n565)
224.00
223.972
-0.012%
1%
Z=0.8 R=.0(n641)
304.00
304.006
0.002%
1%
R=.01 (n645)
304.00
303.975
-0.008%
1%
Z=0.9 R=.0(n721)
396.00
396.010
0.003%
1%
R=.01(n725)

```

\subsection*{396.00}
395.979
-0.005\%
1\%
\(Z=1.0 R=.0(n 801: B)\)
500.00
500.000
0.0000*

00001
\(R=.01\) (n805: C)
500.00
500.000
0.0000*

00001
(*: Imposed temperature)
6.2 Parameters
of execution
Version: 5.03

Machine: SGI - ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 2.28 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA05 Bars cylindrical with density flux

\section*{Date:}

20/09/02
Author (S):

V4.01.005-A Page:
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\section*{7}

Summary of the results
The results obtained are satisfactory, the maximum change is \(0.63 \%\). Modeling 3D (with meshs PENTA15, HEXA20) and modeling AXIS (with meshs TRIA6) give appreciably same results (the grid and the degree of interpolation are identical).

The analytical solution which is an approached solution, supposes that the r/L report/ratio is much higher than
1. For this numerical test, the r/L report/ratio was taken equal to 100.

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.01.006-A Page:
1/8
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\title{
Handbook of Validation \\ V4.01 booklet: Stationary thermics of the axisymmetric structures V4.01.006 document
}

\section*{TPLA06-Cylindrical bar with convection}

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about an axisymmetric problem 2D represented by two modelings, the first using of the elements voluminal, the second of the axisymmetric elements \(2 D\).
Boundary conditions in imposed temperature and of convection are taken into account.
The results resulting from this case test are compared with those provided by VPCS.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

Date:
20/09/02
Author (S):

V4.01.006-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{R \\ Z \\ \(O\) \\ L}
\(R=0.01 \mathrm{~m}\) (ray of the cylinder)
\(L=1 m\)

\section*{1.2}

Properties of material
\[
=33.33 \mathrm{~W} / \mathrm{m}{ }^{\circ} \mathrm{C}
\]

Thermal conductivity

\section*{1.3 \\ Boundary conditions and loadings}

\section*{- Imposed Températures:}
\[
T=0^{\circ} C \text { in } Z=0 .
\]
\[
T=500^{\circ} \mathrm{C} \text { in } \mathrm{Z}=1 .
\]
- Convection on cylindrical surface,
\[
\mathrm{H}=10 \mathrm{~W} / \mathrm{m} 2{ }^{\circ} \mathrm{C},
\]
- \(\boldsymbol{T} \boldsymbol{E}=\boldsymbol{0}^{\circ} \boldsymbol{C}\) (outside temperature).

\subsection*{1.4 Conditions initial}

\section*{Without object.}

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.006-A Page:
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card TPLA06/89 of guide VPCS

R
Z
O
L>> \(R\)
\(\sinh (a z)\)
- Température according to \(Z: T(Z)=T\)
\(1 \sinh (A l)\)
\(2 h\)
where has =
\(\boldsymbol{R}\)
\(\cdot T(Z=0)=0 T(Z=L)=T 1\).

\section*{2.2}

Results of reference
Temperature in \(Z=0 ., 0.1,0.2, \ldots, 0.8,0.9,1.0\)

\section*{2.3}

Uncertainty on the solution
< \(1 \%\)
Approximate analytical solution (approximation: \(T=\) cte, for any \(R\) )

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.006-A Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

3D (PENTA6, HEXA8)
Limiting conditions:
\(\boldsymbol{X}(\boldsymbol{R})\)
- faces \(A B C D, A B E F=\)
0.
\(=30^{\circ}\)
- face DCEF
\(\mathrm{H}=10 . \mathrm{W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\)
Text \(=0^{\circ} \mathrm{C}\)
D
C
- face ADF
\(T=\)
\(0^{\circ} \mathrm{C}\)
- face ECB
\(T=500^{\circ} \mathrm{C}\)
F
E
Cutting:
- 100 elements according to Z

Z
- 3 elements according to

With
B
- 3 elements according to \(X\)
\(y\)
3.2

Characteristics of the grid
A number of nodes:
1313
A number of meshs and types: 300 PENTA6, 600 HEXA8 (and 300 QUAD4)

\subsection*{3.3 Functionalities}

\author{
AFFE_MODELE \\ AFFE \\ THERMICS \\ \(3 D\) \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ \section*{EXCHANGE} \\ \section*{THER_LINEAIRE} \\ EXCIT \\ CHARGE \\ RECU_CHAMP \\ NUME_ORDRE
}

\subsection*{3.4 Remarks}

Voluminal heat CP does not intervene in this test, but must be declared for Code_Aster. One \(C P=1.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}\) takes.

The condition limits \(=0\). is implicit on the free edges.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:

V4.01.006-A Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Reference
Aster \%
difference
tolerance
Temperature ( \({ }^{\circ}\) C)
\(Z=0.0 R=.0(n 1:\) With \()\)
0.0000
0.0000
\(0.0000^{*}\)
.00001
\(R=.01(n 13: D)\)
0.0000
0.0000
\(0.0000^{*}\)
.00001
\(Z=0.1 R=.0(n 131)\)
0.3694
0.3654
\(-1.084 \%\)
\(1 \%\)
\(R=.01(n 143)\)
```

0 . 3 6 9 4
0 . 3 6 4 8
-1.240%
1%
Z=0.2 R=.0(n261)
0 . 9 7 1 8
0 . 9 6 2 1
-0.998%
1%
R=.01 (n273)
0 . 9 7 1 8
0 . 9 6 0 6
-1.154%
1%
Z=0.3 R = .0(n391)
2.1870
2.1679
-0.875%
1%
R=.01 (n403)
2.1870
2 . 1 6 4 4
-1.031%
1%
Z=0.4 R = .0(n521)
4 . 7 8 1 5
4 . 7 4 6 0
-0.743%
1%
R=.01 (n533)
4 . 7 8 1 5
4 . 7 3 8 5
-0.899%
1%
Z=0.5 R = .0(n651)
10.392
10.329
-0.611%
1%
R=.01 (n663)
10.392
10.312
-0.768%

```
```

1%
Z = 0.6 R = .0 (n781)
22.555
2 2 . 4 5 0
-0.468%
1%
R=.01 (n793)
22.555
2 2 . 4 1 4
-0.625%
1%
Z=0.7 R = .0(n911)
4 8 . 9 4 4
4 8 . 7 8 2
-0.331%
1%
R=.01 (n923)
4 8 . 9 4 4
4 8 . 7 0 5
-0.488%
1%
Z = 0.8 R = .0 (n1041)
106.20
106.00
-0.192%
1%
R=.01 (n1053)
106.20
105.83
-0.349%
1%
Z=0.9 R = .0 (n1171)
230.44
2 3 0 . 3 1
-0.056%
1%
R=.01 (n1183)
230.44
2 2 9 . 9 5
-0.214%
1%
Z=1.0 R = .0 (n1301: B)
500.00

```
\(R=.01\) (n1313: \(C\) )
500.00
500.00
0.0000*
.00001
(*: Imposed temperature)

\subsection*{4.2 Parameters}
of execution
Version: 5.03

\section*{Machine: SGI-ORIGIN 2000-R12000}

Obstruction memory:
8 megawords
Time CPU To use: 3.10 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.01.006-A Page:
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\section*{5 Modeling \\ B}

\section*{5.1 \\ Characteristics of modeling}

\section*{AXIS (TRIA3)}

Limiting conditions:
- dimensioned CD
\(H=10 . W / m^{2}{ }^{\circ} \mathrm{C}\)
\(X(R)\)
Text \(=10^{\circ} \mathrm{C}\)
D
C
- dimensioned AD
\(T=\)
\(0^{\circ} \mathrm{C}\)
- dimensioned BC
\(T=500^{\circ} \mathrm{C}\)
Cutting:
\(y\) (Z)
- 150 elements following \(y\)
- 3 elements according to \(X\)

With
B

\section*{5.2}

Characteristics of the grid
A number of nodes:
604
A number of meshs and types: 900 TRIA3 (and 150 SEG2)

\author{
5.3 Functionalities \\ tested \\ Orders
}

AFFE_MODELE
AFFE
THERMICS
AXIS
AFFE_CHAR_THER
TEMP_IMPO
EXCHANGE
THER_LINEAIRE
EXCIT
CHARGE
RECU_CHAMP
NUME_ORDRE

\subsection*{5.4 Remarks}

Voluminal heat CP does not intervene in this test, but must be declared for Code_Aster. One CP \(=1.0 \mathrm{~J} / \mathrm{m3}{ }^{\circ} \mathrm{C}\) takes.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

Titrate:
TPLA06 Bars cylindrical with convection

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

V4.01.006-A Page:
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\section*{6}

Results of modeling \(A\)

\subsection*{6.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )
\(Z=0.0 R=.0\) (n1: With)
0.0000
0.0000
0.0000*
. 00001
\(R=.01\) (n4: D)
0.0000
0.0000
0.0000*
```

.00001
Z=0.1 R=.0(n61)
0 . 3 6 9 4
0 . 3 7 0 3
0.254%
1%
R=.01 (n64)
0 . 3 6 9 4
0 . 3 6 9 7
0.095%
1%
Z=0.2 R = .0(n121)
0 . 9 7 1 8
0 . 9 7 4 1
0.237%
1%
R=.01 (n124)
0 . 9 7 1 8
0 . 9 7 2 6
0.079%
1%
Z=0.3 R = .0(n181)
2.1870
2 . 1 9 1 9
0.224%
1%
R=.01 (n184)
2.1870
2 . 1 8 8 4
0.065%
1%
Z=0.4 R = .0(n241)
4 . 7 8 1 5
4 . 7 9 1 3
0.205%
1%
R=.01 (n244)
4 . 7 8 1 5
4 . 7 8 3 7
0.046%
1%
Z=0.5 R = .0(n301)

```
10.392
```

10.411
0.181%
1%
R=.01 (n304)

```
10.392
10.394
\(0.022 \%\)
1\%
\(Z=0.6 R=.0(n 361)\)
22.555
22.593
0.167\%
1\%
\(R=.01\) (n364)
22.555
22.557
0.008\%
1\%
\(Z=0.7 R=.0(n 421)\)
48.944
49.015
0.145\%
1\%
\(R=.01(n 424)\)
48.944
48.937
-0.013\%
1\%
\(Z=0.8 R=.0(n 481)\)
106.20
106.33
0.126\%
1\%
\(R=.01\) (n484)
106.20
106.16
-0.033\%
1\%
\(Z=0.9 R=.0(n 541)\)
230.44
230.68
0.103\%
\(1 \%\)
\(R=.01\) (n544)
230.44
230.31
-0.056\%
1\%
\(Z=1.0 R=.0(n 601: B)\)
500.00
500.00
0.0000*
. 00001
\(R=.01\) (n604: \(C\) )
500.00
500.00
0.0000*
.00001
(*: Imposed temperature)
6.2 Parameters
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 2.60 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLA06 Bars cylindrical with convection

Date:
20/09/02

\author{
Author (S): \\ C. DURAND, E. SCREW, F. LEBOUVIER Clé
}

7
Summary of the results
The modeling A, carried out in 3D with linear meshs (PENTA15, HEXA8), gives results whose four values (out of 22) exceed the tolerance fixed initially. The maximum change obtained is \(1.24 \%\) for a tolerance of \(1 \%\). This going beyond of the tolerance is observed for values of the temperature close to 0 .

By account modeling B, carried out in AXIS with linear meshs (TRIA3), gives satisfactory results, the maximum change obtained is \(0.25 \%\).

Modeling AXIS is adapted to model this cylindrical bar than modeling
3D. Cutting circonférenciel in 3D is not enough dense to represent the cylinder, and one finer cutting would improve the results.

The results obtained by modeling 3D are regarded as acceptable taking into account grid used.

The analytical solution which is an approached solution, supposes that the r/L report/ratio is much higher than
1. For this numerical test, the r/L report/ratio was taken equal to 100.

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Orthotropic TPLA07 Hollow roll

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\author{
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD
}

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
V4.01.007 document

TPLA07-Orthotropic hollow roll

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about an axisymmetric problem 2D represented by two modelings, the first three-dimensional one, second axisymmetric \(2 D\).

The interest of this case test is to test an orthotropic material subjected to various boundary conditions (flow imposed, convection, linear variation of the outside temperatures).

The results resulting from this case test are compared with those provided by VPCS.

\title{
Handbook of Validation \\ V4.01 booklet: Stationary thermics of the axisymmetric structures \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Orthotropic TPLA07 Hollow roll

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.007-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

Re
IH
Interior ray \(\mathrm{IH}=0.03 \mathrm{~m}\)
External ray \(\mathrm{Re}=0.05 \mathrm{~m}\)
Height \(L=0.40 \mathrm{~m}\)
L
R

\section*{1.2}

Properties of material
\(R=2.89 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}\)
thermal conductivity along the axis \(R\)
\(Z=40.0 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}\)
thermal conductivity along axis Z
circumferential:
unspecified

\section*{1.3}

Boundary conditions and loadings
\(\cdot\) density flux \(=-500 \mathrm{~W} / \mathrm{m}^{2}\) through surface \(\mathrm{Z}=0\). (outgoing flow), \(\cdot\) density flux \(=+500 \mathrm{~W} / \mathrm{m}^{2}\) through surface \(Z=0.4\) (entering flow),
- convection on interior surface: \(\boldsymbol{H}=377.0 \mathrm{~W} / \mathrm{m} 2^{\circ} \mathrm{C}\), \(\cdot\) convection on external surface: \(\boldsymbol{H}=339.3 \mathrm{~W} / \mathrm{m} 2{ }^{\circ} \mathrm{C}\),
- linear variation of the outside temperatures:
on surface \(R\)
E
E
E
I: \(T i=130^{\circ} \mathrm{C}\) in \(\mathrm{Z}=0 . ; \mathbf{T i}=135^{\circ} \mathrm{C}\) in \(Z=0.4(T i=130+12.5 z)\),
on surface \(R\)
E
E
E
\(E: T e=20^{\circ} \mathrm{C}\) in \(\mathrm{Z}=0 . ; T i=25^{\circ} \mathrm{C}\) in \(\mathrm{Z}=0.4(T e=20+12.5 z)\).

\subsection*{1.4 Conditions}
initial
Without object.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Orthotropic TPLA07 Hollow roll
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

:
V4.01.007-A Page:
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\section*{2}
Reference solution

\section*{2.1}
Method of calculation used for the reference solution
The reference solution is that given in card TPLA07/89 of guide VPCS.

Temperature: \(T(R, Z)=-117.46 \log R+12.5 z-311.87\)

\section*{2.2}

Results of reference
\(\cdot\) temperature in \(R=0.03,0.035,0.04\) and 0.05 and for \(Z=0 ., 0.2\) and 0.4 , - density flux on interior and external surface, - density flux following axis Z.

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures

\title{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0 \\ Titrate: \\ Orthotropic TPLA07 Hollow roll \\ \\ Date: \\ \\ Date: \\ 20/09/02 \\ Author (S): \\ C. DURAND, E. SCREW, F. LEBOUVIER Clé \\ : \\ V4.01.007-A Page: \\ 4/6
}

\section*{3 Modeling \\ With \\ 3.1 \\ Characteristics of modeling}

\section*{AXIS (TRIA3)}
\(y\) (Z)
Limiting conditions:

\section*{D}

C
- dimensioned AB
=
500. W/m \({ }^{2}\)
- dimensioned CD
=
-500. W/m \({ }^{2}\)
- dimensioned DA

H =
377. W/m \({ }^{2}{ }^{\circ} \mathrm{C}\)

Tei \(=130+12.5 z\)
- dimensioned BC

H =
339.3 W/m \({ }^{2}{ }^{\circ} \mathrm{C}\)

Tee \(=20+12.5 z\)

\section*{Cutting:}
\(X(\boldsymbol{R})\)
- 40 elements
according to \(y\)
- 4 elements
according to \(X\)
With
B
\(z=0\).
\(z=0.2\)
M42
N201 N202 z=0.4 N204 N205
M282
M2
M242
M41
M281
M80
M320
N101
N102
N104
N105
M1
M241
M40
R
M280
M79
M319
N1
N2
N4
N5
M39
M279
\(r=0.03 r=0.05 r=0.03 r=0.05 r=0.03 r=0.05\)

\section*{3.2}

Characteristics of the grid

\section*{A number of nodes:}

205
A number of meshs and types: 408 TRIA3 (and 88 SEG2)
3.3 Functionalities
tested
Orders

\section*{AFFE_MODELE}

THERMICS
AXIS
DEFI_MATERIAU
THER_ORTH

\author{
AFFE_CARA_ELEM
}

SOLID MASS
ANGL_REP
AFFE_CHAR_THER_F
EXCHANGE
FLUX_REP
THER_LINEAIRE
CARA_ELEM
CALC_CHAM_ELEM
CARA_ELEM

\subsection*{3.4 Remarks}

Voluminal heat CP does not intervene in this test, but must obligatorily be declared. One \(. c p=1.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}\) takes.

The condition limits \(=0\). is implicit on the free edges.
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

Titrate:
Orthotropic TPLA07 Hollow roll

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

V4.01.007-A Page:
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4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )
\(Z=0 . R=0.030(N 1)\)
100.01
99.829
-0.181\%
1.\%
\(R=0.035(N 2)\)
81.90
81.867
-0.040\%
1.\%
\(R=0.040(N 3)\)
66.22
66.229
```

0.014%
1.%
R=0.045 (N4)
52.38
52.422
0.081%
1.%
R=0.050(N5)
4 0 . 0 1
4 0 . 0 9 5
0.212%
1.%
Z=0.2 R = 0.030 (N101)
102.51
102.469
-0.040%
1.%
R=0.035 (N102)
84.40
84.388
-0.014%
1.%
R=0.040 (N103)
6.72
6 8 . 7 1 8
-0.003%
1.%
R=0.045 (N104)
54.88
54.892
0.021%
1.%
R=0.050(N105)
42.51
42.521
0.025%
1.%
Z=0.4 R = 0.030 (N201)
105.01
105.111
0.096%
1.%
R=0.035(N202)

```
86.90
86.909
0.010\%
1.\%
\(R=0.040(N 203)\)
71.22
71.207
-0.018\%
1.\%
\(R=0.045(\mathrm{~N} 204)\)
57.38
57.361
-0.033\%
1.\%
\(R=0.050(N 205)\)
45.01
44.946
-0.142\%
1.\%

Density flux (W/m \({ }^{2}\) )

\author{
Z (z=0.) M1/N1 \\ -500. \\ -511.97 \\ 2.395 \\ 1.\% \\ \(Z(z=0) m .2 / N 1\) \\ -500. \\ -824.47 \\ 64.894 \\ 1.\% \\ Z (z=0.) M241/N5 \\ -500. \\ -346.66 \\ -30.668 \\ 1.\% \\ Z (z=0.2) M40/N101 \\ -500. \\ -499.95 \\ -0.009 \\ 1.\%
}
```

Z (z=0.2) M41/N101
-500.
-499.93
-0.013
1.%
Z (z=0.2) M280/N105
-500.
-499.93
-0.014
1.%
Z (z=0.4) M80/N201
-500.
-828.51
6 5 . 7 0 1
1.%
Z (z=0.4) M319/N205
-500.
-345.65
-30.869
1.%
Z (z=0.4) M320/N205
-500.
-468.64
-6.271
1.%
IH (z=0.) M1/NI
11310.
10381.98
-8.205
1.%
IH (z=0.2) M40/N101
11310.
10450.74
-7.597
1.%
IH (z=0.4) M80/N201
11310.
10520.67
-6.979
1.%
IH (z=0.) M241/N5
6786.
7125.41

```

\title{
4.2 Parameters
}
of execution

Version: 5.03

\section*{Machine: SGI - ORIGIN 2000-R12000}

Obstruction memory:
8 megawords
Time CPU To use: 2.56 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

Orthotropic TPLA07 Hollow roll

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.007-A Page:
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\section*{5}

Summary of the results
The results obtained are satisfactory in temperature but far away from the reference solution to level of flows (especially for flow along axis Z). The maximum departures obtained are as follows:
- \(0.212 \%\) in temperature,
- 65.701\% for flow along axis Z,
- -8.205 for flow according to the ray.

Flow following the axis: the flow of reference is constant, the variations observed are more important with
ends of the cylinder, on the other hand with semi height these variations are very weak (lower than the tolerance).

Flow following the ray: The variations according to the ray are higher than the tolerance, but they could
to be minimized by increasing the discretization along axis \(\boldsymbol{R}\).

Following these results, an anomaly software was emitted under the \(\boldsymbol{n}^{\circ}\) 96-176.
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

TPLA300 Plates circular subjected to a voluminal heat source
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.300-A Page:
1/6
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\author{
Handbook of Validation \\ V4.01 booklet: Stationary thermics of the axisymmetric structures V4.01.300 document
}

TPLA300-Plate circular subjected to a source of voluminal heat

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
The interest of this case test plane 2D is to validate a thermal element under various boundary conditions
(imposed temperature, heat source).
This case test includes/understands two modelings 2D, one axisymmetric, the other planes.
The results are compared with an analytical solution.

Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA300 Plates circular subjected to a voluminal heat source
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.01.300-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{\(R=5.0\)}

With
O
\(Q\)

\section*{1.2}

Properties of material

\section*{\(=0.04 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}\) thermal Conductivity}

\section*{1.3}

Boundary conditions and loadings
- voluminal heat source \(Q=1 \mathrm{~W} / \mathrm{m} 3\),
\(\cdot\) temperature imposed on surface outside \((R=5): T=0^{\circ} \mathrm{C}\).

\subsection*{1.4 Conditions initial}

Without object.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA300 Plates circular subjected to a voluminal heat source
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.300-A Page:
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\section*{2}

Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}

\section*{Analytical solution:}
\(T(R)=6.25(25-r 2)\)
> 2.2

> Results of reference
> Temperature for \(\boldsymbol{R}=0 ., 0.625,1.25,1.875,2.5,3.125,3.75,4.375,5\).

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References bibliographical}

\section*{[1]}
W.K. Liu, T. Belytschko, "Efficient linear and nonlinear heat conduction with has quadrilateral element '", Int. J. num. Meth. Engng, flight 20, n5, pp 931-948, 1984.
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA300 Plates circular subjected to a voluminal heat source
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.01.300-A Page:
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

PLAN (QUAD4)
\(y\)
C
N17
N15
N13
N184
N168
N11
N151
Limiting conditions:
N9

\section*{N133}
- dimensioned \(A B=0\)

N7
N108
- dimensioned CA = 0
- dimensioned \(\boldsymbol{B C} \boldsymbol{T}=0^{\circ} \mathbf{C}\)

N5
N3
B
N1
In N45 N116 N171 N207
\(X\)
N79 N146 N191 N217

\section*{3.2}
Characteristics of the grid
A number of nodes:
217
A number of meshs and types: 192 QUAD4

\subsection*{3.3 Functionalities}
tested

\author{
Orders
}

AFFE_MODELE
THERMICS
PLAN
ALL
AFFE_CHAR_THER
TEMP_IMPO
SOURCE

\section*{THER_LINEAIRE}

EXCIT
CHARGE

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
TPLA300 Plates circular subjected to a voluminal heat source
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

V4.01.300-A Page:
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4
Results of modeling \(A\)
4.1 Values
tested
Identification Reference
Aster \%
difference
tolerance
temperatures ( \({ }^{\circ} \mathrm{C}\) )
\[
R=0.000(N 1)
\]
156.25
156.07
\(R=0.625(\mathrm{~N} 3)\)
\(R=0.625(N 45)\)
153.81
153.63
-0.116
1\%
\(R=1.250\) (N5)
146.48
146.31
-0.117
\(1 \%\)
\(R=1.250(N 79)\)
146.48
146.31
-0.117
\(1 \%\)
\(R=1.875(N 7)\)
134.28
134.10
-0.131
1\%
\(R=1.875\) (N116)
134.28
134.10
-0.131
1\%
\(R=2.500(\mathrm{~N} 9)\)
117.19
116.98
-0.182
1\%
\(R=2.500\) (N108)
117.19
116.82
-0.313
1\%
\(R=2.500(\) N146 \()\)
\begin{tabular}{l}
117.19 \\
116.98 \\
-0.182 \\
\(1 \%\) \\
\(R=3.125\) (N11) \\
95.21 \\
95.04 \\
-0.178 \\
\(1 \%\) \\
\(R=3.125\) (N133) \\
95.21 \\
95.00 \\
-0.216 \\
\(1 \%\) \\
\(R=3.125(N 171)\) \\
95.21 \\
95.04 \\
-0.178 \\
\(1 \%\) \\
\(R=3.750(N 13)\) \\
68.36 \\
68.23 \\
-0.191 \\
\(1 \%\) \\
\(R=3.750(N 151)\) \\
68.36 \\
68.21 \\
-0.214 \\
\(1 \%\) \\
\(R=3.750(N 191)\) \\
68.36 \\
68.23 \\
-0.191 \\
\(1 \%\) \\
\(R=4.375(N 15)\) \\
36.62 \\
36.55 \\
-0.194 \\
\(1 \%\) \\
\(R=4.375(N 168)\) \\
36.62 \\
36.54 \\
\\
(N \\
\\
\hline
\end{tabular}
-0.211
```

1%
R=4.375 (N207)
36.62
36.55
-0.194
1%
R=5.000(N17)
0.00*
0.00
0.00
1%
R=5.000(N217)
0.00*
0.00
0 . 0 0
1%
R=5.000(N184)
0.00*
0 . 0 0
0.00
1%

* Condition limiting

```

\subsection*{4.2 Parameters}
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 1.98 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLA300 Plates circular subjected to a voluminal heat source
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.300-A Page:
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\section*{5}

Summary of the results
The results obtained are very satisfactory, the maximum change is \(0.313 \%\).
Among the points of observation, the most important variation is noted with the N108 node which belongs to the element more deformed grid.

The grid used is that proposed in the reference. A radial grid with the same cutting and with the same meshs should give better results.

This test made it possible to test the taking into account of a source term within meshs QUAD9 with one
modeling AXIS (AFFE_CHAR_THER associated with the key word SOURCE).
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA301 Distribution of temperature in a short cylinder
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.01.301-A Page:
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V4.01 booklet: Stationary thermics of the axisymmetric structures
V4.01.301 document

TPLA301-Distribution of temperature in a cylinder runs

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
The axisymmetric problem 2D aims to validate the axisymmetric thermal elements under temperature imposed in the case of a cylinder court on radial and axial behavior.

It comprises only one modeling (axisymmetric).
The results are compared with a solution based on a graphic estimate.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
5.0

Titrate:
TPLA301 Distribution of temperature in a short cylinder
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
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V4.01.301-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}

\section*{Z}

Tsup

\section*{Z}

E
\(J\)
L
r1
D
I
Tcyl
C
H
H
Y
\(L\)
B
G
G
R
\(X\)
With
F
```

K
Tinf
Points
Z
Points
Z
(r=0)
(x10-3)
(r=r1/2)
(x10-3)
With
0.0
F
0.0
B
38.1
G
38.1
rl=L=0.1524 m
C
76.2
H
7.2
D
114.3
I
114.3
E
152.4
J
152.4
1.2
Properties of material
= 1.7307 W/m. ' C thermal Conductivity
1.3
Boundary conditions and loadings

```

Imposed temperatures:
\(\cdot T\) Tinf \(=\) Tcyl \(=17.778^{\circ} \mathrm{C}\),
- Tsup \(=4.444{ }^{\circ} \mathrm{C}\).

\subsection*{1.4 Conditions}
initial
Without object.

\section*{Handbook of Validation}

V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLA301 Distribution of temperature in a short cylinder
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2
2.1

Method of calculation used for the reference solution
The original reference solution given in the book [bib1] is based on a graphic estimate.
This reference is quoted in the handbook of checking of ANSYS [bib2]

\section*{2.2}

Results of reference

Temperature at points ABCDEFGHIJ

\section*{2.3}

Uncertainty on the solution
Unknown factor, it was not possible to get the original reference (delivers old, more published).

\subsection*{2.4 References \\ bibliographical}
[1]
Schneider, P.J., "Conduction Heat Transfer", Addison-Wesley Publishing Co., Inc. Reading, Mass., 2nd Printing, 1957.
[2]
ANSYS: "Checking manual", 1st edition, June 1, 1976
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA301 Distribution of temperature in a short cylinder
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Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

AXIS (QUAD4)
\(\boldsymbol{y}(\mathrm{Z})\)

\section*{Limiting conditions:}
- dimensioned AE
= 0. W/m2
- dimensioned AK, KL T=-17.778 \({ }^{\circ} \mathrm{C}\) n5 (E)
n15 (J)
n25 (L)
- dimensioned IT
\(T=4.444^{\circ} \mathrm{C}\)
n4 (D)
n14 (I)
Not
\(X\)
\(y\)
Node
With
0.0000 .000

N1
B
0.0000 .381

N2
n3 (C)
n13 (H)
C
0.0000 .762

N3
D
0.0001 .143

N4
E
0.0001 .524

N5
F
0.7620 .000

N11
N2 (B)
\(n 12\) (G)
G
0.7620 .381

N12
H
0.7620 .762

N13
\(X(\boldsymbol{R})\)

I
0.7621 .143

N14
\(J\)
0.7621 .524

N15
n1 (A)
n11 (F)
n21 (K)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes:
25
A number of meshs and types: 16 QUAD4

\subsection*{3.3 Functionalities}
tested
Orders

AFFE_MODELE
THERMICS
AXIS
ALL
AFFE_CHAR_THER
TEMP_IMPO

\section*{THER_LINEAIRE}

\author{
EXCIT
}

CHARGE

\section*{RECU_CHAMP \\ NUME_ORDRE}

\subsection*{3.4 Remarks}

Voluminal heat CP does not intervene in this test, but must obligatorily be declared. One \(C P=2.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}\) takes.

The condition limits \(=0\). is implicit on the free edges.
Limiting conditions, \(T=-17.778^{\circ} \mathrm{C}\) on \(K L\), and \(T=4.444\) on, are incompatible at the point \(L\) (node n25).

Code_Aster applies a "law of overload" which, in this case, consists in taking into account last condition limits entered. The order of assignment of the imposed temperatures thus has large influence on the results obtained.

In the case treated, the temperature on the higher face (IT) is affected after that on the blank of roll (KL).
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLA301 Distribution of temperature in a short cylinder
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

Identification Reference Aster
\% difference
NISA
Front KL
Front KL
Temperature ( \({ }^{\circ} \mathrm{C}\) )

\author{
Nodes \\ n1 T (A) \\ -17.778 \\ -17.778 \\ 0.00\%* - 17.778 \\ N2 T(B) \\ -14.000 \\ -13.79 \\ -1.50\% \\ -13.953 \\ n3 T (C) \\ -9.111 \\ -8.908 \\ -2.27\% \\ -9.151 \\ n4 T (D) \\ -2.889 \\ -2.713 \\ -6.10\% \\ -2.892 \\ n5 T (E) \\ 4.444 \\ 4.444 \\ 0.00\%* \\ 4.444 \\ n11 T (F) \\ -17.778 \\ -17.778 \\ 0.00\%* - 17.778 \\ n12 T (G) \\ -14.889
}
0.74\% -15.179
n13 T (H)
-10.667
-11.005
3.16\% -11.499
n14 T (I)
-4.444
-4.412
-0.72\%
-4.854
n15 T (J)
4.444
4.444
\(0.00 \%\) *
4.444
(*: Imposed temperature)

\subsection*{4.2 Parameters}
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 2.06 seconds
Handbook of Validation
V4.01 booklet: Stationary thermics of the axisymmetric structures HT-66/02/001/A

Titrate:
TPLA301 Distribution of temperature in a short cylinder
Date:
20/09/02
Author (S):

\author{
C. DURAND, E. SCREW, F. LEBOUVIER Clé
}
:
V4.01.301-A Page:
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\section*{5 \\ Summary of the results}

Modeling gives results whose value (on 10) exceeds the tolerance fixed initially
\((5 \%)\). The maximum change obtained is \(-6.10 \%\), it is on the smallest value of reference.
In this test, Code_Aster applies a "law of overload" which in this case consists in taking in count the last condition limits entered. The order of assignment of the imposed temperatures, thus has a great influence on the results obtained.

Calculations were carried out in \({ }^{\circ}\). Determination of the variation, by considering the temperatures in \({ }^{\circ}\) \(F\),
give a maximum value very different from that obtained in \({ }^{\circ} \mathrm{C}\).
A calculation carried out with software NISA gives identical results has those of Aster (checked in case where the temperature imposed on the point \(L\) is of \(4.44^{\circ} \mathrm{C}\) ).

The quality of the results could be improved by carrying out a finer grid, the problem of overload would be always present, but the zone of influence of the temperature imposed on the point \(L\) would be
weaker. The results are regarded as acceptable taking into account modeling carried out (grid and system of unit, law of overload).

Handbook of Validation
\(V 4.01\) booklet: Stationary thermics of the axisymmetric structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
TPLLO1 - Infinite plane wall in linear thermics

Date:
13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

V4.02.001-E Page:
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Organization (S): EDF/SINETICS, AMA

Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures Document: V4.02.001

TPLL01 - Infinite plane wall in linear thermics

\section*{Summary:}

This case test relates to a calculation of stationary thermics linear. It includes/understands 10 modelings which test them
elements 2D and 3D.
This case test is of several interests:
\(\cdot\) for modelings of \(A\) with I, it tests on almost all the elements 3D and 2D (except 2D_AXIS, Lumpés PYRAM and), the calculation of the basic options of linear thermics: "rigidity", "mass", exchange,
imposed flow, imposed temperature,
- in modeling J, one calculates a cartography of space error via option ERTH_ELEM_TEMP

CALC_ELEM on which will rest, in a loop PYTHON, the tool of refinement/déraffinement LOBSTER encapsulated in MACR_ADAP_MAIL.
- The orientation of the wall is unspecified compared to the axes of co-ordinates,
- It is one of the rare case-tests to test elements TETRA10 and QUAD9 in linear thermics, with to combine orders AFFE_CHAR_THER/LIAISON_DDL, and to test INTE_MAIL_3D. Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
HI-23/02/017/A
Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
TPLL01-Infinite plane wall in linear thermics

\section*{Date:}

13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

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\section*{1}

Problem of reference

\subsection*{1.1 Geometry}

\section*{E}
\(S\)
B
The problem corresponds to an infinite wall:
and unspecified
CF
OF
D
\[
\begin{aligned}
& G \\
& F \\
& L=0.05 \mathrm{~m} \\
& m \\
& C=\{0.03,0.0,0.0\} \\
& y \\
& \text { With } \\
& F=\{0.0,0.04,0.0\} \\
& L \\
& \text { With }=\{0.015,0.02,0.0\}
\end{aligned}
\]

\section*{\(X\) \\ C \\ Z}
1.2

Material properties

\section*{\(=0.75 \mathrm{~W} . \mathrm{m}^{\circ} \mathrm{C}\) thermal Conduction}

CP = 2. Voluminal J.m3 \({ }^{\circ}\) C Heat

\section*{1.3}

Boundary conditions and loadings
- [FE] and [CD]: null flow
- [F]: free convection (H=30 W/m2 \(\left.{ }^{\circ} \mathrm{C}, \mathrm{Te}=140^{\circ} \mathrm{C}\right)\)
\(\cdot[A C]\) : imposed temperature \(\mathbf{T i}=100^{\circ} \mathrm{C}\)
\(\cdot[E D]:\) density flux imposed \(i=-1.200 \mathrm{~W} / \mathrm{m} 2\), (outgoing flow)

\subsection*{1.4 Conditions}
initial
To make this stationary calculation, one makes a transitory calculation (except for modelings \(\boldsymbol{A}\) and G) for
which the boundary conditions are constant in time. This makes it possible to test calculations elementary of mass intervening in the first member as well as the second member.
Handbook of Validation

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
6.0

Titrate:
TPLL01 - Infinite plane wall in linear thermics

Date:
13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

\section*{:}

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\section*{2}

Reference solution

\section*{2.1}

Method of calculation used for the reference solution
\(S\)
\(T(S)=M T+(T-T\)
B
With)

\section*{\(S=A M M\) not running}
\(L\)
\(T\)
R
- \(T\)

B
With
= -
Mr.
\(L\)

\section*{2.2}

Results of reference
Temperatures and flow at the points \(A, B, G\).

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References}

Case test VPCS TPLL01.
Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
HI-23/02/017/A
Code_Aster \({ }^{\circledR}\)
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TPLL01-Infinite plane wall in linear thermics

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling \\ Plan (QUAD4, TRIA3)}

One nets part of the infinite wall, such as the field is a square \(O F=C F=L\) with 4 meshs TRIA3 and 2 meshs QUAD4.
\begin{tabular}{l}
\(E\) \\
\(B\) \\
\(X\) \\
\(y\) \\
\(y\) \\
\(N 1\) \\
\(C\) \\
0.03 \\
0 \\
\(D\) \\
0.07 \\
0.03 \\
\(G\) \\
\(F\) \\
\(D\) \\
\(E\) \\
0.04 \\
0.07 \\
\(N 3\) \\
\(F\) \\
0 \\
0.04 \\
With \\
0.0150 .02 \\
\(N 5\) \\
\(B\) \\
0.055 \\
\(N 1\) \\
With \\
\(G\) \\
0.05 \\
\(N 5\) \\
\(C\) \\
\(X\) \\
0 \\
\hline
\end{tabular}

\section*{A number of meshs and types: 2 QUAD4, 4 TRIA3}

\author{
3.3 Functionalities \\ tested \\ Orders \\ Key word factor \\ Single-ended spanner word \\ Argument \\ AFFE_CHAR_THER \\ FLUX_REP \\ EXCHANGE \\ TEMP_IMPO \\ LIAISON_DDL \\ AFFE_MODELE \\ PLAN \\ THERMICS \\ MACRO_MATR_ASSE \\ SOLVEUR \\ "MULT_FRONT" \\ MATR_ASSE \\ RIGI_THER \\ CALC_VECT_ELEM \\ CHAR_THER \\ ASSE_VECT \\ FACT_LDLT \\ RESO_LDLT \\ CALC_CHAM_ELEM \\ FLUX_ELNO_TEMP \\ FLUX_ELGA_TEMP
}

\subsection*{3.4 Remarks}

To test the key word factor LIAISON_DDL, the linear relation was introduced (checked by the
solution):
\(T(G)-T(B)=40\).

\section*{Handbook of Validation}

V4.02 booklet: Stationary thermics of the linear structures
HI-23/02/017/A
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6.0

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TPLL01-Infinite plane wall in linear thermics

Date:
13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

\section*{:}

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested
Identification Reference
Aster \%
difference
T (A) \({ }^{\circ} \mathrm{C} 100\).
100.00
0.00
\(T(B)^{\circ} C 20\).
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G})^{\circ} \mathrm{C} \mathbf{6 0}\).
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) (m
)
```

2
W/m
960.960.00
0 . 0 0
R(R
m). J (m
)
2
W/m
720.720.00
0 . 0 0

```

\section*{Handbook of Validation}

V4.02 booklet: Stationary thermics of the linear structures HI-23/02/017/A

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
TPLL01-Infinite plane wall in linear thermics

\section*{Date:}

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Author (S):
O. BOITEAU, J. Key PELLET

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\section*{5 Modeling}

B

\section*{5.1}

Characteristics of modeling
Plan (QUAD8, TRIA6)
\(E\)
\(B\)
\(X y\)
```

y
N6
C 0.03
0
G
D 0.07
0 . 0 3
F
D
E 0.04
0 . 0 7
N13
F 00.04
To 0.015
0 . 0 2
N22
With
B 0.055
0 . 0 5
N6
N22
G
0.035 0.035 N13
C
X
0

```
5.2
Characteristics of the grid

A number of nodes: 23
A number of meshs and types: 4 TRIA6, 2 QUAD8

\subsection*{5.3 Functionalities}
tested
Orders
Key word factor
Simple key word
Argument
AFFE_CHAR_THER_F

\author{
FLUX_REP \\ EXCHANGE \\ TEMP_IMPO \\ LIAISON_DDL \\ AFFE_MODELE \\ PLAN \\ THERMICS \\ THER_LINEAIRE \\ CALC_CHAM_ELEM \\ FLUX_ELNO_TEMP \\ FLUX_ELGA_TEMP
}

\subsection*{5.4 Notice}

To test the key word factor LIAISON_DDL, the linear relation was introduced (checked by the solution)
\(T(G) T(B)=40\).

\section*{6}

Results of modeling B

\subsection*{6.1 Values}
tested
Identification Reference
Aster \%
difference
T(A) \({ }^{\circ} \mathrm{C} 100\).
100.00
0.00
\(T(B)^{\circ} C 20\).
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \mathrm{C} \mathbf{6 0}\).
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) ( \(m\)
)
2
W/m
960. 960.00
0.00
```

R(R
m). J (m
)
2
W/m
720.720.00
0 . 0 0
Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
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```
Code_Aster \({ }^{\circledR}\)
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TPLL01-Infinite plane wall in linear thermics

\section*{Date:}

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Author (S):
O. BOITEAU, J. Key PELLET
:

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\section*{7 Modeling}

C
7.1

Characteristics of modeling
Plan (QUAD8, TRIA6)
\(X\)
\(y\)
B
\(y\)
C
0.03

0
With

0.0150 .02

N24
B
0.0550 .05

N6
With
G
0.035 0.035 N14

N24
C
\(X\)
0

\section*{7.2}

Characteristics of the grid
A number of nodes: 25
A number of meshs and types: 4 TRIA6, 2 QUAD9

\author{
7.3 Functionalities \\ tested \\ Orders \\ Key word factor \\ Simple key word \\ Argument \\ AFFE_CHAR_THER_F
}

\author{
FLUX_REP \\ EXCHANGE \\ TEMP_IMPO \\ AFFE_MODELE \\ PLAN \\ THERMICS \\ THER_LINEAIRE \\ CALC_CHAM_ELEM \\ FLUX_ELNO_TEMP \\ FLUX_ELGA_TEMP
}

\author{
8 \\ Results of modeling \(C\) \\ \subsection*{8.1 Values} \\ tested \\ \section*{Identification Reference} \\ Aster \% \\ difference \\ \(\boldsymbol{T}(A){ }^{\circ} \boldsymbol{C}\) \\ 100. \\ 100.00 \\ 0.00 \\ \(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \mathrm{C}\) \\ 20. \\ 20.00 \\ 0.00 \\ \(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \mathrm{C}\) \\ 60. \\ 60.00 \\ 0.00 \\ \(\boldsymbol{R}\) ( \(\boldsymbol{R}\) \\ m). \(i\) (m \\ ) \\ 2 \\ W/m \\ 960. 960.00 \\ 0.00 \\ \(\boldsymbol{R}\) ( \(\boldsymbol{R}\) \\ \(m) . J(m\)
}
)
2W/m720. 720.00
0.00
Handbook of Validation
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Date:
13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

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\section*{9 Modeling}

D

\section*{9.1}

Characteristics of modeling
Voluminal (HEXA8)

\section*{E}
\(X y Z\)
N6
\(y\)
\(B\)
C 0.03
00
D
0.070 .030

E
```

0.040.070
F
G
N16
D
F 00.04
0
To 0.015
0 . 0 2
0 N20
B 0.055
0 . 0 5
0 N6
N20
With
G
0.0350.0350
N16
C
X
Z 0

```

\section*{9.2}
```

Characteristics of the grid
A number of nodes: 21
A number of meshs and types: 4 HEXA8 + 20 QUAD4

```

\subsection*{9.3 Functionalities}
```

tested
Orders
Key word factor
Simple key word
Argument
AFFE_CHAR_THER_F
FLUX_REP
EXCHANGE
TEMP_IMPO
AFFE_MODELE
3D
THERMICS

```

\author{
THER_LINEAIRE \\ CALC_CHAM_ELEM \\ FLUX_ELNO_TEMP \\ FLUX_ELGA_TEMP \\ INTE_MAIL_3D \\ POSt_RELEVE
}

\section*{10 Results of modeling \(D\)}
10.1 Values
tested
Identification Reference
Aster \%
difference
T(A) \({ }^{\circ} \mathrm{C}\)
100.
100.00
0.00
\(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \mathrm{C}\)
20.
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \mathrm{C}\)
60.
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) ( \(m\)
)
2
W/m
960. 960.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
\(m) . ~ J(m\)
)
2
W/m
720. 720.00
0.00

\title{
Handbook of Validation \\ V4.02 booklet: Stationary thermics of the linear structures \\ HI-23/02/017/A
}

Code_Aster \({ }^{\circledR}\)
Version
6.0

Titrate:
TPLL01 - Infinite plane wall in linear thermics

Date:
13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

\section*{:}

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\section*{11 Modeling}

E
11.1 Characteristics of modeling

Voluminal (PENTA6)
E
X y Z
N10
\(y\)
B
C 0.03
00
D
0.070 .030

E
0.040 .070

F
G
N11
D
F 00.04

0
To 0.015
0.02

0 N12
B 0.055
0.05

0 N10
N12
With
G
0.0350 .0350

N11
C
\(X\)
Z 0

\subsection*{11.2 Characteristics of the grid}

A number of nodes: 21
A number of meshs and types: 8 PENTA6 + 8 TRIA3 + 16 QUAD4

\subsection*{11.3 Functionalities}
tested
Orders
Key word factor
Simple key word
Argument
```

AFFE_CHAR_THER_F
FLUX_REP
EXCHANGE
TEMP_IMPO
AFFE_MODELE
3D
THERMICS

```

\author{
THER_LINEAIRE
}

CALC_CHAM_ELEM
FLUX_ELNO_TEMP
FLUX_ELGA_TEMP

\section*{12 Results of modeling \(E\)}

\subsection*{12.1 Values}
tested
Identification Reference
Aster \%
difference
\(\boldsymbol{T}(\boldsymbol{A}){ }^{\circ} \boldsymbol{C}\)
100.
100.00
0.00
\(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \mathrm{C}\)
20.
20.00
0.00
\(\boldsymbol{T}(G){ }^{\circ} \mathrm{C}\)
60.
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) ( \(m\)
)
2
W/m
960. 960.00
0.00
\(\boldsymbol{R}(\boldsymbol{R}\)
\(m) . J(m\)
)
2
W/m
720. 720.00
0.00

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Version
6.0

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TPLLO1 - Infinite plane wall in linear thermics

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\section*{13 Modeling}

F

\subsection*{13.1 Characteristics of modeling}

Voluminal (HEXA20)
\(E\)
\(X\)
\(y\)
\(Z\)
\(N 16\)
\(y\)
\(B\)
\(C\)
0.03
0
0
\(N 45\)
\(D\)
0.07
0.03
0
\(E\)
0.04
0.07
0

0
```

F
G
D
F
0
0.04
0
With
0.015 0.02
0
N57
N57
B
0.055 0.05
0
N16
With
G
0.035 0.0350
N45
C
X
Z 0

```
13.2 Characteristics of the grid

A number of nodes: 59
A number of meshs and types: 4 HEXA20 + 20 QUAD8

\subsection*{13.3 Functionalities}
tested
Orders
Key word factor
Simple key word
Argument
AFFE_CHAR_THER_F
FLUX_REP
EXCHANGE
TEMP_IMPO

\title{
AFFE_MODELE
}

PLAN
THERMICS

\section*{THER_LINEAIRE}

CALC_CHAM_ELEM
FLUX_ELNO_TEMP
FLUX_ELGA_TEMP

14 Results of modeling \(F\)

\subsection*{14.1 Values}
tested
Identification Reference
Aster \%
difference
\(\boldsymbol{T}(\mathrm{A}){ }^{\circ} \mathrm{C}\)
100.
100.00
0.00
\(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \boldsymbol{C}\)
20.
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \boldsymbol{C}\)
60.
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) ( \(m\)
)
2
W/m
960. 960.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
\(m) . J(m\)
)
2

\title{
Code_Aster \({ }^{\circledR}\)
}

Version
6.0

Titrate:
TPLL01 - Infinite plane wall in linear thermics

Date:
13/09/02
Author (S):
O. BOITEAU, J. Key PELLET

\section*{:}

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15 Modeling
G

\subsection*{15.1 Characteristics of modeling}

Voluminal (PENTA15)
E
X y Z
N28
\(y\)
B
C 0.03
00
D
0.070 .030

E
0.040 .070

F
\begin{tabular}{|c|}
\hline \(\boldsymbol{G}\) \\
\hline N52 \\
\hline D \\
\hline F 00.04 \\
\hline 0 \\
\hline To 0.015 \\
\hline 0.02 \\
\hline 0 N61 \\
\hline B 0.055 \\
\hline 0.05 \\
\hline 0 N28 \\
\hline N61 \\
\hline With \\
\hline \(G\) \\
\hline 0.0350 .0350 \\
\hline N52 \\
\hline C \\
\hline \(\boldsymbol{X}\) \\
\hline Z 0 \\
\hline
\end{tabular}

\subsection*{15.2 Characteristics of the grid}

A number of nodes: 65

A number of meshs and types: 8 PENTA15 + 8 TRIA6 + 16 QUAD8
15.3 Functionalities
tested

Orders
Key word factor
Simple key word
Argument
AFFE_CHAR_THER
FLUX_REP
EXCHANGE
TEMP_IMPO
AFFE_MODELE
3D
THERMICS
CALC_MATR_ELEM
OPTION
"RIGI_THER"

\author{
NUME_DDl \\ ASSE_MATRICE \\ CALC_VECT_ELEM \\ OPTION \\ "CHAR_THER" \\ ASSE_VECTEUR \\ FACT_LDLT \\ RESO_LDLT \\ PRE_GIBI \\ CALC_CHAM_ELEM \\ FLUX_ELNO_TEMP \\ FLUX_ELGA_TEMP
}

\section*{16 Results of modeling \(G\)}

\subsection*{16.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
\(\boldsymbol{T}(\mathrm{A}){ }^{\circ} \mathrm{C}\)
100.
100.00
0.00
\(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \mathrm{C}\)
20.
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \mathrm{C}\)
60.
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) ( \(m\)
)
2
W/m
960. 960.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). J (m
)
2W/m
\[
\text { 720. } 720.00
\]
\[
0.00
\]

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17 ModelingH
17.1 Characteristics of modeling
Voluminal (TETRA4)
E
\(X y Z\)
N2
\(y\)BC 0.03
00
D0.070 .030
E


\section*{1}

CALC_CHAM_ELEM
FLUX_ELNO_TEMP
FLUX_ELGA_TEMP

18 Results of modeling \(\boldsymbol{H}\)
18.1 Values
tested
Identification Reference
Aster \%
difference
\(\boldsymbol{T}(\mathrm{A}){ }^{\circ} \mathrm{C}\)
100.
100.00
0.00
\(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \mathrm{C}\)
20.
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \mathrm{C}\)
60.
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) (m
)
2
W/m
960. 960.00 0.00
\(\boldsymbol{R}(\boldsymbol{R}\)
m). J (m
)
2
W/m
720. 720.00 0.00

\section*{Handbook of Validation}

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\author{
19 Modeling \\ I \\ 19.1 Characteristics of modeling
}

Voluminal (TETRA10)
E
B
\(X\)
\(y\)
\(y\)
N09
C
0.03

0
D
0.07
0.03

G
F
D
E
0.04
0.07

N04
F
0
0.04
With
0.0150 .02
N01
B
0.0550 .05
N09
With
G
0.035 0.035 N04
N01
C
X
0
19.2 Characteristics of the grid
A number of nodes: 125
A number of meshs and types: 48 TETRA10 + 16 TRIA6
19.3 Functionalities
tested
Orders
Key word factor
Simple key word
Argument
AFFE_CHAR_THER_F
FLUX_REP
EXCHANGE
TEMP_IMPO
AFFE_MODELE
3D
THERMICS
THER_LINEAIRE
PARM_THETA
1.0
CALC_CHAM_ELEM
FLUX_ELNO_TEMP
FLUX_ELGA_TEMP

\section*{20 Results of modeling I}

\subsection*{20.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
\(\boldsymbol{T}(\mathrm{A}){ }^{\circ} \mathrm{C}\)
100.
100.00
0.00
\(\boldsymbol{T}(\boldsymbol{B}){ }^{\circ} \boldsymbol{C}\)
20.
20.00
0.00
\(\boldsymbol{T}(\boldsymbol{G}){ }^{\circ} \boldsymbol{C}\)
60.
60.00
0.00
\(\boldsymbol{R}\) ( \(\boldsymbol{R}\)
m). \(i\) (m
)
2
W/m
960. 960.00
0.00
\(\boldsymbol{R}(\boldsymbol{R}\)
m). J (m
)
2
W/m
720. 720.00
0.00

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\section*{21 Modeling}

J

\subsection*{21.1 Characteristics of modeling}

It is about a case functional test and of data-processing not-regression of the calculation of the indicator of error has
established posteriori in thermics (cf [R4.10.03]). It exhumes a cartography of space error on which will rest, in a loop PYTHON, the tool of refinement/déraffinement LOBSTER encapsulated in MACR_ADAP_MAIL (cf [U7.03.01]).
The calculation of this chart of indicator of error is carried out, via option "ERTH_ELEM_TEMP" of the operator
of postprocessing CALC_ELEM, on a EVOL_THER (provides to the key word RESULT) coming from one
former thermal calculation (linear or not, transient or stationary, isotropic or orthotropic, via THER_LINEAIRE or THER_NON_LINE, cf environment necessary, parameter setting and perimeter
of use [R4.10.03] §6.2/4).
This calculation requires as a preliminary the recourse to option "FLUX_ELNO_TEMP" of
CALC_ELEM which determines
values of the vector heat flux to the nodes (cf example of use [R4.10.03] §6.5).
The indicator consists of fifteen components per element and for a given moment. In this case test, one calculates the fifteen components but the procedure of refinement/déraffinement does not rest
that on the component ERTABS which represents the absolute total space error (cf [R4.10.03] §6.3).
In order to be able post-to treat via POST_RELEVE or GIBI, one needs to extrapolate fields by element in fields with the nodes by element. The addition of option
"ERTH_ELNO_ELEM" (afterwards
the call to "ERTH_ELEM_TEMP") makes it possible to carry out this purely data-processing transformation. For one
moment and a given finite element, it does nothing but duplicate the fifteen components of the indicator on
each node of the element.
This modeling thus constitutes as much an example of use, in a loop PYTHON, possible couplings "calculation of indicator"/"refinement/déraffinement of grid", that a case test of not-regression of options "ERTH_ELEM_TEMP" and "ERTH_ELNO_ELEM" and of their adherence
with the process of mending of meshes.
This case test takes again the characteristics of modeling I and its grid (TETRA10 + TRIA6) associated.

\subsection*{21.2 Functionalities}
tested
Orders

AFFE_CHAR_THER_F
FLUX_REP
EXCHANGE
TEMP_IMPO
AFFE_MODELE
3D
THERMICS
THER_LINEAIRE
CALC_ELEM
FLUX_ELNO_TEMP
ERTH_ELEM_TEMP
ERTH_ELNO_ELEM
MACR_ADAP_MAIL
REFINEMENT

22 Results of modeling \(J\)
22.1 Values
tested
One tests the data-processing not-regression of component ERTREL (relative total space error) of
the indicator of error compared to the V6.2.1 versions of platforms SGI and SUN of Code_Aster and of
the V4.3 version of the software LOBSTER. The relative tolerance is thus severe: \(5.106 \%\).

\section*{Identification \\ Aster}

Tolerance
Value of ERTREL on mesh MA1
4.15918735 10-5 5.10-8
before mending of meshes
Value of ERTREL on node NO4
4.15918735 10-5 5.10-8
before mending of meshes
Value of ERTREL on the M1 mesh
5.48408914 10-6 5.10-7
after mending of meshes
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\section*{23 Summary of the results}

The field solution (linear) belongs to the space of interpolation of all the elements tested. results are thus naturally excellent.

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Code_Aster \({ }^{\circledR}\)
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\section*{Titrate:}

TPLL100 - Anisotropic plane wall in stationary thermics
Date:
23/09/02
Author (S):
C. Key DURAND
:
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Organization (S): EDF/AMA

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TPLL100-Anisotropic plane wall in thermics stationary

\section*{Summary:}

This test the purpose of which relates to it thermal linear stationary and transitory be to validate the anisotropy
Cartesian.
Two modelings are carried out:
- a first into voluminal,
- a second in plan.

The results obtained are in perfect agreement with the analytical values.
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V4.02 booklet: Stationary thermics of the linear structures
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\section*{1}

Problem of reference
1.1 Geometry
```

H
Z 1
D
X1
Y
G1
F
X
With
0
C

```

In the reference mark (X0, Y0, Z0), the points have as co-ordinates:
```

C (0.03; 0; 0)
D (0.07; 0.03; 0)
E (0.04; 0.07; 0)
F (0; 0.04; 0)
To (0.015; 0.02;0)
B (0.055; 0.05; 0)
G (0.035; 0.035; 0)

```
\(F K=C H=D I=E J=0,05 . Z 0\)
CD \(X=r a d\)
Z //Z
1)
0
1
4

\section*{1.2}

Material properties
Anisotropic material, direction privileged along the axes of the reference mark (X1, Y1, Z1):
\(X=1 W^{\circ}\)
\(m C\)
\[
\begin{aligned}
& Y=0.5 \mathrm{~W} /{ }^{\circ} \\
& m C \\
& Z=2 \mathrm{~W} / \circ \\
& m C \\
& C=2 \mathrm{~J} / \mathrm{m} 3 \\
& { }^{\circ} C \\
& p \\
& 1.3 \\
& \text { Boundary conditions and loadings }
\end{aligned}
\]
face FEJK: Outgoing flow of 400 W/m2.
face CDIH: Entering flow of 400 W/m2.
face EDIJ: Outgoing flow of 1.200 W/m2.
face FCHK : Imposed temperature \(100^{\circ} \mathrm{C}\).
Others faces: condition of Neumann.

\subsection*{1.4 Conditions \\ initial}

To make this stationary calculation, a transitory calculation is made for which the boundary conditions are constants in time. This makes it possible to test elementary calculations of mass and rigidity intervening in the first member as well as the second member.

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C. Key DURAND

\section*{2 \\ Reference solution}

\section*{2.1}

Method of calculation used for the reference solution

\section*{Analytical solution.}

Temperature varying linearly according to CD.
Isotherms parallel with faces CHKF and DIJE.

\section*{CD CH CF}
:
locate
```

T

```
= - -
\(y\)
()
cos
\(\sin\)
\(X\)
\(Z\)
0
:
with
\(X=1200 y=400=(X, 1 C D)-\)
\(T(X)\)
\(X\)
=
\(X+T(A)\)
2
2
\(X \cos +Y\)
\(\sin\)
1
that is to say \(T(X)=-\)
\(1600 X+20\)
. \(X=\cos X-\sin y\)
720
that is to say
O
if \(=(C D, X o) .=\sin +\) cos
720
that is to say
Y
\(X\)
\(Z\)
O
2.2

Results of reference
Temperature at the points \(\boldsymbol{A}, \boldsymbol{B}, \boldsymbol{G}\).
Flow following the directions Xo and Yo.
\(T(A)=100\)
\(T(B)=20\)
\(T(G)=60\)
\(X=720\)
\(\boldsymbol{Y}\)
O
\(O=1040\)

\subsection*{2.3 References \\ bibliographical}
[1]
NR. RICHARD: Note technical HM-18/94/0011, "Development of the thermal anisotropy in the software Aster ".

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}
diagram in time, forced on 1 to test the calculation of the second member.
4 elements 3D, HEXA8.

\section*{3.2 \\ Characteristics of the grid}

4 Hexa 8.

\author{
3.3 Functionalities \\ tested \\ Orders \\ Key word factor \\ Single-ended spanner word \\ Argument \\ DEFI_MATERIAU \\ THER_ORTH \\ AFFE_CARA_ELEM
}
```

ANGL_REP
--
NET
AFFE_MODELE
AFFE
MODELING
"3D"
THER_LINEAIRE
TEMP_INIT
STATIONARY
"YES"
INCREMENT
LIST_INST
--
PARM_THETA
l.
--
CARA_ELEM
CALC_NO
OPTION
"FLUX_ELNO_TEMP"
CALC_CHAM_ELEM
--
CARA_ELEM
OPTION
"FLUX_ELNO_TEMP"
OPTION
"FLUX_ELGA_TEMP"
4
Results of modeling A

```

\subsection*{4.1 Values}
```

tested

```

\section*{Identification Reference}
```

Aster \%
0.
0 .
*: imposed temperature

### 4.2 Remarks

The analytical solution being of order 1 and the field represented by the discretization, the code finds, with the errors rounding close, this solution.

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## 5 Modeling

B

## 5.1 <br> Characteristics of modeling

Similar to the modeling $A$, but solved in $2 D$ in plan $C D E F$.

## 5.2 <br> Characteristics of the grid

4 QUAD 4.

### 5.3 Functionalities

tested

## Orders

Key word factor
Single-ended spanner word
Argument
DEFI_MATERIAU
THER_ORTH
AFFE_CARA_ELEM
ANGL_REP
--
NET
AFFE_MODELE
AFFE
MODELING
"PLANE"
THER_LINEAIRE
TEMP_INIT
STATIONARY
"YES"
INCREMENT
LIST_INST
PARM_THETA
1.

CARA_ELEM

CARA_ELEMOPTION

"FLUX_ELNO_TEMP"
OPTION
"FLUX_ELGA_TEMP"
6
Results of modeling B
6.1 Valuestested
Identification Reference
ASTER \%
difference
$T$ (A) N5 *$100^{\circ}$$100^{\circ}$
0.
T(B) N2
$20^{\circ} \mathrm{C}$
$20^{\circ} \mathrm{C}$
0 .$T(G) N 8$$60^{\circ} \mathrm{C}$
$60^{\circ} \mathrm{C}$

0. 

Xo
7207200.
Yo
10401040
0 .
*: imposed temperature

### 6.2 Remarks

The analytical solution being of order 1 and the field represented by the discretization, the code finds, with the errors rounding close, this solution.
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## 7

## Summary of the results

Key word ANGL_REP introduced into order AFFE_CARA_ELEM is thus tested in $3 D$ and plane $2 D$ on an anisotropic problem of thermics.
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V4.02 booklet: Stationary thermics of the linear structures
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLL101 Joule effect heating of a hollow roll
Date:
19/10/01
Author (S):

## Summary:

One imposes electrical currents inside and outside a hollow roll finite length, then one calculate the temperature established under the effect of a heat source produced by Joule effect. The solution of reference is analytical.

The applicability is it thermal linear stationary.
The model is axisymmetric.
The test is carried out on elements QUAD8 and TRIA6.

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5.0

Titrate:
TPLL101 Joule effect heating of a hollow roll
Date:
19/10/01
Author (S):
Key S. TAHERI
:
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1
Problem of reference

### 1.1 Geometry

Axisymmetric

## Z

D
C
R1
RO
$+M$
R
With
B

WITH B
C
D
M
R 1.
2.7182

### 1.2 Properties

of
materials

Electric characteristic: electric conductivity $=1$.
11
m
Thermal characteristics: $=2.102 \mathrm{~W} / m^{\circ} C C P=0$.
1.3

Boundary conditions and loadings
Electric calculation:
$j . n=10$.
on DA
j. $n=$.

36787944 on BC

Thermal calculation
$T=0$. on $D A$
$T=0$. on $B C$
$F L O W=0$. on $A B$
$F L O W=0 . o n C D$

### 1.4 Conditions

initial
Stationary calculation.
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2
Reference solution

```
2.1
Method of calculation used for the reference solution
- Problème elastostatic V potential electric
In volume \(V=0\).
\(j . n=\).
0
on \(C D\) and \(A B\)
```

Boundary conditions NEUMANN j. $n=10$.
on $A D$
j.n
=.
36787944 on BC
electric conductivity $=1$.
$j . n=V$
Axisymmetric solution

```
I
V
R
R
=0 V=0
VL
R R R
og
With
```

The boundary conditions on $A D$ and $B C$ impose:
$V 0=10$.
Note:
The knowledge of A is not necessary for thermal calculation.

- Thermal Problème T the temperature
$T=S$ with a voluminal source $S=(V) 2$
Boundary conditions: $\boldsymbol{T}=\mathbf{0}$. on DA and BC .
$T N=0$ on cd. and $A B$
Axisymmetric solution:
1
2
$T$
v0
$\boldsymbol{R}$
= -
Taking into account the boundary conditions
$\boldsymbol{R} \boldsymbol{R} \boldsymbol{R}$
12
R
2
$v$
$\boldsymbol{R}$
R
$T(R)$

1
0
= -
$L$
$L$
2
og
og
0
$\boldsymbol{R}$
1
$\boldsymbol{R}$
2.2
Results of reference
$T=5.889313102$ (temperature at the point M).
2.3
Uncertainty on the solution
Analytical solution.
Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
5.0
Titrate:
TPLL101 Joule effect heating of a hollow roll
Date:
19/10/01
Author (S):
Key S. TAHERI
:
V4.02.101-B Page:
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## 3 Modeling

With

3.1<br>Characteristics of modeling<br>Element THAXTR6<br>Not M: N200<br>\subsection*{3.2 Characteristics}<br>grid<br>A number of nodes: 305<br>A number of elements: 120

3.3 Functionalities<br>tested<br>Orders<br>Key word factor<br>Single-ended spanner word<br>Argument<br>AFFE_MODELE<br>AFFE<br>MODELING AXIS<br>\% calculation electric

AFFE_CHAR_THER FLUX_REP

## THER_LINEAIRE

## CALC_CHAM_ELEM OPTION "SOUR_ELGA_ELEC"

\% calculation thermal

## FLUX_REP

SOURCE<br>SOUR_CALCULEE<br>THER_LINEAIRE

```
4
Results of modeling \(A\)
```


### 4.1 Values

tested

## Identification Reference

Aster \%

difference
Temperature at the point Mr. N200
5.8893131025 .891298

102
0.034

### 4.2 Remarks

The boundary conditions of the electric problem are all of the conditions of NEUMANN. Nevertheless, the analytical solution is found.

### 4.3 Parameters <br> of execution

Version: 5.4.17

Machine: SGI O-2000

## System:

## IRIX 64

Obstruction memory:
32 Mo
Time CPU To use:
2.0 seconds

Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLL101 Joule effect heating of a hollow roll
Date:
19/10/01
Author (S):
Key S. TAHERI
: $V 4.02 .101-B$ Page:
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5 Modeling
B
5.1

Characteristics of modeling
Element THAXQU8
Not M: N186

### 5.2 Characteristics

grid
A number of nodes: 245
A number of elements: 60

### 5.3 Functionalities

Orders

Key word factor
Single-ended spanner word
Argument
AFFE_MODELE
AFFE
MODELING AXIS
\% calculation electric

AFFE_CHAR_THER FLUX_REP<br>THER_LINEAIRE

CALC_CHAM_ELEM OPTION "SOUR_ELGA_ELEC"<br>\% calculation thermal<br>\title{ AFFE_CHAR_THER TEMP_IMPO }

FLUX_REP

## SOURCE <br> SOUR_CALCULEE

THER_LINEAIRE

6
Results of modeling B

### 6.1 Values

tested
Identification Reference Aster \%differenceTemperature at the point Mr. N186
5.8893131025 .892292
102
0.051
6.2 Parameters
of execution
Version: 5.4.17
Machine: SGI O-2000
System:
IRIX 64
Obstruction memory:
64 Mo
Time CPU To use:
2.00 seconds
Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
5.0
Titrate:
TPLL101 Joule effect heating of a hollow roll
Date:
19/10/01
Author (S):
Key S. TAHERI
:V4.02.101-B Page:6/6
7
Summary of the results
In addition to the test presented, one carried out a calculation on structure (COTHAA). The resultsobtained have
summer compared with those obtained using Code CASTEM 2000. One obtains results very close relations.
Handbook of Validation
V4.02 booklet: Stationary thermics of the linear structures
HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:
1/8
Organization (S): EDF/IMA/MMN

## Handbook of Validation

V4.03 booklet: Stationary thermics of the plates and the hulls
Document: V4.03.100
TPLS100 - Infinite plate subjected to flows
antisymmetric
Summary:
The purpose of this test is to test the model of thermal hull linear with three fields (MODELING:
"HULL" or "COQUE_PLAN") by comparison with an analytical solution, for an infinite plate subjected to one
couple stationary antisymmetric heat flows on its two half-faces, in stationary regime.
The equation of heat is solved in hover, with a linear, isotropic, homogeneous conduction.
Two modelings: With for the finite elements of surface hull (triangles) and B for the linear elements (segments).
One simultaneously tests in each modeling the elementary orders and the total order
THER_LINEAIRE [U4.33.01].
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:
2/8
1
Problem of reference
1.1 Geometry

Z
y
F
E
D
O
X
B
With
B
C
L
Length: $\mathrm{L}=20 \mathrm{~mm}$
Width: $\mathrm{B}=2 \mathrm{~mm}$
Thickness: $\mathrm{H}=4 \mathrm{~mm}$
1.2

Material properties
Conductivity $=4.5 \mathrm{~W} / \mathrm{mm} /{ }^{\circ} \mathrm{C}$
Voluminal heat: $(\mathrm{CP}=0) .\mathrm{J}\left({ }^{\circ} \mathrm{C} . \mathrm{mm} 3\right)$
1.3

Boundary conditions and loadings
Null temperature on the average layer of the plate:
TEMP $=0$.
Flow imposed on the upper surface (ABEF) + flux $+=30 . \mathrm{W} / \mathrm{mm} 2 /{ }^{\circ} \mathrm{C}$
Flow imposed on the upper surface (ABEF) -
flow $=30 . \mathrm{W} / \mathrm{mm} 2 /{ }^{\circ} \mathrm{C}$
Handbook of Validation

V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:
3/8
2
Reference solution
2.1

Method of calculation used for the reference solution
Analytical
For more details to refer to the document [R3.11.01] and the note [bib1].

## 2.2

Results of reference

- Température in higher skin,
- Flux in nodes placed on axis OX in higher skin.
2.3

Uncertainty on the solution
Analytical solution.

### 2.4 References <br> bibliographical

[1]
S. ANDRIEUX, F. VOLDOIRE HI-71/7131 - Formulation of a model of thermics for thin hulls (7/12/90).
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ©

Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)P. MASSIN, F. VOLDOIRE, A.M. DONORE
Key:
V4.03.100-C Page:
4/8
3 Modeling
With
3.1
Characteristics of modeling
yX
N176
N161
N1
N32
N46
X
Cutting:
28 elements in length,
8 elements in width.
Boundary conditions - loading:
TEMP_IMPO (ALL: "yes", TEMP: 0.)
FLUX_REP (Group_Ma: GRSD2,
FLUN_INF: - 30.
FLUN_SUP: 30.)
3.2
Characteristics of the gridA number of nodes: 969
A number of meshs and types: 448 meshs TRIA6
3.3 Functionalities
tested
Orders
KeysAFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU
THER
[U4.23.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"THERMAL"
"HULL"
ALL
[U4.22.01]
AFFE_CHAR_THER_F
TEMP_IMPO
FLUX_REP
[U4.25.02]
CALC_MATR_ELEM
RIGI_THER
[U4.41.01]
CALC_MATR_VECT
CHAR_THER
[U4.42.01]
NUME_DDL
[U4.42.02]
ASSE_MATRICE
[U4.41.02]
ASSE_VECTEUR
[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.52.02]
AFFE_CHAR_THER
TEMP_IMPO
ALL
[U4.25.02]
FLUX_REP
GROUP_MA
THER_LINEAIRE
CARA_ELEM
[U4.33.01]
CALC_ELEM
RESULT
[U4.61.02]
NIVE_COUCHE
CARA_ELEM
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:
5/8
4
Results of modeling A
4.1 Values
tested
Identification
Reference
Aster
\% difference
TEMP_SUP:
node N201 $(10 ., 0)$
13.3321
13.3310
0.008
node N176 $(5.15,0)$
13.2565
13.2564
0.001
node N161 (2.8, 0)
12.7462
12.7458
0.003
node N1 (0. , 0.)
6.6666
6.6666
0.001
node N32 $(28,0.25)$
0.5870
0.5872
0.027
node N46 $(5.15,0)$
0.07679
0.07693
0.177

Flow component X:
N176 node
net M297
0.2992
0.2907
2.84
net M289
0.2992
0.2780
7.09
net M290
0.2992
0.2799
6.44
average value
0.2992
0.283
5.46

N161 node
net M265
2.287
2.186
4.43
net M266
2.287
2.197
3.92
net M273
2.287
2.242
1.96
average value
2.287
2.208
3.44

N1 node
net M1
25.98
25.94
0.15
net M225

## Contents of the file results

- Températures with the nodes,
- heat flow to the nodes in higher wall, - values tested deferred above.


### 4.2 Parameters

of execution
Version: 3.06
Machine: CRAY C90
System UNICOS:

## 8.4

Obstruction memory:
8 megawords
Time CPU To use:
37.68 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:
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5 Modeling
B

## 5.1

## Characteristics of modeling

y
N201
N1
N71
X
10
$+10$
0
Cutting: 28 linear elements in length, (even progression of meshs that in the grid modeling A).
Boundary conditions - loading:
TEMP_IMPO (ALL: "yes", TEMP: 0.)
FLUX_REP (Group_Ma: GRSD2,
FLUX_INF: - 30.
FLUX_SUP: 30.)
5.2

## Characteristics of the grid

A number of nodes: 57
A number of meshs and types: 28 meshs SEG3

### 5.3 Functionalities

## tested

Orders

## Keys

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU
THER
[U4.23.01]
AFFE_MATERIAU
ALL
[U4.23.02]
AFFE_MODELE
"THERMAL"
"COQUE_PLAN"
ALL
[U4.22.01]
AFFE_CHAR_THER
TEMP_IMPO
ALL

```
[U4.25.02]
FLUX_REP
GROUP_MA
AFFE_CHAR_THER_F
TEMP_IMPO
ALL
[U4.25.02]
FLUX_REP
GROUP_MA
CALC_MATR_ELEM
RIGI_THER
[U4.41.01]
CALC_MATR_VECT
CHAR_THER
[U4.42.01]
NUME_DDL
[U4.42.02]
ASSE_MATRICE
[U4.41.02]
ASSE_VECT
[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.52.02]
THER_LINEAIRE
CARA_ELEM
[U4.33.01]
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
```


## Code_Aster ®

Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:

Results of modeling B
6.1 Values
tested

## Identification

## Reference

## Aster

\% difference
TEMP_SUP:
node N201 (10. , 0)
13.3321
13.331
0.008
node N176 $(5.15,0)$
13.2565
13.2565
node N161 $(2.8,0)$
12.7462
12.7462
node $\mathrm{N} 1(0 ., 0$.
6.6666
6.6666
0.001
node N46 (5.15, 0)
0.07679
0.07683
0.05

## Contents of the files results

- Températures with the nodes on the upper surface, - values tested deferred above.


### 6.2 Parameters

of execution
Version: 3.06
Machine: CRAY C90
System UNICOS:
8.4

Obstruction memory:
8 megawords
Time CPU To use:
5.2 seconds

## Code_Aster © ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS100 Plates infinite subjected to antisymmetric flows
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE, A.M. DONORE

Key:
V4.03.100-C Page:
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7
Summary of the results
In modeling A with meshs TRIA6, one notes that the variations on flows are lower than
$1 \%$, except in the zones where those are very weak.
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:

## P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
1/14
Organization (S): EDF/IMA/MMN
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
Document: V4.03.101
TPLS101 - Infinite plate subjected to an exchange
symmetrical thermics with outside
Summary:

The purpose of this test is to test the model of thermal hull linear with three fields per comparison with analytical solution, for an infinite plate subjected to a couple of conditions of heat exchange with outside, symmetrical compared to the average layer. The equation of heat is solved in hover, with a linear, isotropic, homogeneous conduction.
The results are presented for the finite elements available of thermal surface hull triangles and quadrangles.
Compared to test TPLS100 [V4.03.100], this one makes it possible to check the contribution of the coefficients of exchange to
thermal rigidity, like various methods of assignment of the boundary conditions. Moreover, the solution is such as the temperature is uniform in the thickness.
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)

## P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
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1
Problem of reference
1.1 Geometry

Z
y
F
E
D
O
X
B
With
B
C
L
Length: $\mathrm{L}=20 \mathrm{~mm}$
Width: $\mathrm{B}=2 \mathrm{~mm}$

Thickness: $\mathrm{H}=4 \mathrm{~mm}$
1.2

Material properties
Conductivity $=1000 \mathrm{~W} / \mathrm{mm} /{ }^{\circ} \mathrm{C}$

## 1.3

## Boundary conditions and loadings

- Null Température at the point O , on all the thickness.
- On the higher faces (ABEF) + and lower (ABEF):
coefficient of exchange: $\mathrm{H}=10 \mathrm{~W} / \mathrm{mm} 2 /{ }^{\circ} \mathrm{C}$
outside temperature: Text $=50^{\circ} \mathrm{C}$
- On the faces higher (BCDE) + and lower (BCDE):
coefficient of exchange: $\mathrm{H}=10 \mathrm{~W} / \mathrm{mm} 2 /{ }^{\circ} \mathrm{C}$
outside temperature: Text $=50^{\circ} \mathrm{C}$
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A


## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

## Key:

V4.03.101-C Page:
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2

## Reference solution

## 2.1

Method of calculation used for the reference solution

## Analytical

For more details to refer to the document [R3.11.01] and the note [bib1].

## 2.2

## Results of reference

Temperature in higher, lower skin and average layer.

## 2.3

## Uncertainty on the solution

Analytical solution.

### 2.4 References <br> bibliographical

## [1]

S. ANDRIEUX, F. VOLDOIRE HI-71/7131 - Formulation of a model of thermics for thin hulls (7/12/90).

Handbook of Validation

V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
4/14
3 Modeling
With

## 3.1

Characteristics of modeling
y
N201
X
Cutting:
28 elements in length,
8 elements in width.
Boundary conditions - loading (three calculations for three choices):

- calculation a: scalar loadings and dualisation of the condition of Dirichlet:

TEMP_IMPO (NODE: N1, TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)
EXCHANGE: (GROUP_MA: GRSD1, COEF_H_SUP: 10. , COEF_H_INF: 10.,
TEMP_EXT_SUP: -50., TEMP_EXT_INF: -50.)
(GROUP_MA: GRSD2, COEF_H_SUP: 10. , COEF_H_INF: 10. ,
TEMP_EXT_SUP: 50. , TEMP_EXT_INF: 50.)

- calculation b: loadings constant functions and dualisation of the condition of Dirichlet:
as above, but with constant functions having same values.
- calculation C: scalar loadings and "kinematic" loading:

THER_IMPO: (NODE: N1, TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)

## 3.2

## Characteristics of the grid

A number of nodes: 969
A number of meshs and types: 448 meshs TRIA6

### 3.3 Functionalities

## tested

## Orders

## Keys

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU
THER
[U4.23.01]
AFFE_MODELE
"THERMAL"
"HULL"
ALL
[U4.22.01]
AFFE_CHAR_THER
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_THER_F
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_CINE
THER_IMPO
NODE
[U4.25.05]
CALC_CHAR_CINE
[U4.41.03]
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0
Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)P. MASSIN, F. VOLDOIRE
Key:
V4.03.101-C Page:

4
Results of modeling A
4.1 Values
tested

## Identification

## Reference

## Aster

Aster
Aster
\% difference
calculation has
calculation B
calculation C
N201 node
Temp_sup
49.999
49.9999
49.9999
49.9999
0.002
(-10.,0.)
Temp
49.999
49.9999
49.9999
49.9999
0.002

Temp_inf
49.999
49.9999
49.9999
49.9999
0.002

N176 node
Temp_sup
49.9658
49.9655
49.9655
49.9655
0.001
(-5.15,0.)
Temp
49.9655
49.9655
49.9655
0.001

Temp_inf
49.9658
49.9655
49.9655
49.9655
0.001

N171 node
Temp_sup
49.8888
49.8879
49.8879
49.8879
0.002
(-4.32,0.)
Temp
49.8888
49.8879
49.8879
49.8879
0.002

Temp_inf 49.8888
49.8879
49.8879
49.8879
0.002

N166 node
Temp_sup
49.6631
49.6611
49.6611
49.6611
0.008
(-3.53,0.)
Temp
49.6631
49.6611
49.6611
49.6611
0.008

Temp_inf
49.6631
49.6611
49.6611
49.6611
0.008

N161 node
Temp_sup
49.0542
49.0500
49.0500
49.0500
0.008
$(-2.8,0$.
Temp
49.0542
49.0500
49.0500
49.0500
0.008

Temp_inf 49.0542
49.0500
49.0500
49.0500
0.008

N156 node
Temp_sup
47.556
47.5482
47.5482
47.5482
0.016
(-2.13,0.)
Temp
47.556
47.5482
47.5482
47.5482
0.017

Temp_inf
47.556
47.5482
47.5482
47.5482
0.016

N141 node
Temp_sup
26.700
26.6833
26.6833
26.6833
0.062
(-0.54,0.)
Temp
26.700
26.6828
26.6828
26.6828
0.064

Temp_inf
26.700
26.6833
26.6833
26.6833
0.062

N136 node
Temp_sup
11.830
11.8212
11.8212
11.8212
0.074
(-0.19,0.)
Temp
11.830
11.8206
11.8206
11.8206
0.079

Temp_inf
11.830
11.8212
11.8212
11.8212
0.074

N11 node
Temp_sup
26.700
26.6833
26.6833
26.6833
0.062
(0.54,0.)

Temp
26.700
26.6828
26.6828
26.6828
0.064

Temp_inf
26.700
26.6833
26.6833
26.6833
0.062

N26 node
Temp_sup
47.556
47.5482
47.5482
47.5482
0.016
$(2.13,0$.
Temp
47.556
47.5481
47.5481
47.5481
0.017

Temp_inf
47.556
47.5482
47.5482
47.5482
0.016

## Contents of the file results

- Températures with the nodes of calculation has, - heat flow on the average layer (calculation has), - values tested deferred above (calculations has, B, c).
4.2 Parameters
of execution
Version: NEW 4.00.02
Machine: CRAY C90
System UNICOS:
9.0

Obstruction memory:
16 megawords
Time CPU To use:
54.7 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
6/14
5 Modeling
B
5.1

Characteristics of modeling
y
N387
X
Cutting:
68 elements in length,
2 elements in width.
Boundary conditions - loading (three calculations for three choices):

- scalar loadings and dualisation of the condition of Dirichlet:

TEMP_IMPO (NODE: N1, TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)
EXCHANGE: (GROUP_MA: GRSD1, COEF_H_SUP: 10. , COEF_H_INF: 10. ,

TEMP_EXT_SUP: -50. , TEMP_EXT_INF: -50.)
(GROUP_MA: GRSD2, COEF_H_SUP: 10. , COEF_H_INF: 10. ,
TEMP_EXT_SUP: 50. , TEMP_EXT_INF: 50.)

- loadings constant functions and dualisation of the condition of Dirichlet:
as above, but with constant functions having same values.
- scalar loadings and "kinematic" loading:

THER_IMPO: (NODE: N1,
TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)
5.2

## Characteristics of the grid

A number of nodes: 456
A number of meshs and types: 136 meshs QUAD4
5.3 Functionalities
tested
Orders

## Keys

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU
THER
[U4.23.01]
AFFE_MODELE
"THERMAL"
"HULL"
ALL
[U4.22.01]
AFFE_CHAR_THER
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_THER_F
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_CINE
THER_IMPO
NODE

```
[U4.25.05]
CALC_CHAR_CINE
[U4.41.03]
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
```


## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
7/14
6
Results of modeling B

### 6.1 Values

tested
Identification

## Reference

Aster
Aster
Aster
\% difference
calculation has
calculation B
calculation C
N387 node
Temp_sup
49.999
49.9999
49.9999
49.9999
0.002
(-10.,0.)
Temp
49.999
49.9999
49.9999
49.9999
0.002

Temp_inf
49.999
49.9999
49.9999
49.9999
0.002

N397 node
Temp_sup
49.9658
49.9669
49.9669
49.9669
0.002
$(-5.15,0$.
Temp
49.9658
49.9669
49.9669
49.9669
0.002

Temp_inf
49.9658
49.9669
49.9669
49.9669
0.002

N401 node
Temp_sup
49.8888
49.8913
49.8913
49.8913
0.005
(-4.32,0.)
Temp
49.8888
49.8913
49.8913
49.8913
0.005
49.0594
0.011

N412 node
Temp_sup
47.556
47.5657
47.5657
47.5657
0.020
(-2.13,0.)
Temp
47.556
47.5657
47.5657
47.5657
0.020

Temp_inf
47.556
47.5657
47.5657
47.5657
0.020

N420 node
Temp_sup
26.700
26.7559
26.7559
26.7559
0.209
(-0.54,0.)
Temp
26.700
26.7553
26.7553
26.7553
0.207

Temp_inf
26.700
26.7559
26.7559
26.7559
0.209

## N422 node

Temp_sup
11.830
11.8119
11.8119
11.8119
0.153
(-0.19,0.)
Temp
11.830
11.8106
11.8106
11.8106
0.164

Temp_inf
11.830
11.8119
11.8119
11.8119
0.153

N426 node
Temp_sup
26.700
26.7559
26.7559
26.7559
0.209
(0.54,0.)

Temp
26.700
26.7553
26.7553
26.7553
0.207

Temp_inf
26.700
26.7559
26.7559
26.7559
0.209

N434 node
Temp_sup
47.556
47.5657
47.5657
47.5657
0.020
$(2.13,0$.
Temp
47.556
47.5656
47.5656
47.5656
0.020

Temp_inf
47.556
47.5657
47.5657
47.5657
0.020

### 6.2 Parameters

of execution
Version: NEW 4.01.13
Machine: CRAY C90
System UNICOS:
8.0

Obstruction memory:
16 megawords
Time CPU To use:
8.666 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
8/14

## 7 Modeling

## C

## 7.1

## Characteristics of modeling

Cutting:
30 elements in length,
2 elements in width.
Boundary conditions - loading (three calculations for three choices):

- scalar loadings and dualisation of the condition of Dirichlet:

TEMP_IMPO (NODE: N1, TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)
EXCHANGE: (GROUP_MA: GRSD1, COEF_H_SUP: 10. , COEF_H_INF: 10.,
TEMP_EXT_SUP: -50. , TEMP_EXT_INF: -50.)
(GROUP_MA: GRSD2, COEF_H_SUP: 10. , COEF_H_INF: 10. ,
TEMP_EXT_SUP: 50. , TEMP_EXT_INF: 50.)

- loadings constant functions and dualisation of the condition of Dirichlet:
as above, but with constant functions having same values.
- scalar loadings and "kinematic" loading:

THER_IMPO: (NODE: N1,
TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)

## 7.2

Characteristics of the grid
A number of nodes: 410
A number of meshs and types: 60 meshs QUAD8

### 7.3 Functionalities

## tested

## Orders

## Keys

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU
THER
[U4.23.01]
AFFE_MODELE
"THERMAL"
"HULL"
ALL
[U4.22.01]
AFFE_CHAR_THER
TEMP_IMPO
NODE
[U4.25.02]

## EXCHANGE

GROUP_MA
AFFE_CHAR_THER_F
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_CINE
THER_IMPO
NODE
[U4.25.05]
CALC_CHAR_CINE
[U4.41.03]
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
9/14
8
Results of modeling C
8.1 Values
tested
Identification
Reference
Aster
Aster
Aster
\% difference
calculation has
calculation B
calculation C

## N227 node

Temp_sup

49.999
49.9999
49.9999
49.9999
0.002
(-10.,0.)
Temp
49.999
49.9999
49.9999
49.9999
0.002

Temp_inf
49.999
49.9999
49.9999
49.9999
0.002

N233 node
Temp_sup
49.9658
49.9656
49.9656
49.9656
0.001
(-5.15,0.)
Temp
49.9658
49.9656
49.9656
49.9656
0.001

Temp_inf
49.9658
49.9656
49.9656
49.9656
0.001

N235 node
Temp_sup 49.8888
49.8887
49.8887
0.001
(-4.32,0.)
Temp
49.8888
49.8887
49.8887
49.8887
0.001

Temp_inf
49.8888
49.8887
49.8887
49.8887
0.001

N 237 node
Temp_sup
49.6631
49.6598
49.6598
49.6598
0.007
(-3.53,0.)
Temp
49.6631
49.6598
49.6598
49.6598
0.007

Temp_inf
49.6631
49.6598
49.6598
49.6598
0.007

N238 node
Temp_sup
49.0542
49.0459
49.0459
49.0459
(-2.8,0.)
Temp
49.0542
49.0459
49.0459
49.0459
0.017

Temp_inf
49.0542
49.0459
49.0459
49.0459
0.017

N239 node
Temp_sup
47.556
47.5403
47.5403
47.5403
0.033
(-2.13,0.)
Temp
47.556
47.5403
47.5403
47.5403
0.033

Temp_inf
47.556
47.5403
47.5403
47.5403
0.033

N242 node
Temp_sup
26.700
26.7039
26.7039
26.7039
0.015
(-0.54,0.)
Temp

# 8.2 Parameters 

of execution
Version: NEW 4.01.13
Machine: CRAY C90
System UNICOS:
8.0

Obstruction memory:
16 megawords
Time CPU To use:
9.585 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

## Version

4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:

## P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
10/14
9 Modeling
D

## 9.1

## Characteristics of modeling

Cutting:
30 elements in length,
2 elements in width.
Boundary conditions - loading (three calculations for three choices):

- scalar loadings and dualisation of the condition of Dirichlet:

TEMP_IMPO (NODE: N1, TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)
EXCHANGE: (GROUP_MA: GRSD1, COEF_H_SUP: 10. , COEF_H_INF: 10. ,
TEMP_EXT_SUP: -50. , TEMP_EXT_INF: -50.)
(GROUP_MA: GRSD2, COEF_H_SUP: 10. , COEF_H_INF: 10. ,
TEMP_EXT_SUP: 50. , TEMP_EXT_INF: 50.)

- loadings constant functions and dualisation of the condition of Dirichlet:
as above, but with constant functions having same values.
- scalar loadings and "kinematic" loading:

THER_IMPO: (NODE: N1,
TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)

## 9.2

## Characteristics of the grid

A number of nodes: 470
A number of meshs and types: 60 meshs QUAD9

### 9.3 Functionalities

## tested

## Orders

## Keys

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU

## THER

[U4.23.01]
AFFE_MODELE
"THERMAL"
"HULL"
ALL
[U4.22.01]
AFFE_CHAR_THER
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_THER_F
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_CINE
THER_IMPO
NODE
[U4.25.05]
CALC_CHAR_CINE
[U4.41.03]
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
11/14
10
Results of modeling D

10.1 Values<br>tested<br>\section*{Identification}

## Reference

Aster
Aster
Aster
\% difference

## calculation has

calculation B
calculation C
N227 node
Temp_sup
49.999
49.9999
49.9999
49.9999
0.002
(-10.,0.)
Temp
49.999
49.9999
49.9999
49.9999
0.002

Temp_inf
49.999
49.9999
49.9999
49.9999
0.002

N233 node
Temp_sup
49.9658
49.9656
49.9656
49.9656
0.001
(-5.15,0.)
Temp
49.9658
49.9656
49.9656
49.9656
0.001

Temp_inf
49.9658
49.9656
49.9656
49.9656
0.001

N235 node
Temp_sup
49.8888
49.8887
49.8887
49.8887
0.001
(-4.32,0.)
Temp
49.8888
49.8887
49.8887
49.8887
0.001

Temp_inf 49.8888
49.8887
49.8887
49.8887
0.001

N237 node
Temp_sup
49.6631
49.6598
49.6598
49.6598
0.007
(-3.53,0.)
Temp
49.6631
49.6598
49.6598
49.6598
0.007

Temp_inf
49.6598
49.6598
49.6598
0.007

N238 node
Temp_sup
49.0542
49.0459
49.0459
49.0459
0.017
(-2.8,0.)
Temp
49.0542
49.0459
49.0459
49.0459
0.017

Temp_inf 49.0542
49.0459
49.0459
49.0459
0.017

N239 node
Temp_sup
47.556
47.5403
47.5403
47.5403
0.033
(-2.13,0.)
Temp
47.556
47.5403
47.5403
47.5403
0.033

Temp_inf
47.556
47.5403
47.5403

Temp_sup

26.700
26.7039
26.7039
26.7039
0.015
(0.54,0.)

Temp
26.700
26.7034
26.7034
26.7034
0.013

Temp_inf
26.700
26.7039
26.7039
26.7039
0.015

N249 node
Temp_sup
47.556
47.5403
47.5403
47.5403
0.033
$(2.13,0$.
Temp
47.556
47.5402
47.5402
47.5402
0.033

Temp_inf
47.556
47.5403
47.5403
47.5403
0.033
10.2 Parameters
of execution
Version: NEW 4.01.13
Machine: CRAY C90

## System UNICOS:

8.0

Obstruction memory:
16 megawords
Time CPU To use:
10.858 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
12/14

## 11 Modeling

## E

### 11.1 Characteristics of modeling

## Cutting:

30 elements in length,
2 elements in width.
Boundary conditions - loading (three calculations for three choices):

- scalar loadings and dualisation of the condition of Dirichlet:

TEMP_IMPO (NODE: N1, TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)
EXCHANGE: (GROUP_MA: GRSD1, COEF_H_SUP: 10. , COEF_H_INF: 10.,
TEMP_EXT_SUP: -50. , TEMP_EXT_INF: -50.)
(GROUP_MA: GRSD2, COEF_H_SUP: 10. , COEF_H_INF: 10. ,
TEMP_EXT_SUP: 50. , TEMP_EXT_INF: 50.)

- loadings constant functions and dualisation of the condition of Dirichlet:
as above, but with constant functions having same values.
- scalar loadings and "kinematic" loading:

THER_IMPO: (NODE: N1,
TEMP_SUP: 0. , TEMP: 0. , TEMP_INF: 0.)

### 11.2 Characteristics of the grid

A number of nodes: 590
A number of meshs and types: 120 meshs TRIA7

### 11.3 Functionalities

tested

## Orders

## Keys

AFFE_CARA_ELEM
HULL
ALL
[U4.24.01]
DEFI_MATERIAU
THER
[U4.23.01]
AFFE_MODELE
"THERMAL"
"HULL"
ALL
[U4.22.01]
AFFE_CHAR_THER
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_THER_F
TEMP_IMPO
NODE
[U4.25.02]
EXCHANGE
GROUP_MA
AFFE_CHAR_CINE
THER_IMPO
NODE
[U4.25.05]
CALC_CHAR_CINE
[U4.41.03]
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange

## Date: 01/12/98

Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
13/14
12
Results of modeling $\mathbf{E}$
12.1 Values
tested
Identification
Reference
Aster
Aster
Aster
\% difference
calculation has
calculation B
calculation C
N227 node
Temp_sup
49.999
49.9999
49.9999
49.9999
0.002
(-10.,0.)
Temp
49.999
49.9999
49.9999
49.9999
0.002

Temp_inf
49.999
49.9999
49.9999
49.9999
0.002

N233 node
Temp_sup
49.9658
49.9659
49.9659
0.001
$(-5.15,0$.
Temp
49.9658
49.9659
49.9659
49.9659
0.001

Temp_inf
49.9658
49.9659
49.9659
49.9659
0.001

N 235 node
Temp_sup
49.8888
49.8886
49.8886
49.8886
0.001
(-4.32,0.)
Temp
49.8888
49.8886
49.8886
49.8886
0.001

Temp_inf
49.8888
49.8886
49.8886
49.8886
0.001

N237 node
Temp_sup
49.6631
49.6572
49.6572
49.6572
0.012
(-3.53,0.)
Temp
49.6631
49.6572
49.6572
49.6572
0.012

Temp_inf
49.6631
49.6572
49.6572
49.6572
0.012

N238 node
Temp_sup
49.0542
49.0428
49.0428
49.0428
0.023
(-2.8,0.)
Temp
49.0542
49.0428
49.0428
49.0428
0.023

Temp_inf
49.0542
49.0428
49.0428
49.0428
0.023

N239 node
Temp_sup
47.556
47.5377
47.5377
47.5377
0.038
(-2.13,0.)
Temp
47.556
47.5376
47.5376
47.5376
0.039

Temp_inf
47.556
47.5377
47.5377
47.5377
0.038

N242 node
Temp_sup
26.700
26.7178
26.7178
26.7178
0.067
(-0.54,0.)
Temp
26.700
26.7172
26.7172
26.7172
0.064

Temp_inf
26.700
26.7178
26.7178
26.7178
0.067

N243 node
Temp_sup
11.830
11.7974
11.7974
11.7974
0.275
(-0.19,0.)
Temp
11.830
11.7965
11.7965
11.7965
0.283

Temp_inf
11.830
11.7974
11.7974
11.7974
0.275

N246 node
Temp_sup
26.700
26.7178
26.7178
26.7178
0.067
(0.54,0.)

Temp
26.700
26.7172
26.7172
26.7172
0.064

Temp_inf
26.700
26.7178
26.7178
26.7178
0.067

N249 node
Temp_sup
47.556
47.5377
47.5377
47.5377
0.038
$(2.13,0$.
Temp
47.556
47.5376
47.5376
47.5376
0.039

Temp_inf

# 12.2 Parameters 

8.0

Obstruction memory:
16 megawords
Time CPU To use:
14.679 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS101 Plates infinite subjected to a symmetrical heat exchange
Date: 01/12/98
Author (S)
:
P. MASSIN, F. VOLDOIRE

Key:
V4.03.101-C Page:
14/14
13
Summary of the results
It is noted that the variations on the temperature are weak compared to the reference solution (lower than $0.41 \%$ ).
Meshs QUAD8 and QUAD9 give the same results.
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
1/12
Organization (S): EDF/IMA/MMN, DELTA CAD, SAMTECH
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
Document: V4.03.302
TPLS302 - Distribution of temperature in one
thin section
Summary:
One considers a plate subjected to convection applied to the faces lower and higher and one edge of the plate (width). The temperature is imposed on the opposite edge of the plate.
The goal of this test is to validate the thermal element of hull in conduction in the plan and convection
[R3.11.01] and [U1.01.01]. It also makes it possible to validate the elements of edge (THCOSE2, THCOSE3) in convection.
Because of the boundary conditions and loadings considered, the distribution of temperature is uniform along the width. The results are compared with a solution based on a graphic estimate.
two types of approach give equivalent results.
This test results from the validation independent of version 3 in thermics.
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
2/12
1

## Problem of reference

1.1 Geometry

Y
$\mathrm{L}=101.6$ 10-3m
Z
D
B $=25.410-3 \mathrm{~m}$
Tw
H, Text
With
X
C
L
B
B
1.2

Material properties
$\mathrm{CP}=1 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}$
$=25.961 \mathrm{~W} / \mathrm{m}^{\circ} \mathrm{C}$

## 1.3

## Boundary conditions and loadings

Convection on the lower, higher face:
$\mathrm{H}=85.169 \mathrm{~W} / \mathrm{m} 2^{\circ} \mathrm{C}$
Text $=37.778{ }^{\circ} \mathrm{C}$
and on the end $(\mathrm{X}=\mathrm{L}, \mathrm{BC})$ of the plate:
$\mathrm{h}^{\prime}=2.163 \mathrm{~W} / \mathrm{m}^{\circ} \mathrm{C}$ correspondent with H X B for this test
Temperature imposed on the with dimensions AD: $\mathrm{Tw}=593.333{ }^{\circ} \mathrm{C}$
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)

## P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
3/12
2
Reference solution

## 2.1

## Method of calculation used for the reference solution

The original reference solution given in the book [bib1] is based on a graphic estimate.
Uncertainty on the solution is unknown.
This reference is quoted in the handbook of checking of ANSYS [bib2].
2.2

Results of reference
Temperature at the points of co-ordinates $x / L=0,0.1,0.2, \ldots 0.8,0.9,1$.

### 2.3 References <br> bibliographical

[1]
Kreith, F., "Principles of heat transfer", International Textbook Co., Scranton, Pennsylvania, 2nd Printing, 1959.
[2]
ANSYS: "Checking manual", 1st edition, June 1, 1976
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:

## P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
4/12
3 Modeling
With
3.1

Characteristics of modeling
Because of symmetry of the boundary conditions and loadings, modeling is independent width of the with dimensions AD and BC (the distribution of temperature is uniform in the direction of width).
Modeling: HULL (QUAD8 + SEG3)
y
D
Limiting conditions:
Z

- with dimensions AD
$\mathrm{T}=593.33^{\circ} \mathrm{C}$
W
- higher, lower face and edge BC
$\mathrm{H}=85.169 \mathrm{~W} \mathrm{~m} 2^{\circ} \mathrm{C}$
T
$=37.78^{\circ} \mathrm{C}$
ext.
With
C


## Cutting

- Cotés AB, CD: 5 elements
- Cotés AD, BC: 1 element

B
X
3.2

Characteristics of the grid

There are 28 nodes on the whole
5 meshs QUAD8 on the average surface of the hull
1 mesh SEG3 on the with dimensions BC

### 3.3 Functionalities

tested

## Orders

Key word factor

## Key word

Argument

## Keys

AFFE_CHAR_THER
TEMP_IMPO, EXCHANGE
[U4.25.02]
AFFE_CHAR_THER_F
TEMP_IMPO, EXCHANGE
DEFI_MATERIAU
THER
RHO_CP, LAMBDA
[U4.23.01]
AFFE_MODELE
AFFE
MODELING
"HULL"
[U4.22.01]

## AFFE

PHENOMENON
"THERMAL"
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:

## P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
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| 4 |
| :--- |
| Results of modeling A |
| 4.1 Values |
| tested |
| Identification |
| Reference |
| Aster |
| \% difference |
| Tolerance |
| Localization |
| $\mathrm{T}\left({ }^{\circ} \mathrm{C}\right)$ |
| x/L $=0.0$ |
| 593.333 |
| 593.333 |
| 0.000 |
| $5 \%$ |
| $\mathrm{x} / \mathrm{L}=0.1$ |
| 512.778 |
| 517.947 |
| 1.008 |
| $5 \%$ |
| $\mathrm{x} / \mathrm{L}=0.2$ |
| 446.111 |
| 451.206 |
| 1.142 |
| $5 \%$ |
| $\mathrm{x} / \mathrm{L}=0.3$ |
| 393.333 |
| 395.840 |
| 0.638 |
| $5 \%$ |
| $\mathrm{x} / \mathrm{L}=0.4$ |
| 348.889 |
| 349.657 |
| 0.220 |
| $5 \%$ |
| $\mathrm{x} / \mathrm{L}=0.5$ |
| 312.778 |
| 311.722 |
| 0.338 |
| $5 \%$ |
| $\mathrm{x} / \mathrm{L}=0.6$ |
| 279.444 |
|  |

279.444

### 4.2 Remarks

As envisaged, the distribution of temperature is uniform according to the width. All results obtained are in the interval of allowed tolerance which corresponds to the uncertainty supposed on the results of the graphic estimate.

### 4.3 Parameters

## of execution

Version: 4.01.05
Machine: CRAY C98
Obstruction memory:
8 MW
Time CPU To use:
5.28 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:

TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)

## :

## P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
6/12

## 5 Modeling

## B

## 5.1

## Characteristics of modeling

Because of the boundary conditions and loadings, modeling is independent of the width with dimensions AD and BC (the distribution of temperature is uniform along the width).
Modeling: HULL (QUAD4 + SEG2)
y

## D

Limiting conditions:
Z

- with dimensions AD
$\mathrm{T}=593.33^{\circ} \mathrm{C}$
W
- higher, lower face and edge BC
$\mathrm{H}=85.169 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}$
$\mathrm{T}=37.78^{\circ} \mathrm{C}$
ext.
With
C
Cutting
- Cotés AB, CD:

10 elements

- Cotés AD, BC: 1 element


## B

X

## 5.2

## Characteristics of the grid

One A 22 nodes on the whole
10 meshs QUAD4 on the average surface of the hull
1 mesh SEG2 on the with dimensions BC

### 5.3 Functionalities

tested

## Orders

Key word factor

## Key word <br> Argument

## Keys

AFFE_CHAR_THER
TEMP_IMPO, EXCHANGE
[U4.25.02]
AFFE_CHAR_THER_F
TEMP_IMPO, EXCHANGE
DEFI_MATERIAU
THER
RHO_CP, LAMBDA
[U4.23.01]
AFFE_MODELE
AFFE
MODELING
"HULL"
[U4.22.01]
AFFE
PHENOMENON
"THERMAL"
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
7/12
6
Results of modeling B
6.1 Values
tested

## Identification

## Reference

Aster

## \% difference <br> Tolerance

## Localization

$\mathrm{T}\left({ }^{\circ} \mathrm{C}\right)$
$\mathrm{x} / \mathrm{L}=0.0$
593.333
593.333
0.000

5\%
$\mathrm{x} / \mathrm{L}=0.1$
512.778
518.146
1.047

5\%
$\mathrm{x} / \mathrm{L}=0.2$
446.111
451.267
1.156

5\%
$\mathrm{x} / \mathrm{L}=0.3$
393.333
395.633
0.585

5\%
$\mathrm{x} / \mathrm{L}=0.4$
348.889
349.428
0.155

5\%
$\mathrm{x} / \mathrm{L}=0.5$
312.778
311.457
0.422

5\%
$\mathrm{x} / \mathrm{L}=0.6$
279.444
280.715
0.455

5\%
$\mathrm{x} / \mathrm{L}=0.7$
254.444
256.390
0.765
$\mathrm{x} / \mathrm{L}=0.8$
237.778
237.839
+0.026
5\%
$\mathrm{x} / \mathrm{L}=0.9$
221.111
224.574
1.566

5\%
$\mathrm{x} / \mathrm{L}=1.0$
213.333
216.242
1.364

5\%

### 6.2 Remarks

As envisaged, the distribution of temperature is uniform according to the width. All results obtained are in the interval of allowed tolerance which corresponds to the uncertainty supposed on the results of the graphic estimate.

### 6.3 Parameters

of execution
Version: 4.01.05
Machine: CRAY C98
Obstruction memory:
8 MW
Time CPU To use:
5.19 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:

## 7.1

## Characteristics of modeling

Because of the boundary conditions and loadings, modeling is independent of the width with dimensions AD and BC (the distribution of temperature is uniform along the largeu).

## 7.2

## Characteristics of the grid

There are 33 nodes on the whole
5 meshs QUAD9 on the average surface of the hull
1 meshs SEG3 on the with dimensions BC

### 7.3 Functionalities

## tested

Orders
Key word factor
Key word
Argument

## Keys

AFFE_CHAR_THER
TEMP_IMPO, EXCHANGE
[U4.25.02]

```
AFFE_CHAR_THER_F
TEMP_IMPO, EXCHANGE
DEFI_MATERIAU
THER
RHO_CP, LAMBDA
[U4.23.01]
AFFE_MODELE
AFFE
MODELING
"HULL"
[U4.22.01]
AFFE
PHENOMENON
"THERMAL"
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A
```


## Code_Aster © ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
9/12
8
Results of modeling C
8.1 Values
tested
Identification
Reference
Aster
\% difference
tolerance
Localization
T ( $\left.{ }^{\circ} \mathrm{C}\right)$
$\mathrm{x} / \mathrm{L}=0.0$
593.333

$$
x / L=0.3
$$

$$
393.333
$$

$$
395.841
$$

$$
0.638
$$

$$
5 \%
$$

$$
\mathrm{x} / \mathrm{L}=0.4
$$

$$
348.889
$$

$$
349.658
$$

$$
0.220
$$

$$
5 \%
$$

$$
x / L=0.5
$$

$$
312.778
$$

$$
311.722
$$

-0.338

5\%
$\mathrm{x} / \mathrm{L}=0.6$
279.444
280.993
0.554
$\mathrm{x} / \mathrm{L}=0.7$
254.444
256.673
0.876

5\%
$\mathrm{x} / \mathrm{L}=0.8$
237.778
238.125
0.146

5\%
$\mathrm{x} / \mathrm{L}=0.9$
221.111
224.854
1.693

5\%
$\mathrm{x} / \mathrm{L}=1.0$
213.333
216.516
1.492

5\%

### 8.2 Remarks

As envisaged, the distribution of temperature is uniform according to the width. All results obtained are clearly in the interval of allowed tolerance.

### 8.3 Parameters

of execution
Version: 4.01.12
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
5.436 seconds

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
:
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
10/12
9 Modeling
D
Modeling: HULL (TRIA7 + SEG3)
y
D
Limiting conditions:
Z

- dimensioned AD
$\mathrm{T}=593.33^{\circ} \mathrm{C}$
W
- higher, lower face and edge BC
$\mathrm{H}=85.169 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}$
$\mathrm{T}=37.78^{\circ} \mathrm{C}$
ext.
With
C
Cutting
- Dimensioned AB, CD:

10 elements

- Dimensioned AD, BC:

1 element
B
X
9.1

## Characteristics of modeling

Because of the boundary conditions and loadings, modeling is independent of the width with dimensions AD and BC (the distribution of temperature is uniform along the largeu).
9.2

## Characteristics of the grid

There are 43 nodes on the whole
10 meshs TRIA7 on the average surface of the hull

## 1 meshs SEG3 on the with dimensions BC

### 9.3 Functionalities <br> tested <br> Orders <br> Key word factor <br> Key word <br> Argument

Keys
AFFE_CHAR_THER
TEMP_IMPO, EXCHANGE
[U4.25.02]
AFFE_CHAR_THER_F
TEMP_IMPO, EXCHANGE
DEFI_MATERIAU
THER
RHO_CP, LAMBDA
[U4.23.01]
AFFE_MODELE
AFFE
MODELING
"HULL"
[U4.22.01]
AFFE
PHENOMENON
"THERMAL"
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ©

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)
P. MASSIN, F. LEBOUVIER, A. LAULUSA

Key:
V4.03.302-A Page:
11/12
10
Results of modeling D

### 10.1 Values

## tested

## Identification

## Reference

Aster
\% difference

## tolerance

Localization
$\mathrm{T}\left({ }^{\circ} \mathrm{C}\right)$
$\mathrm{x} / \mathrm{L}=0.0$
593.333
593.333
0.000

5\%
$\mathrm{x} / \mathrm{L}=0.1$
512.778
517.863
0.992

5\%
$\mathrm{x} / \mathrm{L}=0.2$
446.111
451.193
1.139

5\%
$\mathrm{x} / \mathrm{L}=0.3$
393.333
395.862
0.643

5\%
$\mathrm{x} / \mathrm{L}=0.4$
348.889
349.635
0.214

5\%
$\mathrm{x} / \mathrm{L}=0.5$
312.778
311.737
-0.333
5\%
$\mathrm{x} / \mathrm{L}=0.6$
279.444
280.981
0.550

### 10.2 Remarks

As envisaged, the distribution of temperature is uniform according to the width. All results obtained are clearly in the interval of allowed tolerance.

### 10.3 Parameters

## of execution

Version: 4.01.12
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
5.304condes

Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
TPLS302 Distribution of temperature in a thin section
Date: 01/12/98
Author (S)

# P. MASSIN, F. LEBOUVIER, A. LAULUSA 

Key:
V4.03.302-A Page:
12/12
11

## Summary of the results

Results obtained for two modelings (QUAD8 + SEG3 or QUAD + SEG2) in the interval of allowed tolerance ( $<2 \%$ for a tolerance of $5 \%$ ) which corresponds to uncertainty on results of the graphic estimate of reference.
The data of the test could retain a tolerance of $2 \%$.
Handbook of Validation
V4.03 booklet: Stationary thermics of the plates and the hulls
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
5.0

Titrate:
TPLV06 Release of power in a hollow sphere
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.04.006-A Page:
1/6
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

Handbook of Validation<br>V4.04 booklet: Stationary thermics of the voluminal structures<br>V4.04.006 document

TPLV06-Release of power in one
hollow sphere

## Summary:

This test results from the validation independent of version 3 in linear stationary thermics.
It is about a three-dimensional problem which aims to validate the voluminal thermal element subjected to a temperature imposed and on a heat source.

This case test includes/understands a modeling 3D. The results are compared with an analytical solution (VPCS).

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV06 Release of power in a hollow sphere
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.04.006-A Page:
2/6

1

# Problem of reference 

### 1.1 Geometry

## Re

IH
Hollow sphere:

- Interior Ray $\mathbf{I H}=1 \mathrm{~m}$
- External Ray Re=2m

R
O
$Q$

## 1.2

Properties of material

1. $W / m{ }^{\circ} C$

Thermal conductivity

## 1.3 <br> Boundary conditions and loadings

$\cdot T i=T(R=I H)=20^{\circ} \mathrm{C}$,
$\cdot T e=T(R=R e)=20^{\circ} \mathrm{C}$,
$\cdot Q=100 \mathrm{~W} / \mathrm{m} 3$.
1.4 Conditions
initial
Without object.
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

TPLV06 Release of power in a hollow sphere
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.04.006-A Page:
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## 2 <br> Reference solution

## 2.1

Method of calculation used for the reference solution
The reference solution is that given in card TPLV06/89 of guide VPCS.

- Température according to $R$ :


## R2-R2

```
I)
|
R
T=T
I
+
-(R2-R2
I
I)
6
```


## - Density flux according to R:

$d T$
$2 Q$
2

1
1
1
$=-4$
$=-$
$(2$
$R-$
2
$R)$
-
-2
$R$
$R$
$E$
$I$
$I$

## 2.2

Results of reference
Temperature in $R=1.25$; 1.5 and 1.75 m

## 2.3

Uncertainty on the solution
Analytical solution.

### 2.4 References <br> bibliographical

## [1]

Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV06 Release of power in a hollow sphere
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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4/6

## 3 Modeling

With

## 3.1 <br> Characteristics of modeling <br> 3D (HEXA8)

N101
G
Limiting conditions:
C
N106
N105

- faces ABCD, EFGH,
$A B F E, D C G H=0$
N111
N110
- faces $A D H E, B C G F T=20^{\circ} C$

F
N1
N116
N115
N6
X
N120
N121
N11
B
N5
Z
H
N16
N125
D
N10
E
N15
N21
N20
C
With
N25
$30^{\circ}$
$30^{\circ}$

3.2<br>Characteristics of the grid<br>A number of nodes:<br>125<br>\subsection*{3.3 Functionalities}<br>tested<br>Orders<br>AFFE_MODELE<br>THERMICS<br>3D<br>ALL<br>\title{ AFFE_CHAR_THER }<br>TEMP_IMPO<br>SOURCE<br>\section*{THER_LINEAIRE}<br>EXCIT<br>CHARGE<br>RECU_CHAMP<br>NUME_ORDRE

A number of meshs and types: 64 HEXA8

```
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0
```

Titrate:
TPLV06 Release of power in a hollow sphere

## Date:

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:

V4.04.006-A Page:
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## 4

Results of modeling $A$

### 4.1 Values

tested
Identification Reference
Aster \%
difference
tolerance
Temperature ( ${ }^{\circ} \mathrm{C}$ )
$R=1.25$ (N16)
30.625
30.471
-0.504
1\%
$R=1.25$ (N116)
30.625
30.471
-0.504
1\%
$R=1.25$ (N20)
30.625
30.462
-0.532
$1 \%$
$R=1.25$ (N120)

1\%
$R=1.50$ (N11)
32.500
32.337
-0.500
$1 \%$
$R=1.50$ (N111)
32.500
32.337
-0.500
$1 \%$
$R=1.50$ (N15)
32.500
32.335
-0.507
1\%
$R=1.50($ N115 $)$
32.500
32.335
-0.507
$1 \%$
$R=1.75$ (N6)
28.482
28.379
-0.362
1\%
$R=1.75$ (N106)
28.482
28.379
-0.362
$1 \%$
$R=1.75$ (N10)
28.482
28.382
-0.351
1\%
$R=1.75$ (N110)
28.482
28.382
-0.351

### 4.2 Parameters

of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 2.00 seconds
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV06 Release of power in a hollow sphere
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.04.006-A Page:
6/6

## 5

Summary of the results
The results obtained are satisfactory, the maximum change obtained is $0.53 \%$.
Modeling 3D used to model this sphere is correct.
The quality of the results could be still improved in:

- carrying out a finer grid of the portion of sphere,
- choosing quadratic elements for better approximating the reference solution.

Handbook of Validation
5.0

Titrate:
Orthotropic Cubic TPLV07

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.04.007-A Page:
1/6
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
V4.04.007 document

TPLV07 - Cubic orthotropic

## Summary:

This test results from the validation independent of version 3 in linear stationary thermics.
It validates the voluminal thermal elements under conditions of imposed flow, of convection but also of linear variation of the outside temperature.

The results are compared with an analytical solution (VPCS).

## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Orthotropic Cubic TPLV07

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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## 1

Problem of reference

### 1.1 Geometry

has
Cubic of edge $=0.2 \mathrm{~m}$ has
Center cube $=(0 ., 0 ., 0$.
Z
$y$
$X$
$O$

## 1.2

Properties of material
$X=1.0 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}$ thermal Conductivity along axis $X$
$y=0.75 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}$ thermal Conductivity along the axis $y$
$Z=0.50 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}$ thermal Conductivity along axis $Z$

## 1.3

Boundary conditions and loadings

- density flux normal:
$N=60 \mathrm{~W} / \mathrm{m}^{2}$ face $\mathrm{y}=-0.1$ (entering flow),
$N=-60 \mathrm{~W} / \mathrm{m}^{2}$ face $y=0.1$ (outgoing flow),
$N=30 \mathrm{~W} / \mathrm{m}^{2}$ face $\mathrm{Z}=-\mathbf{0 . 1}$ (entering flow),
$N=-30 \mathrm{~W} / \mathrm{m}^{2}$ face $\mathrm{Z}=0.1$ (outgoing flow),
$\cdot$ convection on the faces $X=0.1$ and $X=0.1: H=15 \mathrm{~W} / \mathrm{m} 2^{\circ} \mathrm{C}$,
- linear variation of the outside temperatures,
- Text $=30-80 y-60 z$ face $X=-0.1$,
- Text $=15-80 y-60 z$ face $X=$
0.1.


### 1.4 Conditions initial

Without object.

## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
Orthotropic Cubic TPLV07

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
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2
Reference solution

## 2.1

Method of calculation used for the reference solution
The reference solution is that given in card TPLV07/89 of guide VPCS.
Analytical solution.
$T(X, y, Z)=a x+b y+c z+D$
$=-45 x-80 y-60 z+22.5$
Z
Not T $\left({ }^{\circ} \mathrm{C}\right)$
L
$K$
O
22.5

M
With
35.0

B
26.0

I
$y$
$J$
C
10.0

G
D
C
D
19.0

E
30.5

HOFX
F
18.0

E
G
14.5

With
B
H
27.0

R
$Q$
$I$
29.0
$S$
$J$
20.0
$K$
4.0
$L$
13.0
$N R$
$P$
$M$
16.5
$N R$
41.0
$P$
32.0
$X=45 \quad \mathrm{~W} / \mathrm{m}^{2}=$ constant
$Q$
16.0
$R$
25.0
$y=60 \mathrm{~W} / \mathrm{m}^{2}=$ constant
$S$
28.5
$Z=30 \mathrm{~W} / \mathrm{m}^{2}=$ constant
2.2

Results of reference
Temperature at the points quoted in the table above.

## 2.3 <br> Uncertainty on the solution

Analytical solution.

### 2.4 References <br> bibliographical

## [1]

Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7

## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures
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## 3 Modeling <br> With

## 3.1

Characteristics of modeling
3D (HEXA8)

## Limiting conditions:

Z

- face NPJI

L
K
$N=60 \mathrm{~W} / \mathrm{m}^{2}$

- face RQKL
$N=-60 \mathrm{~W} / \mathrm{m}^{2}$
- face NPQR

I

```
J
N=30 W/m 2
- face IJKL
N=-30 W/m 2
y
- face NRLI
H=15 W/m 2 ' C
T
X
ext. = 30-80y-60z
O
- face PQKJ
H=15 W/m ' }\mp@subsup{}{}{\circ}\textrm{C
Text =15-80y-60z
Cutting:
R
Q
-6 elements according to X
-6 elements following y
NR
P
-6 elements according to Z
```

3.2
Characteristics of the grid
A number of nodes: ..... 343
A number of meshs and types: 216 HEXA8 (and 216 QUAD8)
3.3 Functionalities
tested
Orders
DEFI_MATERIAUTHER_ORTH
AFFE_MODELE

## THERMICS

3D
ALL

AFFE_CARA_ELEM

SOLID MASS
ANGLE_REP
0. 0.0.

DEFI_NAPPE
AFFE_CHAR_THER_F
FLUX_REP
EXCHANGE

THER_LINEAIRE<br>CARA_ELEM<br>EXCIT<br>CALC_CHAM_ELEM<br>CARA_ELEM<br>FLUX_ELNO_TEMP

### 3.4 Remarks

Voluminal heat CP does not intervene in this test, but must be declared for Code_Aster. One $C P=1.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}$ takes.

## Handbook of Validation

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Author (S):

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## 4

Results of modeling $A$

### 4.1 Values

tested
Identification Reference
Aster
Relative variation (\%)
Absolute deviation ( ${ }^{\circ} \mathrm{C}, \mathrm{W} / \mathrm{m} 2$ )
difference
tolerance
difference
tolerance
Not
Node
$T$
$\left({ }^{\circ} \mathrm{C}\right)$

O
N169
22.5
22.500
$0.000 \%$ 1.\% 2.50E-7 0.5
${ }^{\circ} \mathrm{C}$
With
N5
35.0
35.000 0.000\%
1.\%
1.03E-7
0.5

B
N301

| 26.0 |
| :--- |
| 26.000 |
| $1 . \%-7.12 E-8$ |
| 0.5 |
| $C$ |
|  |
| $N 337$ |
| 10.0 |
| 10.000 |
| $0.000 \%$ |
| $1 . \%$ |
| $-7.72 E-8$ |
| 0.5 |
| $D$ |
|  |
| $N 49$ |
| 19.0 |
| 19.000 |
| $0.000 \%$ |
| $1 . \%$ |
| $1.00 E-7$ |
| 0.5 |
| $E$ |
| $N 151$ |
| 30.5 |
| 3.500 |
| $1 . \%$ |
| $2.54 E-7$ |
| 0.5 |
| $F$ |
|  |
| $N 316$ |
| 18.0 |
| $18.000 \%$ |
| $0.000 \%$ |
| $1 . \%$ |
| $-7.42 E-8$ |
| 0.5 |
| $G$ |
| $N 196$ |
| 14.5 |
| 14.500 |
| $0.000 \%$ |
|  |

1.\%
2.46E-7
0.5

H
N24
27.0
27.000
0.000\%
1.\%
1.02E-7
0.5

I
N1
29.0
29.000
0.000\%
1.\%
1.14E-7
0.5

J
N298
20.0
20.000
0.000\% 1.\% 4.97E-7 0.5

K
N340
4.0
4.000 0.000\%
1.\% -4.64E-8
0.5

L
N44
13.0
13.000
0.000\%
1.\%
1.07E-7
0.5

M

| $N 172$ |
| :--- |
| 16.5 |
| 16.500 |
| $0.000 \%$ |
| $1 . \%$ |
| $2.89 E-7$ |
| 0.5 |
| $N R$ |
| $N 2$ |
| 41.0 |
| 41.000 |
| $0.000 \%$ |
| $1 . \%$ |
| $9.66 E-8$ |
| 0.5 |
| $P$ |
| $N 297$ |
| 32.0 |
| 32.000 |
| $1 . \% .000 \%$ |
| 0.5 |
| $Q$ |
| $N 338$ |
| 16.0 |
| 16.000 |
| $0.000 \%$ |
| $1 . \%$ |
| $-8.38 E-8$ |
| 0.5 |
| $R$ |
| $N 43$ |
| 25.0 |
| 25.000 |
| $0.000 \%$ |
| $1 . \%$ |
| $9.59 E-8$ |
| 0.5 |
| $S$ |
| $N 173$ |
| 28.5 |
| 28.500 |
| $0.000 \%$ |
|  |

1.\%
2.37E-7
0.5

Not maill
Node
(W/m2)

## E

X K m211 N340
45.0
45.000
0.000\%
1.\%
-1.73E-6
$0.5 \mathrm{~W} / \mathrm{m} 2$
X F m201 N316 45.0
45.000
0.000\%
1.\%
-1.13E-6
0.5

X O m129 N169
45.0
45.000
0.000\%
1.\%
-1.36E-6
0.5
y K m211 N340
60.0
60.000
0.000\%
1.\%
-2.43E-6
0.5
y F m201 N316
60.0
60.000
0.000\% 1.\% 2.96E-8 0.5

### 4.2 Parameters

of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 3.30 seconds
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V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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## 5

Summary of the results
The results obtained are excellent. The computed values by Aster are identical to the values of reference. That is "a normally awaited" result since the field solution which is linear belongs to the space of interpolation of the element tested.

This test made it possible to test the following orders:

- DEFI_NAPPE allowing to define a variation in the external temperature according to
$X$-coordinate $X$ and of the ordinate $y$,
- DEFI_MATERIAU associated with key word THER_ORTH, allowing to define the characteristics of one
orthotropic material,
$\cdot$ - AFFE_CARA_ELEM associated with the MASSIVE key word, allowing to define the axes of orthotropism.


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TPLV100 Rolls subjected to boundary conditions
Date:
22/12/98
Author (S):
X. DESROCHES Key

Handbook of Validation<br>V4.04 booklet: Stationary thermics of the voluminal structures<br>Document: V4.04.100

TPLV100-Roll subjected to conditions
with the nonaxisymmetric limits

## Summary:

It is about a test in stationary thermics with modeling of Fourier.
This test validates all the elements of Fourier in thermics ( 5 different modelings) with various types of boundary conditions: imposed temperature, exchange, imposed flow, heat source.

The interest of the test, in addition to the validation of Fourier thermics, lies in the following points: - comparison between the results and an analytical solution on various harmonics of Fourier (1, 2 and
$3)$,

- homogeneity of the elements between them.


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1
Problem of reference

### 1.1 Geometry

### 1.2 Properties

of

## materials

$$
=1 \mathrm{~W} / \mathrm{m}{ }^{\circ} \mathrm{C}
$$

1.3
Boundary conditions and loadings
[EA]: imposed temperature
$\boldsymbol{T}=\boldsymbol{T o}=0 .{ }^{\circ} \mathrm{C}$
[BC]: imposed flow
$=O=2 . W / m 2{ }^{\circ} \mathrm{C}$
[CD]: exchange
$h=2 . W / m 2{ }^{\circ} C$
Text $=2 .{ }^{\circ} \mathrm{C}$
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X. DESROCHES Key
:
2V4.04.100-C Page:3/14
Reference solution
2.1
Method of calculation used for the reference solution
$T(R, Z)=,r 2 \cos L$

## with L number of the harmonic of Fourier

```
\(-T=(l 2-4) \cos L=S\)
- \(2 r \cos L\)
R
T=0.+(lrsinl)
[
on \(A B]\) and [ED]
\(\boldsymbol{R} \boldsymbol{R}\)
0
\(=N=0\).
[
on BC]
0
\(=2 R=2\).
[
on CD]
2
\(N R=2 R=\)
\((2 R 2-R 2)=(h T e x t-T)\)
R
2
from where \(H\)
\(=2\).
R
\(T\)
2
ext.
\(=2 R\)
\(=2\).
```

Only the source term varies according to the harmonic $(S l(R, Z)=l 2-4)$.
In following modelings, one will solve the problem on harmonics 1, 2 and 3.

Results of reference
Temperatures and flow at the points $B, C, D, F, G$.
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures HI-75/01/010/A

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## 3 Modeling

With

## 3.1

Characteristics of modeling

## AXIS-FOURIER (TRIA6)

$y$ (Z)
R Z node
To 0.0. N1
G
B 1.0. N7
E
D
C 1.0 .5
N8
D 1.1. N9
C
E 0.1. N3
F 0.5
0. N4
$X(R)$
With
G 0.5

1. N6

F
B

The axes of description of the grid are $X(R)$ and $y(Z)$.
Mode - Fourier: 1 T $(A)=0$.
$S=-3$.
on all the field
[BC]:
$=2$.
[CD]:
$H=2 . \operatorname{Text}=2$.

### 3.2 Characteristics

grid
A number of nodes: 25.
A number of meshs and types: 8 TRIA6

### 3.3 Functionalities

tested
Orders

Keys
THERMAL AFFE_MODELE
"AXIS_FOURIER"
ALL
[U4.22.01]
AFFE_CHAR_THER TEMP_IMPO
NODE
[U4.25.02]
FLUX_REP
GROUP_MA

## EXCHANGE

GROUP_MA

SOURCE<br>ALL<br>CALC_MATR_ELEM "RIGI_THER"<br>MODE_FOURIER<br>[U4.41.01]<br>CALC_VECT_ELEM "CHAR_THER"

[U4.41.02]<br>ASSE_MATRICE

[U4.42.02]
ASSE_VECTEUR
[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM "FLUX_ELNO_TEMP"
MODE_FOURIER
[U4.61.01]
COMB_CHAM_NO COMB_FOURIER
[U4.53.02]
COMB_CHAM_ELEM COMB_FOURIER
[U4.53.03]
POST_RELEVE CHAM_GD
"EXTRACTION"
[U4.74.03]
3.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM.

The use of orders COMB_CHAM_NO and COMB_CHAM_ELEM key word COMB_R is not one recombination of Fourier but a simple validation of this key word.

## Handbook of Validation

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## 4

Results of modeling $A$

### 4.1 Values

tested
Identification
Reference
Aster \%
difference
= 0
$T(B)$
1.
0.9981
-0.19
$T$
(F)
0.25
0.2484
-0.66
$\boldsymbol{R}$ (B)
-2-1.993
-0.36
$\boldsymbol{R}(F)$
-1. -0.9924
-0.76
(B)

1. 0.9996
-0.04
(F)
0.50 .4982
-0.37
Z (B)
2. -5.

10-3-
$Z(F)$
0. 7.

10-4-
$=45$
$\boldsymbol{T}(\boldsymbol{B})$
0.7071
0.7057
-0.192
$T$
(F)
0.177
0.1756
-0.65
$\boldsymbol{R}$ (B)
-1.414-1.4018
-0.87

```
R(F)
-0.7071 -0.6848 3.15
```

(B)
$-0.707-0.70690 .027$
(F)
-0.3535-0.3512-0.65
Z (B)
0.0 .36
10-3-
$Z(F)$
0.0 .12
10-2
$=135$
$\boldsymbol{T}(\boldsymbol{B})$
-0.707
-0.7057
0.19
$T$
(F)
-0.177
-0.1756
0.65
$\boldsymbol{R}(\boldsymbol{B})$
1.4141 .4018
-0.87
$\boldsymbol{R}(F)$
0.7070 .685
-3.15
(B)
-0.707-0.7069 0.027
(F)
-0.3535-0.3533 0.06

Z (B)
0. -0.36

10-3-
$Z(F)$
0. 0.12

10-2

### 4.2 Remarks

The values of flows to the nodes are realised on the elements containing this node.
It is noticed that the exact solution is not found. This is with the fact that numerical integration thermal matrix of rigidity is approximate (formula at 3 points of GAUSS). If one were used formulate at 6 points, one would find the solution exactly.

### 4.3 Parameters

of execution
Version: 4.00.02

Machine: CRAY C90
System UNICOS:
8.04

Obstruction memory:
8 megawords
Time CPU To use:
7.0 seconds

Handbook of Validation
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Version
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Author (S):
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## 5 Modeling

B

## 5.1

Characteristics of modeling

## AXIS-FOURIER (QUAD8)

$y$ (Z)<br>$R Z$ node<br>To 0.0. N1<br>G<br>B 1. O. N4<br>E<br>D<br>C 1. 0.5<br>N5<br>D 1. 1. N6<br>C<br>E 0.1. N3<br>F 0.5<br>0. N7<br>$X(R)$<br>With<br>G 0.5<br>1. N12<br>F<br>B

The axes of description of the grid are $X(R)$ and $y(Z)$.
Mode - Fourier: $2 T(A)=0$.
No the term source bus $S L(R, Z)$
$=$.
0 for $L=2$
$[B C]:=2$.
$[C D]: H=2$. Text $=2$.

### 5.2 Characteristics

grid
A number of nodes: 13.
A number of meshs and types: 2 QUAD8

5.3 Functionalities<br>tested<br>\section*{Orders}<br>\section*{Keys}<br>"THERMAL" AFFE_MODELE "AXIS_FOURIER"<br>ALL<br>[U4.22.01]<br>AFFE_CHAR_THER TEMP_IMPO<br>NODE<br>[U4.25.02]<br>FLUX_REP<br>GROUP_MA

## EXCHANGE

GROUP_MA

CALC_MATR_ELEM "RIGI_THER"
MODE_FOURIER
[U4.41.01]

[U4.42.02]<br>ASSE_VECTEUR

[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM "FLUX_ELNO_TEMP"
MODE_FOURIER
[U4.61.01]

### 5.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM.
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```
6
Results of modeling B
```

6.1 Values
tested
Identification Reference
Aster \%
difference
T(B) 1. 1.
0.
T(C) 1. 1.
0.
T(D) 1. 1.
0.
$T(F) 0.250 .25$
0.
$\boldsymbol{T}(\boldsymbol{G}) 0.250 .25$
0.
$\boldsymbol{R}$ (B)
-2. -2.
0.
$R(C)$
-2. -2.
0.
$R(D)$
-2. -2.
0.
$\boldsymbol{R}$ (F)
-1. -1.
0.
$\boldsymbol{R}(G)$
-1. -1.
0.
(B)
2. 2.
0.
(C)
2. 2.
0.
(D)
2. 2.
0.
2.10-15
0.
$Z(C)$
0.
-1.2 10-14
0.

Z (D)
0.
-1.2 10-13
0.
$Z(F)$
0.
-1.4 10-14
0.
$Z(G)$
0.
-3.7 10-15
0.

### 6.2 Remarks

The analytical solution is found exactly.

### 6.3 Parameters

of execution
Version: 4.00.02

## Machine: CRAY C90

System UNICOS:
8.04

Obstruction memory:
8 megawords
Time CPU To use:
3.8 seconds

## Handbook of Validation

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Version
5.0

Titrate:
TPLV100 Rolls subjected to boundary conditions
Date:
22/12/98
Author (S):
X. DESROCHES Key
:
V4.04.100-C Page:
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## 7 Modeling

C

## 7.1

Characteristics of modeling

## AXIS-FOURIER (QUAD9)

$y$ (Z)
R Z node
To 0.0. N1
G
B 1.0.N4
E
D
C 1. 0.5
N5
D 1.1. N6
C
E 0.1. N3
F 0.50. N7$X(R)$WithG 0.51. N12
F
B
The axes of description of the grid are $X(R)$ and $y(Z)$.
Mode - Fourier: $3 \boldsymbol{T}(\boldsymbol{A})=\mathbf{0}$.
$S=5$.
on all the field
[BC]:
$=2$.
[CD]:
$H=2$. Text $=2$.
7.2 Characteristicsgrid
A number of nodes: 15.
A number of meshs and types: 2 QUAD9
7.3 Functionalities
tested
Orders
Keys"THERMAL"AFFE_MODELE "AXIS_FOURIER"
ALL
[U4.22.01]AFFE_CHAR_THER TEMP_IMPO
NODE
[U4.25.02]
FLUX_REP
GROUP_MA

## EXCHANGE

GROUP_MA

SOURCE<br>ALL<br>CALC_MATR_ELEM "RIGI_THER"<br>MODE_FOURIER<br>[U4.41.01]<br>CALC_VECT_ELEM "CHAR_THER"

[U4.41.02]
ASSE_MATRICE
[U4.42.02]
ASSE_VECTEUR
[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM "FLUX_ELNO_TEMP"
MODE_FOURIER
[U4.61.01]

### 7.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM.

## Handbook of Validation

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HI-75/01/010/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:

## Date:

22/12/98
Author (S):
X. DESROCHES Key

```
:
V4.04.100-C Page:
```

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## 8 <br> Results of modeling $C$

### 8.1 Values

tested
Identification Reference
Aster \%
difference
T(B) 1. 1.
0.

T(C) 1. 1.
0.

T(D) 1. 1.
0.
$T(F) 0.250 .250$.
T(G) 0.250 .25
0.
$\boldsymbol{R}$ (B)
-2. -2.
0.
$R(C)$
-2. -2.
0.
$R(D)$
-2. -2.
0.
$\boldsymbol{R}(F)$
-1. -1.
0.
$\boldsymbol{R}$ (G)
-1. -1.
0.
(B)
3. 3.
0.
(C)

## 3. 3.

0. 

(D)
3. 3.
0.
(F)
1.51 .5
0.
(G)
1.51 .5
0.
$Z$ (B)
0.
1.2 10-14
0.
$Z(C)$
0.
$5.510-14$
0.
$Z$ (D)
0.
4.6 10-15
0.
$Z(F)$
0.
-1.1 10-15
0.
$Z(G)$
0.
1.8 10-14
0.
8.2 Remarks

The analytical solution is found exactly.

### 8.3 Parameters

of execution

Version: 4.00.02

Machine: CRAY C90
System UNICOS:
8.04

Obstruction memory:
8 megawords
Time CPU To use:
3.9 seconds

Handbook of Validation
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Code_Aster ${ }^{\circledR}$
Version
5.0

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9 Modeling
D
9.1

Characteristics of modeling
AXIS-FOURIER (QUAD4)
$y$ (Z)
50 subdivisions in $R$
R Z node
To 0.0. N1

```
G
D
B 1. 0. N151
E
C 1. 0.5
N152
D 1. 1. N153
C
E 0.1. N3
F 0.5
0. N76
X(R)
G 0.5
1. N78
With
F
B
The axes of description of the grid are X (R) and y (Z).
Mode - Fourier: 2 T(A)=0.
S=0.
on all the field
[BC]:
=2
[CD]:
H=2. Text = 2.
```


### 9.2 Characteristics

grid
A number of nodes: 153
A number of meshs and types: 100 QUAD4
9.3 Functionalities
tested
Orders
Keys
"THERMAL"AFFE_MODELE "AXIS_FOURIER"
ALL

[U4.22.01]<br>AFFE_CHAR_THER TEMP_IMPO<br>NODE<br>[U4.25.02]<br>FLUX_REP<br>GROUP_MA

EXCHANGE
GROUP_MA

SOURCE<br>ALL<br>CALC_MATR_ELEM "RIGI_THER"<br>MODE_FOURIER<br>[U4.41.01]<br>CALC_VECT_ELEM "CHAR_THER"

[U4.41.02]
ASSE_MATRICE
[U4.42.02]
ASSE_VECTEUR
[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT
[U4.51.02]
CALC_CHAM_ELEM "FLUX_ELNO_TEMP"
MODE_FOURIER
[U4.61.01]
9.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM.

# Handbook of Validation <br> V4.04 booklet: Stationary thermics of the voluminal structures <br> HI-75/01/010/A 

Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

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10 Results of modeling $D$
10.1 Values
tested
Identification Reference
Aster \%
difference
T(B) 1. 0.9998
-2.10-3
T (C) 1. 0.9998
-2.10-3
T (D) 1. 0.9998
-2.10-3
T(F) 0.25
0.2498
-0.02
T(G) 0.25
0.2498
-0.02
$\boldsymbol{R}$ (B)
-2. -1.9800 1.
$R(C)$
-2. -1.9800 1.
$R(D)$
-2. -1.9801 1.
$\boldsymbol{R}$ (F)
-1. -1.0000 4. 10-3
$\boldsymbol{R}$ (G)
-1. -1.0000 4. 10-3
(B)
2. 2.0000 4. 10-3
(C)
2. 2.0000 4. 10-3
(D)
2. 2.0001 5. 10-3
(F)

1. 1.0000 4. 10-3
(G)
2. 1.0000 4. 10-3
$Z$ (B)
3. 

-2.10-5
-
$Z(C)$
0.
-2.10-5
-
$Z$ (D)
0.
-2.10-5
$Z(F)$
0.
-2.10-8
$Z(G)$
0.
-2.10-8

### 10.2 Remarks

The bad precision recorded on $R(B), R(C), R(D)$ is explained by the fact why $B, C$ and $D$ are nodes of the edge, therefore flows are not realised on adjacent elements in the direction variation in temperature (direction $R$ ).

This phenomenon is not found on, because is balanced by $1 / r$.
10.3 Parameters
of execution
Version: 4.00.02

Machine: CRAY C90
System UNICOS:
8.04

Obstruction memory:
8 megawords
Time CPU To use:
5.8 seconds

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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV100 Rolls subjected to boundary conditions
Date:
22/12/98
Author (S):
X. DESROCHES Key
:
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## 11 Modeling

E
11.1 Characteristics of modeling

## AXIS-FOURIER (TRIA3)

$y$ (Z)

## 50 subdivisions in $R$

R Z node
To 0. 0. N1
G
D
B 1. 0. N151
E
C 1.0 .5
N152
D 1. 1. N153
C
E 0.1. N3
F 0.5
0. N76
$X(R)$
G 0.5

1. N78

With
F
B

The axes of description of the grid are $X(R)$ and $y(Z)$.
Mode - Fourier: $2 \boldsymbol{T}(A)=0$.
$S=0$.
on all the field
[BC]:
$=2$.
[CD]:
$H=2$. Text $=2$.

### 11.2 Characteristics

grid
A number of nodes: 153
A number of meshs and types: 200 TRIA3

### 11.3 Functionalities

tested
Orders
Keys"THERMAL"AFFE_MODELE "AXIS_FOURIER"ALL
[U4.22.01]
AFFE_CHAR_THER TEMP_IMPO
NODE
[U4.25.02]
FLUX_REP
GROUP_MA

## EXCHANGE

GROUP_MA

SOURCE<br>ALL<br>CALC_MATR_ELEM "RIGI_THER" MODE_FOURIER<br>[U4.41.01]<br>CALC_VECT_ELEM "CHAR_THER"

[U4.41.02]
ASSE_MATRICE

[U4.42.02]<br>ASSE_VECTEUR

[U4.42.03]
FACT_LDLT
[U4.51.01]
RESO_LDLT

[U4.51.02]<br>CALC_CHAM_ELEM "FLUX_ELNO_TEMP"<br>MODE_FOURIER<br>[U4.61.01]

### 11.4 Remarks

The number of the mode of Fourier not affecting the loading, key word MODE_FOURIER is not necessary in order CALC_VECT_ELEM.
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Code_Aster ${ }^{\circledR}$
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12 Results of modeling $E$
12.1 Values
tested
Identification Reference
Aster \%
difference
T(B) 1. 0.99950 .049
T (C) 1. 0.99997 .103
T(D) 1. 1.00030 .033
$T(F) 0.25$
0.2500 9.10-3

T(G) 0.25
0.2498 -0.077
$\boldsymbol{R}$ (B)
-2. -1.977
-1.14
$R(C)$
-2. -1.9819

### 0.90

$\boldsymbol{R}$ (D)
-2. -1.9856
0.72
$\boldsymbol{R}$ (F)
-1. -0.993
0.68
$\boldsymbol{R}$ (G)
-1. -1.007
0.68
(B)
2. 1.9992 -0.04
(C)
2. 2.0000 -
(D)
2. 2.00080 .04
(F)

1. 1.00040 .04
(G)
2. $0.9995-0.05$

Z (B)
0.
-4.10-3
$Z(C)$
0.
-4.10-3
Z (D)
0.
-4.10-3
$Z(F)$
0.
1.10-3
$Z(G)$
0.
1.10-3

### 12.2 Remarks

The bad precision recorded on $R(B), R(C), R(D)$ is explained by the fact why $B, C$ and $D$ are nodes of the edge, therefore flows are not realised on adjacent elements in the direction variation in temperature (direction $R$ ).

This phenomenon is not found on, because is balanced by $1 / r$.

### 12.3 Parameters

of execution
Version: 4.00.02

Machine: CRAY C90
System UNICOS:
8.04

Obstruction memory:
8 megawords
Time CPU To use:
6.1 seconds

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X. DESROCHES Key
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13 Summary of the results
This problem is correctly solved:

- whatever the number of harmonic of Fourier, - by the various types of elements (degree 1 or 2).

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Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
TPLV101 - Stationary thermics with exchange between walls
Date:
03/06/03
Author (S):
Key J.P. LEFEBVRE
:
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Organization (S): EDF-R \& D /AMA

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
Document: V4.04.101

TPLV101 - Stationary thermics with condition of exchange between walls in opposite

## Summary:

This elementary test makes it possible to deal with stationary problem in thermics bringing into play two fields separated by imposing a boundary condition of the type exchanges between walls.

For modelings presented here, the results obtained by Code_Aster are identical to the reference calculated analytically.

Handbook of Validation<br>V4.04 booklet: Stationary thermics of the voluminal structures HT-66/03/008/A

## Code_Aster ${ }^{\circledR}$

Version
6.4

Titrate:
TPLV101 - Stationary thermics with exchange between walls
Date:
03/06/03
Author (S):
Key J.P. LEFEBVRE

## : <br> V4.04.101-B Page:

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1
Problem of reference

### 1.1 Geometry

H
$G$
$H$
1
1
2
$G 2$
$D$
$D$
$D$
1
$C 1$
2
$C 2$
$L$
$J 1$
$J 2$
$I$
$I$
1
1
2
$Z$
$y$
$F 1$


#### Abstract

F E 2 1 E2 X A1 B With 1 2 B2 $L$ $L$ Height L = 3. $m$ Width $I=1 . m$


## 1.2 <br> Material properties

Voluminal heat $C P=0$.
Thermal conductivity $K=1 . W / m^{\circ} C$

## 1.3 <br> Boundary conditions and loadings

Outgoing flow through plan B1F1G1C1 identical to flow
entering through plan A2E2H2D2
Temperature imposed in A1
$T=0 .{ }^{\circ} \mathrm{C}$
Temperature imposed in B2
$T=4.5^{\circ} \mathrm{C}$
Normal flow imposed on plan B2F2G2C2
$=3 . \mathrm{W} / \mathrm{m} 2$
Normal flow imposed on plans C1G1H1D1 and C2G2H2D2
$=6 . \mathrm{W} / \mathrm{m} 2$
Normal flow imposed on plans E1F1G1H1 and E2F2G2H2
$=2 . \mathrm{W} / \mathrm{m} 2$
Source imposed in field 1
$S$

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2
Reference solution

## 2.1 <br> Method of calculation used for the reference solution

One has a simple analytical solution, since it is about exhiber a harmonic function and to adjust the source associated in each field:

- in field 1: $T(X, y, Z)=T(1$

With)
2
2
2
$+X+y+Z$, (in the reference mark of A1 origin),
1

- in field 2: $T(X, y, Z)=T(2$

With)
2
2
2
$+X+y+Z$, (in the reference mark of origin $A$
2
2).

One deduces the values from them from s1 and $s 2, s 1=6 ., s 2=5 . \mathrm{W} / \mathrm{m} 3$.

## 2.2 <br> Results of reference

Temperatures at the points of plans B1F1G1C1 and A2E2H2D2

## 2.3 <br> Uncertainty on the solution

Analytical solution.
Handbook of Validation
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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Modeling 2D:
D1

```
C
D
1
2
C2
N14
N33
N16
N36
N9
N28
NII
N31
N3
N22
N6
N26
N2
N19
With
With
1
B1
2
B
2
3.2
Boundary conditions and loadings
Outgoing flow through wall B1C1 identical to entering flow
through wall A2D2
Temperature imposed in A1
T=0. ' C
Temperature imposed in B2
T=4.5}\mp@subsup{}{}{\circ}\textrm{C
Normal flow imposed on wall B2C2
= 3.W/m2
Normal flow imposed on plans C1D1 and C2D2
= 6.W/m2
```

Source imposed in field 1
S

Source imposed in field 2
s2

## 3.3

Characteristics of the grid
6 QUAD8
36 nodes

## 3.4 <br> Functionalities tested

Order

Key word factor
Simple key word
Argument
DEFI_MATERIAU THER
LAMBDA

CP

AFFE_CHAR_THER SOURCE

TEMP_IMPO
$F L U X \_R E P$

## ECHANGE_PAROI

LIAISON_GROUP<br>STATIONARY THER_LINEAIRE TEMP_INIT<br>"YES"<br>INCREMENT

Handbook of Validation<br>V4.04 booklet: Stationary thermics of the voluminal structures HT-66/03/008/A

## Code_Aster ${ }^{\circledR}$

Version
6.4

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Date:
03/06/03
Author (S):
Key J.P. LEFEBVRE

## :

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## 4 <br> Results of modeling $A$

### 4.1 Values

tested

## Identification

Reference
Aster \%
difference
Temperature
node N2 (B1) 1.00
1.00
0.0

N3 node
2.00
2.00
0.0

N6 node
1.25
1.25
0.0

N11 node

N26 node

N31 node
4.25
4.25
0.0

N28 node
6.00
6.00
0.0

N36 node
8.25
8.25
0.0
node N33 (D2) 11.00
11.00
0.0

### 4.2 Remarks

The functions of form of element QUAD8 being of order 2, it is natural to obtain the solution of reference which is expressed in the form of a polynomial of order 2.

The command file deposited contains a list of moments and calls the order THER_LINEAIRE to carry out a transitory calculation which is not of interest, the coefficient of voluminal heat being taken equal to 0 .

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03/06/03
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Key J.P. LEFEBVRE
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## 5 Modeling <br> B

## 5.1 <br> Characteristics of modeling

## Modeling 3D:

A30B44
B1
A10
A35
B40
A39
B28
A7
A15
B8
A3
B4
5.2
Characteristics of the grid
6 HEXA20
88 nodes
5.3
Functionalities tested
Order
Key word factorSimple key word
Argument
DEFI_MATERIAUTHER LAMBDA
$C P$
AFFE_CHAR_THER SOURCE
TEMP_IMPO

# AFFE_CHAR_THER_F ECHANGE_PAROI 

LIAISON_GROUP

THER_LINEAIRE
STATIONARY TEMP_INIT
"YES"
INCREMENT

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## Code_Aster ${ }^{\circledR}$

Version
6.4

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## 6

Results of modeling B

### 6.1 Values

tested

## Identification

Reference

```
Aster
% difference
Temperature
```

node A2 (B1) 1.00
1.00
0.0
node A3 (F1) 2.00
2.00
0.0
A7 node
3.00
3.00
0.0
Al0 node
1.25
1.25
0.0
Al5 node
2.25
2.25
0.0
A22 node
5.00
5.00
0.0
A26 node
3.25
3.25
0.0
A30 node
5.25
5.25
0.0
node A35 (G1) 11.00
11.00
0.0
A39 node
8.25
8.25
0.0
node B1 (A2) 2.00
2.00
0.0
B44 node
11.25
11.250.0

### 6.2 Remarks

The functions of form of element HEXA20 being of order 2, it is natural to obtain the solution of reference which is expressed in the form of a polynomial of order 2.

The command file deposited contains a list of moments and calls the order THER_LINEAIRE to carry out a transitory calculation which is not of interest, the coefficient of voluminal heat being taken equal to 0 .

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## 7 <br> Summaries of the results

Two modelings with elements of order 2 lead in an exact way to the solution analytical and the establishment of the boundary conditions of the type ECHANGE_PAROI validates.

## Handbook of Validation

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## Code_Aster ${ }^{\circledR}$

Version
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Titrate:
TPLV102 - Transport of heat by convection

Date:
03/06/03
Author (S):
J.P. LEFEBVRE, G. BERTRAND Clé
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# Handbook of Validation 

V4.04 booklet: Stationary thermics of the voluminal structures
Document: V4.04.102

TPLV102-Transport of heat by convection in a parallelepiped

## Summary:

This functionality was developed in the code in order to be able to test the nonsymmetrical matrices.
Stationary thermal calculation is carried out on elements of the quadrangle type to 4 nodes.

## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures HT-66/03/008/A

Code_Aster ${ }^{\circledR}$
Version
6.4

## Titrate:

TPLV102-Transport of heat by convection

Date:<br>03/06/03<br>Author (S):<br>J.P. LEFEBVRE, G. BERTRAND Clé<br>:<br>V4.04.102-B Page:<br>2/6<br>\section*{1<br><br>Problem of reference}<br>\subsection*{1.1 Geometry}

One considers the plane thermal problem of a square cavity (on side equal to 1 ) where heat propagate:

- by convection (i.e the particles constituting the medium of the cavity move at a speed $U$ supposed here constant); speed $U$ is supposed to form an angle of $67.5^{\circ}$ with axis $X$, - by conduction.


## 1.2 <br> Material properties

One takes $C P=1$
106
= -
from where a diffusivity $==-$
106
CP
U. L
and as one takes $U=$ one has the Peclet number $p=$
= +
1
106
(L is the length
characteristic, here $L=1$.).

## 1.3

Boundary conditions and loadings
On segments $A B$ and $B C$, one imposes a temperature $T=1$.
On segment $A E$, one imposes a temperature $T=0$.
On the 2 other sides, one in the condition by defect, namely, one is with null flow.
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Code_Aster ${ }^{\circledR}$
Version
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Titrate:
TPLV102-Transport of heat by convection

Date:
03/06/03
Author (S):
J.P. LEFEBVRE, G. BERTRAND Clé
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2
Reference solution
2.1

Method of calculation used for the reference solution
The reference solution is that recommended by Hughes and Brooks in their article referred to bibliographical [bib1].

One can take as exact solution the field of temperature of the border projected upstream on
the border downstream according to the direction speed.

## 2.2

Results of reference
One tests the temperatures on the border between points $E$ and $D$.

## 2.3 <br> Uncertainty on the solution

Analytical solution.

### 2.4 References <br> bibliographical

## [1]

T.J.R. HUGHES, A. BROOKS "A multidimensional design with No crosswind diffusion" T.J.R. HUGHES ED., Finite Element Methods for convection doninated flows, AMD vol. 34 (ASME, New York (1979)).

Handbook of Validation<br>V4.04 booklet: Stationary thermics of the voluminal structures HT-66/03/008/A

Code_Aster ${ }^{\circledR}$
Version
6.4

Titrate:
TPLV102-Transport of heat by convection

Date:
03/06/03
Author (S):

J.P. LEFEBVRE, G. BERTRAND Clé

```
:
```

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## 3 Modeling

With

## 3.1 <br> Characteristics of modeling

Modeling is plane: the grid consists of 100 square elements QUAD4 of equal sizes, and 50 elements SEG2 on the borders.

- the temperature of 0.0 is imposed on the GROUP_NO d2,
$\cdot$ the temperature equal to 1.0 is imposed on the GROUP_NO C1 and C4.


## 3.2 <br> Characteristics of the grid

50 SEG2, 100 QUA4

3.3 Functionalities<br>tested<br>Orders

```
AFFE_MODELE AFFE
MODELING "PLAN"
AFFE_CHAM_NO
SIZE
"DEPL_R"
AFFE_CHAR_THER CONVECTION SPEED
(AFFE_CHAM_NO)
MACRO_MATR_ASS MATR_ASS
OPTION
"RIGI_THER_CONV_D"
CALC_VECT_ELEM
OPTION
"CHAR_THER"
```


## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures HT-66/03/008/A

## Code_Aster ${ }^{\circledR}$

Version
6.4

Titrate:
TPLV102-Transport of heat by convection

Date:
03/06/03
Author (S):
J.P. LEFEBVRE, G. BERTRAND Clé
:

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## 4

Results of modeling $A$

### 4.1 Values

tested

## Identification Reference

```
Aster %
difference
T(N31) x=1.0
1.
1.0009
0.0009
T (N29) x=0.8
1.
0.9920
0.008
T (N27) x=0.6
1. 1.0495 0.0495
T(N25) x=0.4
1. 0.8450.155
T(N23) x=0.2
0.0.055 0.155
T(N1) x=0.
0.0.
0.155
```

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## Code_Aster ${ }^{\circledR}$

Version
6.4

Titrate:
TPLV102 - Transport of heat by convection

Date:
03/06/03
Author (S):
J.P. LEFEBVRE, G. BERTRAND Clé
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5
Summary of the results

Good nonsymmetrical matrix installation of for a plane thermal problem.
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV103 - Infinite cylinder in anisotropic stationary thermics
Date:
23/09/02
Author (S):
C. Key DURAND
:
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Organization (S): EDF/AMA

## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures
Document: V4.04.103

## TPLV103 - Infinite cylinder in stationary thermics anisotropic

## Summary:

This test the purpose of which relates to it thermal linear stationary and transitory be to test the anisotropy cylindrical.

Two modelings are carried out:

- a first into voluminal, - a second in plane 2D.

The results obtained are in perfect agreement with the analytical values.
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Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV103-Infinite cylinder in anisotropic stationary thermics
Date:
23/09/02
Author (S):
C. Key DURAND
:
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1
Problem of reference
1.1 Geometry

C

## B

1/4 of cylinder

```
G
F
E
With
I
0
H
D
X0
```

In the reference mark (X0, Y0, Z0), the points have as co-ordinates:
$C(0 ; 2,1)$
$D(2 ; 0 ; 0)$
E (0; 2; 0)
$F(1 ; 0 ; 1)$
O (0;0;0)
To (2;0;1)
B (2; 2; )
1
$G(0 ; 1 ; 1)$
$H(1 ; 0 ; 0)$
$I(0 ; 1 ; 0)$

## 1.2

Material properties
Anisotropic material, direction privileged along the axes of the cylindrical reference mark $(U, U, U$ $\boldsymbol{R}$

## Z).

3
$R=1$
$=0.5$
$Z=3 W /^{\circ}$
$m C$
$C=2 \mathrm{~J} / \mathrm{m}^{\circ} \mathrm{C}$
1.3

## Boundary conditions and loadings

face AFHD:
Temperature imposed on $100^{\circ} \mathrm{C}$
face CGIE:
Temperature with $0{ }^{\circ} \mathrm{C}$ others faces:
Neumann

### 1.4 Conditions <br> initial

To make this stationary calculation, a transitory calculation is made for which the boundary conditions are
constants in time. This makes it possible to test elementary calculations of mass and rigidity intervening in the 1st member as well as 2nd.

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2
Reference solution

## 2.1

Method of calculation used for the reference solution
Analytical solution.

Temperature varying linearly in.
in $(R, Z)$
$T$ () [
$=T(C)-T(A)] 2$
$+T(A)$
(A)

1
$Y=$
$T$
1
2
=-
$\boldsymbol{T C - T A}$.
R
$\boldsymbol{R}(\boldsymbol{A})$ [()
()]

## 2.2

Results of reference
Temperatures at points A and B, flow following Y to point A.

100
T ()
$T o=100$
T ()
B $=50$
(A) $Y=$
15.915

2

## 2.3

Uncertainty on the solution
Analytical solution.
2.4 Referencesbibliographical
[1]NR. RICHARD: "Development of the thermal anisotropy in the software Aster", Notetechnique HM-18/94/0011.
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3 Modeling
With
3.1
Characteristics of modelingdiagram in time forced on 1 to test the calculation of the second member in transient.
3.2
Characteristics of the grid
Regulated in 250 HEXA8 ( 5 elements on the edges HD and DM, 10 elements on DF) by IDEAS.
3.3 Functionalities ..... tested

Orders<br>Key word factor<br>Single-ended spanner word<br>Argument<br>DEFI_MATERIAU<br>THER_ORTH<br>AFFE_CARA_ELEM<br>SOLID MASS<br>ANGL_AXE<br>(0. , 90.)<br>SOLID MASS<br>ORIG_AXE<br>(0., 0., 0.)<br>THER_LINEAIRE<br>PARM_THETA<br>0.8<br>CARA_ELEM<br>CALC_CHAM_ELEM<br>CARA_ELEM<br>OPTION<br>"FLUX_ELNO_TEMP"<br>\section*{4}<br>Results of modeling $A$<br>4.1 Values<br>tested<br>Identification Reference<br>Aster \%<br>difference<br>$T(A) * N 1$<br>100<br>100<br>0<br>T (B) N133<br>50

### 4.2 Remarks

The symmetry of the grid makes that the solution $T$ with the nodes of the network is exact, but in elements, the extrapolated solution is not exact.

Flow is calculated by Aster at the points of integration of the elements then deferred to the nodes by extrapolation. As flow is not uniform, this extrapolation involves a difference enters calculation and reference.

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## 5 Modeling

B
5.1

Characteristics of modeling
Similar to the modeling A, but solved in plan HIED.

## 5.2

Characteristics of the grid
Grid IDEAS with 50 QUAD4 and 66 nodes.

5.3 Functionalities<br>tested<br>Orders<br>Key word factor<br>Single-ended spanner word<br>Argument<br>DEFI_MATERIAU<br>THER_ORTH<br>AFFE_CARA_ELEM<br>SOLID MASS<br>ORIG_AXE<br>(0., 0.)<br>THER_LINEAIRE<br>PARM_THETA<br>0.8<br>--<br>CARA_ELEM<br>CALC_CHAM_ELEM<br>CARA_ELEM<br>--<br>OPTION<br>"FLUX_ELNO_TEMP"<br>\section*{6}<br>Results of modeling B<br>6.1 Values<br>tested<br>Identification Reference<br>Aster \%

```
difference
T(A)*N6
100
100
0
T (B) N36
5 0
5 0
0
R() R
WITH Y
15.915515.950
0.22
*: imposed temperature
```


### 6.2 Remarks

The symmetry of the grid makes that the solution $T$ with the nodes of the network is exact. But in elements, the extrapolated solution is not exact.

Flow is calculated by Aster at the points of integration of the elements then deferred to the nodes by extrapolation. As flow is not uniform, this extrapolation involves a difference enters calculation and reference.

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Summary of the results
Key words ANGL_AXE and ORIG_AXE introduced into order AFFE_CARA_ELEM are tested in 3D and plane 2D for an anisotropic problem of thermics.
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Version
6.4

Titrate:
TPLV105-Stationary nonlinear thermics in pointer
Date:
11/05/04
Author (S):
C. DURAND, F. LEBOUVIER Key
:
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Organization (S): EDF-R \& D /AMA, DeltaCAD

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
Document: V4.04.105

TPLV105 - Stationary nonlinear thermics in
locate mobile: simulation of the Varestrain test

## Summary:

This test presents thermal simulation by finite elements of the Varestraint test. This test of weldability is
employee to characterize resistance to the hot cracking of materials.
This test makes it possible to test a nonlinear thermal problem formulated in a pointer under condition of stationnarity.

In this test only one modeling is carried out, it acts of a thermal modeling PLAN associated with operator THER_NON_LINE_MO allowing to calculate the nonlinear stationary response with one mobile loading.

Handbook of Validation

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Version
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1
Problem of reference

### 1.1 Geometry

The studied geometry is a 200 mm length parallelepipedic plate, of 60 mm width and 7 mm thickness.

Torch TIG
Feel displacement
torch

## Thickness

Width of
Length of
test-tube
the test-tube
the test-tube
7 mm
60 mm
200 mm

## 1.2

Properties of material

The material considered is a forged austenitic stainless steel of 316L type (Z2CND17-12).
For nonlinear thermal modeling thermal conductivity and the product density
heat-storage capacity C vary according to the temperature. Their values are given in table below:

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## :

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## $T$

C
Temperature $\left({ }^{\circ} \mathrm{C}\right)$
Thermal conductivity
(J/(
3
mm C
${ }^{\circ}$ ) )
Enthalpy
(W/(mm C
${ }^{\circ}$ )
(
3
J/mm)
20
14.0 E-3
36.00 E-4
0.0

## 5014.4

E-3

$$
100
$$

15.2 E-3
39.05 E-4
15015.8

E-3
200
16.6 E-3
41.63 E-4
25017.3

E-3
300
17.9 E-3
43.00 E-4
35018.6

E-3
400
19.2 E-3
43.90 E-4
45019.9

E-3
500
20.6 E-3
44.50 E-4
55021.2

E-3
600
21.8 E-3
44.95 E-4
65022.4

E-3

## Thermal conductivity

)
2
/
C
0.05
m
/
m

```
W
0 . 0 4
I
that (
m
0 . 0 3
0 . 0 2
T
I
v
I
T
é
T
her
0 . 0 1
onduc
C
0
0
200
4 0 0
6 0 0
800
1000
1200
1 4 0 0
1600
Temperature ( }\mp@subsup{}{}{\circ}\mathrm{ C)
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```

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## density $X$ specific heat

0.0050
0.0048
)
0.0046

C
$1 .{ }^{\circ}$
m
0.0044

J
$/ m$
$p$
0.0042

R
H
O
C
0.0040
0.0038
0.0036

0
200
400
600
800
1000
1200
1400
1600
Temperature $\left({ }^{\circ} \mathrm{C}\right)$

## Enthalpy <br> 9 <br> 8 <br> 7 <br> 2

```
)
m
6
J
/
m
5
E(
pi
4
T
hal
N
3
E
2
0
0
200
4 0 0
6 0 0
800
1000
1 2 0 0
1 4 0 0
1 6 0 0
Temperature ( }\mp@subsup{}{}{\circ}\textrm{C}
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```

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1.3

Boundary conditions
The parameters of welding are presented in the table below:

V<br>I<br>$U$<br>(Tape speed<br>Diameter of electrode<br>(Intensity)<br>(Voltage)<br>part)<br>200A<br>13 V<br>$14 \mathrm{~cm} / \mathrm{min}$<br>3 mm

The upper surface of the plate is subjected to the action of a torch. This torch is placed at center plate, to 15 mm of the edge, and parallel to moves its length at constant speed $(14 \mathrm{~cm} / \mathrm{min})$ until 85.5 mm of the edge corresponding to the position of extinction of the torch.

### 1.4 Conditions

initial
None.

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Titrate:
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Date:

## 2

Reference solution

## 2.1 <br> Method of calculation

Method of calculation of the density flux constant to impose, boundary conditions imposed, as well as the test results are presented in [bib1].

## 2.2

Sizes and results of reference
Temperatures on the higher and lower face of the plate close to the torch

### 2.3 References <br> bibliographical

[1]
D. BUI: "Thermal Simulations by finite elements of the Varestraint test", Note HI74/97/09

## Handbook of Validation

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TPLV105 - Stationary nonlinear thermics in pointer
Date:
11/05/04
Author (S):
C. DURAND, F. LEBOUVIER Key
3 ModelingWith
3.1
Characteristics of modeling
Y Modeling "PLAN" (QUAD8):
WITH LSUP B
I LSUP J X
DC1
DC2
LINF
82. mm
90. mm
Central zone of the grid
Torch
B LZA I
The center of the torch is placed has $x=86$. mm of the left edge.
Boundary conditions:

- Côté DC1: adiabatic condition (flux=0)
- Côté DC2: imposed temperature $20^{\circ} \mathrm{C}$
- Surface upper LSUP: this part is made up on the sides AB and IJ
- Nonlinear Flow imposed QINOX (see table and figure below)
- Convectif Exchange: $\mathrm{H}=15 x 10-6 \mathrm{~W}\left(\mathrm{~mm}^{2} .{ }^{\circ} \mathrm{C}\right)$, Text $=20^{\circ} \mathrm{C}$.
- Lower Surface: LINF
- Nonlinear Flow imposed QINOX (see table and figure below)
- Convectif Exchange: $\mathrm{H}=15 x 10-6 \mathrm{~W}\left(\mathrm{~mm}^{2} .{ }^{\circ} \mathrm{C}\right)$, Text $=20^{\circ} \mathrm{C}$.
- Tape speed of the $V=-2.33 \mathrm{~mm} / \mathrm{s}$ part
Loading: Density flux brought by the torch
- Density flux imposed $Q=19.62 \mathrm{~W} / \mathrm{mm}^{2}$ on the with dimensions LZA (B I)
Temperature
QINOX
Temperature
QINOX
$\left({ }^{\circ} \mathrm{C}\right)$
Nonlinear flow
$\left({ }^{\circ} \mathrm{C}\right)$
Nonlinear flow
(W/mm ${ }^{2}$ )
(W/mm ${ }^{2}$ )
20 0.00E+00
800 -7.96E-02
100-1.76E-03
900-1.08E-01200-5.04E-03
1000-1.46E-01
300-9.80E-031100 -1.92E-01400-1.63E-02
1200-2.48E-01
500-2.59E-02
1300-3.17E-01600-3.89E-02
1400-3.99E-01700-5.64E-02
1500-4.97E-01
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Date:
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Author (S):<br>C. DURAND, F. LEBOUVIER Key<br>:<br>V4.04.105-A Page:<br>8/10

## Non-linear flow (QINOX)

0
200
400
600
800
1000120014001600
0
-0.1
2
) -0.2
m
/m
W-0.3
$X$
(
L
$U$
F-0.4
-0.5
-0.6
Temperature $\left({ }^{\circ} \mathrm{C}\right)$

## 3.2 <br> Characteristics of the grid <br> A number of meshs: 144 (QUAD8) <br> A number of nodes: 565

3.3 Functionalities<br>tested<br>Orders

DEFI_MATERIAU<br>THER_NL LAMBDA<br>BETA

AFFE_MODELE AFFE
"THERMAL" PHENOMENON
MODELING
"PLANE"
AFFE_CHAR_THER_F
TEMP_IMPO GROUP_NO
TEMP
FLUX_NL
GROUP_MA
FLUN
EXCHANGE
GROUP_MA
COEF_H
TEMP_EXT
FLUX_REP
GROUP_MA
FLUN
CONVECTION
SPEED
THER_NON_LINE_MO CONVERGENCE
CRIT_TEMP_RELA
CRIT_ENTH_RELA
ITER_GLOB_MAXI
TEMP_INIT
MODEL
CHAM_MATER
EXCIT

## CHARGE

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3.4Sizes tested and results
Identification
Size Reference
Aster
\% Difference
$\left({ }^{\circ} \mathrm{C}\right)$$\left({ }^{\circ} \mathrm{C}\right)$
N1 ( $X=82, Y=0$ )
TEMP
1755.01756.110.063
N2 ( $X=83, Y=0$ )
TEMP1920.0
1919.29
0.037
N3 ( $X=84, Y=0$ )
TEMP
1910.0

```
1908.63
0 . 0 7 2
N7 (X=88, Y=0)
TEMP
1494.0
1493.46
0.036
N8 (X=89,Y=0)
TEMP
1300.0
1297.51
0 . 1 9 1
N173 (X=46.93, Y=0)
TEMP
1160.0
1155.76
0.365
N174 ( }X=57.36,Y=0
TEMP
1215.0
1213.32
0 . 1 3 9
N175 (X=67.79, Y=0)
TEMP
1295.0
1291.86
0 . 2 4 3
N478 ( }X=10.43,Y=-7
TEMP
1007.0
1001.09
0 . 5 8 7
N522 ( X=52.14, Y=-7) TEMP
989.0
9 8 2 . 3 9
0 . 6 6 8
N559 (X=0,Y=-7)
TEMP
980.0
973.92
0 . 6 2 1
```


### 3.5 Remarks

In the table below we present the position of the nodes in the reference mark (xy) of the torch.

```
Nodes located
Nodes located on the left of
Nodes located on the left of under the torch (Zone 1) torch (Zone 2)
torch and in lower part of plate (Zone 3)
N1: \(x=-4 \mathrm{~mm}, y=0\)
N173: \(X=-39.0 \mathrm{~mm}, y=0\)
N478: \(X=-86.0 \mathrm{~mm}\)
N2: \(x=-3 \mathrm{~mm}, y=0\)
N174: \(X=-28.6 \mathrm{~mm}, y=0\)
N522: \(X=-81.0 \mathrm{~mm}\)
N3: \(x=-2 m m, y=0\)
N175: \(X=-18.2 \mathrm{~mm}, y=0\)
N559: \(X=-18.2 \mathrm{~mm}\)
N7: \(x=2 \mathrm{~mm}, y=0\)
N8: \(x=3 \mathrm{~mm}, y=0\)
```

$\boldsymbol{Y}$<br>$y$<br>Center torch<br>Zone 1<br>Zone 2<br>$X$<br>$X$<br>Zone 3

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Code_Aster ${ }^{\circledR}$

## Version

6.4

## Titrate:

TPLV105-Stationary nonlinear thermics in pointer

## Date:

11/05/04
Author (S):
C. DURAND, F. LEBOUVIER Key

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Code_Aster ${ }^{\circledR}$
Version
4.0

Titrate:
Stationary nonlinear Thermal TPLV106 in pointer

## Date:

07/12/98

## Author (S):

## F. WAECKEL, B. NEDJAR

Key:
V4.04.106-A Page:
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Organization (S): EDF/IMA/MMN
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## Document: V4.04.106

TPLV106-Stationary nonlinear thermics

## in pointer

## Summary:

This elementary test makes it possible to treat a reducible three-dimensional example with a problem a variable
of space in stationary nonlinear thermics in pointer (problem of convection-diffusion).
It also makes it possible to check the taking into account of a solid phase shift/liquid by Code_Aster.
The reference solution is analytical and the variations with the results obtained by Code_Aster are lower
to $1 \%$. The problem is modelled in the plane case.
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HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
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Titrate:
Stationary nonlinear Thermal TPLV106 in pointer
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Author (S):
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1
Problem of reference

### 1.1 Geometry

That is to say a bar moving, at the speed V , the right of conditions of temperatures imposed in $\mathrm{X}=0$ and $\mathrm{X}=\mathrm{L}$ expressed in a fixed reference frame (compared to the bar moving).
$T L=1000^{\circ} \mathrm{C}$
TT
$1=585^{\circ} \mathrm{C}$
$T$
$T=615^{\circ} \mathrm{C}$
2
$T 0=200^{\circ} \mathrm{C}$
liquid phase
solid phase
$\mathrm{X}=\mathrm{L}=1 \mathrm{~m}$
$\mathrm{X}=0$
V

### 1.2 Properties

## materials

- thermal conductivity is constant: $\mathrm{K}=150 \mathrm{~W} / \mathrm{m}^{\circ} \mathrm{C}$
- the function enthalpy is such as:

C
$T$
;
TT
$S$
1
$(T)=C$
$T 1+C$

```
S
sl (T
T)
T
T
T
2
C T1 + C
2-1+
;
2
S
sl (T
T) Cl(T T)
T
T2
```

with the following values:
$C=C=1$
$7 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}$
S
L
310
$C=8333$
$107 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}$
sl
$T 1=$
-
585 C
$T 2=$
。
615 C
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HI-75/98/040 - Ind A

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
Stationary nonlinear Thermal TPLV106 in pointer
Date:
07/12/98
Author (S):
F. WAECKEL, B. NEDJAR

Key:
V4.04.106-A Page:
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(T) 8

J/m3
7
6
5
$C l=C s$
4
3
Csl
2
1
Cs
00
200
400
600
800
1000
1200
1400
1600
$T^{\circ} C$
585
615
1.3

Boundary conditions and loadings
Temperatures imposed at the ends
$T=$
${ }^{\circ} \mathrm{C}$
for
$X=$

## Code_Aster ${ }^{\circledR}$

Version
4.0

Titrate:
Stationary nonlinear Thermal TPLV106 in pointer
Date:
07/12/98
Author (S):
F. WAECKEL, B. NEDJAR

Key:
V4.04.106-A Page:
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2

## Reference solution

## 2.1

## Method of calculation used for the reference solution

The result of reference is of the semi-analytical type. The equation 1D to be solved is as follows:
V
(T), - KT
$=$
$X$
, $x x$
0

## éq 2.1-1

with ( $T$
$=0$
=
$x=0$ )
$T$ and
$T x=L$ )
TL
by integrating the equation [éq 2.1-1] one obtains:
V (
$d T$
T) -
$=A$
éq 2.1-2
K
$d x$
V
where $A$ is a constant depending on the boundary conditions, of the report/ratio and of the function

K
enthalpy ( $T$ ).
This constant will be analytically given.
The equation [éq 2.1-2] led to:
(
$T X$ )
$d T$
$X=$
éq 2.1-3
V
With +
(T)

T0
K
who must check:
TL
$d T$
$L=$

## éq 2.1-4

V
WITH + (T)
T0
K
Knowing $T T, L V$
, $T$ and ( $T$ )
$0, L$
, the equation [éq 2.1-4] must give the value of the constant
of integration $A$.

However, it is difficult (even impossible) to determine this constant analytically, from where resort to a numerical resolution of the equation [éq 2.1-4] to determine $A$.
With the facts of the case ( $T, T, T, T, C=C, C \ldots$
0
L
1
2
$S$
$L$
sl
), we obtained the solution
(physics) of $A$ which takes the value $A=294,9117$.
From this constant, the analytical solution of the problem [éq 2.1-1] is analytical.
Handbook of Validation
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## Code_Aster ®

Version
4.0

Titrate:
Stationary nonlinear Thermal TPLV106 in pointer
Date:
07/12/98
Author (S):
F. WAECKEL, B. NEDJAR

Key:
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2.2

Results of reference
X-coordinate
Temperature
0.6
387.98514
0.7
451.51001
0.725
469.72232
0.750
488.97505
0.775
509.32766

## 3.1

Characteristics of modeling
Modeling 2D

## 3.2

Characteristics of the grid
80 QUAD8

### 3.3 Functionalities

tested
Orders
Key word factor
Single-ended spanner word

## Argument

Keys
DEFI_MATERIAU
THER_NL
LAMBDA
[U4.23.01]
BETA
THER_NON_LINE_MO

## CONVERGENCE

CRIT_TEMP_RELA:
1.E-4
[U4.33.04]
CRIT_ENTH_RELA:
1.E-4

ITER_GLOB_MAXI:
130
MODEL
CHAM_MATER
EXCIT
CHARGE
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HI-75/98/040 - Ind A

## Code_Aster ®

Version
4.0

Titrate:
Stationary nonlinear Thermal TPLV106 in pointer
Date:
07/12/98
Author (S):
F. WAECKEL, B. NEDJAR

Key:
V4.04.106-A Page:
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4
Results of modeling A
4.1 Values
tested
Identification

## Reference

Aster
\% difference
Temperature
N80 (X = 0.9875)
956.515
956.884
+0.039
N79 (X = 0.9750)
914.222

```
914.888
+0.073
N78 (X = 0.9625)
8 7 3 . 0 8 7
8 7 3 . 9 8 2
+0.103
N77 (X = 0.9500)
833.079
8 3 4 . 1 3 7
+0.127
N76 (X = 0.9375)
794.167
7 9 5 . 3 2 6
+0.146
N75 (X = 0.9250)
756.322
757.235
+0.159
N74 (X = 0.9125)
7 1 9 . 5 1 6
7 2 0 . 7 0 1
+0.165
N73 (X = 0.9000)
6 8 3 . 7 1 2
6 8 4 . 8 3 4
+0.164
N69 (X = 0.8500)
577.631
576.682
0 . 1 6 4
N67 (X = 0.8250)
553.587
553.507
0.014
N65 (X = 0.8000)
530.842
531.519
+0.128
N63 (X = 0.7750)
5 0 9 . 3 2 7
510.657
+0.261
N61 (X = 0.7500)
```


### 4.2 Parameters

of execution
Version: 4.0.5
Machine: CRAY C90
Obstruction memory:
8 MW
Time CPU To use:
59 seconds
5

## Summary of the results

The results are very satisfactory with variations with the reference solution lower than $1 \%$.
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HI-75/98/040 - Ind A

Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

TPLV304 Distribution of the temperature in a bar of square section
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

Handbook of Validation<br>V4.04 booklet: Stationary thermics of the voluminal structures<br>V4.04.304 document

TPLV304-Distribution of the temperature in one
bar square section

## Summary:

This test results from the validation independent of version 3 in linear stationary thermics.
It aims to validate the voluminal thermal elements under conditions of convection and of imposed temperature.

The reference solution is based on an analytical approach.
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures

## HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

## Titrate:

TPLV304 Distribution of the temperature in a bar of square section Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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## 1

Problem of reference

### 1.1 Geometry

Z<br>$L=203.2 \times 10-3 \mathrm{~m}$<br>$B=25.4 \times 10-3 \mathrm{~m}$<br>$T w$<br>X<br>H, Your<br>B<br>L<br>B<br>$\boldsymbol{Y}$

1.2

Properties of material
$=43.2675 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}$
Thermal conductivity

## 1.3

Boundary conditions and loadings
$\cdot$ temperature imposed on the face $y=0 ~ T w=37.78{ }^{\circ} \mathrm{C}$,
$\cdot=0$ on the face $y=L$,

- convection on the others faces:
$\mathrm{H}=5.678 \mathrm{~W} / \mathrm{m} 2{ }^{\circ} \mathrm{C}$,
- Your $=-17.780^{\circ} \mathrm{C}$.
1.4 Conditionsinitial
Without object.
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
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Code_Aster ${ }^{\circledR}$
Version
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Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé:V4.04.304-A Page:
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2Reference solution
2.1
Method of calculation used for the reference solution
The original reference solution given in the book [bib1] is based on an analytical approach.
This reference is quoted in the handbook of checking of ANSYS [bib2]
2.2
Results of reference
Temperature on the face $y=L$


## 2.3

Uncertainty on the solution
Unknown factor, it was not possible to get the original reference (delivers old, more published).

### 2.4 References <br> bibliographical

## [1]

ANSYS: "Checking manual", 1st edition, June 1, 1976
[2]
Kreith, F., "Principles of heat transfer", International Textbook Co., Scranton, Pennsylvania, 2nd Printing, 1959.
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV304 Distribution of the temperature in a bar of square section
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C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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## 3 Modeling <br> With

## 3.1 <br> Characteristics of modeling <br> 3D (HEXA27)

```
y
E
Face BFGC:
With
C
G
N102 N104 N103
N100
N108
N101
Limiting conditions:
B
X
F
N97
N99
N98
- face AEHD
T=37.78}\mp@subsup{}{}{\circ}\textrm{C
- face BFGC
= 0
- faces ABCD, ABFE,
H=5.678 W/m 2}\mp@subsup{}{}{\circ}\textrm{C
EFGH, DCGH
Text =-17.78 ' C
```


## 3.2 <br> Characteristics of the grid <br> A number of nodes: <br> 153 <br> A number of meshs and types: 8 HEXA27 (and 32 QUAD9)

### 3.3 Functionalities

tested
Orders

## THERMICS

3D
ALL

## AFFE_CHAR_THER

TEMP_IMPO

## EXCHANGE

## THER_LINEAIRE

EXCIT
CHARGE

## RECU_CHAMP <br> NUME_ORDRE

### 3.4 Remarks

Voluminal heat CP does not intervene in this test, but must be declared for Code_Aster. One $C P=1.0 \mathrm{~J} / \mathrm{m} 3^{\circ} \mathrm{C}$ takes.

The condition limits $=0$. is implicit on the free edges.

## Handbook of Validation

V4.04 booklet: Stationary thermics of the voluminal structures
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## Code_Aster ${ }^{\circledR}$

Version
5.0

## Titrate:

TPLV304 Distribution of the temperature in a bar of square section
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Author (S):
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4

# Results of modeling $A$ 

4.1 Values<br>tested<br>Identification Reference<br>Aster<br>Relative variation (\%)<br>Absolute deviation ( ${ }^{\circ} \mathrm{C}$ )

## difference tolerance difference tolerance

Temperature
$\left({ }^{\circ} \mathrm{C}\right)$
at the end of the bar
0.5
medium BF
N99
0.5

F N98
20.329
20.295
-0.166
$1 \%$
-0.0338
0.5
medium $F \boldsymbol{G}$
N101

$$
20.329
$$

20.327
-0.010
$1 \%$
-0.0021
0.5

G N103
20.329
20.295
-0.166
$1 \%$
-0.0338
0.5
medium GC
N104
20.329
20.327
-0.010
$1 \%$
-0.0021
0.5

C N102
20.329
20.295
-0.166
1\%
-0.0338
0.5

## medium CB

N100
20.329
20.327
-0.010
1\%
-0.0021
0.5
medium of the face
N108
20.329
20.359
0.146

1\%
0.0297
0.5

### 4.2 Remarks

The small differences which remain correspond to a variation in temperature in the section observed. What is in conformity with the modelled physical phenomenon.

### 4.3 Parameters <br> of execution

Version: 5.03

Machine: SGI-ORIGIN 2000-R12000

## Obstruction memory:

8 megawords
Time CPU To use: 2.09 seconds
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV304 Distribution of the temperature in a bar of square section
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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## 5

## Summary of the results

The results obtained are very satisfactory, the maximum change is $-0.166 \%$. Principal interest of it test is to check mesh HEXA27.

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV305 Heat gradient in a cylinder (Fourier)
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

# Handbook of Validation <br> V4.04 booklet: Stationary thermics of the voluminal structures <br> V4.04.305 document 

## TPLV305-Heat gradient in a cylinder (Fourier)

## Summary:

This test results from the validation independent of version 3 in linear stationary thermics.
It validates the thermal axis_fourier and voluminal elements with for boundary conditions of the temperatures imposed according to a harmonic function (mode 1).

It comprises two modelings, one 3D and the other using of the thermal elements axis_fourier.
The interest of this test is the validation of the thermal elements axis_fourier and the order COMB_CHAM_NO.

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:

### 1.1 Geometry

Z
D
$J$
I
H
C
ro
$y$
O
1.524

With
E
F
G
B
$y$
Z
$r 0=6.096 m$

## 1.2

Properties of material
$=1.7307 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}$
Thermal conductivity

## 1.3

Boundary conditions and loadings
The limiting condition is applied to the external surface of the cylinder, it breaks up into:

- a symmetrical limiting condition of revolution associated with harmonic 0:

CL1: $T 0=-17.778^{\circ} \mathrm{C}$

- a symmetrical limiting condition compared to associated harmonic 1:

CL2: $T 1 \cos =44.444 \cos \left({ }^{\circ} C\right)$

T0
$y$
$O$
$y$

- T1
$\boldsymbol{O}$

T1
$X$
$X$
T0 T1 cos

### 1.4 Conditions <br> initial <br> Without object. <br> Handbook of Validation <br> V4.04 booklet: Stationary thermics of the voluminal structures <br> HT-66/02/001/A

Code_Aster ${ }^{\circledR}$
Version
5.0

Titrate:
TPLV305 Heat gradient in a cylinder (Fourier)

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

## 2

Reference solution

## 2.1 <br> Method of calculation used for the reference solution

The original reference solution given in the book [bib1] is based on an analytical approach. This reference is quoted in the handbook of checking of ANSYS [bib2]

## 2.2

Results of reference

- Température at the points $A, E, F, G, B$ for mode 0 (CL1),
- Température at the points $A, E, F, G, B$ mode 0 and mode 1 recombined (CL1+CL2) for $=0^{\circ}$, $45^{\circ}, 90^{\circ}$ and $180^{\circ}$.


## 2.3 <br> Uncertainty on the solution

Unknown factor, it was not possible to get the original reference (delivers old, more published).

### 2.4 References <br> bibliographical

[1]
Kreith, F., "Principles of heat transfer", International Textbook Co., Scranton, Pennsylvania, 2nd Printing, 1959.
[2]
ANSYS: "Checking manual", 1st edition, June 1, 1976
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
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## Titrate:

TPLV305 Heat gradient in a cylinder (Fourier)
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Author (S):
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## :

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## 3 Modeling

With

## 3.1

Characteristics of modeling
AXIS_FOURIER (QUAD4)

```
y(Z)
D
J
I
H
C
1.5
X(R)
```

With
E
F
G
B
Limiting conditions:
Points
$X$
nodes
With
0.000
N1, N2

- dimensioned $A B, C D=0$
E
1.524

```
N3, N4
-dimensioned BC
F
3.048
N5, N6
. mode 0 T = -17.778
G
4 . 5 7 2
N7, N8
. mode (0 +1) T = -17.778+44.444 cos
B
6.096
N9, N10
```

3.2

Characteristics of the grid

A number of nodes:

10

A number of meshs and types: 4 QUAD4

### 3.3 Functionalities

tested
Orders

AFFE_MODELE
THERMICS
AXIS_FOURIER
ALL
AFFE_CHAR_THER
TEMP_IMPO
CALC_MATR_ELEM
RIGI_THER
MODE_FOURIER
CALC_VECT_ELEM

NUME_DDL RENUM "RCMK"<br>ASSE_MATRICE MATR_ELEM<br>NUME_DDL<br>ASSE_VECTEUR<br>VECT_ELEM<br>NUME_DDL<br>FACT_LDLT<br>MATR_ASSE<br>RESO_LDLT<br>MATR_FACT<br>CHAM_NO<br>COMB_CHAM_NO<br>COMB_FOURIER<br>NUME_MODE<br>TYPE_MODE<br>\section*{ENG}<br>COMB_R

## Handbook of Validation

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Code_Aster ${ }^{\circledR}$
Version
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TPLV305 Heat gradient in a cylinder (Fourier)
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20/09/02

\author{
Author (S): <br> C. DURAND, E. SCREW, F. LEBOUVIER Clé <br> : <br> V4.04.305-A Page: <br> 5/8 <br> ```
4 <br> Results of modeling $A$

``` \\ \subsection*{4.1 Values} \\ tested \\ Relative variation \% \\ Absolute deviation \\ Identification Reference \\ Aster difference tolerance difference tolerance \\ Temperature \\ \(\left({ }^{\circ} \mathrm{C}\right)\) \\ CL1 \((\) mode \(=0)\)
}

N1, N2
-17.778
-17.778
0.000 1\%
-1.14e-12
0.01

N3, N4
-17.778
-17.778
0.000 1\%
-9.09e-13
0.01

N5, N6
-17.778
-17.778
```

0.000 1%
-6.82e-13
0 . 0 1
N7, N8
-17.778
-17.778
0.000 1%
-3.41e-13
0 . 0 1
N9, N10 *
-17.778
-17.778
0 . 0 0 0 ~ 1 \% ~
0.000e+0
0 . 0 1
CL1+CL2 (mode 0 and 1)

```
\(=0^{\circ} \mathrm{N} 1, \mathrm{~N} 2\)
-17.778
-17.778
\(0.0001 \%\)
\(-1.14 e-12\)
0.01
\(N 3, N 4\)
-6.667
-6.667
\(0.0001 \%\)
\(1.820 e-8\)
0.01
\(N 5, N 6\)
4.444
4.444
0.000
\(1 \%\)
\(3.650 e-8\)
0.01
\(N 7, N 8\)
15.556
15.555
15.555
\begin{tabular}{l}
-0.006 \\
\(1 \%\) \\
\(-1.000 e-3\) \\
0.01 \\
\(N 9, N 10 * 26.667 \quad 26.666-0.004\) \\
\(1 \%\) \\
\(-1.000 e-3\) \\
0.01 \\
\(=45^{\circ} N 1, N 2\) \\
-17.778 \\
-17.778 \\
\(0.0001 \%\) \\
\(-1.14 e-12\) \\
0.01 \\
\(N 3, N 4\) \\
-9.921 \\
-9.921 \\
\(0.0031 \%\) \\
\(-3.370 e-4\) \\
0.01 \\
\(N 5, N 6\) \\
-2.064 \\
-2.065 \\
\(0.0331 \%\) \\
\(-6.730 e-4\) \\
0.01 \\
\(N 7, N 8\) \\
5.792 \\
5.792 \\
0.000 \\
\(1 \%\) \\
\(-1.040 e-5\) \\
0.01 \\
\(N 9, N 10\) \\
13.649 \\
13.649 \\
-0.003 \\
\(1 \%\) \\
\(-3.460 e-4\) \\
0.01 \\
\(=90^{\circ} N 1, N 2\) \\
-17.778 \\
-17.778 \\
\hline
\end{tabular}
1\%
-1.000e-3
0.01
N9, N10 * 26.667 26.666-0.004
1\%
-1.000e-3
0.01
\(=45^{\circ} \mathrm{N} 1, \mathrm{~N} 2\)
- 17.778
-17.778
0.000 1\%
-1.14e-12
0.01
N3, N4
-9.921
-9.921
\(0.0031 \%\)
-3.370e-4
0.01
N5, N6
-2.064
-2.065
\(0.0331 \%\)
-6.730e-4
0.01
N7, N8
5.792
5.792
0.000
1\%
-1.040e-5
0.01
N9, N10
13.649
13.649
-0.003
1\%
-3.460e-4
0.01
\(=90^{\circ} \mathrm{N} 1, \mathrm{~N} 2\)
-17.778
-17.778

\author{
0.000 1\% \\ \(-1.14 e-12\) \\ 0.01 \\ N3, N4 \\ -17.778 \\ -17.778 \\ \(0.0001 \%\) \\ -9.09e-13 \\ 0.01 \\ N5, N6 \\ -17.778 \\ -17.778 \\ 0.000 1\% \\ -5.68e-13 \\ 0.01 \\ N7, N8 \\ -17.778 \\ -17.778 \\ 0.000 1\% \\ -2.27e-13 \\ 0.01 \\ N9, N10 \\ -17.778 \\ -17.778 \\ 0.000 1\% \\ 2.27e-13 \\ 0.01 \\ \(=180^{\circ} \mathrm{N} 1, \mathrm{~N} 2\) \\ -17.778 \\ -17.778 \\ 0.000 1\% \\ -1.14e-12 \\ 0.01 \\ N3, N4 \\ -28.889 \\ -28.889 \\ 0.000 1\% \\ -1.820e-8 \\ 0.01 \\ N5, N6 \\ -40.000 \\ -40.000 \\ 0.000 1\%
}
```

-3.650e-8
0.01
N7, N8
-51.111
-51.111
0.000 1%
1.040e-6
0.01
N9, N10
-62.222
-62.222
0.000 1%
2.27e-13
0.01

* imposed temperatures

```

\subsection*{4.2 Parameters}
of execution
Version: 5.03

Machine: SGI - ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 3.12 seconds

\section*{Handbook of Validation}

V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
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:

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\section*{5 Modeling}

B

\section*{5.1}

\section*{Characteristics of modeling}

3D (PENTA6, HEXA8)
Z
\(=180^{\circ}\)
N155
\(=90^{\circ}\)
N273
N145
N265
N139
N274
N243
N133
N197
N266
N244
N123
N69
N49
N57
N65
N198
N53
\(=45^{\circ}\)
\(X\)
N124
N17
N54
N5
N3
N18
```

z=0
z=1.524
N6
N70
N69
y
N4
45
N50
N49
Limiting conditions:
N58
N57
N66
N65

- external face (N8, N155, N273)
N134
N133
T=-17.778 + 44.444 cos
90
N140
N139
- face interns (N8, N123, N273)
N146
N145
N155
N154
= 0

```

\section*{5.2}
```

Characteristics of the grid
A number of nodes:
274
A number of meshs and types: 128 (16 PENTA6, 112 HEXA8)

```

\subsection*{5.3 Functionalities}
```

tested
Orders

```

\author{
AFFE_MODELE \\ AFFE \\ THERMICS \\ 3D \\ \section*{AFFE_CHAR_THER} \\ TEMP_IMPO \\ \section*{THER_LINEAIRE} \\ EXCIT \\ CHARGE \\ RECU_CHAMP \\ NUME_ORDRE
}

\subsection*{5.4 Remarks}

Calculations were carried out by considering complete loading CL1+CL2:
Timp \(=-17.778+44.444 \cos\)
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
TPLV305 Heat gradient in a cylinder (Fourier)
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.04.305-A Page:
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6
Results of modeling B

\subsection*{6.1 Values}
tested

\author{
Relative variation \% \\ Absolute deviation \\ Identification Reference \\ Aster difference tolerance difference tolerance CL1+CL2
}

\section*{Temperature}
\(\left({ }^{\circ} \mathrm{C}\right)\)
= 0 N123, N124
-17.778
-17.778
0.000 1\%
-3.330e-5
0.01

N53, N54
-6.667
-6.667
0.000 1\%
1.330e-7
0.01

N17, N18
4.444
4.444
0.001

1\%
2.840e-5
0.01

N5, N6
15.556
15.555
-0.006
\(1 \%\)
-9.730e-4
0.01

N3, \(N 4 * 26.667\)
26.666
```

-0.004
1%
-1.000e-3
0 . 0 1
= 45 N69, N70
-9.921
-9.921
0.003 1%
-3.460e-4
0.01
N49, N50
-2.064
-2.065
0.031 1%
-6.480e-4
0 . 0 1
N57, N58
5.792
5 . 7 9 2
0.001
1%
8.240e-5
0 . 0 1
N65, N66* 13.649
13.649 0.000 1%
-5.68e-13
0 . 0 1
= 90 N133, N134
-17.778
-17.778
0.000 1%
-3.750e-5
0 . 0 1
N139, N140
-17.778
-17.778
0.000 1%
-5.030e-5
0 . 0 1
N145, N146
-17.778
-17.778
0.000 1%

```
```

-6.990e-5
0.01
N155, N156*
-17.778
-17.778
0.000 1%
9.09e-13
0 . 0 1
= 180 N197, N198
-2.889
-2.889
0.000 1%
-6.440e-5
0.01
N243, N244
-40.000
-40.000
0.000 1%
-7.680e-5
0.01
N265, N266
-5.1111
-5.1111
0.000 1%
-5.210e-5
0.01
N273, N274*
-62.222
-62.222
0.000 1%
+6.82e-13
0.01

* imposed temperatures

```

\subsection*{6.2 Parameters}
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000

\section*{Obstruction memory:}

8 megawords
Time CPU To use: 2.19 seconds
Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLV305 Heat gradient in a cylinder (Fourier)
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.04.305-A Page:
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\section*{7}

Summary of the results
Two modelings carried out (AXIS_FOURIER and 3D) give excellent results, the variation maximum is \(\mathbf{- 0 . 0 0 6 \%}\) for two modelings

This test made it possible to test in AXIS_FOURIER order COMB_CHAM_NO with the operands following:
- COMB_FOURIER to calculate the temperature in an angle given, \(\cdot C O M B \_R\) to carry out a linear combination of modes 0 and 1.

Handbook of Validation
V4.04 booklet: Stationary thermics of the voluminal structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:

TPLP01 Field in L with geometrical singularity

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.001-A Page:
1/8
Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures
V4.05.001 document

TPLP01-Field in L with singularity
geometrical

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about a problem plane 2D presenting two modelings, one planes, the second voluminal one.

The objective is to validate, in the presence of a geometrical singularity, the plane thermal elements and 3D with
for boundary condition an imposed temperature.
The results are compared with those provided by VPCS.

\author{
Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP01 Field in L with geometrical singularity

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.05.001-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
0.8

F
E
0.4
0.2
0.8

C
D
0.2

With
B
0.4

\section*{Dimensions in meters}

\section*{1.2}

Properties of material
1. W/m. \({ }^{\circ} \mathrm{C}\)

\section*{thermal conductivity}

\title{
1.3 \\ Boundary conditions and loadings \\ \(\cdot\) side [AF] imposed Temperature \(T p=10^{\circ} \mathrm{C}\), \\ - side [OF] imposed Temperature \(T p=0^{\circ} \mathrm{C}\), \(\cdot\) side [AB], [BC], [CD], [EF], flow \(=0\).
}

\subsection*{1.4 Conditions initial}

Without object.

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

TPLP01 Field in L with geometrical singularity

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.001-A Page:
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in card TPLP01/89 of guide VPCS.

\section*{2.2}

Results of reference
Temperature at the points of a squaring of with dimensions \(0.2 m X 0.2 m\).

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7
[2]
G.T. Symm-, National Physical Laboratory Division of Numerical Analysis and Computing, treatment of singularities in solution of Laplace' \(S\) equation by integral year equation method, NPL Carryforward NAC Jan 31, 1973.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP01 Field in L with geometrical singularity

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

PLAN (QUAD8)
\(y\)
F
E
N23
N37
N45
Limiting conditions:
N21
N35
N43
- dimensioned AF:
\(T=10^{\circ} \mathrm{C}\)
- dimensioned:
\(T=0^{\circ} \mathrm{C}\)
- dimensioned AB, BC, CD, EF: =0

N19
N33
N41
C
D
N17
N31
\(X\)
N15
N29
With
B

\section*{3.2 \\ Characteristics of the grid \\ A number of nodes: \\ 53 \\ A number of meshs and types: 12 QUAD8}

\author{
3.3 Functionalities \\ tested \\ Orders \\ AFFE_MODELE \\ THERMICS \\ PLAN \\ ALL \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ \section*{THER_LINEAIRE} \\ EXCIT \\ CHARGE \\ RECU_CHAMP \\ NUME_ORDRE
}

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLP01 Field in L with geometrical singularity

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.001-A Page:
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```

4
Results of modeling A

```

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )
\(X=0.2 \quad y=0.0(\) (N15)
9.316
9.283
-0.357
\(1 \%\)
\(y=0.2\) (N17)
9.001
9.108
1.186
\(1 \%\)
\(y=0.4\) (N19)
8.514
8.519
0.054
\(1 \%\)
\(y=0.6(N 21)\)
8.018
8.015
-0.037
\(1 \%\)
\(y=0.8(N 23)\)
7.869
7.883
0.177
0.177
1\%
\(y=0.2\) (N31)
8.640
8.669
0.334
1\%
\(y=0.4\) (N33)
6.667
6.667
-0.005
1\%
\(y=0.6\) (N35)
5.680
5.666
-0.254
\(1 \%\)
\(y=0.8\) (N37)
5.495
5.519
0.443
1\%
\(X=0.6 y=0.4(N 41)\)
2.972
2.963
-0.310
1\%
\(y=0.6\) (N43)
2.881
2.877
-0.132
```

1%
y=0.8(N45)
2.816
2.834
0.650
1%
4.2 Parameters
of execution

```

Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 2.25 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP01 Field in L with geometrical singularity

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.001-A Page:
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\section*{5 Modeling \\ B}

\section*{5.1}

Characteristics of modeling

Limiting conditions:
N29
N36
F
N20
E
- face AGLF:
\(T=10^{\circ} \mathrm{C}\)
N18
N27
N34
- face DJKE:
\(T=0^{\circ} \mathrm{C}\)
- faces ABHG, BHIC:
\(=0\)
N17
N28
N33
- faces CDJI, FEKL:
\(=0\)
N16
I
N26
N31
J
N15
N25
N32
N13
C N24
D
Thickness \(=0.2 \mathrm{~m}\)
N14
N23

\section*{G}

N11
N21
H
N12
N22
\(X\)
With
B
Z

\section*{5.2 \\ Characteristics of the grid}

A number of nodes:
42
A number of meshs and types: 12 HEXA8

\subsection*{5.3 Functionalities}
tested
Orders

AFFE_MODELE
THERMICS
3D
ALL
AFFE_CHAR_THER
TEMP_IMPO

\section*{THER_LINEAIRE}

EXCIT
CHARGE
RECU_CHAMP
NUME ORDRE

\title{
Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A \\ Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0 \\ \\ Titrate: \\ \\ Titrate: \\ TPLP01 Field in L with geometrical singularity \\ Date: \\ 20/09/02 \\ Author (S): \\ C. DURAND, E. SCREW, F. LEBOUVIER Clé \\ : \\ V4.05.001-A Page: \\ 7/8 \\ \\ 6 \\ \\ 6 \\ Results of modeling B \\ \\ 6.1 Values \\ \\ 6.1 Values \\ tested
}

Identification Reference
Aster difference
tolerance
Temperature ( \({ }^{\circ}\) C)
\(X=0.2 y=0.0(N 11)\)
9.316
9.294
-0.239
1\%
(N12)
9.316
9.294
-0.239
1\%

0.139-

1\%
\(y=0.2\) (N23)
8.640
8.661
0.247
\(1 \%\)
(N24)
8.640
8.661
0.247
\(1 \%\)
\(y=0.4\) (N25)
6.667
6.667
-0.005
1\%
(N26)
6.667
6.667
-0.005
\(1 \%\)
\(y=0.6(N 27)\)
5.680
5.669
-0.188
1\%
(N28)
5.680
5.669
-0.188
\(1 \%\)
\(y=0.8\) (N29)
5.495
5.502
0.123

1\%
(N30)

\subsection*{6.2 Parameters}
of execution
Version: 5.03

\section*{Obstruction memory:}

8 megawords
Time CPU To use: 2.00 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP01 Field in L with geometrical singularity

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.001-A Page:
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\section*{7}

Summary of the results
Two modelings give results whose certain values exceed the fixed tolerance initially (1\%):
- for modeling A (PLANE with meshs QUAD8), the maximum change is \(1.19 \%\) (going beyond for only one value out of the 13 tested), - for modeling B (3D with meshs HEXA8), the maximum change is \(2.7 \%\) (going beyond for two values out of 26 tested).

The modeling of the geometrical singularity (presence of an important heat gradient close to singularity) is represented better with quadratic elements (modeling A).

For two modelings, the precision should be improved by using a finer grid (more important refinement in the zone of the geometrical singularity).

The results are regarded as acceptable taking into account modelings carried out.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures

\title{
Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0 \\ Titrate: \\ Orthotropic Square TPLP02 \\ \\ Date: \\ \\ Date: \\ 20/09/02 \\ Author (S): \\ C. DURAND, E. SCREW, F. LEBOUVIER Clé \\ : \\ V4.05.002-A Page: \\ 1/6
}

Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\author{
Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ V4.05.002 document
}

TPLP02-Orthotropic square

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about a problem plane 2 D represented by only one modeling (plane).
The functionalities tested are the use of plane thermal elements, of an orthotropic material, three limiting types of conditions:
- convection
-
linear variation of the outside temperatures,
- flow
imposed.
The results are compared with an analytical solution (VPCS).
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Orthotropic Square TPLP02

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.05.002-A Page:
2/6

1
Problem of reference

\subsection*{1.1 Geometry}
has
Cubic of edge \(=0.2 \mathrm{~m}\) has

Center cube \(=(0.0 .0)\)
Y
\(X\)
\(O\)

\section*{1.2}

Properties of material
\(X=1.0 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}\)
thermal conductivity along axis \(X\)
\(y=0.75 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}\) thermal conductivity along the axis \(y\)

\section*{1.3 \\ Boundary conditions and loadings}
- density flux:
\(y=60 \mathrm{~W} / \mathrm{m}^{2}\)
face \(y=-0.1\) (entering flow)
\(y=-60 \mathrm{~W} / \mathrm{m}^{2}\)
face \(y=0.1\) (outgoing flow)
\(\cdot\) convection on the faces \(X=-0.1\) and \(X=0.1: H=15 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\),
- linear variation of the outside temperatures:
- Text \(=30-80 y\) face \(X=-0.1\),
- Text \(=15-80 y\) face \(X=0.1\).

\subsection*{1.4 Conditions}
initial
Without object.

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:

\author{
Date:
}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

V4.05.002-A Page:
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}

The reference solution is that given in card TPLP02/89 of guide VPCS Analytical solution.
\(T(X, y, Z)=a x+b y+D=-45 x-80 y+22.5\)

D
G
C
Not T ( \({ }^{\circ} \mathrm{C}\) )
Figure 0.1
O
22.5

With
35.0
\(\boldsymbol{Y}\)
Figure 0.2
B
26.0

C
10.0

X
H
F
D
19.0
```

O
E
30.5
F
18.0
G
14.5
H
27.0
With
E
B
X=45 W/m}\mp@subsup{}{}{2}=\mathrm{ constant
y=60 W/m}\mp@subsup{}{}{2}=\mathrm{ constant

```
2.2

Results of reference
Temperature at the points located on the figure above.

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Guide validation of the software packages of structural analysis. French company of Mechanics, AFNOR 1990 ISBN 2-12-486611-7
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
Orthotropic Square TPLP02

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{PLAN (QUAD4)}
\(y\)
\(D\)
N66 G
C
Limiting conditions:
N11
N121
m100
m96 m95
m91
- dimensioned AB \(y=60 \mathrm{~W} / \mathrm{m}^{2}\)
- dimensioned \(C D y=-60 \mathrm{~W} / \mathrm{m}^{2}\)
- dimensioned \(B C H=15 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\)

Text \(=15-80 y\)
- dimensioned \(A D H=15 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\)

Text \(=30-80 y\)
m60
m56
m51 F
```

O
X
H
N6
N116
m50
N61 m45
m41
N1
m10
m6
m5
ml N111
With
N56 E
B

```

\section*{3.2}

\section*{Characteristics of the grid}

A number of nodes:
121
A number of meshs and types: 100 QUAD4

\subsection*{3.3 Functionalities}
tested

\author{
Orders
}

\author{
AFFE_MODELE \\ THERMICS \\ PLAN \\ \(A L L\) \\ DEFI_MATERIAU \\ THER_ORTH \\ AFFE_CARA_ELEM \\ SOLID MASS
}

\author{
DEFI_FONCTION \\ NOM_PARA: " \(X\) " \\ NOM_PARA: " \(Y\) " \\ AFFE_CHAR_THER_F \\ EXCHANGE \\ FLUX_REP \\ THER_LINEAIRE \\ EXCIT \\ CARA_ELEM \\ CALC_CHAM_ELEM \\ CARA_ELEM \\ FLUX_ELNO_TEMP \\ Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
Orthotropic Square TPLP02

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.002-A Page:
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

\section*{Aster difference tolerance \\ Temperature \({ }^{\circ} \mathrm{C}\)}
\(T(O) 22.5\)
22.5
0.0

1\%
T (A) 35.0
35.0
0.0

1\%
\(T\) (B) 26.0
26.0
0.0

1\%
\(T(C) 10.0\)
10.0
0.0

1\%
\(T(D) 19.0\)
19.0
0.0

1\%
\(T(E) 30.5\)
30.5
0.0

1\%
\(T(F) 18.0\)
18.0
0.0

1\%
\(T(G) 14.5\)
14.5
0.0

1\%
\(T(H) 27.0\)
27.0
0.0

1\%
Flow
\(W / m^{2}\)
```

X(A)
45.045.0 0.0 1%
X(H)
45.045.0 0.0 1%
X(D)
45.0 45.0 0.0 1%
X(B)
45.045.0 0.0 1%
X(F)
45.045.0 0.0 1%
X(C)
45.0 45.0 0.0 1%
y(A)
60.060.0 0.0 1%
y(E)
60.060.0 0.0 1%
y(B)
60.060.0 0.0 1%
y(D)
60.060.0 0.0 1%
y(G)
60.060.0 0.0 1%
y(C)
60.060.0 0.0 1%

```

\subsection*{4.2 Parameters of execution}

Version: 5.03

\author{
Machine: SGI-ORIGIN 2000-R12000
}

Obstruction memory:
8 megawords
Time CPU To use: 2.40 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

Orthotropic Square TPLP02

Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.05.002-A Page:
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\section*{5 \\ Summary of the results}

The results obtained are excellent. The computed values by Aster are identical to the values of reference. That is "a normally awaited" result since the field solution which is linear belongs to the space of interpolation of the element tested.

This test made it possible to test the following orders:
- DEFI_FONCTION associated with operand NOM_PARA, allowing to define a variation of outside temperature according to the \(X\)-coordinate or of the ordinate,
- DEFI_MATERIAU associated with key word THER_ORTH, allowing to define the characteristics of an orthotropic material,
- AFFE_CARA_ELEM associated with the MASSIVE key word, allowing to define the axes of orthotropism.

Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP300 Plates rectangular: convection, imposed temperature
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures V4.05.300 document

TPLP300 - Rectangular plate: convection, imposed temperature

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about a problem plane \(2 D\) represented by only one modeling (plane).
The functionalities tested are the use of plane thermal elements under conditions limit temperature imposed and of convection.

The results are compared with those provided by NAFEMS.

\author{
Handbook of Validation
}

V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

Titrate:
TPLP300 Plates rectangular: convection, imposed temperature
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\section*{:}

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1
Problem of reference

\subsection*{1.1 Geometry}

D
C
dimensions in meters
1.0
\(\boldsymbol{Y}\)
E
\(X\)
0.2

With
B
0.6

\section*{1.2}

Properties of material
\(=52 \mathrm{~W} / \mathrm{m} .{ }^{\circ} \mathrm{C}\)
Thermal conductivity

\section*{1.3}

Boundary conditions and loadings
- imposed temperature with dimensions \([A B]: T p=100^{\circ} C\),
- density flux \(=0\) on the with dimensions one [DA],
- convection on the with dimensions ones [BC] and [CD],
\(H=750 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\),
- Text \(=0^{\circ} \mathrm{C}\).

\subsection*{1.4 Conditions}
initial
Without object.

\section*{Handbook of Validation}
\(V 4.05\) booklet: Stationary thermics of the plane structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\)}

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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The reference solution is that given in the card "TEST \(n^{\circ}\) T4" of the tests of reference published by NAFEMS.

\section*{2.2}

Results of reference
Temperature at the point \(\boldsymbol{E}\) : \(\boldsymbol{T}=18.3^{\circ} \mathrm{C}\)
2.3

Uncertainty on the solution
Nonavailable on card NAFEMS

\subsection*{2.4 References \\ bibliographical}
[1]
NAFEMS (the National Agency for Finite Element Methods Standard and (the U.K.)): "The standard NAFEMS Benchmarcks '", TNSB rév 3, October 1990.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
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Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP300 Plates rectangular: convection, imposed temperature
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Author (S):
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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

PLAN (QUAD4)

\section*{Boundary conditions:}
\(y\)
n71 (D)
n77 (C)
- Dimensioned AB:
\(T\)
\(=100^{\circ} \mathrm{C}\)
- Dimensioned BA:
=
- Dimensioned BC, CD: Text \(=0^{\circ} \mathrm{C}\)

H
\(=750 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\)
Points
\(X\)
Y
Nodes
E
0.6
0.2

N21

With
0.0
0.0

N1
n28
B
0.6
0.0

N7
n21 (E)
C
0.6
1.0

N77
D
0.0
1.0

N71
n14
X
N2 n3 n4 n5 n6

\section*{3.2}
Characteristics of the grid
A number of nodes:
77
A number of meshs and types: 60 QUAD4 (16 SEG2)

\subsection*{3.3 Functionalities}
tested
Orders

\author{
AFFE_MODELE \\ THERMICS \\ PLAN \\ ALL \\ AFFE_CHAR_THER \\ TEMP_IMPO
}

\section*{EXCHANGE}

COEF_H

\section*{THER_LINEAIRE}

\author{
EXCIT
}

CHARGE
```

RECU_CHAMP
NUME_ORDRE

```

\section*{Handbook of Validation}
```

V4.05 booklet: Stationary thermics of the plane structures HT-66/02/001/A

```

\section*{Code_Aster \({ }^{\circledR}\)}

Version
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TPLP300 Plates rectangular: convection, imposed temperature
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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )

At the point E: N21
18.3
17.954
-1.89
1\%

\subsection*{4.2 Parameters \\ of execution}

Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:

Titrate:
TPLP300 Plates rectangular: convection, imposed temperature
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{5}

Summary of the results
Modeling gives a result which exceeds the tolerance fixed initially. The maximum change obtained is \(1.9 \%\), to compare with the tolerance of \(1 \%\).

In this test, the heat gradients are more important close to the point \(B\) (imposed temperature and convection), a finer grid in this zone would improve quality of the results.

The results are regarded as acceptable taking into account the type of mesh (QUAD4) and of density of the grid used

The interest of this test is its origin NAFEMS.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP301 Plates square with imposed temperature distributed sinusoïdalement Date:

V4.05 booklet: Stationary thermics of the plane structures

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about a problem plane 2D represented by two modelings, one planes, the second hull.
The functionalities tested are as follows:
- plane thermal element,
- thermal element hull,
- limiting conditions: sinusoidal distribution of the imposed temperature

The results are compared with an analytical solution.

\section*{Handbook of Validation}
\(V 4.05\) booklet: Stationary thermics of the plane structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
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20/09/02
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1
Problem of reference

\subsection*{1.1 Geometry}
Y
has \(=1 \mathrm{~m}\)
\(D\)
\(C\)
\(I\)
\(H\)
Poin
Poits
T
\(X\)
\(Y\)
has
\(B\)
\(E\)
0,0
0.1
50,0
0.1
00
\(G\)
\(E\)
\(F\)
0,0
0.1
0.1
50,0
0.2
25
\(F\)
\(G\)
0,0
0.1
50,0
0.3
50
\(G\)
\(H\)
0,0
0.2
50,0
0.1
75
\(F\)
\(H\)
\(I\)
0,0
0.2
50,0
1.2
00
0,02
\(X\)
0,03
With
\(E\)
\(B\)
0,03
\(T p=04\)
ain \((X)\)

\section*{1.2 \\ Properties of material}
1. \(W / m .{ }^{\circ} C\)

Thermal conductivity

\section*{1.3}

Boundary conditions and loadings
- side [AB]
imposed temperature \(T p=\sin (X)\),
- side [BC]
imposed temperature \(\boldsymbol{T 0}=0^{\circ}\),
- side [CD]
imposed temperature \(\boldsymbol{T 0}=0^{\circ}\),
- side [BA]
imposed temperature \(\boldsymbol{T 0}=0^{\circ}\).

\subsection*{1.4 Conditions \\ initial \\ Without object. \\ Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP301 Plates square with imposed temperature distributed sinusoïdalement Date:
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\section*{Author (S):}
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
Analytical solution:
\(T(X, y)=\sinh [(1.0\) there \()] \sin (X) / \sinh ()\)

\title{
2.2 \\ Results of reference \\ Temperature at the points \(E, F, G, H, I\)
}

\section*{2.3}

Uncertainty on the solution
Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
W.K. Liu, T. Belytschko, "Efficient linear and nonlinear heat conduction with has quadrilateral element '', Int. J. num. Meth. Engng, flight 20, n5, pp 931-948, 1984.

\section*{Handbook of Validation}
\(V 4.05\) booklet: Stationary thermics of the plane structures
HT-66/02/001/A

\section*{Code_Aster \({ }^{\circledR}\) \\ Version \\ 5.0}

\section*{Titrate:}

TPLP301 Plates square with imposed temperature distributed sinusoïdalement Date:
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Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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\section*{3 Modeling}

With
3.1

Characteristics of modeling

\section*{HULL (TRIA3)}
```

y
0 . 5
n17 (D)
n153 (I)
Limiting conditions:

- dimensioned AE
T= 涪(X)
n149(H)
- dimensioned JD, DA T= O O
- dimensioned EJ:
= 0
n145 (G)
Not
X
y
Node
E
0.5

0. 

n137
F
0 . 5
0 . 2 5
n141
n141 (F)
G
0.5
0 . 5
n145
H
0.5
0 . 7 5
n149
I
0 . 5
I.
n153
n1 (A)
n137 (E)
n1

```

\section*{3.2 \\ Characteristics of the grid}

A number of nodes:
153
A number of meshs and types: 256 TRIA3
3.3 Functionalities
tested

Orders

\section*{AFFE_MODELE}

THERMICS

HULL

AFFE_CHAR_THER
TEMP_IMPO
TEMP_SUP, TEMP
TEMP_INF

AFFE_CARA_ELEM
HULL

THER_LINEAIRE
CARA_ELEM
EXCIT

RECU_CHAMP
NUME_ORDRE

\subsection*{3.4 Remarks}

The imposed temperature, distributed sinusö̈dalement on \(A E\), entered node by node.
The data of voluminal heat CP is obligatory for Code_Aster (although without influence in this simulation). One takes \(C P=1 . \mathrm{J} / \mathrm{m3}{ }^{\circ} \mathrm{C}\).

The condition limits \(=0\). is implicit on the free edges.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP301 Plates square with imposed temperature distributed sinusoïdalement Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

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\section*{4}

Results of modeling \(A\)

\subsection*{4.1 Values}
tested

\section*{Identification Reference}

Aster difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )

\section*{E Node n137 skin inf.}
1.0
1.00000
0.00\%*
\(1 \%\)
E Node n137 skin moy.
1.0
1.00000
0.00\%*

1\%
E Node n137 skin sup.
1.0
1.00000
\(0.00 \%\) *
1\%
F Node n141 skin inf.
0.45269
0.45379
0.24\%

1\%
F Node n141 skin moy.
0.45269
0.45379
0.24\%
\(1 \%\)
F Node n141 skin sup.
0.45269
0.45379
0.24\%

1\%
G Node n145: skin inf.
0.19927
0.20019
0.46\%

1\%
G Node n145: skin moy.
0.19927
0.20019
0.46\%

1\%
G Node n145: skin sup. 0.19927
```

0.20019
0.46%
1%
H Node n149: skin inf.
0 . 0 7 5 2 2
0 . 0 7 5 6 9
0.63%
1%
H Node n149: skin moy.
0 . 0 7 5 2 2
0 . 0 7 5 6 9
0.63%
1%
H Node n149: skin sup.
0 . 0 7 5 2 2
0 . 0 7 5 6 9
0.63%
1%
I Node n153: skin inf.
0.0
0 . 0 0 0 0 0
-2.E17*
1.E4
I Node n153: skin moy.
0 . 0
0 . 0 0 0 0 0
2.E17*
1.E4
I Node n153: skin sup.
0.0
0 . 0 0 0 0 0
1.E17*
1.E4

```

\subsection*{4.2 Parameters}
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:

8 megawords
Time CPU To use: 2.36 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP301 Plates square with imposed temperature distributed sinusoïdalement Date:
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Author (S):
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\section*{5 Modeling}

B
5.1

Characteristics of modeling

\section*{PLAN (TRIA3)}
\(y\)
0.5
n17 (D)
n153 (I)
Limiting conditions:
- dimensioned \(A E\)
\(T=\sin (X)\)
n149 (H)
- dimensioned JD, DA \(T=0^{\circ} C\)
- dimensioned EJ:
= 0
n145 (G)
Not
\(X\)
\(y\)
Node
```

E
0.5
0.
n137
F
0.5
0 . 2 5
n141
n141 (F)
G
0.5
0 . 5
n145
H
0 . 5
0 . 7 5
n149
I
0.5
1.
n153
n1 (A)
n137 (E)
n1
n35
n69
n103
n137
X
n18
n52
n86
n120

```

\section*{5.2}

Characteristics of the grid
A number of nodes:
153
A number of meshs and types: 256 TRIA3

\subsection*{5.3 Functionalities}
tested

Orders

\author{
AFFE_MODELE \\ THERMICS \\ PLAN \\ ALL \\ AFFE_CHAR_THER \\ TEMP_IMPO \\ \section*{THER_LINEAIRE} \\ EXCIT \\ CHARGE
}

\subsection*{5.4 Remarks}

The data of voluminal heat CP is obligatory for Code_Aster (although without influence in this simulation). One takes \(C P=1 . \mathrm{J} / \mathrm{m} 3{ }^{\circ} \mathrm{C}\).

The condition limits \(=0\). is implicit on the free edges.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures

\section*{HT-66/02/001/A}

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLP301 Plates square with imposed temperature distributed sinusoïdalement Date:
20/09/02
Author (S):
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\title{
6 \\ Results of modeling B
}

\subsection*{6.1 Values}
tested

\section*{Identification Reference}

Aster difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )

\section*{E: Node n137}
1.0
1.00000
0.00\%*

1\%
F: Node n141
0.45269
0.45379
0.24\%

1\%
G: Node n145
0.19927
0.20019
0.46\%

1\%
H: Node n149
0.07522
0.07569
0.63\%

1\%
I: Node n153
0.0
0.00000
-1.E17*
1.E4

\subsection*{6.2 Parameters}
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 2.05 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
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20/09/02
Author (S):
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7
Summary of the results
2 modelings carried out, HULL and PLAN with meshs TRIA3 give results satisfactory, the maximum change obtained is \(0.63 \%\). Results found for two modelings are identical. The interest of this test is to compare the results obtained with an analytical solution.

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLP302 Plates rectangular with imposed temperature
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
V4.05.302 document

TPLP302 - Rectangular plate with temperature imposed

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics. It is about a problem plane 2 D represented by a modeling hull.

The functionalities tested are as follows:
- thermal element hull,
- limiting conditions: imposed temperature.

The results are compared with an analytical solution.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP302 Plates rectangular with imposed temperature
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.302-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(Y\)
\(L\)
\(D\)
\(C\)
\(L=0.2 \mathrm{~m}\)
\(L=2 \mathrm{~m}\)
has
\(=0.05 \mathrm{~m}\) has
has
Points
\(X\)
\(Y\)
\(E\)
\(J\)
0.100 .15
E
H

\section*{1.2}

Properties of material
\(\overline{1} \mathrm{~W} / \mathrm{m}^{\circ} \mathrm{C}\)

\author{
Thermal conductivity
}

\section*{1.3 \\ Boundary conditions and loadings}
- Imposed Température:
dimensioned \([B C]\) and \([A D] T=0^{\circ} C\),
dimensioned \([A B] T=100^{\circ} C\).

\section*{- Imposed Flux:}
dimensioned \([C D]=0\)

\author{
1.4 Conditions initial \\ Without object. \\ Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
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Titrate:
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Author (S):
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2
Reference solution

\section*{2.1 \\ Method of calculation used for the reference solution}
```

4T

```
\([-(2 n+1) y / l]\)
p
E
(2n+)
1 X
\(T(X, y)=\)

\title{
2.2 \\ Results of reference
}

Temperature at the points \(E, F, G, H, I, J, K\)

\section*{2.3 \\ Uncertainty on the solution}

Analytical solution.

\subsection*{2.4 References \\ bibliographical}
[1]
J.R. Welty, E.C. Wicks, R.E. Wilson, "Fundamentals of momentum heat and mass transfer", third edition, John Wiley \& Sounds, 1983.
Handbook of Validation

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\section*{3 Modeling}

With

\section*{3.1 \\ Characteristics of modeling}

\section*{HULL (TRIA6)}
\(y\)
Limiting conditions:

\section*{D}

M
- dimensioned Al
\(=100^{\circ} \mathrm{C}\)
- dimensioned \(A D\)
\(=0^{\circ} \mathrm{C}\)
. 1
- dimensioned LM
\(=0\)
- dimensioned DM
\(=0\)
2.

Points
X
Y
Nodes
E
0.050 .05
\[
0.050 .15
\]
\[
N 57
\]

H

N23
I
0.100 .10
N41
\(J\)
0.100 .15
N59
X
\(K\)
0.100 .20
N77
With
\(L\)
Cutting:
    - 4 elements
    according to \(X\)
- 40 elements
according to \(y\)

\section*{3.2 \\ Characteristics of the grid}

A number of nodes:
729
A number of meshs and types: 320 TRIA6

\subsection*{3.3 Functionalities}
tested
Orders

\section*{AFFE_MODELE}

\section*{THERMICS}

HULL
ALL
AFFE_CHAR_THER_F
TEMP_IMPO
TEMP_SUP
TEMP_INF
TEMP
THER_LINEAIRE
CARA_ELEM
EXCIT
CHARGE
RECU_CHAMP
NUME_ORDRE

\subsection*{3.4 Remarks}

Limiting conditions, \(T=100^{\circ} \mathrm{C}\) on AB , and \(T=0^{\circ} \mathrm{C}\) on AD , are incompatible at point \(A\). It Code_Aster applies a "law of overload" which, in this case, consists in taking into account last condition limits entered. The order of assignment of the imposed temperatures thus has large influence on the results obtained.

In the treated case, the temperature assigned to point A is of \(0^{\circ} \mathrm{C}\).
Handbook of Validation
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HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
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Titrate:
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Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{V4.05.302-A Page:}

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\section*{4 \\ Results of modeling \(A\)}

\subsection*{4.1 Values}
tested
Identification Reference
Aster difference
tolerance
Temperature ( \({ }^{\circ} \mathrm{C}\) )

\section*{N21 (surface_supérieure) 43.496}
43.499
0.007

1\%
N21 (surface_moyenne) 43.496
43.499
0.007

1\%
N21 (surface_inférieure) 43.496
43.499
0.007

1\%
N39 (surface_supérieure) 18.978
18.957
-0.112
1\%
N39 (surface_moyenne) 18.978
18.957
-0.112
1\%
N39 (surface_inférieure) 18.978
18.957

N57 (surface_supérieure) 8.559
8.554
-0.057
1\%
N57 (surface_moyenne) 8.559
8.554
-0.057
1\%
N57 (surface_inférieure) 8.559
8.554
-0.057
1\%
N23 (surface_supérieure) 54.467
54.514
0.087

1\%
N23 (surface_moyenne) 54.467
54.514
0.087

1\%
N23 (surface_inférieure) 54.467
54.514
0.087

1\%
N41 (surface_supérieure) 26.096
26.096
-0.001
1\%
N41 (surface_moyenne) 26.096
26.096
-0.001
1\%
N41 (surface_inférieure) 26.096
26.096
-0.001
1\%
N59 (surface_supérieure) 12.032
12.025
-0.061
1\%
N59 (surface_moyenne) 12.032

N59 (surface_inférieure) 12.032
12.025
-0.061
1\%
N77 (surface_supérieure) 5.499
5.496
-0.063
1\%
N77 (surface_moyenne) 5.499
5.496
-0.063
1\%
N77 (surface_inférieure) 5.499
5.496
-0.063
1\%
4.2 Parameters
of execution
Version: 5.03

Machine: SGI-ORIGIN 2000-R12000
Obstruction memory:
8 megawords
Time CPU To use: 3.19 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP302 Plates rectangular with imposed temperature
Date:
20/09/02

Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.05.302-A Page:
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\section*{5}

Summary of the results
Modeling HULL with meshs TRIAG gives very satisfactory results, the variation maximum obtained is \(0.11 \%\). The interest of this test is of:
- to test meshs TRIA6 in HULL,
- to compare the results compared to an analytical solution.

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
V4.05.303-A Page:
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Organization (S): EDF/AMA, EDF/UTO/SIS, Delta CAD

\author{
Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ V4.05.303 document
}

TPLP303 - Distribution of the temperature in section of a conduit of chimney

\section*{Summary:}

This test results from the validation independent of version 3 in linear stationary thermics.
It is about a problem plane 2D represented by seven modelings mixing each one several types elements.

The functionalities tested are as follows:
- plane thermal element,
- voluminal thermal element,
- limiting condition: convection.

The interest of the test lies in the mixture of different elements.
The results are compared with an analytical solution.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit
Date:
20/09/02

\author{
Author (S): \\ C. DURAND, E. SCREW, F. LEBOUVIER Clé
}
:
V4.05.303-A Page:
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1
Problem of reference

\subsection*{1.1 Geometry}
\(=1.2192 \mathrm{~m}\) have
\(B=0.3048 \mathrm{~m}\)
\(12 C\)
\(Z\)
\(\boldsymbol{Y}\)
10
11
B
\(X\)
has
7
8
9
H
B
D
I Ti
4
5
6
has
He \(\boldsymbol{T e}\)
With
B
1
2
3

Properties of material

\section*{= \\ 1.7307 Thermal W/m \({ }^{\circ}\) C Conductivity}
1.3
Boundary conditions and loadings
- Interior Surface: \(\boldsymbol{h i}=68.135 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\); \(\mathbf{T i}=37.78{ }^{\circ} \mathrm{C}\),
\(\cdot\) External Surface: \(\mathrm{He}=17.034 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C} ; \mathbf{T e}=-17.78^{\circ} \mathrm{C}\).

\subsection*{1.4 Conditions \\ initial \\ Without object. \\ Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit

\section*{Date:}

20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

\section*{:}

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2
Reference solution

\section*{2.1}

Method of calculation used for the reference solution
The original reference solution given in the book [bib1] is based on a method of relieving numerical. This reference is quoted in the handbook of checking of ANSYS [bib2].

\section*{2.2}

Results of reference
Temperature at the points \(n^{\circ} 1\) to 11.

\section*{2.3}

Uncertainty on the solution
Unknown factor, it was not possible to get the original reference (delivers old, more published).

\subsection*{2.4 References \\ bibliographical}

\section*{[1]}

Kreith, F., "Principles of heat transfer", International Textbook Co., Scranton, Pennsylvania, 2nd Printing, 1959.
[2]
ANSYS: "checking manual", 1st edition, June 1, 1976
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
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5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit
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:
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\section*{3 Modeling}

With
3.1
Characteristics of modeling
PLAN (TRIA3, QUAD4)
C
N3N6
\(y\)
N9
N2
N11
N5
N12
N8
N1
B
D
    N4
    \(45^{\circ}\)
    N10
    N7
    Boundary conditions:
    With
    - dimensioned AD: Text \(=37.778{ }^{\circ} \mathrm{C}\)
    \(H=68.135 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\)
    0.3
    -dimensioned BC: Text \(=-17.778{ }^{\circ} \mathrm{C}\)
    \(H=17.034 \mathrm{~W} / \mathrm{m}^{2}{ }^{\circ} \mathrm{C}\)
    \(X\)
    - dimensioned \(A B, C D=0\)

\section*{3.2}

Characteristics of the grid
A number of nodes:
12
A number of meshs and types: 6 (5 QUAD4, 1 TRIA3)

\subsection*{3.3 Functionalities}

AFFE_MODELE
THERMICS
PLAN
ALL

AFFE_CHAR_THER
EXCHANGE
THER_LINEAIRE
EXCIT
CHARGE

RECU_CHAMP
NUME_ORDRE

\section*{Handbook of Validation}

V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
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5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
:
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4
Results of modeling \(A\)

\subsection*{4.1 Values}

\author{
Relative variation \% \\ Absolute deviation \\ Identification Reference \\ Aster difference \\ tolerance \\ difference tolerance \\ Temperature ( \({ }^{\circ} \mathrm{C}\) )
}

\section*{Points}

N1 30.889
29.795
-3.541
2\%
-1.09
0.5

N2
-1.333
-2.528
89.632

2\%
-1.19
0.5

N3
-15.167
-16.036
5.729
\(2 \%\)
-0.869
0.5

N4 34.000
34.718
2.111
\(2 \% 0.7180 .5\)
N5 8.611
```

8.566
-0.518
2%
-0.045
0 . 5
N6
-11.278
-10.810
-4.152
2%
-4.152
0 . 5
N734.278
34.114
-0.480
2%
-0.164
0 . 5
N8 12.556
12.716
1.274
2%
0.160
0 . 5
N9
-7.611
-8.152
7.111
2%
-0.541
0 . 5
N10 13.500
13.973
3.506
2% 0.473 0.5
N11
-5.889
-5.909
0.336 2% -0.02
0 . 5
N12
-5.444
-5.377

```

\subsection*{4.2 Parameters \\ of execution}

Version: 5.03

Machine: SGI - ORIGIN 2000-R12000

Obstruction memory:
8 megawords
Time CPU To use: 2.36 seconds
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

TPLP303 Distribution of the temperature in the section of a conduit
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

V4.05.303-A Page:
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\section*{5}

Complementary modelings \(B, C, D, E, F\) and \(G\)

\section*{Modeling b:}
- Maillage identical to that described in the card of modeling, on \(1 / 8\) of the structure, but with quadratic elements,
- Système of unit ( \(\left.{ }^{\circ} C, W, m, S\right)\).

It is noted that the quadratic interpolation improves the results, the maximum change is \(\mathbf{4 9 . 1 6 \%}\) for
the value of reference nearest to 0 .

\section*{Modeling C:}
- Finer Maillage (22 QUAD8 + 4 TRIA6), on 1/8 of the structure,
- Système of unit ( \({ }^{\circ} \mathrm{C}, \mathrm{W}, \mathrm{m}, \mathrm{S}\) ).

It is noted that compared to modeling B, the maximum change does not decrease but increases (54.58\%).

\section*{Modeling D:}
- Maillage identical to that described in the card of modeling, on \(1 / 8\) of the structure,
- Système of English unit ( \({ }^{\circ}\) F, Btu, feet, hr).

It is noted that the maximum relative variation decreases in an important way (33.29\%), this variation is not more
located at the same place. On the other hand it is always located on the value of reference nearest to 0.

\section*{Modeling E:}
- Maillage identical to that described in the card of modeling, on \(1 / 8\) of the structure, but with quadratic elements,
- Système of English unit ( \({ }^{\circ}\) F, Btu, feet, hr).

It is noted that the quadratic elements improve the results by with a linear modeling (maximum change of 6.5\%).

\section*{Modeling F:}
- cutting identical to that described in the card of modeling (12 QUAD4) but on 1/4 of the structure,
- Système of English unit ( \({ }^{\circ}\) F, Btu, feet, hr).

It is noted that this grid with linear elements (without TRIA3) is much more precise, the maximum change is \(5.27 \%\).

Modeling G:
- Découpage identical to that described in the card of modeling, but on 1/4 of the structure and with quadratic elements,
- Système of English unit ( \({ }^{\circ}\) F, Btu, feet, hr).

It is noted that this grid (without TRIA3) is much less precise than at the time of modeling F, the maximum change is \(6.26 \%\).

\title{
Handbook of Validation \\ V4.05 booklet: Stationary thermics of the plane structures \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé

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\section*{6}

Results of modelings \(B, C, D, E, F\) and \(G\)
On the figure below we present the points of observation (for more details to see the card of test corresponding).
(calculations carried out by Mr. CLERK EDF/DER/ADE). We grayed the upper deviations than tolerance (2\%).

\author{
Calculations carried out out of \(W, m,{ }^{\circ} C\) \\ Mod* has \\ Mod* \(\boldsymbol{B}\) Mod \({ }^{*}\) C Mod \({ }^{*}\) has \\ Points Réf ASTER \\ Variation \\ \% \\ ASTER \\ Variation \% ASTER \\ Variation \% \\ NISA \\ Variation \% \\ 134.27834 .114 \\ -0.480 \\ 34.141 \\ -0.401 34.181-0.283 34.114-0.480 \\ 213.50013 .9733 .506 \\ 13.277 \\ -1.653 13.277-1.649 13.973 \\ 3.506 \\ 3-5.444-5.377 \\ -1.229 \\ -5.685 \\ 4.421-5.681 4.357-5.377-1.229 \\ 434.00034 .7182 .111 \\ 33.715 \\ -0.840 33.847 -0.450 34.718 \\ 2.111 \\ 512.55612 .7161 .274 \\ 12.140 \\ -3.314 12.056-3.982 12.716 \\ 1.274 \\ 6 -5.889-5.909 0.336 \\ -6.258 \\ 6.258 -6.283 6.694-5.909 0.336 \\ 730.88929 .795 \\ -3.541 \\ 29.440 \\ -4.690 29.290 -5.177 29.795-3.541 \\ 88.6118 .566
}
```

-0.518
7.057
18.049
7.012 -18.566 8.566 -0.518
9 -7.611 -8.152 7.111
-8.283
8.693-8.300 9.049 -8.152 7.111
10-1.333 -2.528 89.632
-1.988
49.164 -2.061 54.578 -2.528 89.632
11 -11.278 -10.810 -4.152
-11.571 2.597 -11.579 2.671 -10.810 -4.152
12 -15.167 -16.036 5.729 -15.273 0.701 -15.287 0.791 -16.036 5.729

* Modeling
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A

```

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TPLP303 Distribution of the temperature in the section of a conduit
Date:
20/09/02
Author (S):
C. DURAND, E. SCREW, F. LEBOUVIER Clé
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\section*{Calculations carried out in Btu, feet, \({ }^{\circ}\) F}

\author{
MOD D \\ MOD E \\ MOD F \\ MOD G \\ Points Réf ASTER Variation \% \\ ASTER \\ Variation \% \\ ASTER \\ Variation \% ASTER Variation \\ \% \\ 193.7 \\ \(93.404-0.316\) 93.453-0.264 93.454-0.262 \\ 93.450 \\ -0.267 \\ 256.3 \\ 57.1521 .51355 .898 -0.713 56.8721 .017 \\ 55.884 \\ -0.739 \\ 322.2 \\ 22.3210 .545 21.767-1.948 22.2200 .090 \\ 21.761 \\ -1.975 \\ 493.2 \\ \(94.4921 .38692 .686-0.55294 .1491 .018\) \\ 92.688 \\ -0.549
}
554.6
\(54.8890 .52953 .852-1.37054 .568-0.590\)
53.821
-1.427
621.4
\(21.364-0.16820 .736-3.10120 .936-2.167\)
20.725
-3.155
787.6
85.631 -2.247 84.992-2.977 96.572-1.173
85.064
-2.894
847.5
\(47.419-0.16944 .702-5.89045 .003-5.257\)
44.523
-6.268
918.3
\(17.326-5.32317 .091-6.50717 .335-5.273\)
17.237
\(-5.807\)
\(1029.627 .450-7.26428 .421\)-3.983 29.8991 .010
27.978
-5.480
\(1111.712 .5427 .20111 .172-4.50911 .560-1.194\)
11.272
-3.659
\(124.73 .135-33.2854 .509-4.0894 .8242 .633\)
4.452
-4.932

From these 7 analyses, we can make the following observations:
- the grid suggested in the test probe (5 QUAD4 + 2 TRIA3) is not adapted. For to bring closer the reference solution there are two possibilities:
to use the quadratic grid on \(1 / 8\) of the structure,
to use a linear grid without triangle on \(1 / 4\) of the structure,
- the choice of the system of unit to important considerable in the calculation of the relative variation, - for same modeling \((A)\) the results between Code_Aster and NISA are identical.

\section*{Summary of the results}

The modeling carried out on \(1 / 8\) of the structure gives results of which many values exceed the tolerance fixed initially ( \(2 \%\) ). The maximum change obtained is \(89 \%\), it is on smaller value of reference. The analysis of the isotherms shows that those are not perpendiculars with right-hand side cd., the condition of symmetry is not observed.

To find an explanation to these important differences, several complementary modelings (cf were carried out annexes B). The conclusions are as follows:
- the change of the system of units \(\left({ }^{\circ} \mathrm{C}->^{\circ} \mathrm{F}\right)\) makes it possible to decrease the maximum change with one
value of \(33 \%\),
- modeling with quadratic elements (and in \({ }^{\circ} \mathrm{F}\) ) improves the results, the variation maximum is \(6.8 \%\),
- the modeling of one \(1 / 4\) of the structure with only of the QUAD4 (and in \({ }^{\circ} \mathrm{F}\) ) improves them results, the maximum change is \(5.27 \%\),
- the modeling A, carried out with software NISA, gives results identical to those of Code_Aster.

Moreover, it was not possible to get the original reference (delivers of Kreith) quoted in handbook of checking of ANSYS. Method of acquisition of the reference solution and its uncertainty are thus not known.

The results are regarded as acceptable taking into account the points evoked above.
However it will be necessary to seek complementary elements on the reference solution.
Handbook of Validation
V4.05 booklet: Stationary thermics of the plane structures
HT-66/02/001/A
Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TTLLO1 - Thermal shock on an infinite wall

Date:
30/08/02
Author (S):
J. Key PELLET

V4.21.001-F Page:
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\author{
Handbook of Validation
}

V4.21 booklet: Transitory thermics of the linear structures
Document: V4.21.001

TTLL01 - Thermal shock on an infinite wall

\section*{Summary:}
- Thermique linear transient,
\(\cdot\) elements 2D and 3D ( 7 modelings),
- interests of the test:
test the algorithm of linear thermics transitory with change of step of time,
-
imposed temperature (with discontinuity),
filing of some not of time.
- The shock is modelled in 2 different ways:
by a linear slope: \(T=100\). in 103 second,
by true a discontinuity of imposed temperature.
Handbook of Validation

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TTLL01 - Thermal shock on an infinite wall

Date:
30/08/02
Author (S):
J. Key PELLET
:
V4.21.001-F Page:
2/24

1
Problem of reference

\subsection*{1.1 Geometry}

\section*{B}

With
\(A^{\prime}\)
M1
M2
\(X\)
0
\(2 L\)
\(A A=2 L=2 m\)
\(X(M 1)=0.2 \mathrm{~m}\)
\(X(m 2)=0.8 m\)

\section*{1.2}

Material properties
\[
\begin{aligned}
& =1 \mathrm{~W} / \mathrm{m}{ }^{\circ} \mathrm{C} \\
& \mathrm{CP}=1 \mathrm{~J} / \mathrm{m3}{ }^{\circ} \mathrm{C} \\
& 1.3 \\
& \text { Boundary conditions and loadings }
\end{aligned}
\]
\(\cdot A: T(0, T)=T p=100^{\circ} C\)
for \(T>0\)
\(\cdot A^{\prime}: T(2 L, T)=T p=100^{\circ} \mathrm{C}\)

\subsection*{1.4 Conditions}
initial
\(T(X, 0)=0^{\circ} C\) for any \(X\)

\section*{1.5}

Specified concerning modelings
Discretization in time (T):
The thermal shock requires a "fine" discretization in time nearly \(\boldsymbol{T}=0\).
The goal of the test being to validate the various elements (various modelings), we have chosen a single discretization in time:

10 steps
for [0.
```

,
1.D3] is $T=104 S$

```

9
not for
[1 D3
1.D2]
that is to say
\(T=103 S\)
9 steps
for
[1.D2
,
1.D1]
that is to say
\(T=102 S\)
9
not for
[1.D1
'1.]
that is to say
```

T=101 S
10
not for
[1.
2.]
that is to say
T=101 S
The shock is defined in two different ways:
· for modeling B, it acts of a true shock (Tp is discontinuous):
T
p()
To=0.
T+
p()
To = 100.

- for modelings A, C, D, E, F, G, it acts of a linear slope:
T
p(A)t==
0

0. 

T
p(A)t=-3=
10
100.
Handbook of Validation
V4.21 booklet: Transitory thermics of the linear structures
HT-66/02/001/A

```

Code_Aster \({ }^{\circledR}\)
Version
5.0

\section*{Titrate:}

\section*{TTLL01 - Thermal shock on an infinite wall}

\section*{Date:}

30/08/02
Author (S):
J. Key PELLET
:
V4.21.001-F Page:
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\section*{2 \\ Reference solution}

\section*{2.1 \\ Method of calculation used for the reference solution}
\(T(X, T)\) -
\(T p\)
4
1
\(N X\)

\section*{N2}
\(=\sin\)
exp-
. \(t\)
0
\(T-T\)
\(N\)
\(L\)
\(L\)
\(C\)
\(p\)
\(N\)
2
2
\(=\)
2
\(p\)
\(X=\)
\(X\)-coordinate
\(T=\) time
0
\(T=\) températur initial

\section*{E}
\(T p=\) températur imposed

\section*{2.2}

Results of reference
Temperatures at the points M1 \((X=0.2)\) and \(m 2(X=0.8)\), and at various moments ( \(T=0.1,0.2,0.7\) and 2.0).

The values of reference are those given in guide VPCS.

\section*{2.3 \\ Uncertainty on the solution}

Numerical series.

\subsection*{2.4 References \\ bibliographical}
[1]
J.F. SACCADURA: Initiation with the thermal transfers, Paris, Technique and documentation (1982).

\title{
Handbook of Validation \\ V4.21 booklet: Transitory thermics of the linear structures \\ HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TTLL01 - Thermal shock on an infinite wall

Date:
30/08/02
Author (S):
J. Key PELLET
:
V4.21.001-F Page:
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3 Modeling
With

\section*{3.1 \\ Characteristics of modeling}

\section*{QUAD8}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height \(H=1.0\) with only one layer of elements.
\[
L=0.05
\]

\section*{Limiting conditions}

\section*{D}

C
on [BC], [AB] and [cd.]: \(J=0\)

\section*{H}
on [AD]: Tp is imposed
With
M1

\section*{Initial conditions}

\author{
3.2 \\ Characteristics of the grid \\ A number of nodes: 103 \\ A number of meshs and types: 20 QUAD8
}

\subsection*{3.3 Functionalities}
tested
Orders

\section*{THER_LINEAIRE}

LIST_INST

\section*{RECU_CHAMP}

INST

\author{
Handbook of Validation \\ V4.21 booklet: Transitory thermics of the linear structures HT-66/02/001/A
}

Code_Aster \({ }^{\circledR}\)
Version
5.0

Titrate:
TTLL01 - Thermal shock on an infinite wall

\section*{Date:}

30/08/02
Author (S):
J. Key PELLET

V4.21.001-F Page:
5/24

4
Results of modeling \(A\)

\subsection*{4.1 Values}
tested

Identification Reference
Aster \%
difference
M1 ( \(X=0.2\) ) N9
\(T=0.1\)
65.48
65.294
-0.28
\(T=0.2\)
75.58
75.814
+0.31
\(T=0.7\)
93.01
92.867
-0.15
\(T=2.0\)

\title{
Handbook of Validation \\ V4.21 booklet: Transitory thermics of the linear structures HT-66/02/001/A
}

\section*{Code_Aster \({ }^{\circledR}\)}

Version
5.0

\section*{Titrate:}

TTLL01 - Thermal shock on an infinite wall

\section*{Date:}

30/08/02
Author (S):
J. Key PELLET

\section*{5 Modeling \\ B \\ 5.1 \\ Characteristics of modeling}

\section*{QUAD8}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height \(H=1.0\) with only one layer of elements.
\[
L=0.05
\]

Limiting conditions

\section*{D}

C
on [BC], [AB] and [cd.]: = 0
H
on [AD]: Tp is imposed \(T p=100^{\circ} \mathrm{C}\)
With
M1
M2
B
\(T p\)
\(100{ }^{\circ} \mathrm{C}\)
20 elements
0
\(T\)
points
nodes
Initial conditions
M1
N9
One affects the temperature directly of M2

\section*{N33}
\(100^{\circ} \mathrm{C}\) at moment 0 .

\author{
5.2 \\ Characteristics of the grid \\ A number of nodes: 103 \\ A number of meshs and types: 20 QUAD8
}

\subsection*{5.3 Functionalities}
tested

\section*{Orders}

\author{
THER_LINEAIRE \\ TEMP_INIT \\ CHAM_NO \\ RECU_CHAMP \\ INST \\ AFFE_CHAM_NO
}

\section*{Handbook of Validation}

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\section*{6}

Results of modeling B

\subsection*{6.1 Values}

\section*{Identification Reference}

Aster \%
difference
M1 ( \(X=0.2\) ) N9
\[
\begin{aligned}
& T=0.1 \\
& 65.48 \\
& 65.369 \\
& -0.17 \\
& T=0.2 \\
& 75.58 \\
& 75.841
\end{aligned}
\]
0.35
\(T=0.7\)
93.01
92.875
-0.14
\(T=2.0\)
99.72
99.700
-0.02

M2 \((X=0.8)\) N33
\(T=0.1\)
8.09
8.113
0.28
\(T=0.2\)
26.37
25.872
-1.89
\(\boldsymbol{T}=0.7\)
78.47
78.071
-0.51
\(T=2.0\)
99.13
99.078
-0.05

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\section*{7 Modeling \\ C}
7.1

Characteristics of modeling

\section*{HEXA8}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height and a thickness \(H=1.0\) with only one layer of elements.

\section*{\(L=0.05\)}

Limiting conditions
D
C
on [BC], [AB] and [cd.]: = 0

\section*{H}
on [AD]: Tp is imposed
With
M1
M2
B H
\(T p\)
\(100{ }^{\circ} \mathrm{C}\)
20 elements HEXA8
0
\(T\)
103 S
points
nodes

\section*{Initial conditions}

M1

\section*{N21 with N24}
\(\boldsymbol{T}=\boldsymbol{0}^{\circ} \mathrm{C}\)
M2
N69 with N72
One fixes here the duration of the shock at 103 S .

\section*{7.2}

Characteristics of the grid

A number of nodes: 84
A number of meshs and types: 20 HEXA8

\subsection*{7.3 Functionalities}
tested

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\section*{8 \\ Results of modeling \(C\)}

\subsection*{8.1 Values}
tested

\section*{Identification Reference \\ Aster \% \\ difference \\ M1 ( \(X=0.2\) ) N21}
\[
\begin{aligned}
& T=0.1 \\
& 65.48 \\
& 65.31 \\
& -0.26 \\
& T=0.2 \\
& 75.58 \\
& 75.81 \\
& +0.31 \\
& T=0.7 \\
& 93.01 \\
& 92.87 \\
& -0.15 \\
& T=2.0 \\
& 99.72 \\
& 99.70 \\
& -0.02
\end{aligned}
\]
M2 \((X=0.8)\) N69
\(T=0.1\)
8.09
7.98
-1.31
\(T=0.2\)
26.37
25.76
-2.30
\(T=0.7\)
78.47
78.05
-0.53
\(T=2.0\)
99.13
99.08
-0.05

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\section*{9 Modeling \\ D}
9.1

Characteristics of modeling

\section*{HEXA20}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height and a thickness \(H=1.0\) with only one layer of elements.
\(L=0.05\)
Limiting conditions
D
C
on [BC], [AB] and [cd.]: \(=0\)

\section*{H}
on [AD]: Tp is imposed
With
M1
M2
B H
Tp
```

100 *
20 elements HEXA20
O
T
103 S
points
nodes
Initial conditions
M1
N57 with N64
T= 0' C
M2
N201 with N208
One fixes here the duration of the shock at 103 S.

```

\section*{9.2}

Characteristics of the grid
A number of nodes: 248
A number of meshs and types: 20 HEXA20

\subsection*{9.3 Functionalities}
tested

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10 Results of modeling \(D\)
10.1 Values
tested

\section*{Identification Reference}

Aster \%
difference
M1 ( \(X=0.2\) ) N57
\(T=0.1\)
65.48
65.29
-0.28
\(T=0.2\)
75.58
75.81
+0.31
\(T=0.7\)
93.01
92.87
-0.15
\(T=2.0\)
99.72
\[
T=0.1
\]
\[
8.09
\]
\[
8.04
\]
\[
-0.67
\]
\[
T=0.2
\]
\[
26.37
\]
\[
25.79
\]
\[
-2.20
\]
\[
T=0.7
\]
\[
78.47
\]
\[
78.05
\]
\[
-0.54
\]
\[
T=2.0
\]
\[
99.13
\]
\[
99.08
\]
\[
-0.05
\]

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11 Modeling
E

\subsection*{11.1 Characteristics of modeling}

\section*{PENTA6}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height and a thickness \(H=1.0\) with only one layer of elements. Each cube is cut out in 2 pentahedrons.
```

$L=0.05$

```

\section*{Limiting conditions}
```

D
C
on [BC], [AB] and [cd.]:=0
H
on [AD]: Tp is imposed
With
M1
M2
B H
Tp
100 *}\textrm{C
20 elements PENTA6
0
T
103 S
points
nodes
Initial conditions
M1
N21 with N24
T=0}\mp@subsup{}{}{\circ}\textrm{C

```

N69 with N72
One fixes here the duration of the shock at 103 S .
11.2 Characteristics of the grid

A number of nodes: 84
A number of meshs and types: 40 PENTA6
11.3 Functionalities
tested

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\section*{12 Results of modeling \(E\)}

\subsection*{12.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
M1 ( \(X=0.2\) ) N21
\(T=0.1\)
65.48
65.31
-0.26
\(T=0.2\)
75.58
75.81
+0.31
\(T=0.7\)
93.01
92.87
-0.15
\(T=2.0\)
99.72
99.70
-0.02

\section*{M2 ( \(X=0.8\) ) N69}
\(T=0.1\)
8.09
7.98
-1.31
\(T=0.2\)

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\section*{13 Modeling}

F

\subsection*{13.1 Characteristics of modeling}

\section*{PENTA15}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height and a thickness \(H=1.0\) with only one layer of elements. Each cube is cut out in 2 pentahedrons.

\section*{\(L=0.05\)}

\section*{Limiting conditions}
D
\(C\)
on \([B C],[A B]\) and \([c d]:.=0\)
\(H\)
on \([A D]:\) Tp is imposed
With
\(M 1\)
\(M 2\)
\(B H\)
\(T p\)
\(100^{\circ} \mathrm{C}\)
20 elements PENTA15
0
\(T\)
\(103 S\)
points
nodes
Initial conditions
M1
N62 with N70
\(T=0^{\circ} \mathrm{C}\)
M2

M2
N218 with N226
One fixes here the duration of the shock at 103 S.

\subsection*{13.2 Characteristics of the grid}

A number of nodes: 269
A number of meshs and types: 40 PENTA15

\subsection*{13.3 Functionalities tested}

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\section*{14 Results of modeling \(F\)}

\subsection*{14.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
M1 ( \(X=0.2\) ) N62
\(T=0.1\)
\(T=0.2\)
75.58
75.81
+0.31
\(T=0.7\)
93.01
92.87
-0.15
\(T=2.0\)
99.72
99.70
-0.02

\section*{M2 \((X=0.8) N 218\)}
\[
\begin{aligned}
& T=0.1 \\
& 8.09 \\
& 8.04 \\
& -0.67 \\
& T=0.2 \\
& 26.37 \\
& 25.79 \\
& -2.20 \\
& T=0.7 \\
& 78.47 \\
& 78.05 \\
& -0.54 \\
& T=2.0 \\
& 99.13 \\
& 99.08 \\
& -0.05
\end{aligned}
\]

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15 Modeling
G

\subsection*{15.1 Characteristics of modeling}

\section*{TETRA4}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height and a thickness \(H=1.0\) with only one layer of elements. Each cube is cut out in 5 tetrahedrons.
L \(=0.05\)
Limiting conditions
D
\(C\)
on [BC], [AB] and [cd.]: = 0
H
on [AD]: Tp is imposed
With
M1
M2
B H
\(T p\)
```

100 *}\textrm{C
20 elements TETRA4
0
T
103 S
points
nodes
Initial conditions
M1
N12,
N17
T=0}\mp@subsup{}{}{\circ}\textrm{C
M2
N48,
N53
One fixes here the duration of the shock at 103 S.

```
15.2 Characteristics of the grid

A number of nodes: 84
A number of meshs and types: 100 TETRA4

\subsection*{15.3 Functionalities}
tested

Orders

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STATIONARY

\section*{IMPR_RESU}

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AFFE_CHAR_THER_F
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16 Results of modeling \(G\)
16.1 Values
tested

\section*{Identification Reference \\ Aster \% \\ difference \\ M1 ( \(\mathrm{X}=0.2\) )}
\(T=0.1 \mathrm{~N} 12\)
65.48
65.37
-0.17
N17
65.49
65.27
-0.33
\(T=0.2 \mathrm{~N} 12\)
75.58
75.84

\author{
+0.34 \\ N17 \\ 75.58 \\ 75.80 \\ +0.29 \\ T=0.7 N12 \\ 93.01 \\ 92.88 \\ -0.14 \\ N17 \\ 93.01 \\ 92.86 \\ -0.16 \\ \(T=2.0 \mathrm{~N} 12\) \\ 99.72 \\ 99.70 \\ -0.02 \\ N17 \\ 99.72 \\ 99.70 \\ -0.02
}
\(M 2(X=0.8)\)
\(T=0.1 \mathrm{~N} 48\)
8.09
8.08
-0.11
N53
8.09
7.97
-1.43
T=0.2 N48
26.37
25.85
-1.96
N53
26.37
25.74

\subsection*{16.2 Remarks}

At the beginning of transient, one observes slightly different values between the nodes located in a plan \(X=\) constant (<3 per 1000). This anomaly seems to be due to modeling in tetrahedrons with 4 nodes. The results remain nevertheless correct compared to the other elements 3D.

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\section*{17 Modeling \\ \(J\) \\ 17.1 Characteristics of modeling}

\section*{TETRA4_D}

One nets only half the thickness of the wall by reason of symmetry; modeling is made under a height and a thickness \(H=1.0\) with only one layer of elements. Each cube is cut out in 5 tetrahedrons.

One uses modeling "3D_DIAG" applied to TETRA4, which corresponds to the lumpage of stamp of thermal mass.

\section*{\(L=0.05\)}

Limiting conditions
\(D\)
\(C\)
on \([B C],[A B]\) and \([c d]:.=0\)

\section*{H}
on [AD]: Tp is imposed
With
M1
M2
B H
Tp
\(100{ }^{\circ} \mathrm{C}\)
20 elements TETRA4_D
0
\(T\)
103 S
points
nodes

\section*{Initial conditions}

M1
N12,
N17
\(T=0^{\circ} \mathrm{C}\)
M2
N48,
N53
One fixes here the duration of the shock at 103 S .
17.2 Characteristics of the grid

A number of nodes: 84
A number of meshs and types: 100 TETRA4
17.3 Functionalities
tested

Orders

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TEMP_IMPO

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\section*{18 Results of modeling J}

\subsection*{18.1 Values}
tested

\section*{Identification Reference}

Aster \%
difference
M1 ( \(X=0.2\) )
\(T=0.1 \mathrm{~N} 12\)
65.48
65.34
-0.21
N17
65.49
65.24
-0.36
T=0.2 N12
75.58
75.84
+0.34
N17
75.58
75.80
+0.29
T=0.7 N12
93.01
92.87
-0.15
N17
93.01
92.86
-0.16
\(T=2.0 \mathrm{~N} 12\)
99.72
99.70
-0.02
N17
99.72
99.70
-0.02
\(M 2(X=0.8)\)
\(T=0.1 \mathrm{~N} 48\)
8.09
8.18
+1.16
N53
8.09
8.08
-0.15
\(T=0.2 \mathrm{~N} 48\)
26.37
25.90
-1.77
N53
26.37
25.79
-2.20
\(T=0.7 \mathrm{~N} 48\)
78.47
78.06
-0.52
N53
78.47
78.02
-0.57
\(T=2.0 \mathrm{~N} 48\)
99.13

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[^0]:    Beam
    Length
    Moment of inertia
    The beams are square sections on side 0.01m:
    $A B$
    L
    With $=$

[^1]:    Plan average Plaq2
    $d=0.1 \mathrm{~m}$

